DRAFT ENVIRONMENTAL IMPACT REPORT

FASHION SQUARE EXPANSION PROJECT ENV 2007-9914-EIR SCH # 2007071103

APPENDICES VOLUME II

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SEPTEMBER 2008

TABLE OF CONTENTS APPENDICES

VOLUME 1I

APPENDIX F PHASE 1 ENVIRONMENTAL SITE ASSESSME
--

APPENDIX G STORMWATER QUALITY

APPENDIX H URBAN DECAY REPORT

APPENDIX I TRAFFIC STUDY

APPENDIX J EMPLOYMENT CALCULATIONS

APPENDIX K SUPPLEMENTAL TRAFFIC INFORMATION FOR PROJECT AND ALTERNATIVES

- K-1 AVERAGE TRIP LENGTH CALCULATIONS
- K-2 TRAFFIC COUNT CONFIRMATION
- K-3 ALTERNATIVES ANALYSIS

APPENDIX L ZONING (Q) CONDITIONS

APPENDIX M MITIGATION MONITORING PROGRAM

APPENDIX F

PHASE 1 ENVIRONMENTAL SITE ASSESSMENT

PHASE I

ENVIRONMENTAL SITE ASSESSMENT

Westfield Shopping Mall Fashion Square 14006 Riverside Drive Sherman Oaks, California

February, 2008

Prepared for:

Westfield Corporation Inc. 11601 Wilshire Boulevard, 12th Floor Los Angeles, California 90025

TRG Project #7278

TABLE OF CONTENTS

PHASE I ENVIRONMENTAL SITE ASSESSMENT

Westfield Shopping Mall Fashion Square 14006 Riverside Drive Sherman Oaks, California

1.0	EXE	CUTIVE SUMMARY	1				
2.0	INT	INTRODUCTION2					
	2.1	Purpose					
	2.2	Scope of Work	2				
**	2.3	Involved Parties	3				
3.0	GEN	VERAL PROPERTY CHARACTERISTICS	3				
	3.1	Location	3				
	3.2	Adjacent Properties	3				
	3.3	Property Description					
4.0	ENV	IRONMENTAL SETTING	4				
	4.1	Regional Physiographic and Geologic Conditions	4				
	4.2	Soil Conditions					
	4.3	Groundwater Conditions	5				
5.0	RES	ULTS OF INVESTIGATION	6				
	5.1	Property Inspection Observations	6				
	5.2	Adjacent Property and Vicinity Observations	7				
	5.3	Results of Regulatory Agency List Review and File Research	7				
	5.4	Property History/Land Use Review	12				
	5.5	Synopsis of Previous Environmental Investigations					
•	5.6	Interviews	16				
6.0	CON	NCLUSIONS	17				
7.0	LIM	ITATIONS	17				
8.0	PRO	FESSIONAL CREDENTIALS	17				
9 0	REF	FRENCES	19				

FIGURES

Figure 1 - Site Location Map

Figure 2A - Site Plot Plan

Figure 2B - Lease Plan – First Level

Figure 2C - Lease Plan – Levels 2 and 3

Figure 3 - EDR Historical Topographic Map Report

Figure 4 - EDR Aerial Photo Decade Package

APPENDICES

Appendix A - Photographs of Property & Vicinity Taken During Site Visit

Appendix B - Owner Questionnaire Responses

Appendix C - Sanborn Fire Insurance Map Search, City Directory Findings and

Environmental Lien Search

Appendix D - Krazan & Associates, Inc. Geotechnical Engineering Investigation

Appendix E - EDR Environmental Database Report and Multi Tenant Facility

Report

PHASE I ENVIRONMENTAL SITE ASSESSMENT

Westfield Shopping Mall -Fashion Square 14006 Riverside Drive Sherman Oaks, California

1.0 EXECUTIVE SUMMARY

In December 2007, a Phase I Environmental Site Assessment (Phase I ESA), consistent with American Society for Testing Materials (ASTM) Standard E1527-05 and the U.S. Environmental Protection Agency Region 9 Standard 40 CFR Part 312 for All Appropriate Inquiries (AAIs), was performed for Westfield Shopping Mall - Fashion Square, a multi-tenant retail site located at 14006 Riverside Drive in Sherman Oaks, California (the Property, see **Figure 1 – Site Location Map**). The Property consists of an enclosed shopping mall, a separate building housing various retail stores and a bank, two parking garages, and a large paved parking area. The Property is 35 +/- acres and is developed with approximately 974,245 square feet of building area (see **Figure 2A – Site Plot Plan**). The buildings are occupied by several different retail stores and restaurants (see **Figure 2B – Lease Plan – First Level** and **Figure 2C – Lease Plan – Levels 2 and 3**).

The subject Property is located along the south side of Riverside Drive between Woodman Avenue at the east and Hazeltine Avenue at the west. US Highway 101 (Ventura Freeway) runs east to west just south of the Property. Area landmarks include the Los Angeles River which runs along the south side of the 101 Freeway and the Notre Dame High School campus northeast of the Property. The base of the Santa Monica Mountains is about one half mile south of the Property. The immediate area surrounding the Property is mixed commercial and residential development.

Historic investigations of the Property reviewed by TRG included a Phase I Environmental Site Investigation by Hillman Environmental Group LLC (Hillman, 2002) and a Geotechnical Investigation

Report by Krazan & Associates (Krazan 2006 -see Appendix D). The Hillman Phase I concluded that Asbestos Containing Materials were identified in two tenant spaces (Radio Shack and Erik's Shoes) in the mall and it documented several drums of diesel stored outdoors without secondary containment. In the geotechnical report, no discolored soils or odors were noted in the 10 soil borings advanced in the Krazan investigation, and first groundwater was encountered at depths ranging from 34-44 feet below ground surface (ft bgs). The geotechnical work was carried out to determine the suitability of site soils for future mall expansions.

Based on historical research, the Property reconnaissance, and interviews performed during this investigation, one recognized environmental condition (REC) exists at the Property. According to a past environmental investigation of the Property (Hillman, 2002), approximately 450 square feet of friable asbestos ceiling materials were found in the Erik's Shoes shop. During this investigation, the previously tested asbestos containing material was again observed and is considered a REC, requiring proper management in-place and notification to the tenants of that shop. The asbestos containing floor tiles once found in the Radio Shack unit were not found during TRG's site visit, and were likely removed as a part of the remodel by the new tenant White House/Black Market.

Two off-Site and five on-Site areas of potential environmental concerns were identified by the Environmental Database Review. A 76-Tosco Service Station adjacent to the northeast of the Property at 13650 Riverside Drive is reported to have released significant quantities of hydrocarbons to the soil and groundwater. This service station is considered a potential off-Site REC to the Property because the extent of the impact to groundwater has not yet been defined. However, concerns regarding the 76-Tosco site are mitigated by the fact that the owners of 76 Station are actively investigating and remediating the impacts under the oversight of the Los Angeles Regional Water Quality Control Board (LARWQCB). Located adjacent to the north of the Property, a Former Chevron station was operated 14061 Riverside Dr.. Following the removal of three USTs and one waste oil tank in March 1986, soils beneath the excavation were found to be impacted by lead, benzene, toluene, ethylbenzene and

xylenes. Between 1986 and 1989, a total of nine groundwater monitoring wells were installed on the property. Two wells (WB-8 and WB-9), located at the southern most portion of the site, detected maximum concentrations of TPH-G at 330 ug/L and benzene at 92 ug/L. Testing was never performed for MTBE on any of the wells and there were no wells installed further to the south. A vapor extraction system consisting of two vapor extraction wells was installed and was operated from 1991 to 1992. An estimated 6,100 pounds of hydrocarbons were removed over this period of time. Following the subsurface remediation, TPH-G and benzene concentrations dropped to below laboratory detection limits in all 10 wells. Quarterly groundwater monitoring proceeded until 1994, when the case was given regulatory closure by the LARWQCB. The reported groundwater gradient showed flow to the south at a very slow rate. Although this site has been given regulatory closure, it is considered a potential environmental concern due to the fact that there was never any southerly delineation of the plume prior to the installation of the remediation system, MTBE was never tested for in any of the groundwater samples and the reported gradient is towards the south in the direction of the subject Property.

Five current tenants generate small quantities of hazardous waste as a part of their operations. The Ritz/Kitz Camera is a small quantity generator of hazardous wastes related to photo processing. The chemical use and storage area of this tenant's space appeared to be clean and small scale. Sephora, a cosmetic store, was also listed as a small quantity generator of hazardous waste (acetone and alcohols) but in very limited quantities. LensCrafters is a small quantity generator of organic solids hazardous wastes. The wastes are stored in a five gallon container and picked up for disposal on an as needed basis. No violations were found in the database review. As observed by TRG, Bloomingdale's is a generator of hazardous wastes from two categories (used grease from the restaurant and returned/damaged cosmetics). Both these wastes are stored in 55 gallon drums and picked up for off Site disposal or recycling, Macy's also generates a small amount of hazardous waste identical to the waste from Bloomingdale's. Their waste is stored in a 55 gallon drum and is picked for off-Site disposal/recycling. The five on-Site waste generators only produce small quantities of relatively

innocuous wastes, and the tenant waste handling areas observed by TRG appeared to be well maintained, thus they pose only a minor potential environmental concern to the property.

Cleaning chemicals used by the Property maintenance staff were observed in small quantities and were stored properly with no evidence of spills. Backup generators were also observed in walled open air area on the southern portion of the Property, with an associated above ground diesel tank located on concrete. According to mall staff, the diesel tank is double walled. Minor staining was observed near the diesel tank. One 55 gallon drum of oil was observed to be stored without secondary containment in the area of freight elevators. Provided that these fuels are handled using best management practices, they are not considered an issue of environmental concern at this time.

2.0 INTRODUCTION

2.1 Purpose

This study was designed to evaluate whether RECs are present at or in the vicinity of commercial Property located at 14006 Riverside Drive in the City of Sherman Oaks, County of Los Angeles, California (see **Figure 1 - Site Location Map**). This Phase I was performed according to ASTM Standard E1527-05 and U.S. Environmental Protection Agency Region 9 Standard 40 CFR Part 312 for All Appropriate Inquiries (AAIs).

2.2 Scope of Work

The scope of work conformed to the requirements of Phase I Environmental Site Assessments as specified by the ASTM Standard E1527-05 and included:

 A Property inspection by a qualified environmental assessor to observe and assess Property characteristics of potential environmental concern;

- Observation of adjacent properties and the Property vicinity by a qualified environmental assessor to identify and assess Property characteristics of potential concern;
- Review of regulatory agency files;
- Review of Property history/land use to identify potential uses that may have contributed to the
 presence of environmental concerns at the Property;
- Review of previous environmental investigations;
- An environmental lien search;
- Development of this report.

2.3 Involved Parties

Westfield Corporation, Inc. (the Client) contracted with The Reynolds Group (TRG) to conduct this Phase I Environmental Site Assessment. Mr. Peter Nyquist, attorney for Westfield Corporation, Inc. represented the Client in this matter. The purpose of this investigation was to determine what RECs may exist at the subject Property in order to secure financing for planned future mall expansions. Felix Gonzales, Operations Manager for Fashion Square, accompanied the Reynolds Group representatives on a tour of the Property and answered questions during this investigation. Keitha Mills, a General Manager for Westfield, and Jonathan Krausche, Development Manager for Westfield also responded to TRG questions during this Phase I investigation. The current legal owners of the subject Property is Sherman Oak Fashion Associates, LLC.

3.0 GENERAL PROPERTY CHARACTERISTICS

Figure 1 shows the Property location, Figure 2A – Site Plot Plan is a plot plan of the Property, Figure 2B – Lease Plan – First Level is a lease plan showing the first level of the shopping mall, Figure 2C – Lease Plan – Levels 2 and 3 is a lease plan showing levels 2 and 3 of the shopping mall, Figure 3 includes historical topographic maps of the area, and Figure 4 consists of historical aerial photographs of the vicinity. Photographs of the Property and the adjacent properties taken by TRG in December 2007, are included in Appendix A of this report. The owner questionnaire responses are found in Appendix B. Historical Sanborn Insurance Maps and a City (telephone) Directory, which list past usages of the Property and surrounding sites, are found in Appendix C.

3.1 Location

The Property is situated in a mixed commercial and residential neighborhood of Sherman Oaks, California, along the south side of Riverside Drive between Woodman Avenue at the east and Hazeltine Avenue at the west. US Highway 101 (Ventura Freeway) runs east-west just south of the Property. Area landmarks include the Los Angeles River which runs along the south side of Interstate and Notre Dame High School, northeast of the Property. The base of the Santa Monica Mountains is about one half mile south of the Property.

3.2 Adjacent Properties

The immediate vicinity is dominated by commercial and residential uses. North of the Property is a residential neighborhood composed primarily of apartment buildings with an area of single family dwellings further north. Parcels along Riverside Drive, both west of Hazeltine Avenue and east of Woodman Avenue, are primarily commercial use. Notre Dame High School is at the northeast corner of Woodman Avenue and Riverside Drive and a Downey Savings office is found on the northwest

corner of this intersection. A 76-Tosco Gasoline Service Station (LUST site) is found just south of Notre Dame High, adjacent to the northeast of the Property. South of the 76 Tosco Station, on Woodman Avenue are some small retail shops and an office building. West of the Property, at the southwest corner of the intersection of Riverside Drive and Hazeltine Avenue is a Sunkist office building (LUST site). A Trader Joes store is found at the at the northwest corner of Riverside and Hazeltine, and a Los Angeles Department of Water & Power facility at the northeast corner of this intersection. Located to the adjacent to the north west of the Property at 14061 Riverside Drive is a Former Chevron Service Station which is a closed Leaking Underground Storage Tank (LUST) case.

3.3 Property Description

The Property is developed with a shopping mall, two parking garages, a separate building that houses retail stores and a bank, and a large area of paved parking. The Property is 35 +/- acres and is developed with approximately 974,245 square feet of existing building area (see **Figure 2A – Site Plot Plan**). The rear of the shopping center runs along Riverside Drive with all of the pedestrian entrances to the mall along the south side of the building facing the parking areas. The main shopping mall building is located along Riverside Drive from Hazeltine Avenue toward the east. It is constructed of poured in place concrete with frame and stucco sections. Floors and decking are primarily made of steel beams and light weight concrete.

Based on files reviewed at the City of Los Angeles Department of Building and Safety, the subject Property was originally developed with the McKinley Home for Boys and an elementary school. The existing buildings were developed on the site between 1961 and 1996. The buildings have been in use as a shopping mall since they were constructed. The original mall consisted of a Bullocks store and several small retail outlets. The old Bullocks store is now the Macy's store. The original building did not extend all the way to Hazeltine Avenue. The current Bloomingdale's store and the other retail stores at that end of the mall were added during later remodeling of the mall. The shopping center was

expanded and remodeled in 1977, and it was converted into an enclosed mall in 1989. In the process of expansion and enclosure of the mall the pedestrian entrances were changed to the south side of the building. The shopping center was further remodeled in 1994 and 1996. The exterior of the building is a combination of painted concrete and painted stucco. Interior finishes include painted gypsum board walls and ceilings with 2' x 4' and 2' x 2' ceiling panels. Floors are covered with a combination of ceramic tile, wood flooring, terrazzo flooring, and some areas of vinyl flooring. In addition to the main mall building, there is a building at the east end of the center which is occupied by retail stores and a Bank of America branch.

The mall has approximately 185-190 retail tenants of various types including several restaurants (see **Figures 2B and 2C**). The anchor tenants are a Macy's store at the east end of the main building and a Bloomingdale's store at the west end of the main building.

Water and electrical services are provided to the Property by the Los Angeles Department of Water & Power, and sewer services are provided by the Los Angeles Department of Public Works. Floor drains noted throughout the shopping mall reportedly drain to the sanitary sewer. Three sumps for collection of subsurface water in storm events are reportedly located beneath the building and loading dock areas. The pumps are said to be pumped to the sewer, but have never been used according to Site personnel. Trash disposal services to the Property are provided by IEM and Crown Disposal Services Corp. Large trash compactors were observed in four locations, and some minor staining of the concrete was noted below the compactors.

4.0 ENVIRONMENTAL SETTING

4.1 Regional Physiographic and Geologic Conditions

The Property is located at approximately 640-650 feet above mean sea level on land that slopes at a

gentle rate downward to the south and southeast. The base of the Santa Monica Mountains is about one half mile south of the Property. The Santa Monica mountains rise rapidly to the south of Ventura Boulevard. The Property is located within the southern portion of the San Fernando Valley several miles northwest of the Civic Center area of the City of Los Angeles. The San Fernando Valley is a large valley surrounded by the Santa Monica Mountains on the south, the Simi Hills on the west, the Santa Susana and San Gabriel Mountains on the north, and the Verdugo Mountains on the east. The valley is drained by the Los Angeles River system. The Los Angeles River runs along the south side of the valley and is adjacent to the south side of the 101 Freeway south of the Property. Primary deposits in the valley are alluvial deposits of recent age and are classified as Yolo soils according to the General Soils Map of Los Angeles published by the United States Department of Agriculture, Natural Resource Conservation Service in December 1969. Bedrock deposits beneath the alluvial soils are Miocene age marine and nonmarine deposits.

4.2 Soil Conditions

The soils in the area of the Property are unconsolidated. Recent alluvium derived from rocks and deposits of various ages in the mountains surrounding the valley. The soils in the area are well drained and slope gently toward the east and southeast. Generally, the soil profile consists of loamy and silty sand with gravelly sand at lower depths. Runoff is slow to moderate and the hazard of erosion is minor. In a September 2006 a Geotechnical Engineering Investigation by Krazan & Associates, Inc. (see **Appendix D**) was undertaken and the Subject Property. This work involving advancing 10 soil borings to depths from 6 to 50 feet below ground surface to determine if the soils were geotechnically stable for future mall expansion activities. This work found that the soils consisted of five feet of fill underlain by alluvium consisting of clayey silt, sandy silt, silty clay, silty sand and sand. There were no odors or discoloration note in any of the Krazan soil boring logs.

4.3 Groundwater Conditions

Groundwater in the general vicinity of the Property tends to flow in a northeasterly direction. The accuracy of the groundwater direction may be influenced by seasons and local groundwater extraction or infusion. According to the Upper Los Angeles River Area Watermaster Groundwater Contour Map, depth to groundwater in the area of the Property measures approximately 40-50 feet below ground surface, although some shallower perched groundwater zones may be present. In their Geotechnical Engineering Investigation, Krazan & Associates discovered groundwater at 34 ft., 43.5 ft., and 44.5 ft. below ground surface in three soil borings (see **Appendix D**).

5.0 RESULTS OF INVESTIGATION

5.1 <u>Property Inspection Observations</u>

On December 28, 2007, representatives of The Reynolds Group conducted an inspection of the Property. Property photographs are included in **Appendix A**. The inspection was performed from the northwest corner of the Property in a counter clockwise direction. The subject Property is described in detail in section 3.3 of this report.

The shopping mall has approximately 185-190 tenants with a Macy's store as an anchor tenant at the east end of the main building and a Bloomingdale's store as an anchor tenant at the west end of the main building. Tenants in the building at the east end of the shopping center at the southwest corner of Woodman Avenue and Riverside Drive include a Ross Dress 4 Less, a Linens & Things store, K B Toy Works, and a Bank of America branch.

The two previously identified (Hillman, 2002) area of asbestos containing materials (ACM) were examined by TRG during the Site visit. The Radio Shack noted in the 2002 Phase I was no longer present. It has been replaced by a White House/Black Market clothing store, with the interior of the

unit extensively updated and remodeled. No evidence of the 50 square feet of 12 x 12 floor tiles containing ACM were noted during the site visit, although no asbestos sampling was undertaken The ACM ceiling material was observed to still be present in the Erik's Shoes back room. Some patching of the ceiling materials was noted. It is very important and an operations and maintenance manual (O & M Manual) for this ACM be implemented any time work is to be done in the ACM containing material. No other ACM observations were made on the Property and a full asbestos survey is beyond the scope of this Phase I.

The main building has two floors in the area between Macy's and Bloomingdale's with stairways, escalators & elevators for the use of shopping patrons. Macy's store has four floors and Bloomingdale's has three floors. In addition to the elevators for the shopping patrons, there are several freight elevators. A fifty five gallon drum of oil was observed in one of the equipment rooms for the freight elevators. No stains or other evidence of leaking were observed around the drum of oil. A representative of Kone Elevators indicated that the hydraulic oil used in the elevators did not contain PCB's.

The mall is heated by electric powered, roof mounted HVAC equipment. The common areas are heated and cooled by equipment owned and maintained by the mall and there are individually owned and operated HVAC units for each of the tenants. Electricity, drinking water and sewer services are provided by the city of Los Angeles through the Los Angeles Department of Water & Power and the Los Angeles Department of Public Works. Natural gas is provided by The Gas Company. Solid waste disposal is provided by IEM and Crown Disposal Services Corp. A representative of IEM who maintains the trash compactors, indicated that the hydraulic fluid in the compactors does not contain PCB's. Floor drains located throughout the mall drain to the sewer system. There are three sump pumps located in the mall. Two are located in a subterranean storage area and the third is located adjacent to one of the loading docks along Riverside Drive. In addition to the loading dock with the sump pump, there are additional loading docks along Riverside Drive and along the front of the mall facing the parking areas.

There are two emergency generators located in an equipment area across from the multi story parking

garage along the front of the mall. Next to the generators is an above ground storage tank for diesel

fuel to run the generator. According to Mall staff, this tank is double walled. Minor staining was

observed on the concrete around the storage tank.

The only other chemicals stored on the site are cleaning fluids and other janitorial and maintenance

supplies. Those supplies are stored on shelves in a locked house keeping storage room and a locked

maintenance room.

The parking areas around the buildings and the loading dock areas are paved with asphalt and concrete

which is in generally good condition. Borings were noted in the south central portion of the parking

lot, in locations that correspond to those discussed in the Krazan, 2006 geotechnical report.

There are 12 pad mounted electrical transformers located along Riverside Drive and along the

parking garage areas south of the building. The transformers (IS 2627-1-01 & 02 through IS 2627-6-

01 & 02) are the property of the Los Angeles Department of Water & Power and they would be

responsible for any maintenance on their transformers. A request has been sent to the LADWP

regarding the possible presence of PCB's in the transformers. Several additional transformers are

located within the shopping mall building. All of those transformers are "dry type" transformers and,

as such, are not suspect of containing PCB's.

No pungent or noxious air emissions were noted during the Property inspection.

No standing water was observed at the subject Property.

The Reynolds Group

Westfield Shopping Mall, 14006 Riverside Dr., Sherman Oaks, CA

Page 12

No adverse environmental conditions were observed during the Property inspection.

5.2 Adjacent Property and Vicinity Observations

Adjacent properties are described in Section 3.2 of this report. Visually, no obvious issues of environmental concern were noted during the investigation as viewed from public right of ways Sixteen (16) sites within one mile of the Property are listed in the Environmental Data Resources (EDR) environmental disclosure report (see section 5.3 below and **Appendix E**). One site listed in the EDR report presents a potential environmental concern to the Property for reasons detailed in section 5.3 and 5.4 below.

5.3 Results of Regulatory Agency List Review and File Research

TRG retained Environmental Data Resources, Inc. (EDR), to provide a list of facilities within the Property vicinity that are currently under review, management, or notification by a regulatory agency as indicated in an "Environmental Disclosure Report." The full "Environmental Disclosure Report" is included in **Appendix** E. The following summarizes the surrounding sites identified in the agency databases by the EDR report as well as the results of regulatory agency files reviews. Depending on the database, and in compliance with ASTM Standard E1527, the search distance for each of the database reviewed is between 0.25 and 1.0 miles from the subject Property.

All sites located in the immediate vicinity of the Property found in the EDR environmental disclosure report and City Directory, are not issues of environmental concern to the subject Property for reasons detailed in section 5.3, except for the 76-Tosco Service Station located east of the Property at 13650 Riverside Drive, which is considered a potential environmental concern.

Subject Property:

The subject Property was listed on four (4) of the databases searched by EDR.

Sephora Stores, HAZNET, RCRA – SQG, FINDS: Sephora Stores is a small quantity generator of hazardous wastes. The wastes are stored in a five gallon container and are picked up for recycling or disposal on an as needed basis. No violations found.

Kits/Ritz Camera, RCRA-SQG, FINDS: Kits Camera has a one hour photo developing service that generates a small quantity of hazardous waste from the film developing process. Silver is recovered from the waste developing fluids and then those fluids are dumped into the sewer system. The recovered silver is picked up for recycling on an as needed basis. No violations were found in the database review and TRG noted that the film development chemical use area was clean and well maintained at Ritz Camera.

LensCrafters, HAZNET, EMI: LensCrafters is a small quantity generator of organic solids hazardous wastes. The wastes are stored in a five gallon container and picked up for disposal on an as needed basis. No violations were found in the database review.

Bloomingdale's is not included in the attached EDR list. However, Bloomingdale's is a generator of hazardous wastes from two categories as observed by TRG on the site visit. Bloomingdale's Manager Aaron showed TRG a 55 gallon of used grease from the restaurant which is picked up for recycling by Baker Industries on an as needed basis. According to Aaron, returned and damaged cosmetics and fragrances are also classified as hazardous waste. Those wastes are also stored in a 55 gallon drum and picked up for disposal by Smurfit Co. on an as needed basis

Macy's also generates a small amount of hazardous waste identical to the waste from

Bloomingdale's. Their waste is stored in a 55 gallon drum and is picked up by Clean Harbors Co., Inc. on an as needed basis. Information was provided by the onsite Manager Michele.

JP Mechanical, HAZNET: JP Mechanical is listed as a small quantity generator of waste oil and mixed oil hazardous waste at the subject Property. Reportedly, the wastes were stored in a five gallon container and picked up for recycling or disposal on an as needed basis. City Freehold was also listed on the HAZNET database as a small quantity generator of inorganic solids hazardous wastes. The wastes were stored in a five gallon container and are picked up for recycling or disposal on an as needed basis. Neither of these tenants were observed at the time of the site visit and neither historic telephone directories or a tenant report listed either of these businesses at the Property. According to Westfield staff, City Freehold may have been a prior owner or management company for the Property.

Surrounding Sites:

The United States Environmental Protection Agency (EPA), National Priority List (NPL): a listing of hazardous waste generators which are, or proposed to be, EPA-enforced Superfund sites. No cases are identified on the NPL, Proposed NPL or Delisted NPL databases within a one-mile search distance of the Property. Further, the subject Property is not listed as an NPL Recovery Site.

CERCLIS): contains data on potentially hazardous waste sites that have been reported to the USEPA by states, municipalities, private companies and private persons, pursuant to Section 103 of the CERCLA. CERCLIS contains sites which are either proposed to or on the NPL and sites which are in the screening and assessment phase for possible inclusion on the NPL. No CERLIS cases are identified within a ½-mile search distance of the subject Property.

Comprehensive Environmental Response, Compensation, and Liability Information System Source, No Further Action Planned (CERCLIS-NFRAP): As of February 1995, CERCLIS sites designated "No Further Remedial Action Planned" (NFRAP) have been removed from CERLIS. NFRAP sites may be sites where, following an initial investigation, no contamination was found, contamination was removed quickly without the need for the site to be placed on the NPL, or the contamination was not serious enough to require Federal Superfund action or NPL consideration. No CERCLIS-NFRAP cases are identified within a 1/2 -mile search distance from the subject Property.

Corrective Action Report (CORRACTS): identifies hazardous waste handlers with Resource Conservation and Recovery Act (RCRA) corrective action activity. No cases are identified on the CORRACTS list within a one-mile search distance from the subject Property.

Resource Conservation and Recovery Act (RCRIS): is a database which includes selected information on sites that generate, store, treat, or dispose of hazardous waste as defined by the Act. The list is developed and maintained by the EPA. The purpose of this listing is to summarize registration information and does <u>not</u> imply that contamination has occurred on the property, but does identify potential sources of contamination.

Three (3) small quantity generator (SQG) sites are identified within 1/4 mile of the Property. They have been identified as:

Burbank Medical Clinic Inc., 13739 Riverside Drive, located adjacent to the north northeast of the site, is a small quantity generator with no violations found.

Former Chevron Station #9-1683, 14061 Riverside Drive located adjacent to the north northwest, former gasoline station site. Site is now closed. The former Chevron station is discussed in more

detail in the LUST section below.

High Tech Auto, located adjacent to the west of the Property at 4774 Woodman Avenue, is a small

quantity generator with no violations found.

No violations have been reported at the 2 current RCRIS SQG sites and, thus, they are not likely an

issue of environmental concern to the Property.

Emergency Response Notification System (ERNS): This listing is a database of incident

notification information regarding incidents of reported releases of oil and hazardous substances.

The search includes only the subject property/target property (TP). The subject property was not

listed on the ERNS list.

Hazardous Material Incident Report System (HMIRS): contains information on reported

hazardous material incidents (accidental releases or spills). The search includes only the subject

property/target property (TP). The subject property is not listed on the HMIRS list.

Hist Cal-Sites: This database contains both known and potential hazardous substance sites. The

source is the California Department of Toxic Substance Control. A review of the HIST Cal-Sites list,

as provided by EDR revealed that there are no HIST Cal-sites within approximately 1 mile of the

Property.

Cortese List: This older database, which is not currently updated by the State of California,

identifies public drinking water wells with detectable levels of contamination, hazardous substance

sites selected for remedial action, sites with known toxic material identified through the abandoned

site assessment program, sites with USTs having a reportable release and all solid waste disposal

facilities from which there is known migration. The source is the California Environmental

The Reynolds Group Westfield Shopping Mall, 14006 Riverside Dr., Sherman Oaks, CA

Page 17

Protection Agency/Office of Emergency Information. A review of the Cortese list, as provided by EDR, revealed that there are four (4) Cortese sites within a half mile radius of the Property. All of these sites are also found in the LUST database and are discussed in the LUST section below. The Cortese sites include:

- The 76-Tosco Gasoline Service Station found on Woodman Avenue and Riverside Drive
- The former Chevron station at 14061 Riverside Drive adjacent to the north of the Property
- The Sunkist Growers site, adjacent to the west of the property and
- The Fashion Square Car Wash (LUST Site) located just south of the Ventura Freeway on Woodman Avenue

The former Chevron and Sunkist LUST cases are closed and the 76-Tosco and Fashion Square sites are still undergoing assessment/remediation. For a more detailed description of these site found near the Property, please see Section 5.3 below.

Proposition 65 (**Notify 65**): contains facility notifications about any release which could impact drinking water and thereby expose the public to a potential health risk. One case, the 76-Tosco Gasoline Service Station at Woodman and Riverside, is identified in the Notify 65 database within a one-mile search distance of the subject Property. This site is discussed in the LUST section below.

Toxic Pits: identifies sites suspected of containing hazardous substances where cleanup has not yset been completed. <u>No Toxic Pit cases</u> are identified within a one-mile search distance of the subject Property.

Waste Management Unit Database (WMUDS/SWAT): is composed of a number of databases and is used by the State Water Resources Control Board staff and the Regional Water Quality Control Boards for program tracking and inventory of waste management units. No WMUDS/SWAT cases are identified within a ½-mile search distance of the subject Property.

SWRCY: is a listing of recycling facilities in California. A review of the SWRCY list, as provided

by EDR, has revealed that there are no SWRCY sites within a half mile of the Property.

Leaking Underground Storage Tanks (LUST) list: This is a list that compiles the State Water

Resources Control Board (SWRCB) and Regional Water Quality Control Board (RWQCB)

identified facilities that have had unauthorized releases from underground storage tanks (USTs) and

non-tank spills in the area. Four (4) LUST sites are listed within a ½-mile search distance from the

subject Property.

• The 76 -Tosco Gasoline Service Station found on Woodman Avenue and Riverside Drive

• The former Chevron station at 14061 Riverside Drive adjacent to the north of the Propert

• The Sunkist Growers site, adjacent to the west of the property and

• The Fashion Square Car Wash (LUST Site) located just south of the Ventura Freeway on

Woodman Avenue

The former Chevron and Sunkist LUST cases are closed and the 76-Tosco and Fashion Square sites

are still undergoing assessment/remediation. For a more detailed description of these site found near

the Property, please see Section 5.3 below.

Underground Storage Tanks (UST): is a list of active registered underground storage tanks. No

sites within the ¼ mile search distance from the Property are listed with USTs.

California Bond Expenditure Plan (CA-BEP): Department of Health Services developed a site-

specific expenditure plan as the basis for an appropriation of Hazardous Substance Cleanup Bond

Act funds. It is not updated. No cases are identified within a one-mile search distance of the subject

Property in the CA-BEP database.

California Facility Inventory Database (CA-FID UST): contains a historical listing of active and

The Reynolds Group Westfield Shopping Mall, 14006 Riverside Dr., Sherman Oaks, CA

Page 19

inactive underground storage tank locations from the State Water Resource Control Board. One site

is listed in the CA-FID database within ¼ mile from the Property.

Current occupant, 14061 Riverside Drive, (former Chevron Station) - see Section 5.3 for more

details.

Hazardous Substance Storage Container Database (HIST UST): is a historical listing of UST

sites provided by the State Water Resources Control Board. One site is listed in the HIST UST

database within \(\frac{1}{4} \) mile of the Property.

14061 Riverside Drive, (former Chevron Station) discussed further in Section 5.4 below.

Dry Cleaners is a database listing dry cleaning facilities that have EPA ID numbers. No dry

cleaning business is reported to be operating within 1/4 mile of the Property.

Spills, Leaks, Investigations and Cleanups (CA SLIC) is a database which includes sites that have

experienced an unauthorized discharge. Other than UST sites discussed above, there are no SLIC

cases listed within ½ mile of the Property.

Envirostor: EnviroStor database identifies sites that have known contamination or sites for which

there may be reasons to investigate further. A review of the Envirostor list, as provided by EDR,

revealed that there is one Envirostor site within approximately one mile of the Property. The site is

Joy's Dry Cleaners at 13313 Moorpark Street. The site is located more than one half mile from the

Property at a lower elevation and likely down gradient from the Property. Due its distance from the

Property and the presence of the Los Angeles River between the sites, Joe Cleaners is not considered

to be a issue of environmental concern to the Property.

US Brownfields: The EPA's listing of Brownfields properties addressed by Cooperative Agreement

The Reynolds Group

Westfield Shopping Mall, 14006 Riverside Dr., Sherman Oaks, CA

Page 20

Recipients and Brownfields properties addressed by Targeted Brownfields Assessments. No cases were identified on the Brownfields list within approximately ½ mile of the Property.

California Hazardous Material Incident Report System (CHMIRS): contains information on reported hazardous material incidents (accidental releases or spills). No cases were identified in the CHMIRS database within a one-mile search distance of the subject Property.

Solid Waste Information System (SWIS): The California Integrated Waste Management Board maintains an inventory of both open, as well as closed and inactive, solid waste disposal facilities and transfer stations pursuant to the Solid Waste Management and Resource recovery Act of 1972. No facilities were identified on the SWIS list within a ½-mile search distance of the subject Property.

RESPONSE: is a database that identifies confirmed release sites where DTSC is involved in remediation efforts. No cases are listed within approximately 1.0 mile of the Property.

EDR Manufactured Gas Plants: is a listed of plants that used coal to manufacture gas to be distributed used as fuel. No cases were found within 1.0 miles of the Property.

EDR Historical Auto Stations: is a historical listing of potential auto gas stations within 0.25 miles of the Property. No sites have been listed within 0.25 miles of the Property.

EDR Historical Cleaners: is a historical listing of businesses that may have included dry cleaning businesses. No sites were listed within 0.25 miles of the Property.

Indian Reservation Database: is a listing of Indian administered lands. No Indian Reservation sites are listed within one mile of the subject Property.

Orphaned Sites: Due to poor or inadequate address information, one site was not able to be mapped within the EDR database search and is identified as "Orphan" (see Orphan Summary on page 11 of the EDR Geocheck Report in **Appendix E**). The Orphan site is:

1X McKesson Drug Company: No address is given and the site was not observed in the vicinity of the Property.

Environmental Liens: A search of Title Records conducted by EDR revealed no evidence of any environmental liens currently existing against the Property. A copy of the Environmental Lien Report can be found in **Appendix C**.

5.4 Property History/Land Use Review

5.4.1 Aerial Photographs

Aerial photographs for the Property vicinity were provided by EDR and are included as **Figure 4** of this report. The photographs were reviewed in order to ascertain historical land uses that may have been responsible for the generation/or storage of potentially hazardous materials. Photographs were available for review for the years 1928, 1938, 1947, 1956, 1965, 1976, 1989, 1994, and 2002. The following is a brief summary of photograph observations:

<u>1928</u>

The Property is developed with the McKinley Home for Boys in the central area of the site, and there is a school at the northeast corner of the Property. The school is identified as the Woodman School in Sanborn Maps and old Topographic Maps. The western portion of the Property is undeveloped. The areas around the Property are all undeveloped with some agricultural usage visible. South of the Los Angeles River south of the Property the area shows scattered residential development.

1938

The subject Property is as it appeared in 1928. South of the Property the residential development has continued south of the river. North, northeast and east of the Property the area is still undeveloped. West of the Property the area is mostly undeveloped with buildings visible along the west side of Hazeltine Avenue north of the river. Those buildings are identified in Sanborn Maps as the Eunice Knight Saunders School.

1947

The Property remains as it was in 1928 and 1938. The area north of the Property is mostly undeveloped with some development beginning in the northwest quadrant of the intersection of Woodman Avenue and Riverside Drive. The initial development of Notre Dame High School is visible in the northeast portion of the intersection. East of the Property the area is undeveloped near Riverside Drive with some residential development farther south along Woodman Avenue. South of the Property the freeway has not been developed, but the residential development south of the river has continued with only a few scattered undeveloped areas. The areas west and northwest of the Property remain mostly undeveloped with the Saunders School west of Hazeltine Avenue and north of the river.

1956

The Property is still developed with the boy's home and the school as it was in earlier photographs. The area south of the Property and south of the river is almost fully developed with residential dwellings

1965

The boy's home and the school have been removed from the Property and the initial development of the existing shopping mall has been completed. The anchor tenant building

(Bullocks) at the east side of the main building has been built, and the retail store spaces west of the Bullocks location have also been completed. The western portion of the Site is used for parking, with only one smaller structure on the southwest corner of the property, which is no longer there. The areas north and south of the subject Property are in residential use, and the areas east of the Property appears to be in commercial use. The Saunders School west of

the subject Property has been removed and that site is not developed.

1976

The subject Property is unchanged from the 1968 photograph. The area around the Property is also mostly unchanged except that the existing office building has been built west of the Property.

1989

The shopping mall on the subject Property has been expanded to its current size. The western portion of the mall is now covered, not open-air separate stores. The building on the southwest corner of the Property is now gone and has been replaced by the current parking structure on the southwestern portion of the property. The areas around the site are unchanged from 1976.

1994

There are no visible changes to the subject Property. The area around the Property has no significant changes from 1989.

2002

The subject Property and vicinity appears as they are today.

5.4.2 Topographic Maps

Historical topographic maps for the Property vicinity were provided by EDR and are included as **Figure 3** of this report. The reviewed maps dated 1900, 1901, 1902 showed the area around the site as undeveloped land with none of the surrounding roads. By 1926 Riverside Drive was located along the northern edge of the Property and Woodman Avenue was also in place.. Based upon these maps, there were originally two small hills on the eastern and western sides of the subject Property north of the river.

A Topographic Maps from 1953 showed the Mc Kinley Home for Boy's buildings and the Woodman Avenue School buildings on the subject Property. The area around the site was mostly developed, but the freeway south of the Property had not been built. To the west, the Saunders School was shown. On the 1966 topographic map, the school at the Property has been replaced with the original shopping center buildings. The school to the west is also absent. In 1966, the 101 freeway is seen for the first time. The 1972 topographic map 1972 also showed the original portion of the shopping mall in place and the existing office building west of the Property.

5.4.3 Sanborn Maps

Because the area around the site has been developed for an extended period, Sanborn Maps were available for several dates from 1955 through 1969 (see **Appendix C**). Those maps showed the Property developed with the McKinley Home for Boys in 1955 and 1960, with the Saunders Nursery School adjacent to the west. The 1960 map shows the Ventura Freeway adjacent south of the Property. Maps from 1963 through 1969 showed the initial portion of the shopping center on the Property with the Bullocks store as the anchor tenant. The eastern portion of the Property is not shown in the Sanborn Maps, nor are land uses adjacent to the east seen.

5.4.4 City Directory Search and Multi-Tenant Retail Facility Report

City directories and Tenant Reports were reviewed for the subject Property as provided by EDR and is attached in **Appendix C** and **Appendix E** The Property address first appears in the City Directory in 1985 as four different businesses. In 1990 it first appears as "Sherman Oaks Fashion Square" Los Angeles Joint Board Amalgamated Clothing Workers of America". Subsequent Property listings through 2006 are shown with listings of the mall tenants. Surrounding addresses on Hazeltine Avenue, Huston Street, La Maida Street, Murrieta Avenue, Peach Gove and Riverside show numerous individual names in the earlier listings indicating residential use. Occasionally in earlier listings and increasing beginning in 1970, area addresses include retail and contracting businesses including accounting, legal, gasoline stations, investment firms, travel agency and insurance services.

The Property address first appears in the Multi-Tenant Report in 1971 and identifies a number of retail shops and one restaurant. From 1971 to 2004, the report identifies various tenants which resided within the shopping center.

5.4.5 Agency Records Review

File review were conducted at the following agencies:

Building Department Records_Review - A representative of TRG visited the Los Angeles Building Department on January 3, 2008, to request and review records for the subject Property. There were no records which indicated any recognized environmental concerns to the Property. Based on files reviewed at the City of Los Angeles Department of Building and Safety, the subject Property was originally developed with the McKinley Home for Boys and an elementary school. The existing buildings were developed on the site between 1961 and 1996. The buildings have been in use as a

shopping mall since they were constructed. The original mall consisted of a Bullocks store and several small retail outlets. The old Bullocks store is now the Macy's store. The original building did not extend all the way to Hazeltine Avenue. The current Bloomingdale's store and the other retail stores at that end of the mall were added during later remodeling of the mall. The shopping center was expanded and remodeled in 1977, and it was converted into an enclosed mall in 1989. In the process of expansion and enclosure of the mall the pedestrian entrances were changed to the south side of the building. The shopping center was further remodeled in 1994 and 1996.

Los Angeles City Fire Department – A representative of TRG visited the Los Angeles City Fire Department on January 3, 2008 to review files for one property. Findings of the review are:

76-Tosco Service Station, 13650 Riverside Drive –This gasoline station is at the southeast corner of Riverside Drive and Woodman Avenue, adjacent to the east of the subject Property. A gasoline service station has been located at this address since approximately 1949. A leak was discovered and repaired in the dispenser lines in 1998. The LUST case for that leak was closed in 1999. Substantial contamination was discovered in the soil and groundwater in 2003 with the highest contamination reported to be east and west of the dispenser islands. Groundwater depth was reported to be 25-28 feet bgs with a gradient toward the southeast. A workplan for a further site assessment was prepared in 2004, but there was no indication in the file as to what work may have been done on the site. The file was transferred to the LARWQCB on January 4, 2007, and there were no further entries in the Fire Department file. The results of the LARWQCB file review are discussed below.

Los Angeles Region Water Quality Control Board – A representative of TRG visited the Los Angeles Regional Water Quality Control Board (LARWQCB) to review files for six addresses. Findings of the reviews are as follows:

76- Tosco Service Station, 13650 Riverside Drive - This site, discussed above, is located adjacent to the east of the Property and currently operates three 10,000 gasoline USTs and one 12,000 diesel UST. In 1999, under the oversight of the Los Angeles City Fire Department, 76 tons of hydrocarbon-impacted soil were removed and transferred to a recycling facility. After this 1999 effort, the LAFD granted closure to the site. In 2003, elevated levels of TPH-G, TPH-D, benzene, MTBE and TBA were found in the groundwater

samples at concentrations above Maximum Contamination Levels (MCL) for drinking water. At this time, the case was reopened by the Los Angeles Regional Water Quality Control Board (LARWQCB).. Following preparation of a 2004 workplan, Two groundwater monitoring wells were installed on the 76-Tosco site. The maximum groundwater sample concentrations from 2007 were TPHG at 10,000 ug/L, TPH-D at 5,400 ug/L, benzene at 450 ug/L, toluene at 120 ug/L, ethylbenzene at 810 ug/L, xylenes at 800 ug/L, MTBE at 65 ug/L and TBA at 3,600 ug/L. These levels are also above MCL standards for drinking water. The groundwater gradient direction of this site is unknown and the lateral extent of the hydrocarbons plume has yet to be defined to the west. Groundwater gradient was reported by an earlier consultant to trend towards the east/ southeast following the Los Angeles River, however it is possible that the plume has migrated to the west beneath the subject Property.

Former Chevron Station, 14061 Riverside Drive - Located adjacent to the north of the Property, this site formerly operated as a Chevron Gasoline Station. Following the removal of three USTs and one waste oil tank in March 1986, soils beneath the excavation were found to be impacted by lead, benzene, toluene, ethylbenzene and xylenes. Between 1986 and 1989, a total of nine groundwater monitoring wells were installed on the property. Elevated concentrations of TPH-G (30,000 ug/L) and benzene (9,8000 ug/L-above the MCL) were detected in the monitoring wells. Free product hydrocarbons periodically existed in one of the onsite monitoring wells. Two wells (WB-8 and WB-9), located at the southern most portion of the site detected maximum concentrations of TPH-G at 330 ug/L and benzene at 92 ug/L. Testing was never performed for MTBE on any of the wells and there were no wells installed further to the south. A vapor extraction system consisting of two vapor extraction wells was installed and was operated from 1991 to 1992. An estimated 6,100 pounds of hydrocarbons were removed over this period of time. Following the subsurface remediation, TPH-G and benzene concentrations dropped to below laboratory detection limits in all 10 wells. Two soil borings were advanced to 40 ft. bgs in the confined hydrocarbon impacted area in 1993. All 16 soil samples did not detect the presence of TPH-G or BTEX. Quarterly groundwater monitoring proceeded until 1994, when the case was given regulatory closure by the California Regional Water Quality Control Board (CRWQCB). The groundwater elevation data reveals that the gradient is slightly to the south. Although this site has been given regulatory closure, it is considered a potential environmental concern due to the fact that there was never any southerly delineation of the plume prior to the installation of the remediation system, MTBE was never tested for in any of the groundwater samples and the gradient is towards the south in the direction of the subject Property.

Sunkist Growers, 14130 Riverside Drive- This Site is located adjacent west of the subject Property and operates as Sunkist Growers. The property currently contains two 10,000 gallon USTs. In January 1996, Sunkist Growers reported a release of an estimated 1,100 gallons from the UST system. A Site Assessment was completed in April of 1996 in which

13 exploratory soil borings were advanced around the USTs and associated piping. Three of the borings were converted into groundwater monitoring wells. Elevated soil concentrations of TPH-G, benzene, toluene, ethylbenzene and xylenes were detected above the Los Angeles Fire Department Action Levels. Elevated groundwater concentrations of TPH and BTEX were found to have impacted the groundwater. The elevated concentrations were both above the MCL standards for drinking water. Monitoring wells E-10 and E-11, located on the eastern edge of the site. E-11 detected concentrations of TPH-G at 87 ug/L and benzene at 0.9 ug/L. MTBE was not tested for in any of the groundwater samples. Groundwater monitoring continued for another year, which saw site concentrations decrease. No remedial activities were performed at the site. The site was given "Underground Storage Tank Case Closure" in November 1996 by the LARWQCB. The groundwater gradient measured in three monitoring event between January 1996 and September 1996 showed a general trend towards the west, flowing away from the subject Property. Given this case has been given regulatory closure, and that groundwater has been shown to flow away from the subject property, it is not a recognized as an environmental concern.

Fashion Square Car Wash, 4625 Woodman Avenue- Located approximately one half mile southeast of the subject Property, just south of the Ventura Freeway, the Fashion Square Car Wash operates as a car wash facility with a body shop, a vehicle lubricating shop and has one dispenser island. There are five USTs associated with the dispenser island. Free product was reported at the site in 1988. Since 1995, a total of 21 groundwater monitoring wells have been installed at the site. Elevated maximum concentrations of TPH-G, benzene, toluene, ethylbenzene, xylenes and MTBE, above California MCLs have been detected in the wells. Free product hydrocarbons have been detected in 16 different with free product removal at the Site from the mid to late nineties. In March of 2000, the five original USTs replaced. At this time a soil vapor extraction system was installed. Currently, there are 20 active groundwater monitoring wells, seven of which contain various amounts free product. Since 1995, the groundwater gradient of the Site has consistently been to the east and northeast. As of the latest groundwater monitoring report done in October 2007, the lateral extent of the hydrocarbon plume has not yet been defined in the northeast, north and northwestern directions. Given that the direction of groundwater flow is to the northeast at this site towards the LA River and away from the subject Property, this site is not considered an environmental concern.

76 Station, 14478 Ventura Boulevard- This property is located approximately 1.0 mile southwest of the Site and operates as a gasoline station. This active gas station contains two 10,000 gallon USTs and one 550 gallon waste oil tank. In April 1988, hydrocarbon impacted soil was discovered during trenching activities. Three groundwater monitoring wells were installed in June 1988 to delineate the suspected plume. The three tanks were removed in 1990 and replaced with new tanks. Approximately 380 cubic yards of hydrocarbon impacted soils were removed from the excavation. In February 1991, four soil borings were advanced

on the property. Currently nine groundwater monitoring wells exist at the property. Maximum concentrations of TPH-G (68,000 ug/L), benzene (6,200 ug/L), toluene (210 ug/L), ethylbenzene (7,100 ug/L), and MTBE (50,000 ug/L have been detected in the groundwater since 1992. The groundwater plume has not been fully delineated. The northern most well, MW-5, has current detections of TPH-G and benzene. The overall groundwater gradient detected in monitoring events from 1992 to 2006 been consistently been to the north- northeast. Given the distance between this property and the Site and the fact that it is south of the Los Angeles River, this property is not recognized as an environmental concern.

Former Shell Service Station, 4404 Woodman Avenue- Located approximately three quarters of a mile southeast of the Site, this property operated as a gas station from around 1985 to early 2002. It is now a used car dealership. During a UST removal in 1987, petroleum hydrocarbons were discovered in the soil and groundwater. Between 1985 and 1987 a total of 14 groundwater monitoring wells were installed (B-1 to B-14) to define the extent of the petroleum hydrocarbons. Nine of the 14 wells contained free product and free product recovery was performed on a monthly to semi-monthly basis. remediation took place during this time. A quarterly groundwater monitoring schedule took place until 1998 in which the concentrations of hydrocarbons diminished. In 1998, the Los Angeles Regional Water Quality Control Board issued a No Further Action Letter for the original UST release. Following a divestment of the property by Shell, the case was reopened following the removal of the three USTs and one waste oil tank in 2002. In 2003, two onsite and four offsite monitoring wells (MW-1 thru MW-6) were installed to further delineate the extent of the petroleum hydrocarbons, which are currently being monitored on a quarterly basis. The most recent maximum concentrations of petroleum hydrocarbons in 2007 are TPH-G at 4,400 ug/L, toluene at 0.54 ug/L, MTBE at 280 ug/L, TBA at 160,000 ug/L and DIPE at 340 ug/L. The concentrations of MTBE and TBA exceed Maximum Contamination Levels for drinking water. Historical and recent groundwater gradients show a flow towards the southwest. Given the fact that groundwater flow is in the opposite direction as the subject Property and that the northern extent of the Shell plume is south of the Los Angeles River, this site is not considered an environmental concern.

During the Agency File Review portion of this investigation, two potential environmental concerns were identified at the 76-Tosco Gasoline Station and the Former Chevron Station.. Due to the fact that these site are adjacent to the subject Property, and the extents of the groundwater impacts were not fully delineated in the direction of the Prooperty, they represents a potential off-Site Environmental Concerns.

5.5 Synopsis of Previous Environmental Investigations and Subsurface Investigations

A Phase I Environmental Site Assessment was completed by Hillman Environmental Group LLC of Long Beach, in October 2002. Hillman did not identify any recognized environmental concerns on the subject Property. Further, they did not identify any recognized environmental concerns in the surrounding area or in the EDR Database files that they felt would be a concern to the Property. Hillman identified Ritz Camera, Macy's and Bloomingdale's as generators of hazardous wastes. They found that the wastes were properly labeled and properly disposed on an as needed basis. The only concerns identified by Hillman were suspected asbestos containing materials in two of the tenant spaces that should be the subject of an O&M program. They also found 55 gallon drums of diesel fuel stored outside near the emergency generator that did not have a secondary containment area for protection against an accidental spill. Those drums of diesel fuel have been replaced with an above-ground diesel storage tank, which according to Westfield on-site staff is double walled.

To determine if the site's soils were suitable for future mall expansion into the south portion of the property, a geotechnical investigation was completed by Krazan and Associates, Inc. of Ontario, in September 2006 (see **Appendix D**). In this investigation 10 soil borings were advanced to total depths of 6 to 50 ft bgs. Groundwater was encountered in 3 borings at depths ranging from 34 to 44.5 ft bgs. Unconsolidated fill material was found in the first five feet of many borings, with alluvial sands, silt and clay noted in the native soils. Based on the results of their investigation, Krazan stated that site soils were suitable for future mall expansion, with some removal of unconsolidated fill material required.

5.6 Interviews

Mr. Felix Gonzales, Operations Manager for Westfield Fashion Square and Keitha Mills, General Manager for Westfield Fashion Square, accompanied TRG representatives on the site tour and

answered questions regarding the use of the site and the equipment on the site. Mr. Gonzales provided plot plans for the Property and provided names of contact people for the elevators and for the trash compactors to verify whether or not there are PCB's in the hydraulic oils used in those pieces of equipment. The tour included the janitorial storage rooms, the equipment rooms for all of the elevators and trash compactors, the sump pumps, and the emergency generator. Mr. Gonzales stated that the sump pumps and emergency generator were tested on a regular basis, and that they had not been needed for flooding or emergency power in the last three years since he had been the Operations Manager. Mr. Gonzalez and Ms. Mills noted that there was asbestos located in Erik's Shoes, one of the original stores from the mall. Mr. Jonathan Krausche, Development Manager completed the due diligence questions related to the Property's history and environmental status (see Appendix B).

6.0 CONCLUSIONS

TRG has performed a Phase I Environmental Site Assessment in conformance with the scope and limitations of ASTM Practice E 1527 of the Westfield Shopping Mall - Fashion Square Property located at 14006 Riverside Drive, Sherman Oaks, California. This assessment has revealed one recognized environmental condition (REC) in connection with the Property and one potential off-site REC that may be an issue of concern to the Property. Asbestos located in Erik's Shoes is a REC and must be properly managed to minimize any environmental risk to property and human health. The 76 Gasoline Station located 0.5 miles southeast of the Property is under investigation to define the extent of a petroleum hydrocarbon plume beneath the site. Until the investigation of that case has been completed and the extent of the hydrocarbon plume has been delineated, it presents a potential REC to the Property.

7.0 LIMITATIONS

The conclusions and recommendations presented above are based upon the scope of work outlined in

the above report. Evaluation of potential issues such as air quality, flood plain information, seismic

conditions, mold, radon, lead-based paint, lead in drinking water and wetlands were not included in

the scope of services. The Reynolds Group makes no warranties or guarantees as to the accuracy or

completeness of information obtained from, or compiled by others. It is possible that information

exists beyond the scope of this investigation. Additional information not found or available to The

Reynolds Group at the time of report writing may result in a modification of the conclusions and

recommendations presented. This report is not a legal opinion.

8.0 PROFESSIONAL CREDENTIALS

All work associated with this Environmental Site Assessment was performed by or under the direct

supervision of Gwen Tellegen. Ms. Tellegen has worked in the environmental industry for more

than 15 years and holds several credentials including California Registered Civil Engineer (No.

C58670) and two Master of Science Degrees (Environmental Engineering and Biology) from the

University of Southern California.

Robert Vanderstraeten has worked in the environmental industry for more than 14 years. Mr.

Vanderstraeten has a bachelor's degree in geology and is a California Registered Environmental

Assessor (No. 05364). He performs numerous Phase I Environmental Site Assessments each year.

THE REYNOLDS GROUP

a California corporation by:

Gwen Tellegen, P

California Registered Civil Engineer #C58670

Robert Vanderstraeten, REA I #05364

Project Manager

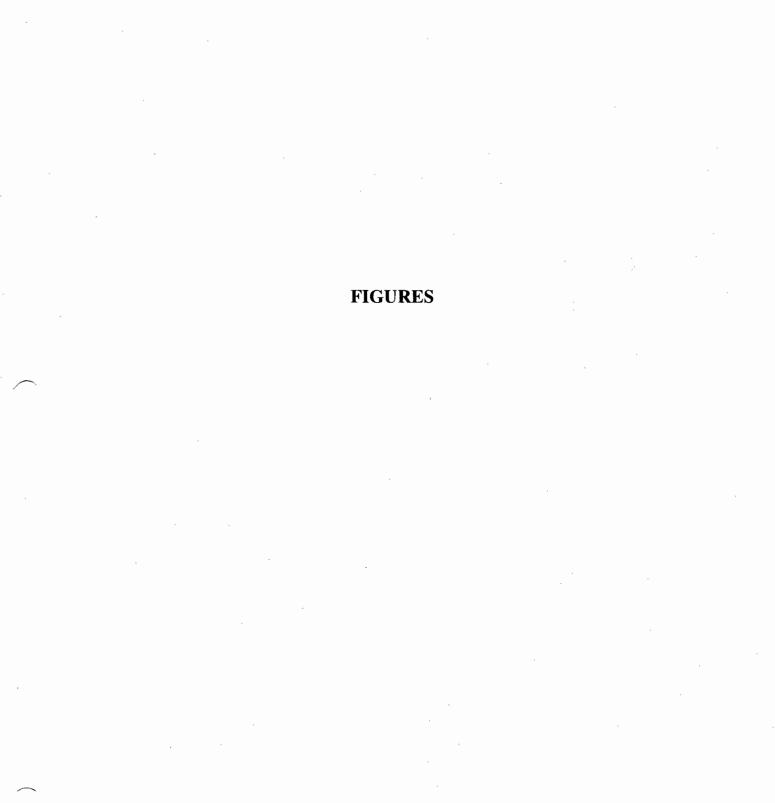
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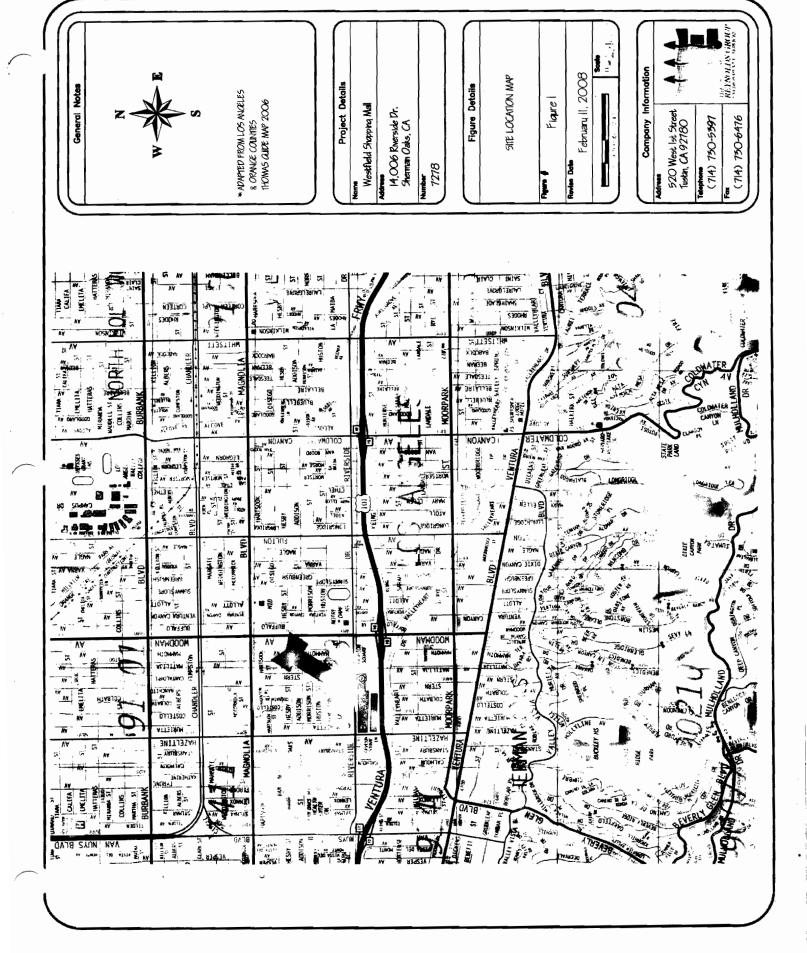
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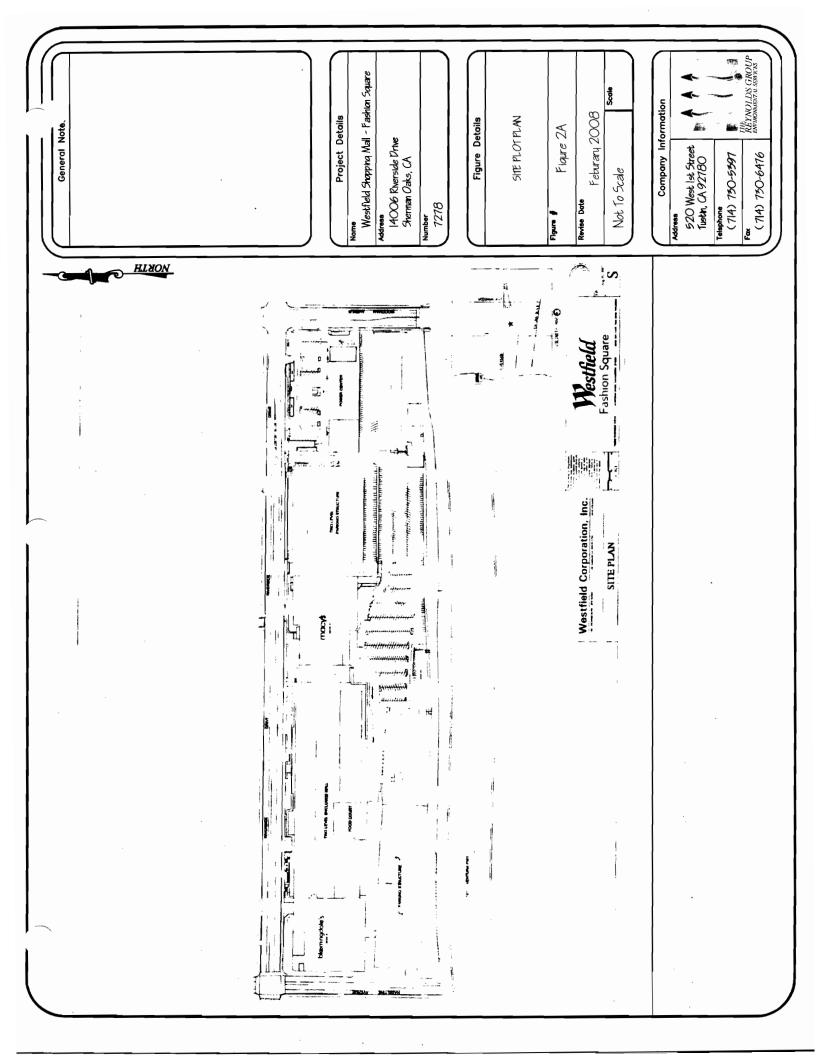
Page 33

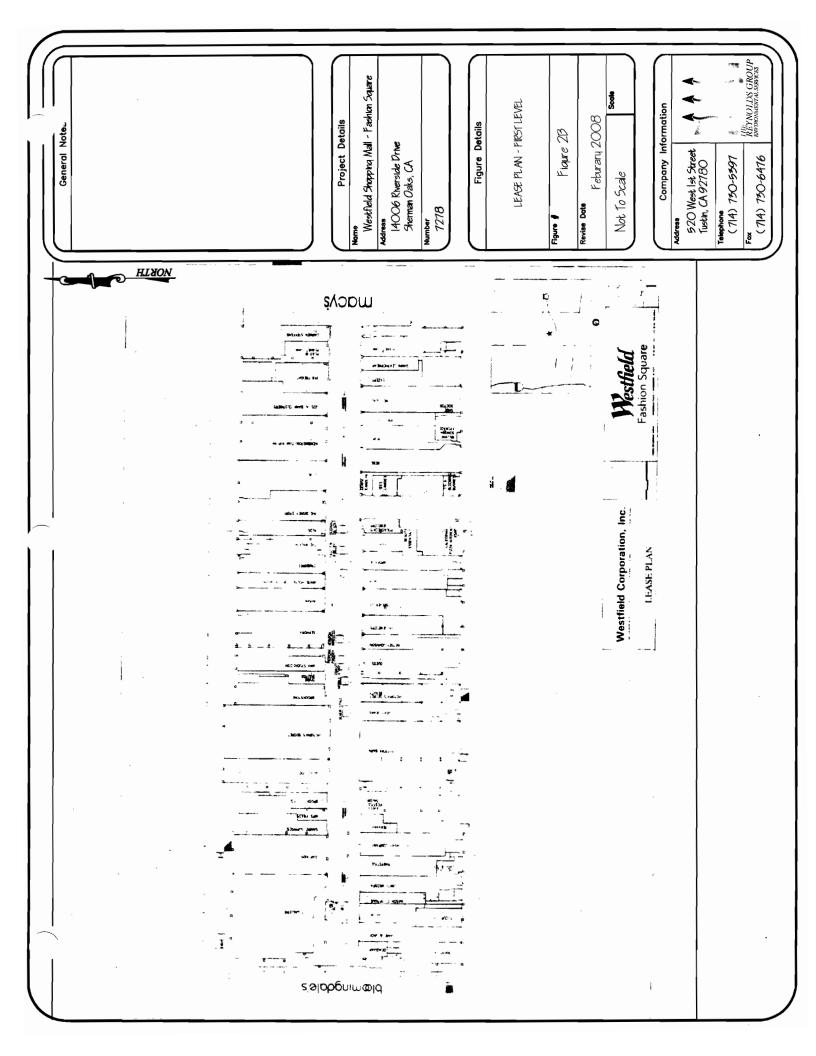
9.0 REFERENCES

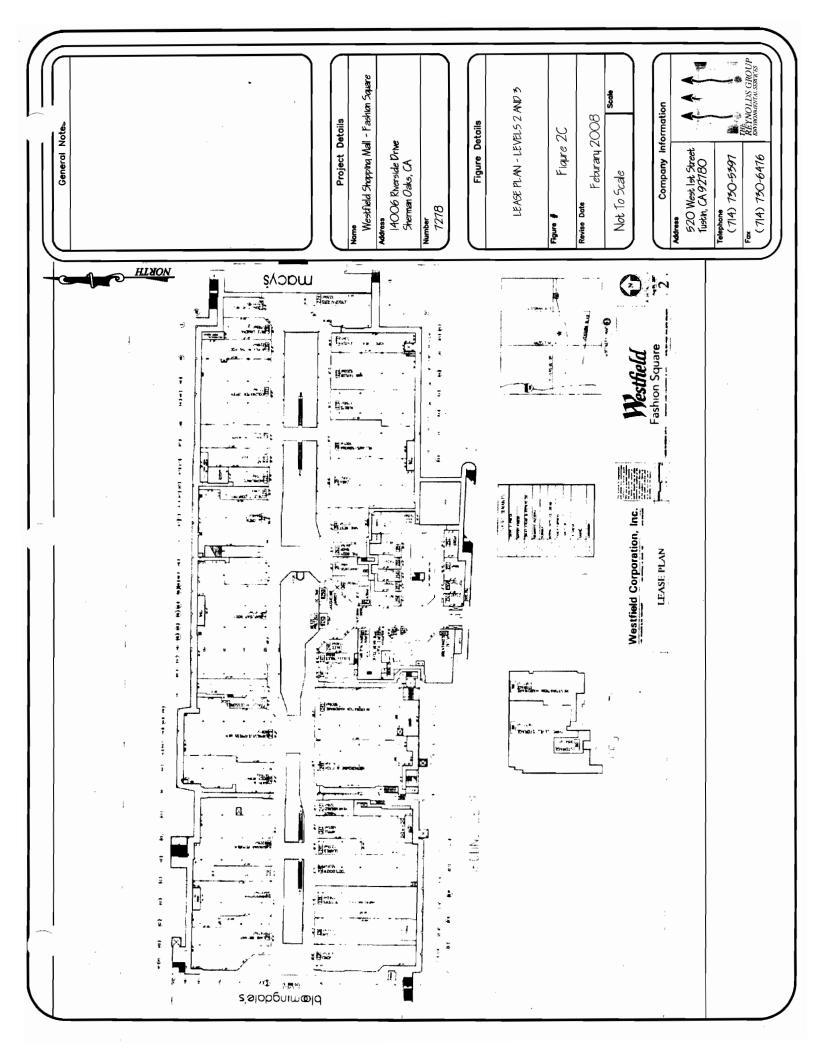
- 1. Thomas Guide, Los Angeles County Street Guide & Directory, 2003.
- 2. Environmental Data Resources, Inc., 440 Wheelers Farms Road, Millford, Connecticut 04616.
- 3. Los Angeles, California Building Department, 201 S. Figueroa Street, Los Angeles, California.
- 4. "Geotechnical Engineering Investigation Report" by Krazan & Associates, Inc., 4221 Brickell Street, Ontario, CA 91761
- 5. "Phase I Environmental Assessment" by Hillmann Environmental Group, LLC, 4510 East Pacific Highway, Suite 200, Long Beach, CA 90804













EDR Historical Topographic Map Report

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number: 2096148.4

December 10, 2007

The Standard in Environmental Risk Information

440 Wheelers Farms Rd Milford, Connecticut 06461

Nationwide Customer Service

Telephone:

1-800-352-0050

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1-800-231-6802

Internet:

www.edrnet.com

EDR Historical Topographic Map Report

Environmental Data Resources, Inc.s (EDR) Historical Topographic Map Report is designed to assist professionals in evaluating potential liability on a target property resulting from past activities. EDRs Historical Topographic Map Report includes a search of a collection of public and private color historical topographic maps, dating back to the early 1900s.

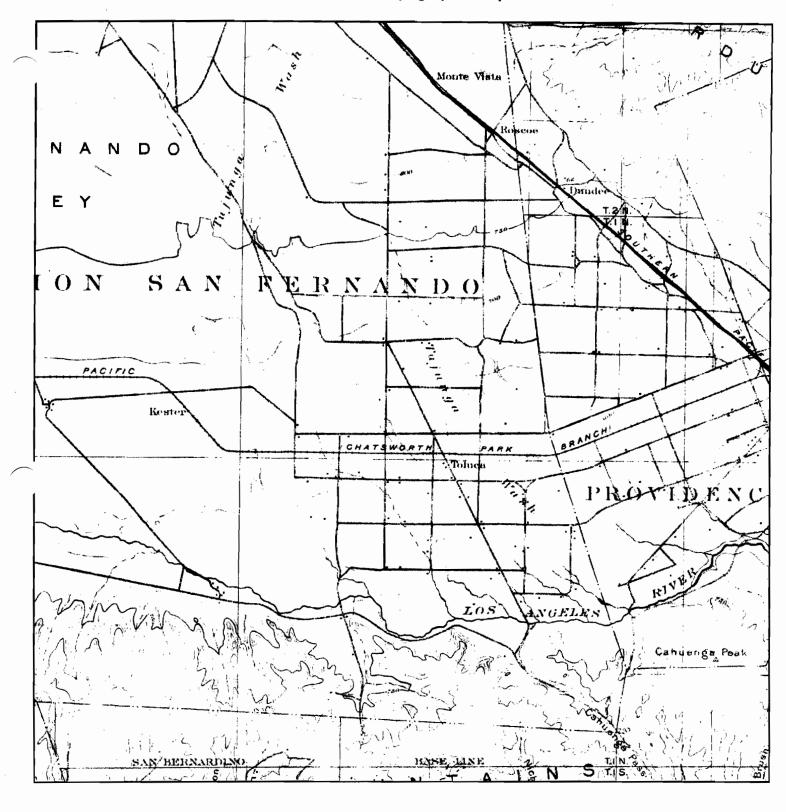
Thank you for your business.
Please contact EDR at 1-800-352-0050
with any questions or comments.

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N

TARGET QUAD

NAME: LOS ANGELES

MAP YEAR: 1900

SERIES:

SCALE:

S: 15 :: 1:62500 SITE NAME:

ME: Westfield Shopping Center

ADDRESS:

14006 Riverside Drive Sherman Oaks, CA 91423

LAT/LONG:

34.1568 / 118.436

CLIENT:

The Reynolds Group

CONTACT:

Gwen Tellegen

INQUIRY#: 2

2096148.4



TARGET QUAD NAME:

SOUTHERN CA SHEET 1

MAP YEAR: 1901

SERIES: SCALE:

60 1:250000

SITE NAME: Westfield Shopping Center 14006 Riverside Drive ADDRESS:

Sherman Oaks, CA 91423

LAT/LONG:

34.1568 / 118.436

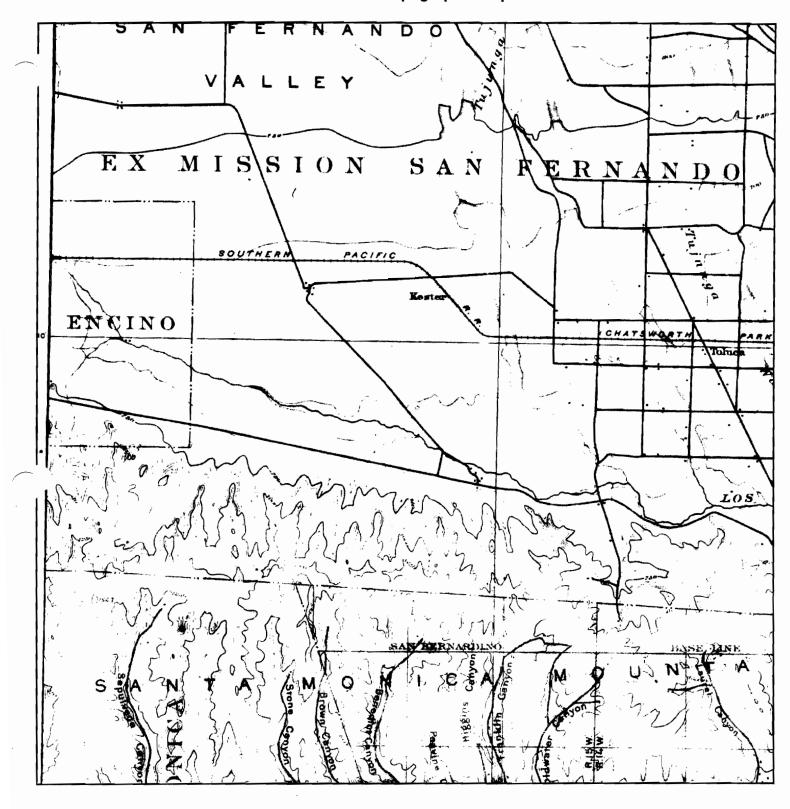
CLIENT:

The Reynolds Group

CONTACT:

Gwen Tellegen

2096148.4 INQUIRY#: **RESEARCH DATE: 12/10/2007**



TARGET QUAD

NAME: SANTA MONICA

MAP YEAR: 1902

SERIES:

15 SCALE: 1:62500

SITE NAME: Westfield Shopping Center

ADDRESS:

14006 Riverside Drive

Sherman Oaks, CA 91423

LAT/LONG: 34.1568 / 118.436 CLIENT:

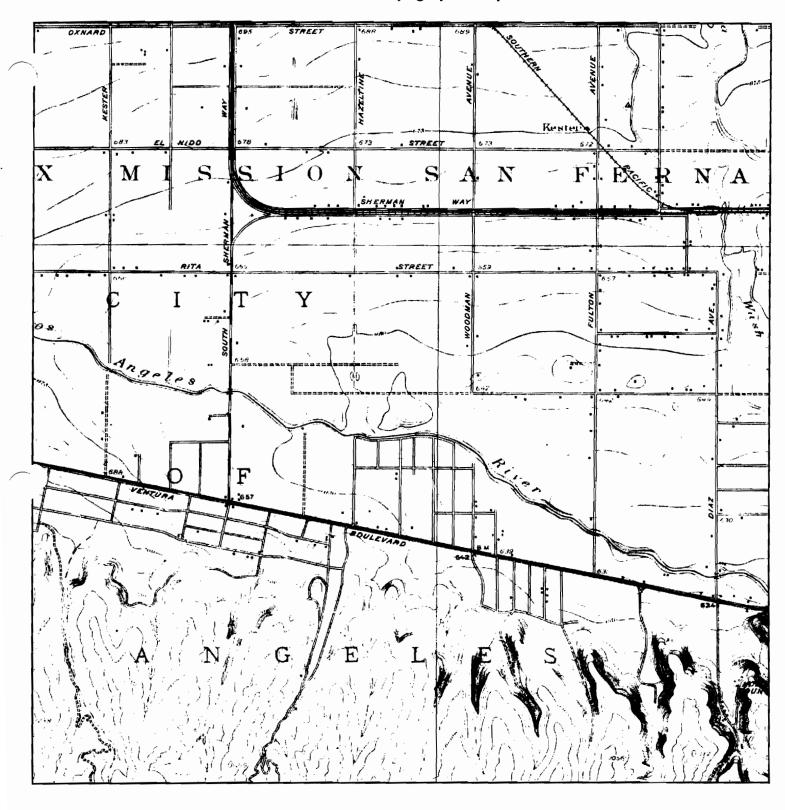
The Reynolds Group

CONTACT:

Gwen Tellegen

INQUIRY#:

2096148.4



TARGET QUAD

VAN NUYS NAME:

6

MAP YEAR: 1926

SERIES:

SCALE:

1:24000

SITE NAME: Westfield Shopping Center

ADDRESS:

14006 Riverside Drive

Sherman Oaks, CA 91423

LAT/LONG:

34.1568 / 118.436

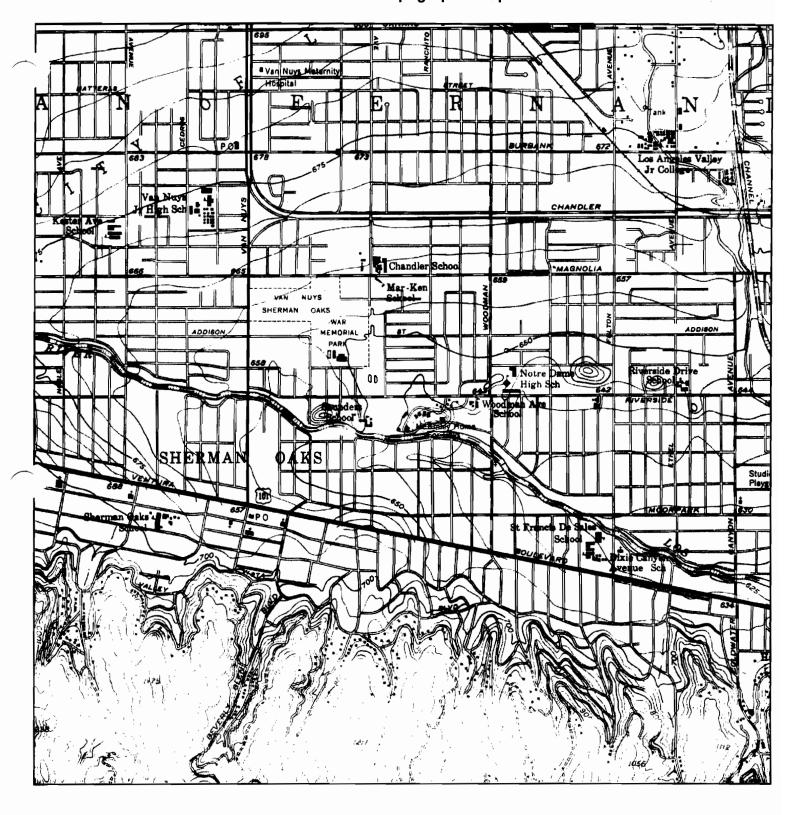
CLIENT:

The Reynolds Group

CONTACT:

Gwen Tellegen

INQUIRY#: 2096148.4



TARGET QUAD

NAME: VAN NUYS

MAP YEAR: 1953

SERIES:

7.5 1:24000 SCALE:

SITE NAME:

Westfield Shopping Center

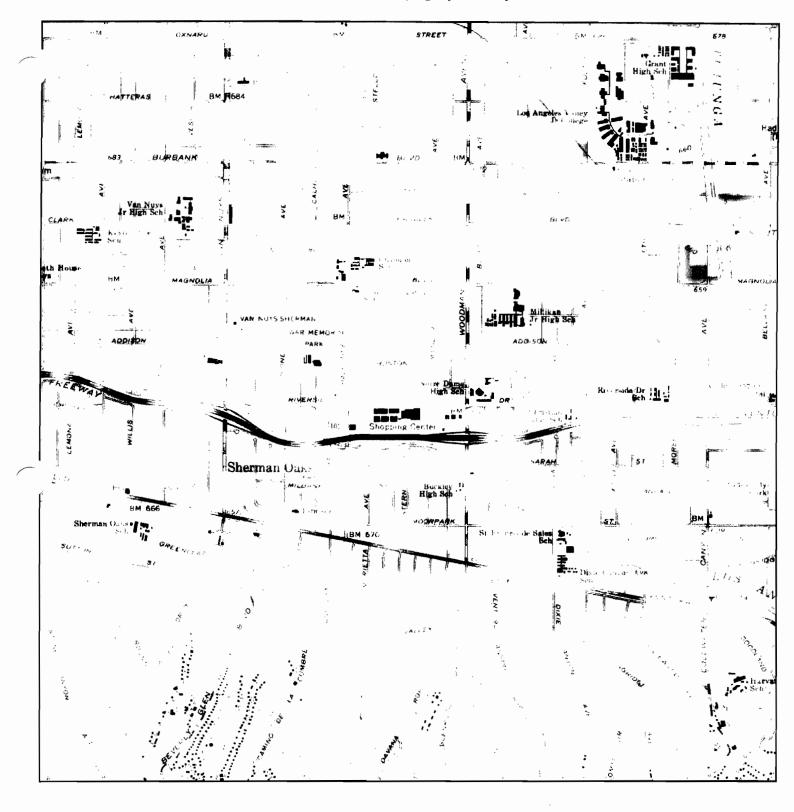
ADDRESS: 14006 Riverside Drive

Sherman Oaks, CA 91423

LAT/LONG: 34.1568 / 118.436 CLIENT: CONTACT: The Reynolds Group

Gwen Tellegen

2096148.4 INQUIRY#:



N N

TARGET QUAD NAME: VAN NUYS MAP YEAR: 1966

SERIES: 7.5 SCALE: 1:24000 SITE NAME: ADDRESS: Westfield Shopping Center

14006 Riverside Drive Sherman Oaks, CA 91423

LAT/LONG: 34.1568 / 118.436

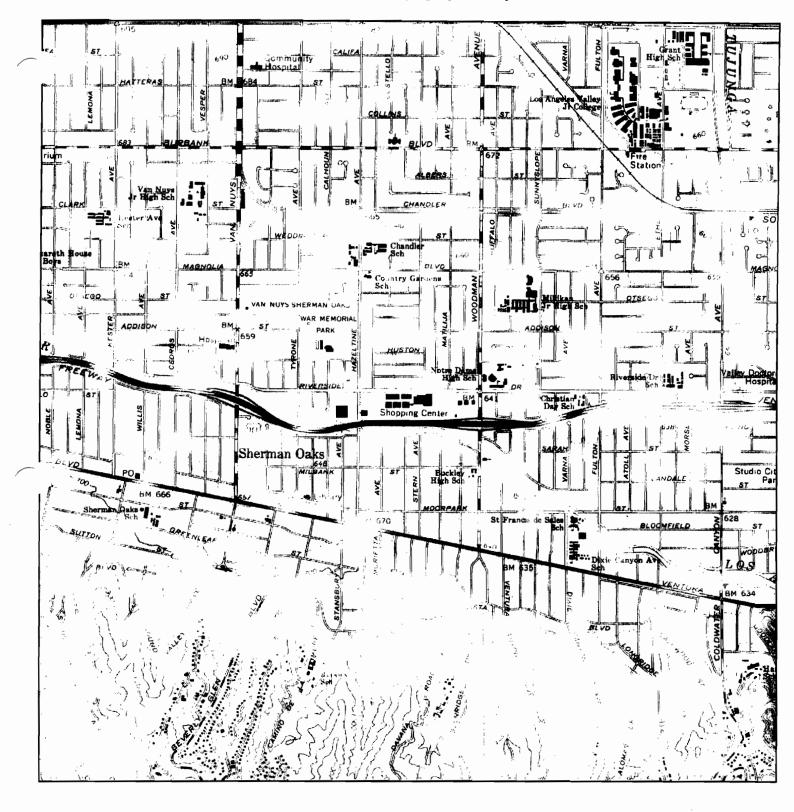
CLIENT:

The Reynolds Group

CONTACT: INQUIRY#:

Gwen Tellegen

INQUIRY#: 2096148.4 RESEARCH DATE: 12/10/2007



TARGET QUAD

NAME: **VAN NUYS MAP YEAR: 1972**

PHOTOREVISED FROM:1966

SERIES:

7.5

SCALE:

1:24000

SITE NAME: ADDRESS:

Westfield Shopping Center

14006 Riverside Drive

Sherman Oaks, CA 91423

LAT/LONG:

34.1568 / 118.436

CLIENT:

The Reynolds Group

CONTACT:

Gwen Tellegen

INQUIRY#:

2096148.4



The EDR Aerial Photo Decade Package

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number: 2096148.5

December 11, 2007

The Standard in Environmental Risk Information

440 Wheelers Farms Road Milford, Connecticut 06461

Nationwide Customer Service

Telephone:

1-800-352-0050

Fax:

1-800-231-6802

Internet:

www.edrnet.com

EDR Aerial Photo Decade Package

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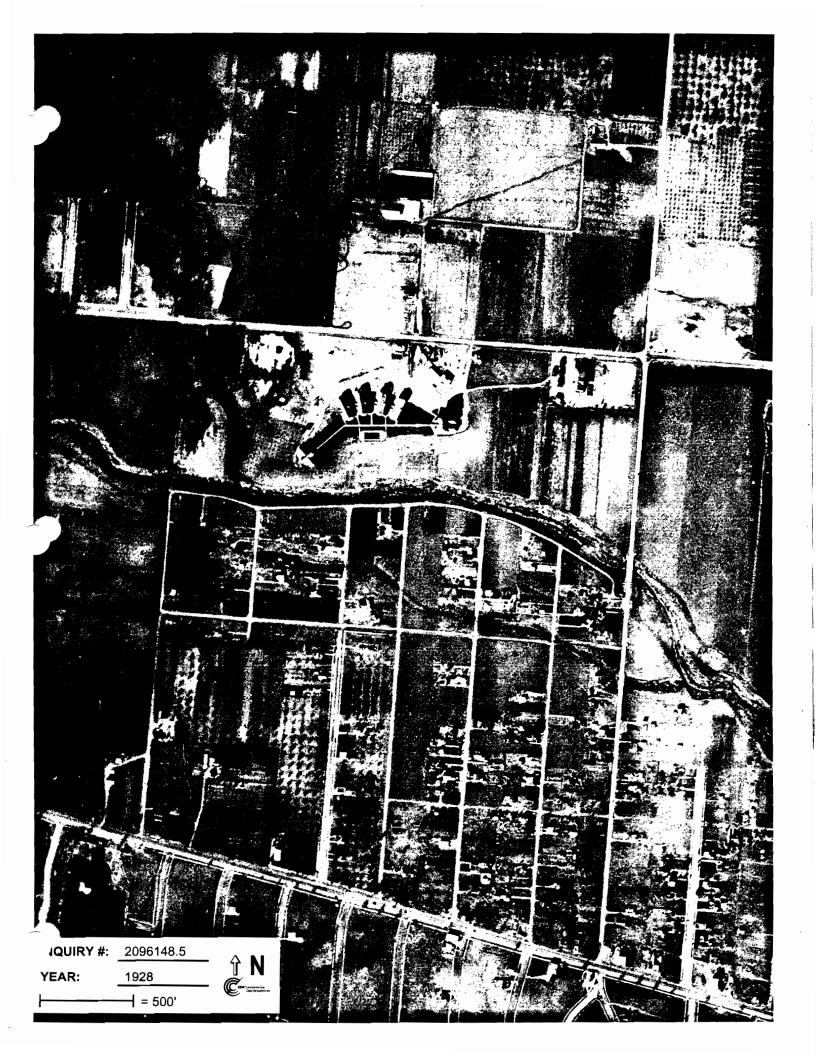
Date EDR Searched Historical Sources:

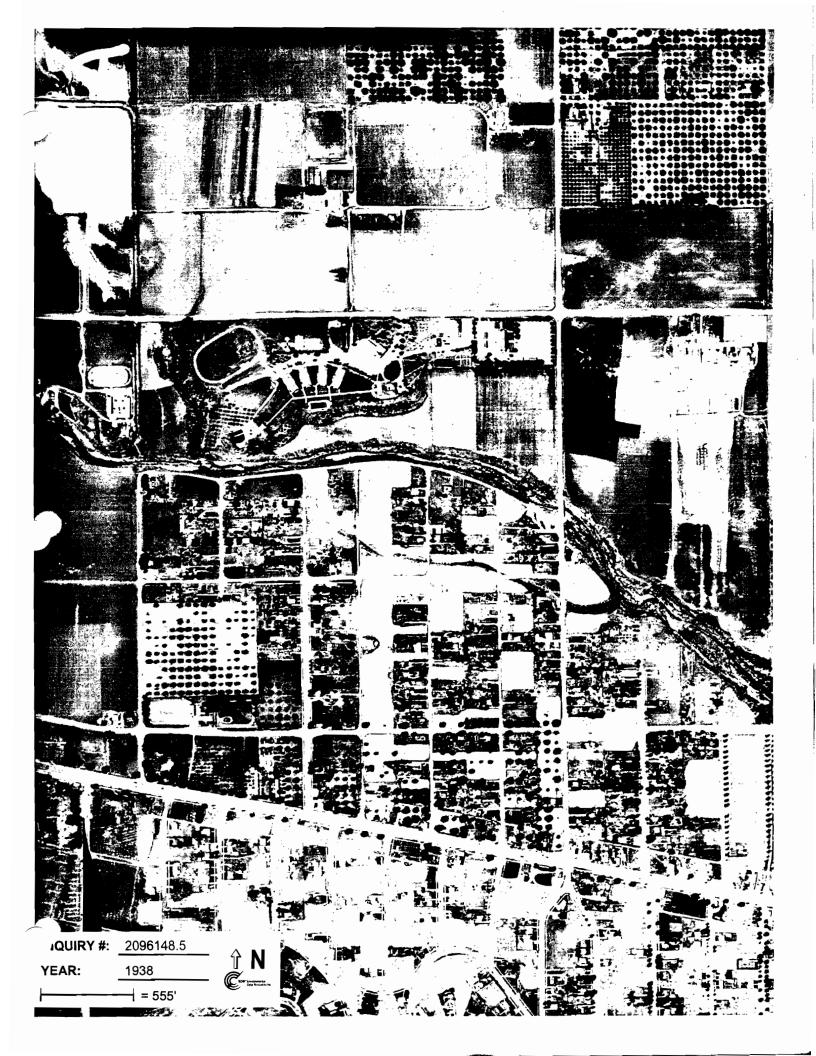
Aerial Photography December 11, 2007

Target Property:

14006 Riverside Drive Sherman Oaks, CA 91423

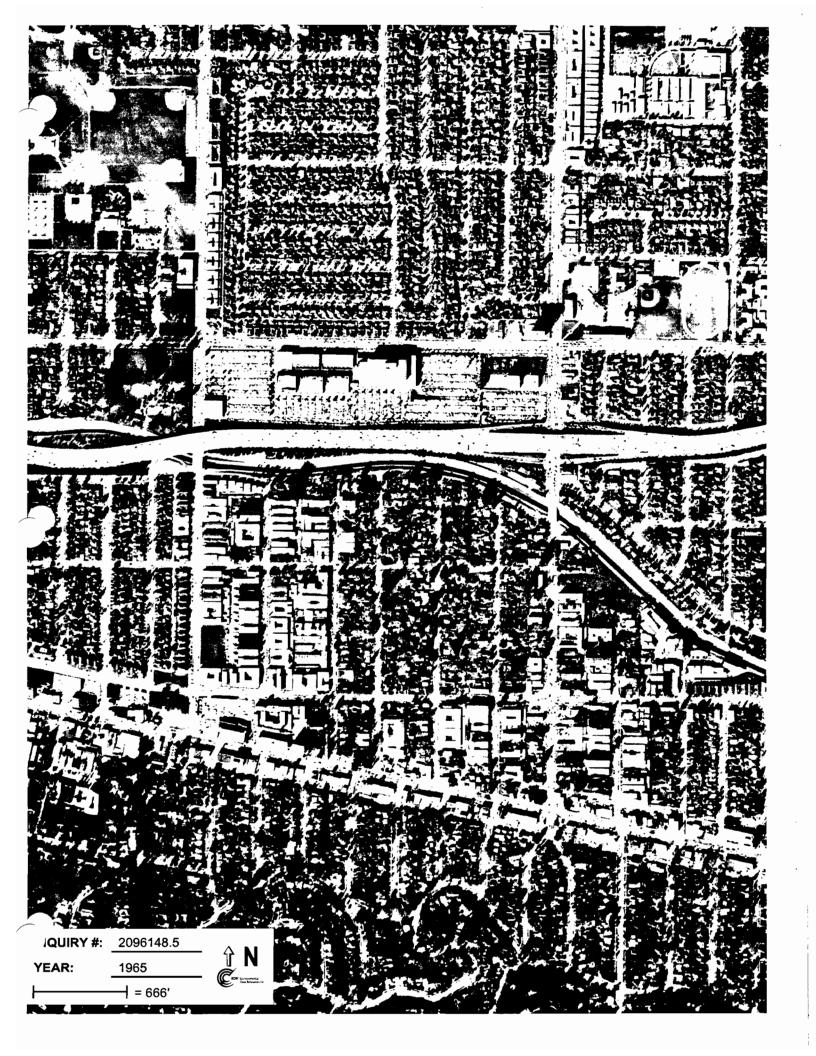
<u>Year</u>	Scale	<u>Details</u>	<u>Source</u>
1928	Aerial Photograph. Scale: 1"=500'	Flight Year: 1928	Fairchild
1938	Aerial Photograph. Scale: 1"=555'	Flight Year: 1938	Laval
1947	Aerial Photograph. Scale: 1"=666'	Flight Year: 1947	Tubis
1956	Aerial Photograph. Scale: 1"=400'	Flight Year: 1956	Fairchild
1965	Aerial Photograph. Scale: 1"=666'	Flight Year: 1965	Fairchild
1976	Aerial Photograph. Scale: 1"=666'	Flight Year: 1976	Teledyne
1989	Aerial Photograph. Scale: 1"=666'	Flight Year: 1989	USGS
1994	Aerial Photograph. Scale: 1"=666'	Flight Year: 1994	USGS
2002	Aerial Photograph. Scale: 1"=666'	Flight Year: 2002	USGS

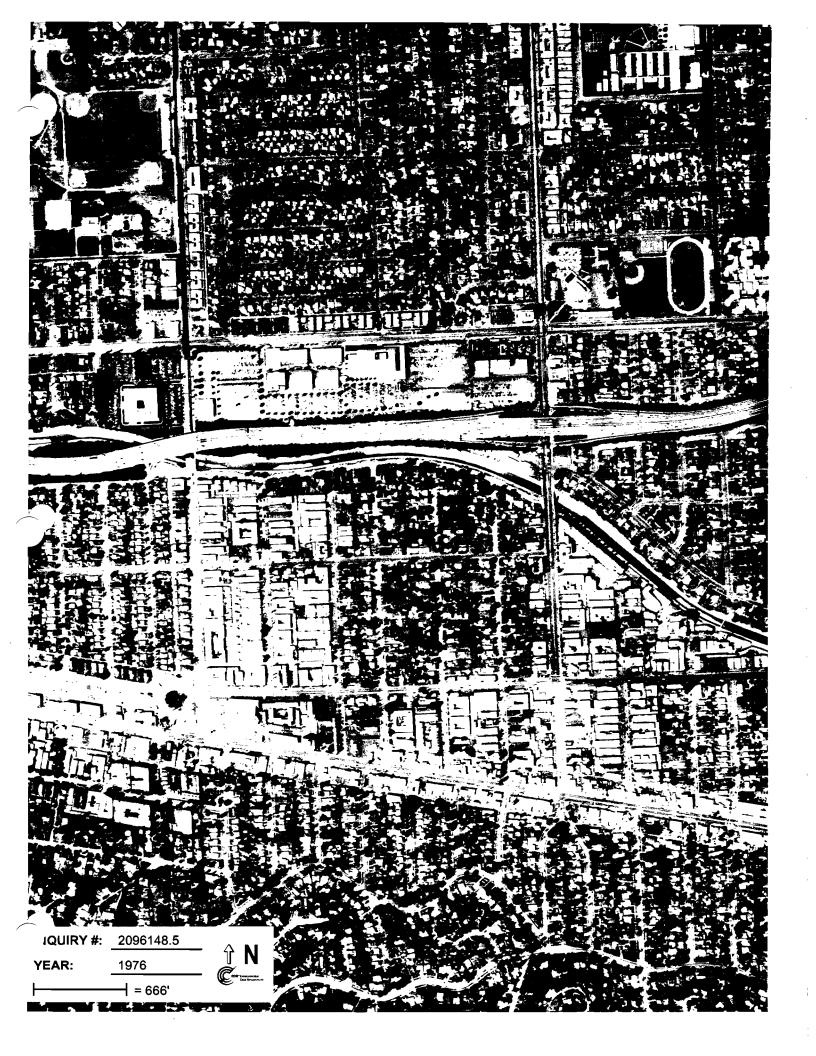


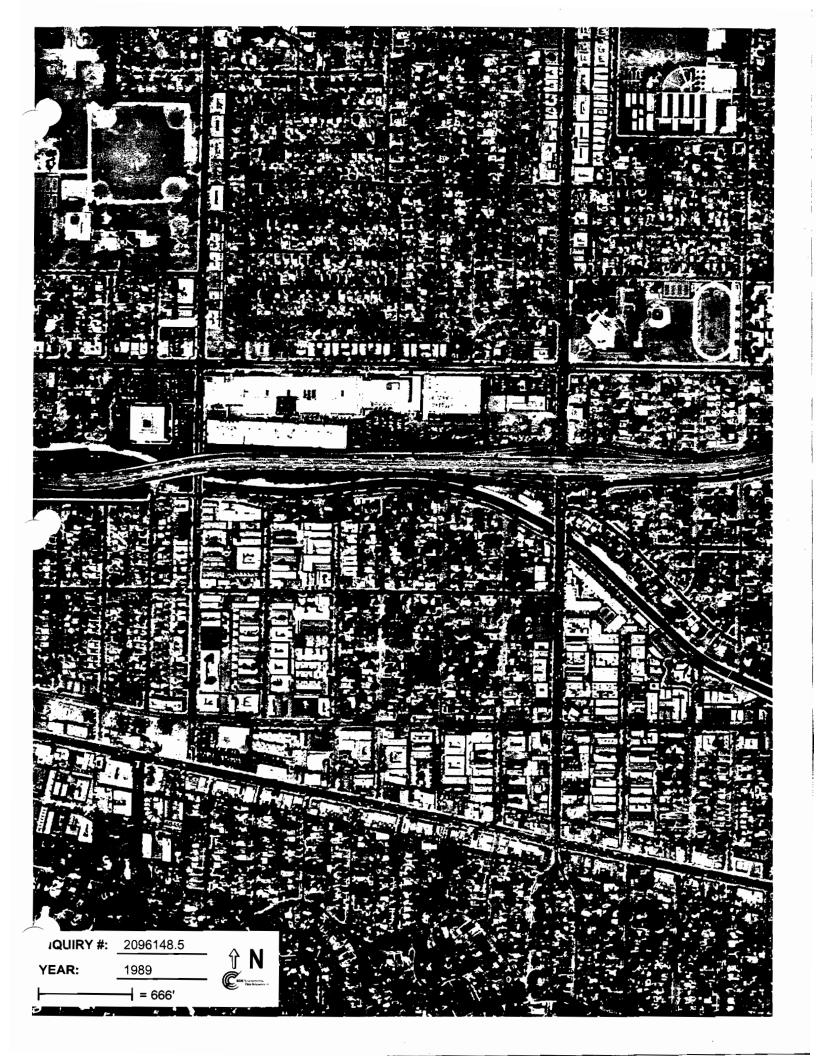


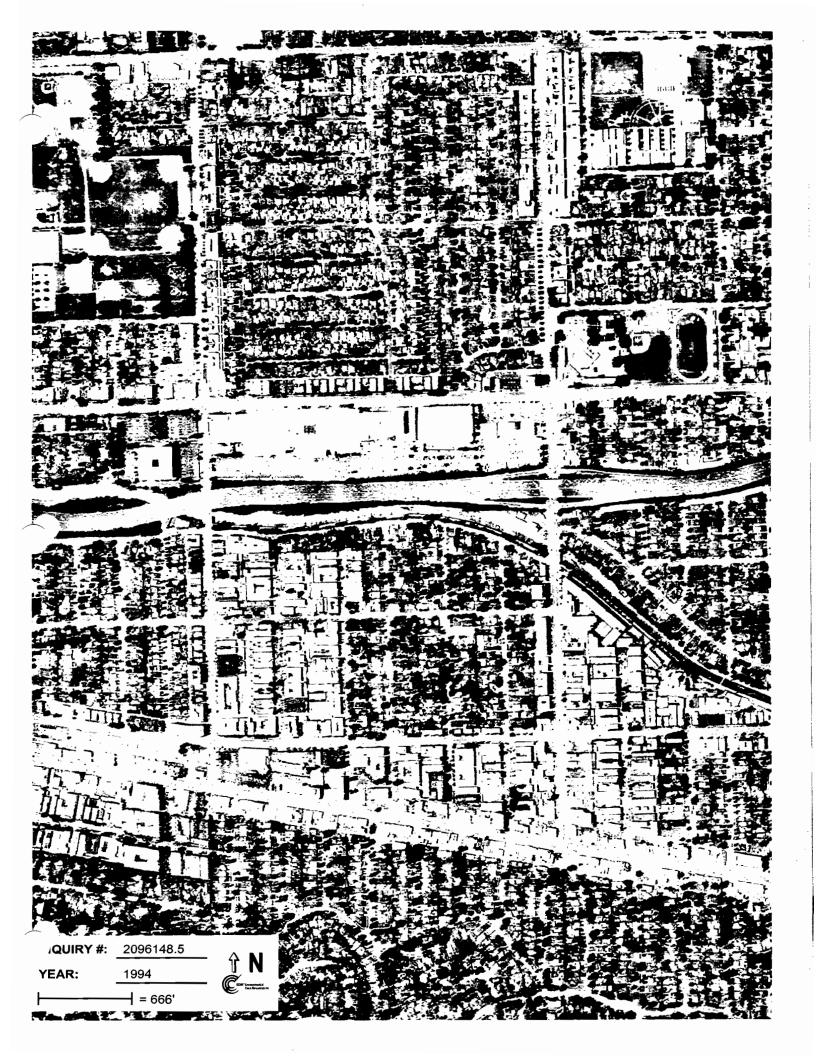














APPENDIX A

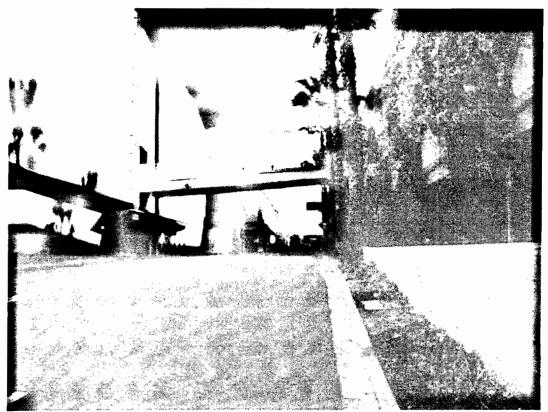
PHOTOGRAPHS OF PROPERTY & VICINITY TAKEN DURING SITE VISIT



Fashion Square Looking Northwest.



2. Parking Lot, Parking Garage and Fashion Looking West.



3. Driveway Entrance To Fashion Square From Hazeltine Avenue Looking East.



4. Typical View Inside Fashion Square.

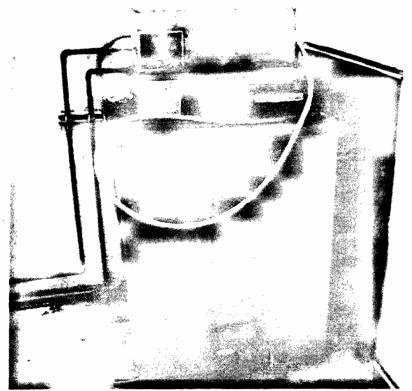
THE REYNOLDS GROUP
Fashion Square, 14006 Riverside Drive, Sherman Oaks, CA



5. Retail Building East End of Fashion Square Looking Southwest.



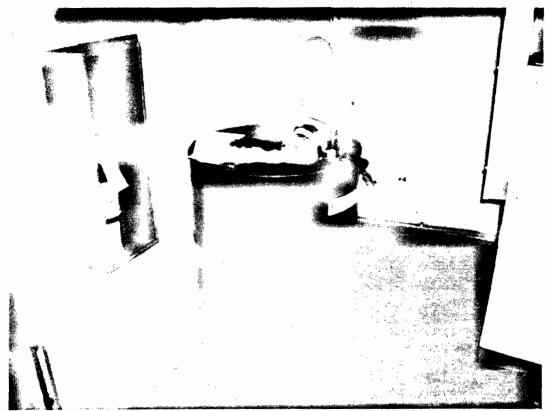
6. Parking Area South Side Of Fashion Square Looking West From Woodman Avenue.



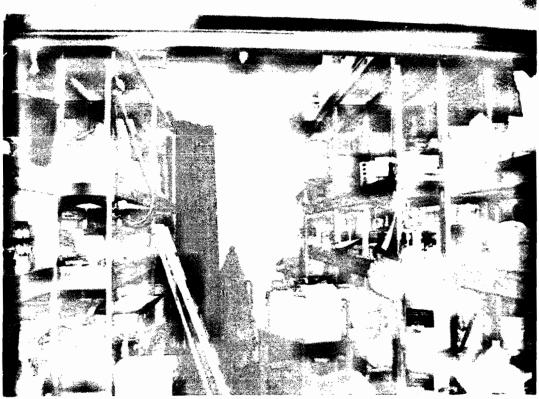
7. D. Tank Near Emergency Generator.



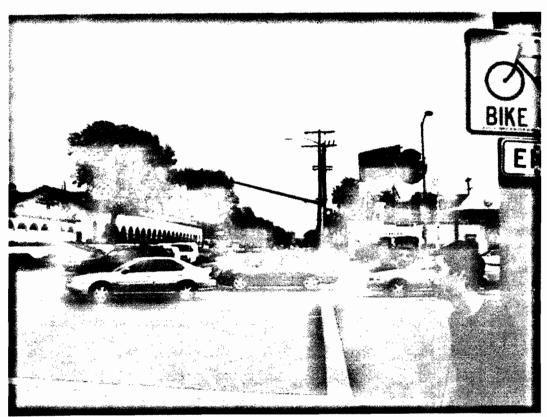
Two ()f The Trash Compactors On The Property.



9. Hydraulic Fuel Container in Elevator Equipment Room.



0. Maintenance Storage Room in Fashion Square.



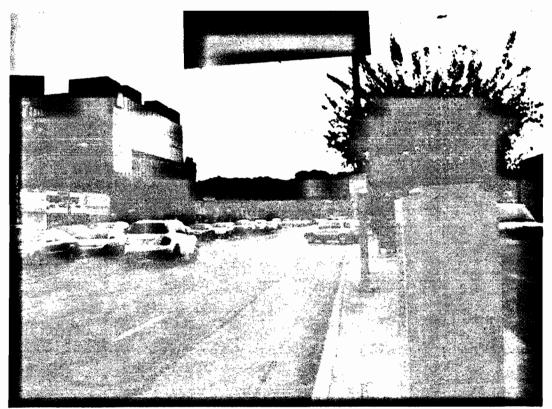
11. Riverside Drive Looking East From Woodman Avenue.



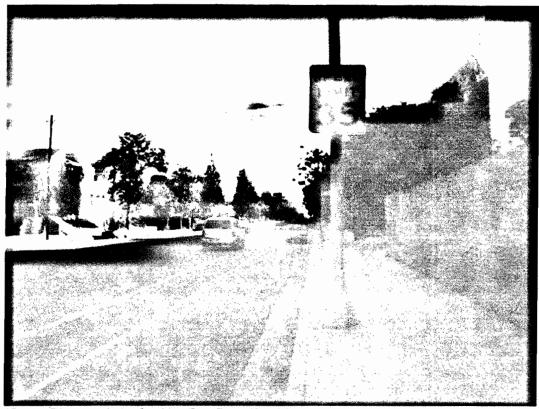
12. Riverside Drive Looking West From Woodman Avenue.



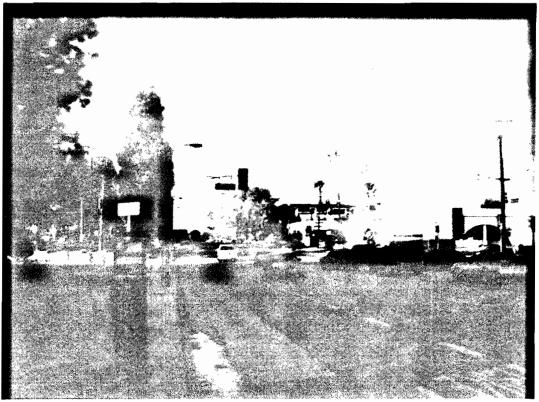
13. Woodman Avenue Looking North From Riverside Drive.



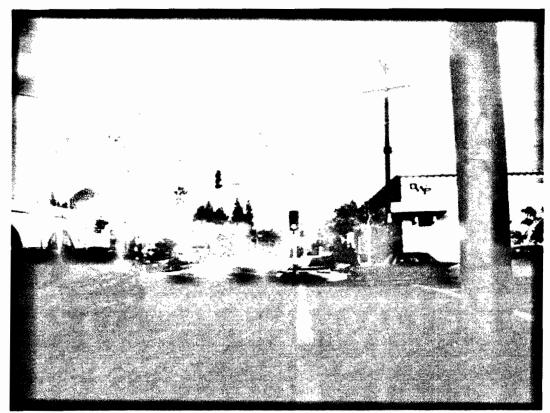
14. Woodman Avenue Looking South From Riverside Drive.



15. Riverside Drive Looking East From Hazeltine Avenue.



16. Riverside Drive Looking West From Hazeltine Avenue.



17. Hazeltine Avenue Looking North From Riverside Drive.



18. Hazeltine Avenue Looking South From Riverside Drive.

APPENDIX B

OWNER QUESTIONNAIRE RESPONSES

SITE ADDRESS: DATE OF RESPONSE: RESPONDENT NAME THE & TITLE: **REYNOLDS GROUP** A California Corporation RESPONDENT SIGNATURE: **CURRENT OWNER PHASE I QUESTIONS (USE** ADDITIONAL SHEETS IF NECESSARY TO COMPLETELY ANSWER THE **FOLLOWING QUESTIONS:** 1. WHO IS THE LEGAL OWNER/ENTITY OF THE PROPERTY? SHERMAN DAK FASHION ASSOCIATES, LLC 2. HOW LONG HAVE YOU OWNED THE PROPERTY? IF LESS THAN 5 YEARS, WHO WAS THE PREVIOUS OWNER? WESTFIELD BOUGHT THE PROPERTY DEC ZOOZ PLEASE PROVIDE HISTORICAL USES AT THE PROPERTY? 3. PRIOR THE SHOPPING CENTER SHOPPING LEMER SINCE 1961. SCHOOL ARE YOU AWARE OF ANY PREVIOUS ENVIRONMENTAL INVESTIGATIONS PERFORMED 4. AT THE PROPERTY? IF SO, PLEASE PROVIDE A COPY OF THE REPORT(S). NO, ONLY PREVIOUS PLASE I DO ANY WELLS, UNDERGROUND STORAGE TANKS, CLARIFIERS, OR HYDRAULIC LIFTS 5. EXIST AT THE PROPERTY? ANY CHEMICAL USE? ANY HAZARDOUS DUMPING? NO, TWO ABOVE GRADE STORAGE FOR ENTRY CONCY GENERATORS ARE YOU AWARE OF ANY EXISTING OR HISTORICAL ENVIRONMENTAL HAZARDS AT 6. THE PROPERTY? ASBES TOS SOME 7. DO ANY ENVIRONMENTAL LIENS, ENGINEERING CONTROLS OR LAND USE RESTRICTIONS EXIST FOR THE PROPERTY? NO DO YOU BELIEVE THE LISTED SALE PRICE OF THE PROPERTY IS "REASONABLE" AND 8. NOT NOTABLY DISCOUNTED DUE TO POTENTIAL ENVIRONMENTAL ISSUES? NO

Please e-mail or fax your response to me just as soon as possible and no later than **January 31, 2007**. You may send it to tellegen@reynolds-group.com or by fax to (760)743-7704. If you have questions, please feel free in calling at (949)743-8242. Thanks very much!

THE REYNOLDS GROUP a California corporation by:

Gwen Tellegen, P.E.

APPENDIX C

SANBORN FIRE INSURANCE MAP SEARCH, CITY DIRECTORY FINDINGS AND ENVIRONMENTAL LIEN SEARCH

Certified Sanborn® Map Report



Sanborn® Library search results Certification # C360-4679-A7E2

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number 2096148.3s

December 10, 2007



The Standard in **Environmental Risk** Information

440 Wheelers Farms Rd Milford, Connecticut 06461

Nationwide Customer Service

Telephone: 1-800-352-0050 1-800-231-6802

Fax: Internet:

www.edrnet.com

Certified Sanborn® Map Report

12/10/07

Site Name:

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Client Name:

The Reynolds Group 520 West 1st Street Tustin, CA 92780-0000

EDR Inquiry # 2096148.3s

Contact: Gwen Tellegen



The complete Sanborn Library collection has been searched by EDR, and fire insurance maps covering the target property location provided by The Reynolds Group were identified for the years listed below. The certified Sanborn Library search results in this report can be authenticated by visiting www.edrnet.com/sanborn and entering the certification number. Only Environmental Data Resources Inc. (EDR) is authorized to grant rights for commercial reproduction of maps by Sanborn Library LLC, the copyright holder for the collection.

Certified Sanborn Results:

Site Name:

Westfield Shopping Center

Address:

14006 Riverside Drive

City, State, Zip:

Sherman Oaks, CA 91423

Cross Street:

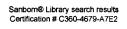
TBD Gwen

P.O. # Project:

TBD Gwen

Certification #

C360-4679-A7E2



Maps Identified - Number of maps indicated within "()"

1969 (1)

1966 (1)

1963 (1)

1960 (1)

1955 (1)

The Sanborn Library includes more than 1.2 million Sanborn fire insurance maps, which track historical property usage in approximately 12,000 American cities and towns. Collections searched:

Library of Congress

✓ University Publications of America

EDR Private Collection

Total Maps:

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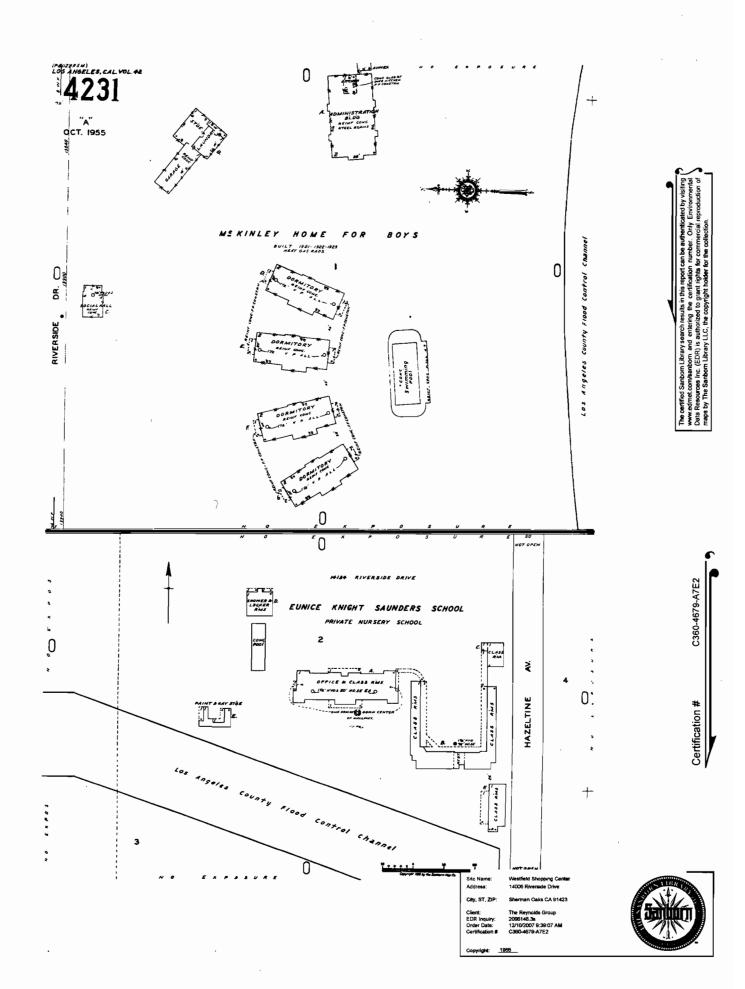
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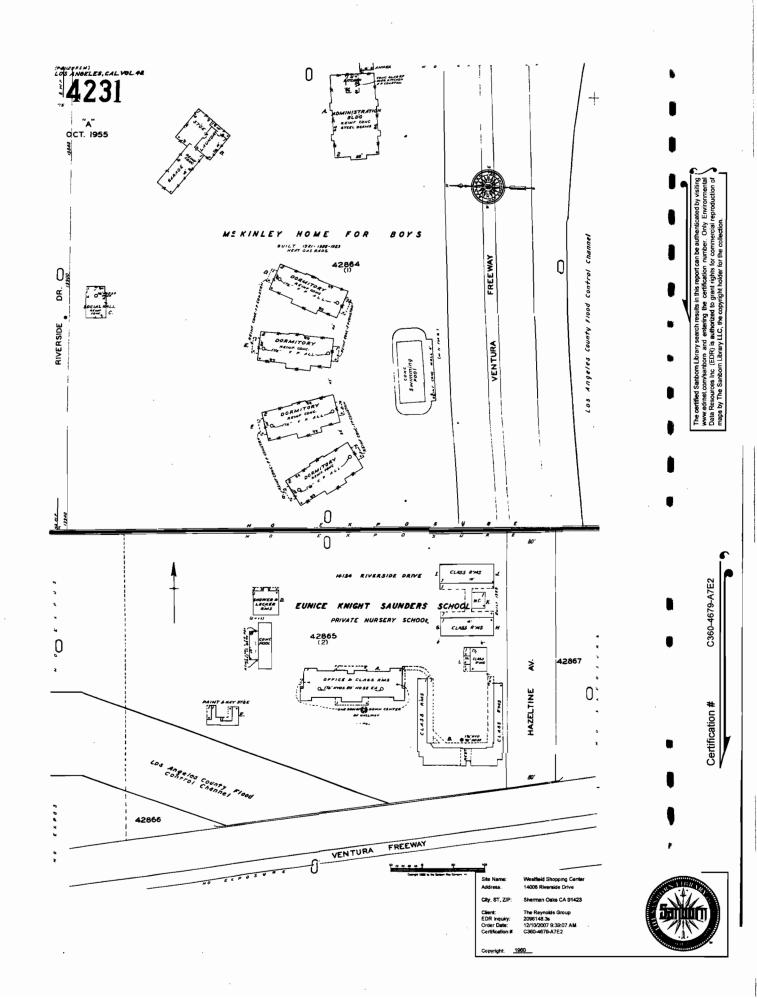
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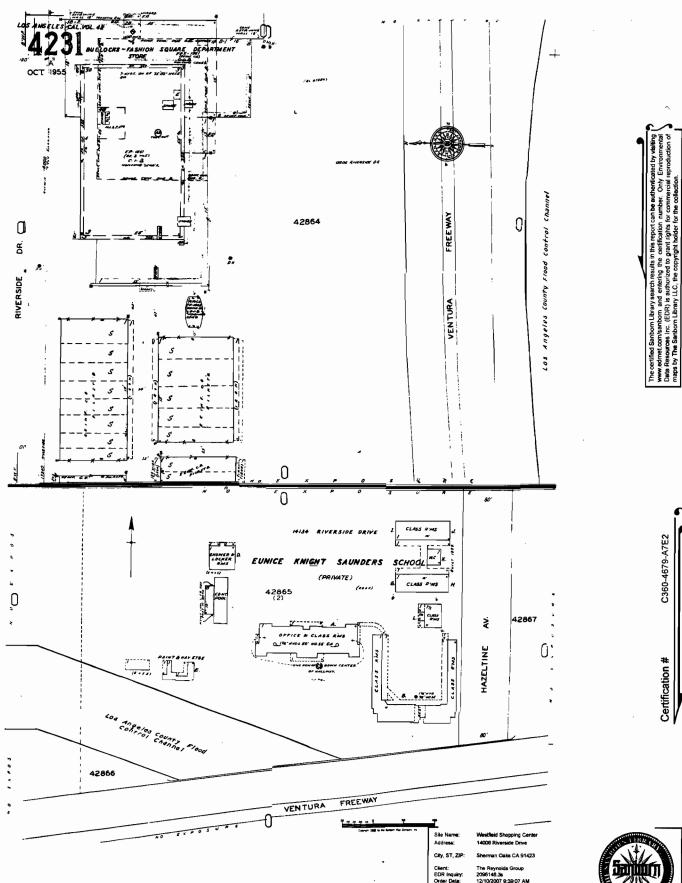
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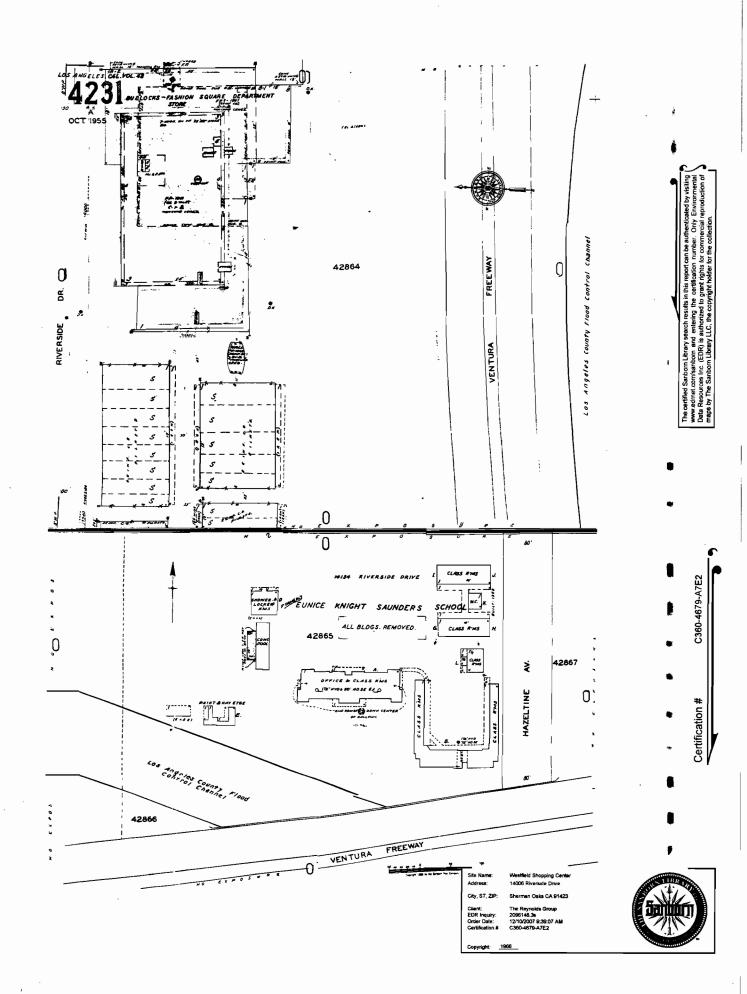
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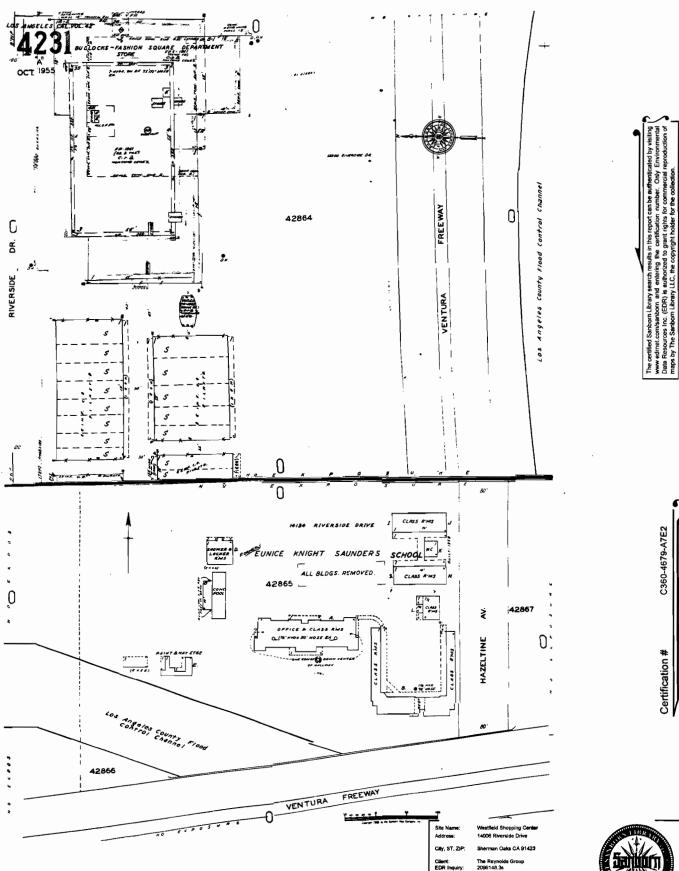
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The EDR-City Directory Abstract

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number: 2096148.6

Friday, December 07, 2007

The Standard in **Environmental Risk** Information

440 Wheelers Farms Road Milford, Connecticut 06461

Nationwide Customer Service

Telephone: 1-800-352-0050

Fax:

1-800-231-6802

Internet:

www.edrnet.com

EDR City Directory Abstract

Environmental Data Resources, Inc.'s (EDR) City Directory Abstract is a screening report designed to assist environmental professionals in evaluating potential liability on a target property resulting from past activities. EDR's City Directory Abstract includes a search and abstract of available city directory data. For each address, the directory lists the name of the corresponding occupant at five year intervals.

Thank you for your business.

Please contact EDR at 1-800-352-0050 with any questions or comments.

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SUMMARY

City Directories:

Business directories including city, cross reference and telephone directories were reviewed, if available, at approximately five year intervals for the years spanning 1920 through 2006. (These years are not necessarily inclusive.) A summary of the information obtained is provided in the text of this report.

This report compiles information by geocoding the subject properties (that is, plotting the latitude and longitude for such subject properties and obtaining data concerning properties within 1/8th of a mile of the subject properties). There is no warranty or guarantee that geocoding will report or list all properties within the specified radius of the subject properties and any such warranty or guarantee is expressly disclaimed. Accordingly, some properties within the aforementioned radius and the information concerning those properties may not be referenced in this report.

Date EDR Searched Historical Sources: December 7, 2007

Target Property:

14006 Riverside Drive Sherman Oaks, CA 91423

<u>Year</u>	<u>Uses</u>	Source
1920	Address Not Listed in Research Source	Los Angeles Directory Co.
1921	Address Not Listed in Research Source	Los Angeles Directory Co.
1923	Address Not Listed in Research Source	Los Angeles Directory Co.
1924	Address Not Listed in Research Source	Los Angeles Directory Co.
1925	Address Not Listed in Research Source	Los Angeles Directory Co.
1926	Address Not Listed in Research Source	Los Angeles Directory Co.
1927	Address Not Listed in Research Source	Kaasen Directory Company Publishers
1928	Address Not Listed in Research Source	Los Angeles Directory Co.
1929	Address Not Listed in Research Source	Los Angeles Directory Co.
1930	Address Not Listed in Research Source	Los Angeles Directory Co.
1931	Address Not Listed in Research Source	Los Angeles Directory Company Publishers
1932	Address Not Listed in Research Source	Los Angeles Directory Co.
1933	Address Not Listed in Research Source	Los Angeles Directory Co.
1934	Address Not Listed in Research Source	Los Angeles Directory Co.
1935	Address Not Listed in Research Source	Los Angeles Directory Co.
1936	Address Not Listed in Research Source	Los Angeles Directory Co.

<u>Year</u>	<u>Uses</u>	<u>Source</u>
1937	Address Not Listed in Research Source	Los Angeles Directory Co.
1938	Address Not Listed in Research Source	Los Angeles Directory Company Publishers
1939	Address Not Listed in Research Source	Los Angeles Directory Co.
1940	Address Not Listed in Research Source	Los Angeles Directory Co.
1942	Address Not Listed in Research Source	Los Angeles Directory Co.
1944	Address Not Listed in Research Source	R. L. Polk & Co.
1945	Address Not Listed in Research Source	R. L. Polk & Co.
1946	Address Not Listed in Research Source	Los Angeles Directory Co.
1947	Address Not Listed in Research Source	Pacific Directory Co.
1948	Address Not Listed in Research Source	Los Angeles Directory Co.
1949	Address Not Listed in Research Source	Los Angeles Directory Co.
1950	Address Not Listed in Research Source	Pacific Telephone
195 1	Address Not Listed in Research Source	Los Angeles Directory Co Publishers
1952	Address Not Listed in Research Source	Los Angeles Directory Co.
1954	Address Not Listed in Research Source	R. L. Polk & Co.
1 9 55	Address Not Listed in Research Source	R. L. Polk & Co.
1956	Address Not Listed in Research Source	Pacific Telephone
1957	Address Not Listed in Research Source	Pacific Telephone
1958	Address Not Listed in Research Source	Pacific Telephone
1960	Address Not Listed in Research Source	Pacific Telephone

1961 Address Not Listed in Research Source 1962 Address Not Listed in Research Source 1963 Address Not Listed in Research Source 1964 Address Not Listed in Research Source 1965 Address Not Listed in Research Source 1966 GTE	
1963 Address Not Listed in Research Source Pacific Telephone 1964 Address Not Listed in Research Source Pacific Telephone	
1964 Address Not Listed in Research Source Pacific Telephone	
, added the Edica in Account of the Control of the	
1965 Address Not Listed in Research Source GTE	
Additional that Electrical in Proposition Country	
1966 Address Not Listed in Research Source Pacific Telephone	
1967 Address Not Listed in Research Source R. L. Polk & Co.	
1969 Address Not Listed in Research Source Pacific Telephone	
1970 Address Not Listed in Research Source R. L. Polk & Co.	
1971 Address Not Listed in Research Source B&G Publications	
1972 Address Not Listed in Research Source R. L. Polk & Co.	
1975 Address Not Listed in Research Source Pacific Telephone	
1976 Address Not Listed in Research Source R.L. Polk & co Pul	blishers
1980 Address Not Listed in Research Source Pacific Telephone	
1981 Address Not Listed in Research Source Pacific Telephone	
1985 **RIVERSIDE DR** Pacific Bell ADEPT RESEARCH (14006) ADER D (14006) ADER HOWARD (14006) TATIANAS INTERIOR MOTIVES (14006)	
1986 Address Not Listed in Research Source Pacific Bell	
1990 **RIVERSIDE DR** Pacific Bell SHERMAN OAKS FASHION SQUARE SHERMAN OAKS (14006)	
1991 **RIVERSIDE DR** Pacific Bell	

Year Uses

Source

1991 (continued)

A PEA IN THE POD MATERNITY REDEFINED (14006)

A PEELING WAEPAPRREMVL (14006)

A PERFECT PEST CONTROL (14006)

A PERFECT PEST CONTROL CO INC (14006)

ACCESSORY LADY (14006)

ADEPT RESEARCH (14006)

ADMINISTRATIVE OFFICE (14006)

AEROPOSTALE (14006)

AEROPOSTALE TOPANGA PLAZA SHOPPING CENTER C PH (14006)

AEROSCOPIC ENGINEERS INC LA (14006)

AMYS HALLMARK (14006)

ANN TAYLORWOMENSAPPRD (14006)

ARTISTIC WEAR (14006)

BACHRACH CLOTHING (14006)

BEBE (14006)

BEBE EN VOGUE IL N BRAND BL GIN (14006)

BEN FRANKLIN ELECTRIC (14006)

BENETTON (14006)

BERNINI (14006)

BOMBAY COMPANY THE (14006)

BRANCH OFFICE (14006)

BRENTANOS (14006)

BROOKS SHOES FOR KIDS (14006)

COFFE E BE AN& TE LE L AF THE (14006)

CACHE (14006)

CALIFORNIA CRISP (14006)

CARITON HAIR INTERNATIONAL INC (14006)

CARITON J GHIS (14006)

CASTLEBY (14006)

CASUAL CORNER (14006)

CELIO LE BEAU TEMPS INC (14006)

CHAYO HAIR SALON (14006)

CIGNAIWOMLYENSAPPAREL (14006)

CITY FREEHOLDS LISA INC (14006)

CITY GIRL ESCORTS (14006)

CLAIRES BOUTIQUE (14006)

COPELANDS SPORTS (14006)

COPELCO LEASING CORP MEDCL EQPT LEASING (14006)

DE JAGER CONSTRUCTION INC (14006)

DEJAUN JEWELERS (14006)

DEVON BECKE (14006)

DI MATTIAS PIZZA & PASTA CAFE (14006)

1991 (continued)

EDDIE BAUER INC (14006)

ERIKS SHOE STORE (14006)

ERIKSEN DARRELL (14006)

ESPRIT (14006)

ETCETERA FASHION ACCESSORIES (14006)

EXPANDING WALL THE (14006)

EXPRESS INC (14006)

FAS HION S QUARE (14006)

FASHION SQUARE DELI (14006)

FASHION SQUARE SHERMAN OAKS (14006)

FIRSTLSSUE (14006)

FLORSHEIM SHOE SHOPS (14006)

FRENCH & CAJUN CAFE (14006)

FROM GLENDALE TELEPHONES CALL (14006)

FROM LA CRESCENTA TELEPHONES CALL (14006)

FUTURE TRONICS INC (14006)

GHQFOR MEN (14006)

GABYS JEWELRY (14006)

GABYS PLUMBING (14006)

GACH DOROTHY LMFCC (14006)

GODIVA CHOCOLATIER INC (14006)

GUESS FACTORY OUTLET (14006)

GYMBOREE STORE (14006)

HAN N AH A DIVIS ION OF FAS HION BAR (14006)

HANNAH AL (14006)

HANNAH CONSTRUCTION CO (14006)

HOFFRTZ FOR CUTLERY (14006)

HOLD EVERYTHING (14006)

HOWARD & PHILS WESTERN WEAR (14006)

IWISH (14006)

IMAGINARIUM (14006)

IMAGINARY CONCEPTS LTD (14006)

JACKS CLASSIC HAMBURGERS (14006)

JOHNSTON & MURPHY SHOP (14006)

JOHNSTON O (14006)

K FINE ART (14006)

K HONG CONSTRUCTION CO (14006)

K JACK ENGINEERING CO INC (14006)

LADY FOOTLOCKER (14006)

LEATHER LAND (14006)

LECHTERS HOUSEWARES (14006)

LEEDS SHOE STORES (14006)

LILLIE RUBIN (14006)

1991 (continued)

LIMITED THE (14006)

LIMITED THE SHERMAN OAKS GALLERIA SH OKS (14006)

LIMITED TOO THE (14006)

MARTIN LAWRENCE GALLERIES (14006)

NANCEE G (14006)

NATURAL WONDERS (14006)

NINE WEST (14006)

PAC SUNWEAR OF CALIF (14006)

PACIFIC EYES &TS (14006)

PANDA EXPRESS (14006)

PARADISE BAKERY (14006)

PEA IN THE POD MATERNITY REDEFINED (14006)

PEABOBY MD (14006)

PEABODY BRENT BARDEN (14006)

PEABODY K O (14006)

PIGEONS (14006)

PIGGOTT CHARLOTTE (14006)

POLACHECKS JEWELERS (14006)

POLACHEK B (14006)

POLACHEK DAVID SEPULVEDA (14006)

POLACHEK P (14006)

PORTER MCTEOD INC CONSTRUCTION SERVICES (14006)

PORTER MELISSA (14006)

PRESTIGIO (14006)

PRESTINE ENTERTAINMENT (14006)

RICHTERS MUSIC BOXES (14006)

RICHTERTAL DONNA & RONA (14006)

RICHVALSZKY JAMES (14006)

RICHWAY S (14006)

RICHWAYS TRADE INTEMATI INC NOR (14006)

ROCKS (14006)

ROYAL PHOTO STUDIO (14006)

SATURDAYS WORLD INC (14006)

SEBASTIANO (14006)

SEBASTIANS FINE SILK FLORALS (14006)

SEES CANDY SHOPS (14006)

SEESE HOWARD N (14006)

SEESE RONALD (14006)

SEESE S (14006)

SEESEE TOM & LIEN TUJ (14006)

SHERMAN OAKS (14006)

SHERMAN OAKS (14006)

Source

1991 (continued)

SHERMAN OAKS FASHION SQUARE ADMINISTRATIVE OFFICE

(14006)

SHERMAN OAKS FASHION SQUARE OFFICE (14006)

SHOE DOCTOR (14006)

SILVANO FASHION FOOTWEAR (14006)

SIMPLY STERLING (14006)

SOCKS DU JOUR (14006)

SPLENDIFEROUS (14006)

SPLETZER BOB (14006)

STORES (14006)

STREAMERS (14006)

SUFLSHINE BEAUTY SUPPLY (14006)

SUNSHINE BEAUTY SUPPLY TOPANGA PLAZA CPK (14006)

TAMARAS (14006)

TAMARAS WOMEON WEAMG APPRD NORTHRIDGE FASHION

CENTER NOR.349 (14006)

TEES (14006)

TEFFETELLER E E (14006)

THINGS REMEMBERED (14006)

THINGS REMEMBERED TOPANGA PTAZA SHOPPING CENTER

CPK .3407586 (14006)

THIS END UP FURNITURE CO (14006)

TIE RACK (14006)

TOPANGA PLAZA CPK P (14006)

WIKIDAL ERNEST (14006)

WIKL WIKI FOOD NC (14006)

WILLIAMS SONOMA INC (14006)

WILLIAMS SONYA (14006)

CITHRS (14006)

FROMTOS ANGELES TELEPHONES CAF (14006)

1995 **RIVERSIDE DR**

Pacific Bell Telephone

A PEA IN THE POD MATERNITY REDEFINED (14006)

ADEPT RESEARCH (14006)

AEROPOSTALE (14006)

AMYS HALLMARK (14006)

ANN TAYLOR WOMENS APPRI (14006)

ARST ALVIN J (14006)

ARTISTIC WEAR (14006)

AUDREY JONES (14006)

BARE CORP (14006)

BARE ESCENTUALS (14006)

BARE ESSENTUALS BARE ESCENTUALS (14006)

BEBE (14006)

BELL COTTAGE (14006)

1995 (continued)

BEN BRIDGE JEWELER (14006)

BENETTON (14006)

BOMBAY COMPANY THE (14006)

BROOKS SHOES FOR KIDS (14006)

BROOKS SHOES FOR KIDS (14006)

BROOKSTONE CO (14006)

BRUMELLE INC (14006)

BWARLES EMPORIUM (14006)

CACHE (14006)

CALIFORNIA CRISP (14006)

CARIMAR (14006)

CARLTON HAIR INTERNATIONAL INC (14006)

CASUAL CORNER (14006)

CIGNAL WOMENS APPAREL (14006)

CITY FREHOLDS USA INC (14006)

CLAIRES BOUTIQUES (14006)

COPELAND VAN K (14006)

DE VORE S Y CLTHRS (14006)

DEJAUN JEWELERS (14006)

DEVON BECKE (14006)

EDDIE BAUER INC (14006)

ERIKS SHOE STORE (14006)

EXPRESS HEATING & COOLING (14006)

EXPRESS INC (14006)

FAS HION S QUARE S HE RMAN OAKS (14006)

FASHION SQUARE DELI (14006)

FIRST INVESTORS TRUST (14006)

FREDERICKS OF HOLLYWOOD (14006)

FUN FACES FOTO GIFTS (14006)

GNC (14006)

GABYS JEWELRY (14006)

GAP KIDS (14006)

GENL NUTRITION CENTER (14006)

GLENDALE (14006)

GODIVA CHOCOLATIER INC (14006)

GOURMET PASTA CO IT THE (14006)

GUEE J ALFRED (14006)

GUESS (14006)

HOLD E VE RYTHIN G (14006)

HOW ARD & PHILS W E S TE RN W E AR (14006)

HANNAH A DIVISION OF FASHION BAR (14006)

HOME SHOP (14006)

I COPELANDS SPORTS (14006)

1995 (continued)

I STRUCTURE (14006)

IMAGINARIUM (14006)

J FIRST ISSUE (14006)

JACKS CLASSIC HAMBURGERS (14006)

JESSICA MC CLINTOCK BOUTIQUE (14006)

KITS CAMERAS (14006)

LADY FOOTLOCKER (14006)

LANE BRYANT (14006)

LECHTERS HOUSEWARES (14006)

LILLIE RUBIN (14006)

LIMITED THE (14006)

LIMITED TOO THE (14006)

MILLER & ASSOCIATES INC (14006)

MISSION RENAISSANCE ART CLASSES (14006)

NANCEE G (14006)

NINE WEST (14006)

NUT KETTLE THE (14006)

PAC SUNWEAR OF CALIF (14006)

PACIFIC EYES & TS (14006)

PANDA EXPRESS (14006)

PARADIS WILLIAM C (14006)

PASTILLE INC (14006)

PEA IN THE POD MATERNITY REDEFINED (14006)

POTTERY BARN (14006)

PRESTIGIO (14006)

RED EARTH BODY CARE INC (14006)

RICHTERS MUSIC BOXES (14006)

ROCKS (14006)

S (14006)

SATURDAYS WORLD INC (14006)

SHERMAN OAKS (14006)

SHERMAN OAKS FASHION SQUARE (14006)

SHERMAN OAKS FASHION SQUARE (14006)

SHERMAN OAKS FASHION SQUARE ADMINISTRATIVE OFFICE (14006)

SHOE DOCTOR (14006)

SILVANO FASHION FOOTWEAR (14006)

SIMPLY STERLING (14006)

STRUCTURAL MATERIALS CO (14006)

STRUCTURAL MATERIALS CO (14006)

SUNSHINE BEAUTY SUPPLY (14006)

SY DEVORE (14006)

THINGS REMEMBERED (14006)

Year Uses Source 1995 (continued) THIS END UP FURNITURE CO (14006) TIC TIME OF SHERMAN OAKS (14006) TIE RACK (14006) **VICTORIA JEWELERS (14006)** VICTORIAS SECRET (14006) WILLIAMS S ON OMA IN C (14006) WET SEAL THE (14006) WIGS TODAY (14006) WIKI WIKI FOOD INC (14006) I PARADISE BAKERY (14006) I STRUCTURE EXPRESS (14006) DEVORE SY CLTHRS SHEMAN OAKS (14006) GTE 1996 Address Not Listed in Research Source 1999 Address Not Listed in Research Source Haines Company 2000 Pacific Bell Telephone Address Not Listed in Research Source 2001 **RIVERSIDE DR** Haines & Company, Inc. ABERCROMBIE&FITCH (14006) ADEPTRESEARCH (14006) ANNA BELLA (14006) ANNTAYLOR (14006) ARTCLASSESMISSION (14006) ARTISTIC WEAR (14006) ARTISTICWEAR (14006) BALYS CLOTHING (14006) BATH&BODYWORKS (14006) **BEAUTY ESSENTIALS (14006) BELLCOTTAGE (14006)** BIG&TALL REPP LTD (14006) BISOUBISOU (14006) **BLOOMING BUSINESS (14006)** BODYSHOP (14006) **BOOKSTORE BRIGHTON (14006) BUILDERS INC JACQUELINEJARROT (14006) BUSINESS A PEAIN THE POD (14006)** BUSINESS J&R GAMBRELL (14006) CA CRISP (14006) CALIFORNIA (14006)

CALIFORNIA INC FASHION SQRDELI (14006)

CARLSJR (14006)

<u>Year Uses</u>

2001 (continued)

CASUAL CORNER (14006)

CENTER GEORGIOU (14006)

CENTERSERVICESINC (14006)

CHAYO HAIR SALON (14006)

CHRISTINA (14006)

CLASSES NAT 9 RALWONDERS A (14006)

CLE 00 CUCCIMARKET (14006)

CLOTHING HOLD EVERY (14006)

COFFEE BEAN&TE (14006)

COLLECTIBLES BROOKSSHOES FOR (14006)

DAVOUDZADEH CLAIRES BOUT 18 UES (14006)

DEJAUN JEWELERS (14006)

DISTRIBUTOR (14006)

DOCTORSOF (14006)

EUROPEAN ANTIQUE (14006)

EYEXAM 20080 F (14006)

FARMS&ORCHARDS CANDELABRA (14006)

FASHION S 9 UARE (14006)

FASHIONSQUARE (14006)

FLORIST FASHION S 9 UARE (14006)

FURNCOLLCTN EXPRESS INC (14006)

GALLERIES (14006)

GAME KEEPER THE (14006)

GAPKIDS (14006)

GAPTHE (14006)

GARDEN WORKS (14006)

GENERALNUTRITION (14006)

GEYORKIANLEO (14006)

GIFTS FSHN SOR ENZOANGIOLINI (14006)

GREAT (14006)

GROUP INC CATHYJEAN (14006)

GYMBOREESTORE (14006)

HA 6 R 9 MAKEOVER (14006)

HALLMARK (14006)

HANA FOODS INC (14006)

HEPBURNS RETAIL 819 986 001 S (14006)

HOMESHOP (14006)

HOUR PHOTO RIVERSIDE DR (14006)

HOUSEWARES LENSCRA (14006)

INC BEBE (14006)

INTERNATIONAL INC CAROLYN DENISE (14006)

ITS A BLOOMING (14006)

JOHNSON BETSEY (14006)

2001 (continued)

JUMPING DOG (14006)

KIDS BROOKSSHOES FOR (14006)

KIDS BROOKSTONECO (14006)

KIDSFOOTLOCKER (14006)

KITS CAMERAS ONE (14006)

LA SALSA (14006)

LADYFOOTLOCKER 818 720 B (14006)

LANEBRYANT B 187 M 9 25 S (14006)

LAVENDERS 818 7 M 4 316 I (14006)

LEAF THE COWBOYS&ANGELS (14006)

LECHTERS 818 08 M (14006)

LIMIT (14006)

LIMIT (14006)

MAISONDOP (14006)

MARTIN LAW (14006)

MASSISKABOB (14006)

METABOLIFE 81 7 BM 96 D I (14006)

MILLERS OUTPOST 818 A (14006)

MISSION RENSNCE ART (14006)

OPTOMETRY EDMUNDSUNIQUE (14006)

RESTAURANTS CARLTON HAIR (14006)

SHERMAN OAKS (14006)

SHERMAN OAKS FASHIONSQUARE (14006)

SHERMAN OAKS FOOTLOCKER (14006)

SHERMNOAKSFSHNSQ A BLOOMING (14006)

SKINAHAIR CARETHE BOMBAY COMPANY (14006)

STEAK&POTATOCO (14006)

STORE BANANA REPUBLIC (14006)

STORE BRENTANOS (14006)

SUPERSTORE ERIKSSHOE STORE (14006)

THE BOSE SHOWCASE (14006)

THE GUESSINC (14006)

2003 Address Not Listed in Research Source

Haines & Company

2004 Address Not Listed in Research Source

Haines Company

2006 **RIVERSIDE DR**

A BLOOMING (14006)

ACCESSORIES DIECASTWORLD (14006)

ALDO SHOES (14006)

AMER EAGLE (14006)

APPLE STORE (14006)

Haines Company

2096148-6

Source

2006 (continued)

ARDEN B (14006)

ASPECTBEAUTY (14006)

ASPECTBEAUTY (14006)

AVEDAKRIZA (14006)

B & M SILVER INC (14006)

BABYSTYLE (14006)

BANANA REPUBLIC (14006)

BATH & BODY (14006)

BEAUTY (14006)

BEAUTY WALKING CO THE (14006)

BLACK MARKET WIENERS (14006)

BLOOMING (14006)

BORDERS EXPRESS (14006)

BRIGHTON (14006)

BUSINESS ABERCROMBIE (14006)

BUSINESS BODYSHOPSKIN (14006)

BUSINESS J CREW (14006)

CARLTON HAIR (14006)

CAUF PAPYRUS (14006)

CHANNEL STORE DISNEY STORE THE (14006)

CHAYO HAIR SALON (14006)

CHICINTHECN TY (14006)

CINNABON (14006)

CLAIRES BOUTIQUE (14006)

CLARKS SHOES (14006)

CLOTHING IR SABLOOMING (14006)

CO Z GALLERIE (14006)

COACH STORE THE (14006)

COLDWATER (14006)

COLLECTIBLES BROOKS SHOES (14006)

CONSULTANTS RAMPAGE (14006)

COOKIES NAARTJIE CUSTOM (14006)

CREATIVE PARIDISE (14006)

CREEK COMFYFEET (14006)

CREPEX PRESS (14006)

DALUIACESSORIES (14006)

DEAGUIAR FINE (14006)

DEJAUN JEWELERS (14006)

DELL INC (14006)

DENISE CAROLYN (14006)

DISCOVERY (14006)

DIVMNE (14006)

EATERY SEPHORA (14006)

2006 (continued)

ENZOANGIOUNI (14006)

ERIKS SHOE STORE (14006)

ESSENTIALS INC BEBE (14006)

EXCEPT (14006)

EXPRESS INC (14006)

FASHION ASSO SHOEDOCTOR (14006)

FASHION SQUARE (14006)

FEFFERMAN ROBERT (14006)

FITCH ADVENTURE INC (14006)

FLORIST FASHION SQUARE (14006)

FOR KIDS BROOKSTONE CO (14006)

FOR KIDS BROOKS SHOES (14006)

GALLERY STORE TALBOTS (14006)

GAP L ODS (14006)

GENERAL (14006)

GEVORKIAN LEO (14006)

GIANNI (14006)

GO WIRELESS INC (14006)

GREATSTEAK (14006)

GROUP INC CATHYJEAN (14006)

HAIR CARE THE BOMBAY (14006)

HANDBAGS KIPUNG (14006)

HANDBAGS WILLIAMS SONOMA (14006)

HARDWARE RITZCAMERAONE (14006)

HARRIS BARBARA (14006)

HEPBURNS RETAIL (14006)

HOUR PHOTO RMATM (14006)

INTERNATIONAL INC CASUAL CORNER (14006)

INTERNATL SUPERSTEAM 4 U (14006)

J JILL THE STORE (14006)

JACQUELINE (14006)

JANIE AND JACK (14006)

JARROT JANES HALLMARK (14006)

JEWELERS DEJAUN JEWELERS (14006)

JEWELERS POTTERYBARN (14006)

JOHNSON BETSEY (14006)

JOSABANK (14006)

JOURNEYS (14006)

KENSINGTON (14006)

KIDS INC NEXTEL RETAIL (14006)

LADYFOOTLOCKER (14006)

LANE BRYANT (14006)

LASALS (14006)

2006 (continued)

LEATHERWEAR GNC (14006)

LOCCITANE (14006)

LOCH PATICK (14006)

LUGGAGE& (14006)

LUGGAGE& (14006)

LUMITY MEDSPA (14006)

MAISON DOPTIQUE (14006)

MASSIS KABOB (14006)

MOODY BEN (14006)

MRS FIELDS (14006)

NINEWEST (14006)

NUMBER (14006)

NUTRITION CENTER GEORGIOU RETAIL (14006)

OAKS FSHN SQ TOKYO GRILL (14006)

OCCHIAUDASOLE (14006)

OLEONGLEE (14006)

OSIMUSAINC (14006)

OUTFITTERS ANN TAYLOR LOFT (14006)

PACSUNWEAROF (14006)

PARFUMERIE (14006)

PETHAVEN (14006)

PIERCING PAGODA (14006)

PLANET FUNK (14006)

POLACHECKS (14006)

POTATO CO THE GUESS INC (14006)

PRETZELMAKER (14006)

PRETZELS WHITEHOUSE (14006)

PROFSNLSECURITY (14006)

RESTORATION (14006)

REYNOLDS MYRON (14006)

SALON&SUPPLY VERCITHE (14006)

SANDWICHES SUNGLASSHUT (14006)

SANRIO SURPRISES (14006)

SANSOIANL 81 IA (14006)

SARA&KATHYS (14006)

SBARRO ITALIAN (14006)

SERVICES SOLUTNS CALIFORNIACRISP (14006)

SHERMAN OAKS (14006)

SHERMAN OAKS FOOTLOCKER (14006)

SHERMAN OAKS T MOBILE SHERMAN (14006)

SHERMN OAKS FSHN SQ 818 FREIGHT (14006)

SHPPNG TWN FSHN (14006)

SQR WETZELS (14006)

Source

Year Uses 2006 (continued) SQUEEZE SWAROVSKI (14006) SUBWAY (14006) SUBWAY (14006) SUPERSTORE (14006) **SURF CITY (14006) TEAVANA (14006) TICTIME OF (14006)** TUMILUGGAGE (14006) VANITY FAIR (14006) VICTORIAS SECRET (14006) **VICTORIAS SECRET (14006)** VICTORIAS SECRET (14006) WESTFIELD (14006) WESTFIELD CORP (14006) WORKS BCBG (14006) **XOZONE (14006)** YANKEE CANDLE (14006) ZALES JEWELERS (14006) A ASATRYNRA IAYEL (14006)

Adjoining Properties SURROUNDING

A TAYLOR MICHAEL (14006)

Multiple Addresses Sherman Oaks, CA 91423

<u>Year</u>	<u>Uses</u>	Source
1920	Address Not Listed in Research Source	Los Angeles Directory Co.
1921	Address Not Listed in Research Source	Los Angeles Directory Co.
1923	Address Not Listed in Research Source	Los Angeles Directory Co.
1924	Address Not Listed in Research Source	Los Angeles Directory Co.
1925	Address Not Listed in Research Source	Los Angeles Directory Co.
1926	Address Not Listed in Research Source	Los Angeles Directory Co.
1927	Address Not Listed in Research Source	Kaasen Directory Company Publishers
1928	Address Not Listed in Research Source	Los Angeles Directory Co.

<u>Year</u>	<u>Uses</u>	Source
1929	Address Not Listed in Research Source	Los Angeles Directory Co.
1930	Address Not Listed in Research Source	Los Angeles Directory Co.
1931	Address Not Listed in Research Source	Los Angeles Directory Company Publishers
1932	Address Not Listed in Research Source	Los Angeles Directory Co.
1933	Address Not Listed in Research Source	Los Angeles Directory Co.
1934	Address Not Listed in Research Source	Los Angeles Directory Co.
1935	Address Not Listed in Research Source	Los Angeles Directory Co.
1936	Address Not Listed in Research Source	Los Angeles Directory Co.
1937	Address Not Listed in Research Source	Los Angeles Directory Co.
1938	Address Not Listed in Research Source	Los Angeles Directory Company Publishers
1939	Address Not Listed in Research Source	Los Angeles Directory Co.
1940	Address Not Listed in Research Source	Los Angeles Directory Co.
1942	Address Not Listed in Research Source	Los Angeles Directory Co.
1944	Address Not Listed in Research Source	R. L. Polk & Co.
1945	Address Not Listed in Research Source	R. L. Polk & Co.
1946	Address Not Listed in Research Source	Los Angeles Directory Co.
1947	Address Not Listed in Research Source	Pacific Directory Co
1948	Address Not Listed in Research Source	Los Angeles Directory Co.
1949	Address Not Listed in Research Source	Los Angeles Directory Co.
1950	**HUSTON RD**	Pacific Telephone

1950 (continued)

WRIGHT JOHN WM R (13958)

GORDON AARON R (13959)

SHELDON PETER J R (13964)

DEVEY ALFRED J MRS R (13965)

HELGESON CLARENCE E R (13970)

SHANK WAYNE C R (13971)

ALTMAN ROBT R (14000)

CANTZ MARVIN S R (14006)

BOUQUET JOS A JR R (14007)

POSEN SAM R (14012)

IGO RICHARD A R (14013)

D ORAZIO R J R (14018)

ATKINSON GEO H R (14024)

SMITH STANLEY J R (14025)

FREDERICK LEONARD L R (14031)

LA MAIDA \$T

HILL HUGH E R (13928)

JOHNSON STELLA CARRELL R (13929)

BAROWITZ LEON R (13934)

HEINEN HILDRETH R (13935)

LOGAN JOHN PR (13940)

BECKER V A R (13941)

BAYLEY H M JR R (13946)

KAMM ALBERT R (13947)

MCCALLON LYNN R (13952)

SUGARMAN EUGENE R (13953)

DORR WALTER O R (13958)

ERWIN RALPH B R (13959)

LENARD ROBT R (13964)

DAMBACHER EMMETT N JR R (13970)

COLLETTI VINCENT PR (13971)

WINEGARDNER D E R (14000)

TROULMAN ROBT W R (14001)

MILLER WM I R (14006)

NATER SAM MRS R (14007)

PETERS J W R (14012)

MANN MAX E R (14018)

WALLACE MARLENE R (14024)

WILKINSON HERBERT D R (14030)

VENN HENRY G R (14031)

MURIETTA AVE

JENKINS DONALD R R (4801)

ROBINSON WM C R (4807)

<u>Source</u>

Pacific Telephone

1950 (continued)

JUILLARD LOUIS GR (4813)

CARTER EUGENE M R (4819)

MCMICHAEL JUDD R (4825)

HAND FRANK E R (4837)

COHEN E H R (4843)

LOCHER MILDAN C R (4849)

MEREDITH M W R (4855)

HAMOVITZ RAY R (4901)

LOWELL MANUEL R (4907)

HANCOCK W L R (4913)

MCCARTHY JOHN E R (4919)

PEACH GROVE ST

FELDMAN BERNARD L R (13928)

PAICH MARTIN L R (13934)

WICKSTROM CARL C R (13935)

TROULMAN JACK R (13940)

HULL KENNETH W R (13941)

EGGETT GORDON E R (13946)

WHITNELL C E R (13947)

BURNS JAS W R (13952)

KUHSE ORVILLE H R (13953)

MARREN MILTON R (13958)

VELLOZZI LARRY R (13959)

NELSON ROBT O R (13965)

SUGARMAN JACK A R (13970) ROCKWELL ROBT G R (13971)

SHERMAN MILTON MRS R (14000)

KERZIN LESTER N R (14001)

TRAPANI JOHN F R (14006)

MOSS MITCHELL R (14007)

CASTELLI CHAS R (14013)

LAYTON ROBT O R (14018)

BEALL GEO R R (14019)

HINCHCLIFFE GORDON R R (14019)

SELLECK ROBT D R (14024)

DEMETROPOULOS ROSEMOND MRS R (14025)

STORM PHILLIP J R (14030)

WOOD JOS F MRS R (14031)

RIVERSIDE DR

COCHRAN EARL G DR R (13911)

HILLEY JACK R (13919)

SCHWARTZ JACK R (13925)

SHILMAN ALFRED E R (13931)

Source

Pacific Telephone

1950 (continued)

LEVENTHAL BERNARD B (13937)

SCOTT TED M R (13941)

SFORZINI ROBT A R (13943)

WALKER CHRISTABEL B R (13949)

LATT SAM R (13965)

NUGER EDW R (13967)

GRIBIN IRA R (13973)

EDMOND JEROME R (14003)

WILLIAMS LANE R (14007)

GIBBONS E J R (14015)

KIMBRELL JAS BR (14019)

BERLINER JOE R (14021)

MARLOWE RAY A R (14025)

MERTENS CAR W R (14031)

HUSTON RD

WRIGHT JOHN WM R (13958)

GORDON AARON R (13959)

SHELDON PETER J R (13964)

DEVEY ALFRED J MRS R (13965)

HELGESON CLARENCE E R (13970)

SHANK WAYNE C R (13971)

ALTMAN ROBT R (14000)

CANTZ MARVIN S R (14006)

BOUQUET JOS A JR R (14007)

POSEN SAM R (14012)

IGO RICHARD A R (14013)

D ORAZIO R J R (14018)

ATKINSON GEO H R (14024)

SMITH STANLEY J R (14025)

FREDERICK LEONARD L R (14031)

LA MAIDA ST

HILL HUGH E R (13928)

JOHNSON STELLA CARRELL R (13929)

BAROWITZ LEON R (13934)

HEINEN HILDRETH R (13935)

LOGAN JOHN PR (13940)

BECKER V A R (13941)

BAYLEY H M JR R (13946)

KAMM ALBERT R (13947)

MCCALLON LYNN R (13952)

SUGARMAN EUGENE R (13953)

DORR WALTER O R (13958)

ERWIN RALPH B R (13959)

<u>Source</u>

Pacific Telephone

Pacific Telephone

1950 (continued)

LENARD ROBT R (13964)

DAMBACHER EMMETT N JR R (13970)

COLLETTI VINCENT P R (13971)

WINEGARDNER D E R (14000)

TROULMAN ROBT W R (14001)

MILLER WM I R (14006)

NATER SAM MRS R (14007)

PETERS J W R (14012)

MANN MAX E R (14018)

WALLACE MARLENE R (14024)

WILKINSON HERBERT D R (14030)

VENN HENRY G R (14031)

MURIETTA AVE

JENKINS DONALD R R (4801)

ROBINSON WM C R (4807)

JUILLARD LOUIS G R (4813)

CARTER EUGENE M R (4819)

MCMICHAEL JUDD R (4825)

HAND FRANK E R (4837)

COHEN E H R (4843)

LOCHER MILDAN C R (4849)

MEREDITH M W R (4855)

HAMOVITZ RAY R (4901)

LOWELL MANUEL R (4907)

HANCOCK W L R (4913)

MCCARTHY JOHN E R (4919)

PEACH GROVE ST

FELDMAN BERNARD L R (13928)

PAICH MARTIN L R (13934)

WICKSTROM CARL C R (13935)

TROULMAN JACK R (13940)

HULL KENNETH W R (13941)

EGGETT GORDON E R (13946)

WHITNELL C E R (13947)

BURNS JAS W R (13952)

KUHSE ORVILLE H R (13953)

MARREN MILTON R (13958)

VELLOZZI LARRY R (13959)

NELSON ROBT O R (13965)

SUGARMAN JACK A R (13970)

ROCKWELL ROBT G R (13971)

SHERMAN MILTON MRS R (14000)

KERZIN LESTER N R (14001)

Source

Pacific Telephone

Pacific Telephone

Year Uses <u>Source</u> 1950 (continued) TRAPANI JOHN F R (14006) MOSS MITCHELL R (14007) CASTELLI CHAS R (14013) LAYTON ROBT O R (14018) **BEALL GEO R R (14019)** HINCHCLIFFE GORDON R R (14019) SELLECK ROBT D R (14024) **DEMETROPOULOS ROSEMOND MRS R (14025)** STORM PHILLIP J R (14030) WOOD JOS F MRS R (14031) Pacific Telephone **RIVERSIDE DR** COCHRAN EARL G DR R (13911) HILLEY JACK R (13919) SCHWARTZ JACK R (13925) SHILMAN ALFRED E R (13931) LEVENTHAL BERNARD B (13937) SCOTT TED M R (13941) SFORZINI ROBT A R (13943) WALKER CHRISTABEL B R (13949) LATT SAM R (13965) **NUGER EDW R (13967) GRIBIN IRA R (13973)** EDMOND JEROME R (14003) WILLIAMS LANE R (14007) **GIBBONS E J R (14015)** KIMBRELL JAS BR (14019) BERLINER JOE R (14021) MARLOWE RAY A R (14025) MERTENS CAR W R (14031) 1951 Address Not Listed in Research Source Los Angeles Directory Co Publishers 1952 Los Angeles Directory Co. Address Not Listed in Research Source R. L. Polk & Co. 1954 Address Not Listed in Research Source 1955 R. L. Polk & Co. Address Not Listed in Research Source 1956 Pacific Telephone **HUSTON RD** GLEASON HAROLD W (13958) GORDON AARON (13959) BUGGE JOHN A (13964) DEVEY ALFRED J MRS R (13965)

1956 (continued)

HELGESON CLARENCE E R (13970)

SHANK WAYNE C R (13971)

ALTMAN ROBT R (14000)

PINCHERLI MANFREDO (14001)

MCGUINNIS FLO (14006)

MCGUINNIS JOE J (14006)

TURNER LLOYD G (14012)

CONTINENTAL IMPORTS AUTOMTV PTS (14013)

KRAUS JOHN W CONTINENTAL IMPORTS AUTOMTV PTS

(14013)

BECK HERBERT J (14018)

SNYDER EDNA MRS (14019)

NOLEN R E (14024)

SMITH STANLEY J R (14025)

SANDERSON JOHN A (14030)

WEISS HARRY C (14031)

HUSTON ST

HULSE DON W HUON (14007)

LA MAIDA ST

BICHLER JOHN (13928)

JOHNSON STELLA CARRELL R (13929)

BAROWITZ LEON R (13934)

HEINEN HILDRETH R (13935)

CHAMPION ALBERT P (13940)

WIDOM BERNARD R (13946)

WAIS ORRIN B (13947)

MCCALLON LYNN R (13952)

SUGARMAN EUGENE R (13953)

DORR WALTER O R (13958)

KEEGAN O ALVA JR (13959)

LENARD ROBT R (13964)

HERMAN GILBERT M (13970)

SAWYER JAS R (13971)

WINEGARDNER D E R (14000)

TROULMAN ROBT W (14001)

MILLER WM I R (14006)

NATER SAM R (14007)

MUSSELMAN JOHN C (14012)

WEISER ANN C (14018)

WEISER EDW A (14018)

WALLACE MARLENE S (14024)

ALBRIGHT FRED L R (14025)

CALCE NINO (14030)

Pacific Telephone

Source

1956 (continued)

VENN HENRY GR (14031)

MURIETTA AVE

CORBETT JAS F R (4801)

ROBINSON WM C (4807)

JUILLARD LOUIS G R (4813)

FOGG LEWIS (4819)

BYERS WM (4825)

STANGE EDITH O (4831)

HAND FRANK E (4837)

LOCHER MILDAN C (4849)

MEREDITH LOIS M (4855)

HAMOVITZ RAY R (4901)

LOWELL MANUEL R (4907)

HANCOCK W L R (4913)

ALARCON ARTHUR L (4919)

PEACH GROVE ST

FELDMAN BERNARD L R (13928)

HASEROT DAVID L (13929)

ABRAMS ALEX (13934)

WICKSTROM CARL C R (13935)

KAZIAN F B (13940)

HULL KENNETH W R (13941)

EGGETT GORDON E (13946)

DUDLEY WM HAYNES (13947)

BURNS JAS W (13952)

KUHSE ORVILLE H R (13953)

YORK HARRY (13958)

HAMLIN BENJ R (13959)

COHEN LOU R (13964)

SUGARMAN JACK A R (13970)

ADELSON DAVID J (14000)

KERZIN LESTER N R (14001)

TRAPANI JOHN F (14006)

MOSS MITCHELL R (14007)

BASTIAN R L R (14012)

CASTELLI CHAS R (14013)

GASTIL RICHARD W R (14018)

HARTE ROY (14019)

SELLECK ROBT D R (14024)

DEMETROPOULOS ROSEMOND MRS R (14025)

WOOD JOS F MRS R (14031)

RIVERSIDE DR

<u>Source</u>

Pacific Telephone

Pacific Telephone

1956 (continued)

GORDON THEO (13909)

MASLER ERNEST G MD (13917)

CATTS WALTER (13919)

MANDELL IRVING R (13923)

BEVERLY ROBT J (13925)

MAAS EVAN (13929)

SHILMAN ALFRED E R (13931)

WHITE THELMA E MRS R (13935)

POPP FRANK JR R (13937)

GRAU HENRY R (13943)

BRUDNEY GEO G (13947)

LACOE NORMAN H (13949)

THOMAS RUSSELL P (13953)

LANG JAS M (13955)

LONG EDWIN S (13959)

LURIE ABRAHAM M (13961)

ZELMAN RUTHE B (13961)

ZELMAN RUTHE B (13961)

LATT SAM R (13965)

LEVINE JESSE (13967)

ESTES LEONARD (13971)

KESSLER JULIUS (13973)

TOWNSEND JEROME B (14001)

WEISBERG V DR (14003)

GREENSEID MAXWELL (14007)

RUBEDOR LESTER L (14009)

ALAN-LEE IVOR (14013)

FISHMAN ANNA MRS R (14019)

HANZAL BRIAN P (14021)

DORNBUSH WAITER J (14025)

HAMAN RICHARD Y (14027)

SOWERS J W (14031)

WEINTRAUH STANLEY (14031)

GIBBONS ELIZABETH B (14033)

1957 Address Not Listed in Research Source

1958

WEISS HARRY C (14031)

HUSTON ST

WEISS HARRY C (14031)

1960 Address Not Listed in Research Source

1961 Address Not Listed in Research Source

Source

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Pacific Telephone

Pacific Telephone

Luskey Brothers & Co

Source

1962 **HAZELTINE AVE**

Pacific Telephone

BRESSICK NORMAN (4824)

KNAACK WM F JR (4824)

LEVI SAMSON H (4824)

DOLE SIDNEY (4832)

ELLSTROM RONALD W (4832)

GONEK LINDA (4832)

GONEK SHELDON (4832)

KENNY WAYNE O (4832)

MERL MAXINE (4832)

ROSENFELD JACK M (4832)

SHAW J JERRY DR (4832)

ALBERTS LIONEL L (4838)

BINGHAM EDWIN TED (4838)

MORTELLARO OLIVA (4838)

ROSEN TILLIE (4838)

VOGLER EDW MRS (4838)

WEBER ESTELLE (4838)

AKOPIAN VICTOR MRS (4846)

ALPER LEONARD (4846)

CARLIN HELEN (4846)

EGGLY MATHEUS (4846)

FRIEDMAN JOS (4846)

HEISER LEONARD F (4846)

KOGUT MAURICE D MD (4846)

MCGINLEY NORMA L (4846)

HUSTON RD

GLEASON HAROLD W (13958)

JOSLIN JAY D (13964)

DEVEY ALFRED J (13965)

HELGESON CLARENCE E (13970)

TREATMAN DAVID (13971)

HAMILTON JOHN S (13987)

HERKAL WALTER H (14000)

MALONEY GORDON E (14012)

NOLEN R E (14024)

SMITH STANLEY J (14025)

SANDERSON JOHN A (14030)

HUSTON ST

ROSSI F (14019)

ROSS CLAUDE F (14031)

ROSS GRACE B (14031)

Pacific Telephone

1962 (continued)

LA MAIDA ST

MACTAGUE CHAS H (13928)

JOHNSON STELLA CARRELL SHERMAN OAKS (13929)

BAROWITZ LEON (13934)

HEINEN HILDRETH (13935)

CHAMPION ALBERT P (13940)

PARR RAYMOND E (13941)

HART ANNETTE J (13946)

MENDELL ERNEST (13947)

MCCALLON LYNN (13952)

SUGARMAN EUGENE (13953)

DORR WALTER O (13958)

KEEGAN O ALVA JR (13959)

LENARD ROBT (13964)

GODLEY FRANCES LA MAIDA (13965)

WALLACE WESLEY V (13970)

SINGER SONDRA A (13971)

WINEGARDNER D E (14000)

TROULMAN JAY H (14001)

MILLER WM I (14006)

NATER SAM (14007)

BERDAN BARBARA M (14012)

BROWN FRANCES MRS (14012)

WALLACE MARIENE (14024)

ALBRIGHT FRED L (14025)

KARAZISSIS NICK (14030)

MRGUDIC ANTE (14031)

MURIETTA AVE

HAAS FRANK E (4801)

ROBINSON WM C (4807)

JUILLARD LOUIS G (4813)

BACHRACH JEROME (4819)

BYERS WM (4825)

STANGE EDITH D (4831)

HAND FRANK E (4837)

ATTERBURY G B (4843)

LOCHER CAROL V (4849)

HAMOVITZ RAY (4901)

SECHOOLER LEONARD J (4907)

HANCOCK W L (4913)

GORDON CRAIG (4919)

PEACH GROVE ST

Source

Pacific Telephone

Pacific Telephone

<u>Year</u> <u>Uses</u>

Source

1962 (continued)

KRASIK BERNARD N (13928)

HASEROT DAVID L (13929)

LUSSIER RAYMOND G (13934)

WICKSTROM CARL C (13935)

KAZIAN F B (13940)

HULL KENNETH W (13941)

EGGETT GORDON E (13946)

DUDLEY WM HAYNES PEACH GROVE (13947)

HENIGMAN MAX N (13952)

REMSON S (13953)

YORK HARRY (13958)

BREWER ROY M (13965)

LURIE NATHAN (13970)

SCHLUND GEO J (13971)

ADELSON DAVID J (14000)

BERNSON I (14001)

GLANTZ JOS R (14006)

MOSS MITCHELL (14007)

NASON CLYDE K (14012)

CASTELLI CHAS (14013)

KEROPIAN HAIG (14018)

HARTMANN WILLARD D (14019)

SELLECK BOB JR (14024)

SELLECK ROBT D (14024)

SELLECK TOM (14024)

DEMETROPOULOS ROSEMOND MRS (14025)

WOOD JOS F (14031)

RIVERSIDE DR

PEDERSON DELVIN (13909)

PEDERSON DORIS (13909)

PASTMAN NORMAN B (13919)

MEYER DONALD (13923)

BAIN SYLVAN A (13925)

GIEBINK GERALD A (13929)

OSTENGAARD ROBT K (13931)

HILL KATHLEEN (13937)

HILL ROBT S (13937)

HELMICK DOROTHY C (13941)

HELMICK LEONARD V (13941)

GERLACH VIRGIL C (13947)

NORTH R A (13949)

KLEINER EZRA E MD (13953)

HELPMAN MARTIN (13959)

<u>Year</u>	<u>Uses</u>	Source
1962 (continued)	
	JOHNSON GENE A (13961)	
	SMITH L J (13965) FOWLER LE ROY E (13967)	
	• •	
	CANDLISH STANLEY L (13973)	
	DESMONDS (14000)	
	CAMPBELL MARILYN (14001) PRICE JOS (14007)	
	ACKER FRANK RICHARD (14009) BUFFO DENNY MRS (14013)	
	WYKES ANN (14015)	
	WYKES DONALD (14015)	
	FISHMAN ANNA MRS (14019)	
	LEVINE NORMAN J (14021)	
	AUDETTE RENE F (14025)	
	WALLER GARY J (14027)	
	MACKENZIE H G (14031)	
	BOB S CHEVRON SERV (14101)	
	CHEVRON SERV STATIONS (14101)	
	DESMONDS (14000)	
	VALLEY FASHION SQUARE STORE (14000)	
1963	Address Not Listed in Research Source	Pacific Telephone
1964	Address Not Listed in Research Source	Pacific Telephone
1965	Address Not Listed in Research Source	GTE
1966	Address Not Listed in Research Source	Pacific Telephone
1967	**RIVERSIDE DR**	R. L. Polk & Co.
	MITNICK JACK (13957)	
1969	Address Not Listed in Research Source	Pacific Telephone
	Address Not Elsted in Nessearch Source	
1970	**HAZELTINE AVE**	R. L. Polk & Co.
	ALDRIDGE DANL T JR (4824)	
	HERKAL WALTER H JR (4824)	
	HORWITZ J (4824)	
	HURT MARGARET E (4824)	
	JACOBS HARRY (4824)	
	NEUSCHOTZ S L (4824)	
	VAN VLIET NELL (4824)	
	ASSIL ABRAHIM (4832)	
	HERZOG ADELINE (4832)	

Source Year Uses 1970 (continued) HERZOG ELLEN A (4832) HERZOG RICHARD C (4832) KRAMER REBECCA (4832) **TRUPP ROY (4832) SWANSON KEITH V (4833) SWONSEN KEITH V (4833) ARNOLD MICHAEL W (4838) CIMINO VINCENT S (4838) NORMAN MAURICE MRS (4838)** VAN DER WEL W G (4838) WATSON LORRAINE M (4838) **WOODINE MEYER (4838)** ATKINSON CHAS (4846) **BLAUSTEIN E E (4846)** MCGINLEY NORMA L (4846) **TOZER ELIZABETH (4846)** R. L. Polk & Co. **HUSTON RD** EPSTEIN SOL (13959) DEVEY ALFRED J (13965) **HELGESON CLARENCE E (13970)** HERKAL WALTER H (14000) SPECCHIERIA LAWRENCE (14006) **MALONEY GORDON E (14012)** JUNG CARSON M (14018) FRIEDMAN LAZAR (14024) ALLES HAROLD F SHERMAN OAKS (14030) **HUSTON ST** R. L. Polk & Co. ANDERSEN SYLVIA ANN (13958) DONG STANLEY B (13964) TASSO BARRY W (13971) GOBET CLAUDE (14019) ROSS CLAUDE F (14031) ROSS GRACE B (14031) R. L. Polk & Co. **LA MAIDA ST** VAN HOUSEN JOHN (13928) **JOHNSON STELLA CARRELL (13929)** CHAMPION ALBERT P (13940) **BRONSTEIN MILDRED (13941) BRONSTEIN PHILIP (13941) BRONSTEIN PHILIP (13941)**

LUKANYI JULIUS (13946) DALPEZ STEVEN L (13947)

1970 (continued)

MCCALLON GREG (13952)

MCCALLON LYNN (13952)

LAWRENCE IIONA (13953)

DORR WALTER O (13958)

KEEGAN O ALVA JR (13959)

SAVITT MALLO M (13964)

GODLEY FRANCES (13965)

WINEGARDNER D E (14000)

TROULMAN JAY H (14001)

MILLER WM I (14006)

NATER SAM (14007)

STAFFORD MICHAEL R (14012)

SCHOTT C D (14018)

BRAUNSTEIN SAM (14025)

MRGUDIC ANTE (14031)

MRGUDIC ANTE (14031)

MURIETTA AVE

BLOSSOMS FLOWERS (4813)

MCCARTHY BILLIE (4819)

LOCHER CAROL V (4849)

HAMOVITZ RAY (4901)

SECHOOLER LEONARD J (4907)

HANCOCK W L (4913)

GORDON CRAIG (4919)

PEACH GROVE ST

HELMERS HAROLD H (13928)

HASEROT BARBARA MRS (13929)

LUSSIER RAYMOND G (13934)

WICKSTROM CARL C (13935)

KAZIAN F B (13940)

HULL KENNETH W (13941)

EGGETT GORDON E (13946)

DUDLEY WM HAYNES (13947)

FRANK S (13952)

REMSON S (13953)

YORK ANNE (13958)

YORK HARRY (13958)

WILSON WM L (13959)

WELLS STANLEY H (13964)

LURIE NATHAN (13970)

SCHLUND GEO J (13971)

BERNSON I (14001)

DUNSMORE RICHARD F (14006)

Source

R. L. Polk & Co.

R. L. Polk & Co.

1970 (continued)

MOSS MITCHELL (14007)

DAVIS S (14012)

CASTELLI CHAS (14013)

O MAHONEY PRINCESS (14018)

NEWMAN MYRON (14019)

SMITH THOS R (14024)

DEMETROPOULOS ROSEMOND MRS (14025)

MCLEAN MICHAEL (14030)

RIVERSIDE DR

LESKO GEO J (13909)

BEN-JAMIN SASSL (13911)

ROMMERSWINKEL KLAUS (13917)

COHEN BARBARA J (13919)

COHEN FANNY (13919)

COHEN GERALD S (13919)

KITAZAKI JAS (13925)

WANKEL WILLIAM E (13929)

DESANTIS JOHN J (13931)

GRABLE FRED (13935)

MCGOWAN CLYDE (13943)

CLARKE WM R (13949)

LOWIEN CARL L (13953)

CARBAUGH GLENN F (13955)

BARTLETT JUANITA (13959)

BORN B (13961)

RUBENS ROBERTA LEE (13965)

SMITH REBECCA S (13965)

NEW ROBT J (13967)

MULVIHILL MARIE D (13973)

AXELROD W G (14001)

HILLEBRAND KENNETH (14007)

REED VIRGINIA S (14009)

GROHMAN GORDON A (14013)

MORGENSTERN FRANK (14015)

MORGENSTERN JOS M (14015)

HURLEY A E (14021)

AUDETTE RENE F (14025)

DIEHL ELMER C (14027)

ROTHMAN BERNARD B (14033)

CHEVRON SERVICE STATIONS SHERMAN OAKS RIVERSIDE &

HAZELTINE (14061)

MAGUIRE JOE CHEVRON SERVICE (14061)

U-HAUL CO DEALERS SHERMAN OAKS (14061)

R. L. Polk & Co.

Source

1970 (continued)

TRANS-COAST SAVINGS & LOAN ASSOCIATION OF OXNARD (14110)

HAZELTINE AVE

ALDRIDGE DANL T JR (4824)

HERKAL WALTER H JR (4824)

HORWITZ J (4824)

HURT MARGARET E (4824)

JACOBS HARRY (4824)

NEUSCHOTZ S L (4824)

VAN VLIET NELL (4824)

ASSIL ABRAHIM (4832)

HERZOG ADELINE (4832)

HERZOG ELLEN A (4832)

HERZOG RICHARD C (4832)

KRAMER REBECCA (4832)

TRUPP ROY (4832)

SWANSON KEITH V (4833)

SWONSEN KEITH V (4833)

ARNOLD MICHAEL W (4838)

VAN DER WEL W G (4838)

WOODINE MEYER (4838)

LA MAIDA ST

PRICE LAWRENCE F (13917)

VAN HOUSEN JOHN (13928)

JOHNSON STELLA CARRELL (13929)

CHAMPION ALBERT P (13940)

BRONSTEIN MILDRED (13941)

BRONSTEIN PHILIP (13941)

BRONSTEIN PHILIP (13941)

DALPEZ STEVEN L (13947)

MCCALLON GREG (13952)

MCCALLON LYNN (13952)

LAWRENCE IIONA (13953)

DORR WALTER O (13958)

KEEGAN O ALVA JR (13959)

GODLEY FRANCES (13965)

WINEGARDNER D E (14000)

TROULMAN JAY H (14001)

MILLER WM I (14006)

NATER SAM (14007)

STAFFORD MICHAEL R (14012)

SCHOTT C D (14018)

BRAUNSTEIN SAM (14025)

<u>Source</u>

R. L. Polk & Co.

R. L. Polk & Co.

<u>Year Uses</u>

1970 (continued)

MRGUDIC ANTE (14031)

MRGUDIC ANTE (14031)

MURIETTA AVE

BLOSSOMS FLOWERS (4813)

MCCARTHY BILLIE (4819)

RIVERSIDE DR

LESKO GEO J (13909)

ROMMERSWINKEL KLAUS (13917)

COHEN BARBARA J (13919)

COHEN FANNY (13919)

COHEN GERALD S (13919)

KITAZAKI JAS (13925)

WANKEL WILLIAM E (13929)

DESANTIS JOHN J (13931)

GRABLE FRED (13935)

MCGOWAN CLYDE (13943)

LOWIEN CARL L (13953)

CARBAUGH GLENN F (13955)

BARTLETT JUANITA (13959)

BORN B (13961)

RUBENS ROBERTA LEE (13965)

SMITH REBECCA S (13965)

NEW ROBT J (13967)

MULVIHILL MARIE D (13973)

HILLEBRAND KENNETH (14007)

GROHMAN GORDON A (14013)

MORGENSTERN FRANK (14015)

MORGENSTERN JOS M (14015)

HURLEY A E (14021)

AUDETTE RENE F (14025)

DIEHL ELMER C (14027)

ROTHMAN BERNARD B (14033)

CHEVRON SERVICE STATIONS SHERMAN OAKS RIVERSIDE &

HAZELTINE (14061)

MAGUIRE JOE CHEVRON SERVICE (14061)

1971 **LA MAIDA**

B&G Publications

MRGUDIC ANTE (14031)

1972 Address Not Listed in Research Source

R. L. Polk & Co.

1975 **HAZELTINE AVE**

Pacific Telephone

DISTRICT TRAVEL OFFICE (4745)

FORECAST LAND INVESTMENT CO (4741)

Source

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R. L. Polk & Co.

1975 (continued)

DEAKINS O K (4824)

HORWITZ J (4824)

HURT MARGARET E (4824)

JACOBS HARRY (4824)

NEUSCHOTZ S L (4824)

VAN VLIET NELL (4824)

ALTONJI J (4832)

ARMSTRONG GERTRUDE A (4832)

RAYBURN SCOTT (4832)

SANDLER JERRY J (4832)

FRAZIN LARRY (4833)

MANKIEWICZ I K (4833)

SMITH ARTHUR H (4833)

HUME C M (4838)

LA MAIDA A (4838)

MOISAN JOHN (4838)

OST RUDY (4838)

PIERCE N C (4838)

YAFFE STEVEN (4838)

HUSTON ST

JERGER TUZO (13958)

RICHARDS MARK (13958)

EPSTEIN SOL (13959)

FEIDEN JENNY (13959)

KOLLER W B (13964) DEVEY ALFRED J (13965)

HELGESON CLARENCE E (13970)

LEVINE B (13971)

HERKAL WALTER H (14000)

SIRACUSA STEVE (14006)

MALONEY GORDON E (14012)

KINSEY CHUCK (14013)

FRIEDMAN LAZAR (14024)

KIBBEE JEFFERSON (14025)

GOODMAN CHAS (14030)

ROSS CLAUDE F (14031)

ROSS GRACE B (14031)

LA MAIDA ST

JOHNSON L W JOHNNY (13929)

M F EXPORTS INC (13929)

HIVELY D L (13934)

CHAMPION ALBERT P (13940)

BRONSTEIN MILDRED (13941)

Source

Pacific Telephone

1975 (continued) BRONSTEIN PHILIP (13941)

LUKANYI JULIUS (13946)

DU BELLIER ALFRED (13947)

MC CALLON GREG (13952)

MC CALLON LYNN (13952)

BULLON GREGORY (13953)

DORR WALTER O (13958)

DHIR SOHAN LAL (13964)

COULTER KENNETH J (13971)

WINEGARDNER D E (14000)

TROULMAN BRENT (14001)

MILLER WM I (14006)

NATER SAM (14007)

FENNEMAN CLIFF (14012)

NAKAYAMA TOSHIHIRO (14018)

MOORER LE ELLA (14019)

BRAUNSTEIN SAM (14025)

KLIGIER ARIE (14030)

MRGUDIC ANTE (14031)

MURIETTA AVE

MC CLAFLIN NEIL (4801)

CRUPI JOS (4813)

HELLIE PAUL (4819)

HAMOVITZ RAY (4901)

HABERMAN NEAL (4907)

GORDON CRAIG (4919)

MURIETTA DR

ATTERBURY G B (4843)

RIVERSIDE DR

CRAVEN P A (13971)

BULLOCKS SHERMAN OAKS (14000)

LITTLE RICHARD A (14003)

SCOTT RICHARD (14007)

STEPHENSON WM E (14009)

LHASA GALLERIES LTD (14012)

JOURNEAY D (14013)

SANCHEZ ALICIA C (14025)

LONNER M (14027)

MC CREIGHT C L (14031)

GILBERT MICHAEL M (14033)

MAGUIRE JOE CHEVRON SERVICE (14061)

RIVERSIDE & HAZELTINE (14061)

<u>Source</u>

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Pacific Telephone

1975 (continued)

PAULS PHARMACY (14101)

PHILIPPINE MASTERCRAFTS (14103)

CREATIVE PUBLICATIONS (14105)

CREATIVE TEACHING CENTER (14105)

BULLOCKS SHERMAN OAKS (14000)

BULLOCK S DEPARTMENT STORES (14000)

1976 **HAZELTINE AVE**

FORECAST LAND INVESTMENT CO (4741)

AMERICAN EXPRESS COMPANY TRAVEL DIVISION (4745)

SHERMAN OAKS (4745)

LA MAIDA ST

M F EXPORTS INC (13929)

RIVERSIDE DR

BULLOCKS SHERMAN OAKS (14000)

1980 **HAZELTINE AVE**

CRELEY JACK (4824)

GHEZZO ANTONETA (4824)

JACOBS HARRY SHERMAN OAKS (4824)

LALEZAR NAIM (4824)

NEUSCHOTZ S L (4824)

ALTONJI J (4832)

EPSTEIN MICHAEL (4832)

FEDOROVICH VALENTIN (4832)

MARTINEZ MICHAEL JR (4832)

O BRIEN STEPHEN P SHERMAN OAKS (4832)

MYERS MARK SHERMAN OAKS (4833)

BENGIS LEONARD (4838)

HUME C M (4838)

LAMAIDA A (4838)

MOISAN JOHN (4838)

OST RUDY (4838)

PEARSON V A (4838)

PIERCE N C (4838)

YAFFE STEVEN (4838)

LA MAIDA ST

MCKEOWN KEITH J (13916)

PRICE JAS M (13917)

MURPHY JAS M A (13923)

SCOTT J A (13929)

HIVELY D L (13934)

PRICE LAWRENCE F (13937)

CHAMPION ALBERT P (13940)

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R.L. Polk & co Publishers

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1980 (continued)

BRONSTEIN MILDRED (13941)

BRONSTEIN PHILIP (13941)

LUKANYI JULIUS (13946)

DU BELLIER ALFRED (13947)

DORR WALTER O (13958)

DHIR SOHAN LAL (13964)

SABLJIC MARKO SHERMAN OAKS (13970)

COULTER KENNETH J (13971)

WINEGARDNER D E (14000)

TROULMAN DOROTHY (14001)

MILLER S (14006)

ORMSBY ALAN (14007)

ROSS KENNETH J (14013)

MOORER LE ELLA (14019)

BRAUNSTEIN SAM (14025)

KARAZISSIS NICOLAS SHERMAN OAKS (14030)

MRGUDIC ANTE (14031)

MURIETTA AVE

ELITE DENTAL CERAMICS (4801)

HELLIE PAUL (4819)

SCHAFFER EPHRAIM (4837)

ATTERBURY G B (4843)

RIVERSIDE DR

HALPERIN HERBERT H (13909)

HYNDS J E (13911)

WAGNER ROBT M (13915)

CASUAL CORNER (13918)

GORDON MAX (13921)

GOLDMAN R SHERMAN OAKS (13923)

KANTER JEROME B (13927)

PIGEONS (13930)

HANCE WILLARD (13933)

AICHOLTS V (13939)

GILLBERG ERIC (13941)

HELFT S LADIES APPAREL (13942)

KAPLAN ALBERT S (13949)

ZIMMERMAN HOWARD (13951)

TEDESCO LOU (13957)

PAGE NORMAN (13961)

FISHER HOWARD (13963)

GRAVELINE R (13965)

HUNTER LEW (13967)

FLORSHEIM SHOE SHOPS (13970)

Source

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Source 5

Year Uses

1980 (continued)

FASHION SQUARE SHERMAN OAKS (13972)

LOUISES FABRICS & QUILTWORKS (13972)

CALLIES WERNER & ALMA (13973)

PHOTO AND SOUND CENTER (13974)

FASHION CONSPIRACY OFC (13976)

FAITHFUL FOOTMAN (13984)

BADGER ELVA ELCTRLGST (14000)

BULLOCK S SHERMAN OAKS (14000)

GREGORY BRUCE (14003)

SWAIN JAS (14007)

LOWY RICHARD E (14013)

MEYERS BARRY SHERMAN OAKS (14013)

CHEMEL RICHARD (14015)

LEH ROBT (14019)

SHERMAN OAKS DELI RESTAURANT (14020)

FOLEY TARA (14021)

JESSER ALBERT (14025)

SISKIN NORMAN J SHERMAN OAKS (14027)

SISKIN S R SHERMAN OAKS (14027)

CORRAL THE (14030)

SCHUR ERIC (14033)

CROW WILLIAM DR OPTMTRST (14034)

DIANES (14038)

SILVERWOOD FASHION SQUARE SHERMAN OAKS (14046)

SIMI S FASHIONS SHERMAN OAKS (14048)

KINNEY SHOES WEST COAST OFCS CHATSWORTH (14050)

BROADWAY DEPARTMENT STORES (14060)

BROADWAY FASHION FORMAL WEAR (14060)

DE LUCA JEWELERS & MANUFACTURERS (14060)

GOROWITA STEVEN DR (14060)

WALDEN BOOKS (14060)

MAGUIRE JOE CHEVRON SERVICE SHERMAN OAKS (14061)

PAUL S PHARMACY (14101)

HUSTON RD

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ATKINSON GEO H R (14024)

1981 **RIVERSIDE DR**

Pacific Telephone

HELFT S LADIES APPAREL (13942)

FAITHFUL FOOTMAN SHERMAN OAKS (13984)

BULLOCK S SHERMAN OAKS SHERMAN OAKS (14000)

GARZA TOURS INTERNATIONAL INC SHERMAN OAKS (14013)

SILVERWOODS (14046)

SIMI S FASHIONS SHERMAN OAKS (14048)

BROADWAY DEPARTMENT STORES STORES (14060)

<u>Source</u>

1981 (continued)

DE LUCA JEWELERS & MANUFACTURERS VAN NUYS (14060)

G T TRAVEL SHERMAN OAKS (14103)

1985 **HAZELTINE AVE** Pacific Bell

FORECAST LAND INVESTMENT CO (4741)

FROM LOS ANGELES TELEPHONES CALL (4741)

BELSHE JUDY (4824)

JACOBS HARRY (4824)

NEUSCHOTZ S L (4824)

BUGACHEVSKY VLADIMIR (4832)

BUGARCIC D (4832)

FELDMAN ISAK (4832)

FELDMAN J (4832)

FELDMAN J (4832)

FELDMAN J 8 (4832)

GERZON YAKDV (4832)

KOGAN EL (4832)

LIVSHIT FANYA (4832)

LIVSHIT REBEKKA (4832)

MALEKZADEH ALI A (4832)

MALEKZADEH MANOOCHEHR (4832)

DEL BENE ALBERT J (4833)

DEL BENE ROBT (4833)

DELBERT BILLY (4833)

DELBERT BILLY (4833)

FLEISCHNER KURT & DOROTHY (4833)

HUME C M (4838)

KING MARRETTE M (4838)

LA MAIDA A (4838)

MOISAN JOHN (4838)

PEARSON V A (4838)

PIERCE N C (4838)

STREETER BRIAN (4838)

CUNNINGHAM J M (4846)

CUNNINGHAM J R (4846)

ELLIS DAVE (4846)

ELLIS DAVE & CATHY (4846)

ROEMERMANN JOHN (4846)

RUSH PATRICK (4846)

SCHOOLER ANNE (4846)

HUSTON ST

EPSTEIN SOL (13959)

LEVINE BL (13971)

HERKAL WALTER H (14000)

Pacific Bell

1985 (continued)

HERKATA PHILIPPUS H (14000)

HERKATA PHILIPPUS H (14000)

MALONEY GORDON E (14012)

MALONEY J (14012)

MALONEY J (14012)

KINSEY K (14013)

FRIEDMAN LAZAR (14024)

FRIEDMAN LEIB & MARCIA (14024)

SHIFLETT S (14025)

GOODMAN CHAS (14030)

LA MAIDA ST

TAHERI SOHRAB (13929)

TAHERPOUR MAHIN (13929)

HIVELY DL (13934)

HIWA L (13934)

CHAMPION J (13940)

LUKANYI JULIUS (13946)

DU BELLIER ALFRED (13947)

YOUNG MARK R (13953)

DORR WALTER O (13958)

DHIR SOHAN LAL (13964)

SABLJIC MARKO (13970)

COULTER J K (13971)

WINEGARDNER D E (14000)

MILLER S (14006)

LEVERHANT MIRIAM (14007)

BRAUNSTEIN SAM (14025)

KARAZISSIS NICOLAS (14030)

MRGUDIC ANTE (14031)

MRGUDICH JOHN (14031)

MURIETTA AVE

ELITE DENTAL CERAMICS (4801)

SCHAFFER EPHRAIM (4837)

ATTERBURY G BL (4843)

HABERMAN NEAL (4907)

GORDON CRAIG (4919)

GORDON D (4919)

GORDON D (4919)

GORDON D (4919)

GORDON D SEP (4919)

GORDON HELEN (4919)

GORDON HELENE CHATS (4919)

Source

Pacific Bell

Pacific Bell

1985 (continued)

PEACH GROVE

MENENDEZ VAL M (13929)

FRIMER SOL (13934)

FRIMKESS D & M (13934)

FRIMKESS S (13934)

WICKSTRAM CARL C (13935)

HULL KENNETH W (13941)

HULL L (13941)

HULL L NOR (13941)

PAPAZIAN G E (13947)

REMSON S (13953)

REISS BERNARD (13958)

WILSON W I (13959)

DAMBROSIO JOS (13970)

MOERKE GARY H (13970)

MOERMAN ERIC R (13970)

MOERMAN HSEPULVEDA (13970)

MOERMAN PAUL (13970)

BOOTH WM (13971)

MOSS MITCHELL (14007)

WAGNER RUTH (14012)

SMITH THOS R (14024)

RIVERSIDE DR

HYNDS J E (13911)

WAGNER ROBT M (13915)

CULBERTSON JOE (13917)

CASUAL COMER (13918)

GORDON MAX (13921)

STREM STEVE E (13923)

HANTER JEROME BL (13927)

MAIN OFC (13930)

SHERMAN OAKS (13930)

DEVORE SY VALLEY SHOP (13932)

WOERSCHING MARCUS BL (13933)

WOESNER EUGENE (13933)

AICHOLTZ V (13939)

FASHION SQUARE SHERMAN OAKS (13942)

FROM LOS ANGELES TELEPHONES CALL (13942)

SHERMAN OAKS (13950)

PICKWICK FASHIONS (13954)

FISHER HOWARD (13963)

GRAVELINE R (13965)

BRITTON MICHAEL (13969)

Source

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Pacific Bell

2096148-6

1985 (continued)

FLORSHEIM SHOE SHOPS (13970)

LOUIS ES FABRICS & QUILTW ORKS (13972)

LOUISES FABRICS & QUILTWORKS (13972)

GARDENS RESTAURANT THE (13982)

GARDENS THE FASHION SQUARE (13982)

GARDER S (13982)

FAITHFUL FOOTMAN (13984)

FROM LOS ANGELES TELEPHONES CALL (13984)

BADGER ELVA EICTRLGST (14000)

BULLOCKS (14000)

BULLOCKS TRAVEL BUREAU (14000)

FROM GLENDALE BURBANK AREA CALL (14000)

TRAVEL BUREAU (14000)

GREGORY BRUCE (14003)

GREGORY BRUCE D (14003)

PHOTO & SOUND CENTER (14004)

SWAIN JAS (14007)

SWAIN JAS W (14007)

SWAIN JOS R & DELIA SYLM (14007)

COVERT K (14015)

COVERT SANDI L (14015)

LEH S (14019)

LEHAN DANIEL (14019)

LEHANE JACKIE (14019)

FASHION SQUARE DELI (14028)

FASHION SQUARE DELI (14028)

FASHION SQUARE SHERMAN OAKS (14028)

COUSINS CHILDRENSS STORES INC (14030)

COUSINS CHLTDRENS STORES (14030)

MITCHELL M (14031)

MITCHELL MA (14031)

MITCHELL MC (14031)

WHELAN MATTHEW J (14031)

CROW W ILLIAM DR OPTMTRST (14034)

CROW WILLIAM DR OPTOMETRIST (14034)

HAROWOOD OF PALUS VERDES (14036)

HEARTWOOD OF PALOS VERDES (14036)

TALLBEDS (14036)

SHERMAN OAKS (14038)

FROM LOS ANGELES TELEPHONES CALL (14040)

WILLIAM ROBERTS (14040)

FASHION SQUARE (14046)

SILVERWOODS (14046)

Source

1985 (continued)

FROM LOS ANGELES TELEPHONES CALL (14048)

SIMIS FASHIONS (14048)

SIMIS FASHIONS (14048)

SIMISON ERIN L (14048)

FOOT LOCKER (14050)

FOOT MICHAEL J (14050)

BROADWAY THE (14060)

INDEPENDENT OPTOMETRIST THE BROADWAY (14060)

THE BROADWAY SHERMAN OAKS (14060)

MAGUIRE JOE CHEVRON SERVICE (14061)

PAULS PHARMACY (14101)

G T TRAVE L (14103)

GARZA TOURS INTERNATIONAL INC (14103)

GARZA V (14103)

DISTINCTIVE DRY CLEANERS (14105)

SABRA REALTY (14107)

SABRE REALTY (14107)

YASSKIN PHILIP (14107)

FEELING FANCY (14109)

GRACES MANICURING SALON (14109)

SHERMAN OAKS (14110)

SHERMAN OAKS (14110)

CRYSTAL MAKE UP DESIGN (14111)

JUS JUDY COSMETICS (14111)

RUFFLES HAIR DRESSERS (14111)

RUFFNER J 899 (14111)

1986 **RIVERSIDE DR**

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HELFTS FINE APPAREL FOR WOMEN (13942)

HELFTS FINE APPAREL FOR WOMEN (13942)

GIBRALTAR SAVINGS BRANCH SAVINGS OFFICES BURBANK

(13950)

FAITHFUL FOOTMAN SHERMAN OAKS (13984)

BULLOCK S SHERMAN OAKS SHERMAN OAKS (14000)

BULLOCK S TRAVEL BUREAU (14000)

ROBERTS WM JEWELERS MANUFACTURERS & DESIGNERS

SHERMAN OAK (14040)

BROADWAY DEPARTMENT STORES STORES (14060)

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Pacific Bell

FORECAST LAND INVESTMENT CO SHERMAN OAKS (4741)

RIVERSIDE DR

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HELFT S FINE APPAREL FOR WOMEN (13942)

GIBRALTAR SAVINGS BRANCH SAVINGS OFFICES (13950)

BULLOCK S DEPARTMENT STORES (14000)

2096148-6

Source

1990 (continued)

BULLOCK S TRAVEL BUREAU TORRANCE (14000)

ROBERTS WM JEWELERS MANUFACTURERS & DESIGNERS SHERMAN OAK (14040)

BROADWAY DEPARTMENT STORES STORES (14060)

G T TRAVEL SHERMAN OAKS (14103)

GARZA TOURS INTERNATIONAL INC SHERMAN OAKS (14103)

1991 **HAZELTINE AVE**

Pacific Bell

FORECAST LAND INVESTMENT CO (4741)

FROM LOS ANGELES TELEPHONES CALL (4741)

JACOBS HARRY (4824)

OTTE GARY R (4824)

OTTE PETER (4824)

KOGAN ED (4832)

LLVSHIT REBEKKA (4832)

MALEKZADEH ALI A (4832)

MALEKZADEH MANOOCHEHR (4832)

POCHTAR DAVID (4832)

POCHTER M (4832)

DEL BENE ALBERT J (4833)

DEL BENE ROBT (4833)

LA MATDA A (4838)

PIERCE N C (4838)

SCOVILLE DAN & CAROL (4838)

SCOVILLE J J & ASSOCIATES (4838)

SCOVILLE JJ (4838)

SCOVILLEL .5670330 (4838)

HUSTON ST

Pacific Bell

EPSTEIN SOL (13959)

PARKER LEO (13971)

HERKAL WALTER H (14000)

HERKATA PHILIPPUS H (14000)

HERKATA PHILIPPUS H (14000)

MALONEY GORDON E (14012)

KINSEY K (14013)

KINSEY RICHARD (14013)

FRIEDMAN LAZAR (14024)

LA MAIDA ST

Pacific Bell

CHAMPION J (13940)

BRONSTEIN CALVIN (13941)

DUBEN ALAN& MELANIE (13947)

DU BELLIER ALFRED (13947)

TROULMAN DOROTHY (14001)

TROULMAN RAE (14001)

Year Uses 1991 (continued)

KARAZISSIS NICOLAS (14030)

MURIETTA AVE

ATTERBURY GB (4843) HAMOVLTZ LEE D (4901)

HABERMAN NEAL (4907)

GORDON CRAIG (4919)

PEACH GROVE

MENENDEZ VAL M (13929)

FRIMER SOL (13934)

FRIMER SOL PLUMBING & HEATING COMPANY (13934)

FRLMKESS D& M (13934)

HULL KENNETH W (13941)

PAPAZIAN G E (13947)

WILSON W L (13959)

DAMBROSIO JOS (13970)

MOERKE GARY H (13970)

BOOTH WM (13971)

PUGH RICHARD A (13989)

MOSS MITCHELL (14007)

MOSS NORMAN (14007)

RIVERSIDE DR

GORDON MAX (13921)

ALFARO RAMON (13925)

WOERSCHING MARCUS BL (13933)

VICTORIAS SECRET (13936)

VICTORIAS SECRET BATH (13936)

BERNSTEIN LEONARD M (13937)

AICHOTZ V (13939)

AICKINEDI (13939)

FASHION SQUARE SHERMAN OAKS (13942)

FISHER HOWARD (13963)

HUNG TONY (13977)

BULLOCKS (14000)

BULLOCKS SHERMAN OAKS (14000)

BULLOCKS TRAVEL BUREAU (14000)

TRAVEL BUREAU (14000)

DAY RONALD (14009)

BOROWLRWIN (14025)

SILVERSTEIN C (14025)

SILVERSTEIN CYNTHIA (14025)

SILVERSTEIN D (14025)

SILVERSTEIN DAVID (14025)

Source

Pacific Bell

Pacific Bell

Pacific Bell

2096148-6

1991 (continued)

SILVERSTEIN DENNIS (14025)

SILVERSTEIN E GHIS (14025)

COUSINS CHILDRENS STORES (14030)

SHERMAN OAKS FASHION SQUARE (14030)

MITCHELL M (14031)

MITCHELL M&D GHIS (14031)

MITCHELL M&D GHIS (14031)

WHELAN MATTHEW J (14031)

COOKE MICHAEL (14033)

CROW W ILLIAMI DR OPTMTRST (14034)

CROW WILLIAM DR OPTOMETRIST (14034)

ROBERTS FINE JEWELERS (14040)

ROBERTS WM JEWELERS MANUFACTURERS & DESIGNERS (14040)

WILLIAM ROBERTS (14040)

WILLIAMS A LAVW TER (14040)

WILLIAMS A LK VW TER (14040)

WILLIAMS AB (14040)

SHENNAN OAKS FASHION SQUARE (14046)

SILVERWOODS (14046)

FOOT LOCKER STORE (14050)

FOOT LOCKER STORE (14050)

BROADWAY THE (14060)

GRINGOS (14060)

LANCOME INSTITUT DE BEAUTE (14060)

LANCON R (14060)

THE BROADWAY SHENNAN OAKS (14060)

PAULS PHARMACY (14101)

GTTRAVEL (14103)

GARZA TOURS INTERNATIONAL INC (14103)

DISTINCTIVE DRY CLEANERS (14105)

DISTINCTIVE SERVICES INC (14105)

RED CARPET UNIQUE PROPERTIES (14107)

SHERMAN OAKS OFFICE (14110)

N AILS BY KARLA (14111)

RUFFLES HAIR DRESSERS (14111)

1995 **HAZELTINE AVE**

Pacific Bell Telephone

Source

<u>Uses</u> Source <u>Year</u> 1995 (continued) FORECAST LAND INVESTMENT CO (4741) BEST JAMES M (4824) GONZALEZ SERGIO (4824) JACOBS HARRY (4824) **B KOGAN ED (4832) KOGAN E (4832) KRAVTSOVA ESFIR (4838) ELMORE DAVID (4846)** ITKIS DMITRIY (4846) Pacific Bell Telephone **HUSTON ST** EPSTEIN SOL (13959) **PARKER LEO (13971) TOMLAN DENISE (13971)** HERKAL WALTER H (14000) MALONEY GORDON E (14012) FRIEDMAN LAZAR (14024) **RANDALL M (14025)** SHIFLETT S (14025) GOODMAN CHAS (14030) **LA MAIDA ST** Pacific Bell Telephone **CHAMPION J (13940) BRONSTEIN CALVIN (13941)** DUBELL SAM M (13947) DL BELLIER ALFRED (13947) TROULMAN DOROTHY (14001) ARCADI VICTORIA C DC (14012) KARAYAN VARTEVAR (14030) KARAZISSIS NICOLAS (14030) **MURIETTA AVE** Pacific Bell Telephone PARKT JONG D & EUN (4825) ATTERBURY G B (4843) HAHERMAN NEAL (4907) MAGNUSON LORI (4913) WORSDALE MAGGIE (4913) WORSDTLE MAGGII (4913) **GORDON CRAIG (4919)** Pacific Bell Telephone **PEACH GROVE** MENENDEZ VAL M (13929) FRIMER SOL (13934) FRIMER SOL PLUMBING & HEATING COMPANY (13934) **HULL KENNETH W (13941)**

MOERKE GARY H (13970)

1995 (continued)

Pacific Bell Telephone

<u>Source</u>

RIVERSIDE DR

E H EXCAVATION (13933)

HEZI EMANUEL (13933)

BERNSTEIN LEONARD M (13937)

AICHOLTZ V (13939)

BULLOCKS (14000)

BULLOCKS SHERMAN OAKS (14000)

BULLOCKS TRAVEL BUREAU (14000)

STORES (14000)

WEINERS LUGGAGE & FINE GIFTS (14000)

GARDNER PAUL L (14025)

MITCHELL M (14031)

COOKE MICHAEL (14033)

CROW W MDROPTMTRST (14034)

LUCKE RMAN RDROPTMTRST (14034)

LUCKERMAN R S DR (14034)

STERNBERG E DR SPTMTRAT (14034)

ROBERTS FINE JEWELERS (14040)

ROBERTS WM JEWELERS MANUFACTURERS & DESIGNERS

(14040)

WILLIAM ROBERTS (14040)

FOOT LOCKER STORE (14050)

FOOT LOCKER STORE (14050)

STORES (14050)

BRIDAL (14060)

BROADWAY THE (14060)

STORES (14060)

THE BROADWAY SHERMAN OAKS (14060)

PAULS PHARMACY (14101)

DISTINCTIVE DRY CLEANERS (14105)

UNIQUE REALTY (14107)

CLOTHES FOR FUN (14111)

DIVELY GARY P (14111)

GRACES MANICURING SALON (14111)

N AILS BY KARLA (14111)

SAVARIS SALON (14111)

TK NAILS (14111)

HAZELTINE AVE

Pacific Bell Telephone

FORECAST LAND INVESTMENT CO SHERMAN OAKS (4741)

RIVERSIDE DR

Pacific Bell Telephone

BULLOCK S DEPARTMENT STORES (14000)

BROADWAY DEPARTMENT STORES (14060)

<u>Year</u>	<u>Uses</u>	Source
1996	Address Not Listed in Research Source	GTE
1999	Address Not Listed in Research Source	Haines Company
2000	Address Not Listed in Research Source	Pacific Bell Telephone
2001	**HAZELTINE AVE**	Haines & Company, Inc.
	FORECAST LAND CORP (4741)	
	FORECAST LAND INVST (4741)	
	X RIVERSIDE DR (4741)	
	AZIMINITO (4824)	
	KELLERDANA (4824)	
	SABHERWALINDERI H (4832)	•
	REILLYJON O (4833)	
	VANTAYLOR LOU IS (4833)	
	CHAVOLFIROVZ (4835)	
	BETANCOURTEDGAR (4838)	
	FELDMANSAN IORD (4838)	
	HUSTON	Haines & Company, Inc.
	EPSTE 1 NSOI (13958)	
	SEIDMAN BARBARA (13964)	
	TOLENTINOFRDERNCK (13970)	
	RHOADESGREGORY (13971)	
	X RANCHI TO AV (13971)	
	PINCHERLIHAZEL 00 B (14001)	
	HERKALWALLER H (14005)	•
	SIRACUSASIEVE (14006)	
	HOROWITZJEFFREY (14012)	
	MCMILLENBEVERIY (14013)	
	JUNGCARSAN (14018)	
	RAHNAMAMASOUD (14019)	
	FRIEDMANLAZAR (14024)	
	DAVIESMARK (14025)	
	SHIFLETTS (14025)	
	GOOOMANCHAS (14030)	
	BALDERRAMAFRANK (14031)	
	X MURIETTAAV (14031)	
	LA MAIDA	Haines & Company, Inc.
	OCASINOGENE (13928)	
	SAYADIANLARIS (13929)	
	CAMPBELLMRCHAEL (13934)	
	D 4 G 7 / 4 G 1 / G 1 G 1 G 1 G 1 G 1 G 1 G 1 G 1 G	

RACZYNSKIRAY (13935)

2001 (continued)

GORDONJOHN (13940)

BRONSTEINK (13941)

KAITBARBARA H (13946)

DUBELLIERHELGA (13947)

LAFFITTEDIANNE (13953)

WERNERKETH (13958)

DODYLE EDWARD (13959)

PHILLIPSANDREW (13964)

CAIOZZOMICHAEL (13965)

ROCKEY RONALD (13970)

SABICMARTA (13970)

COULTERJODY (13971)

X MURIETTA AV (13971)

WINEGARDNER DONALD (14000)

MANLAPAZJOSE (14006)

MILLER SYLVIA (14006)

LEVERHANTEVELYN (14007)

XXXX (14012)

ROSSMICHELLE (14013)

MOORERLEELLA (14019)

KENTMARTN (14024)

SMITH KENNELH (14024)

BRAUNSTEIN DOROLHY (14025)

8 KARAZISSISNICALAS (14030)

MRGUDICANTE (14031)

WEALTH CODE (14031)

MURIETTA AVE

ATTORNEYJOHN (4801)

ATTY&COUNSELOR (4801)

PALADIN ATTORNEYJOHN (4801)

PALADIN PALADINJOHN (4801)

ROBINSON GLONA (4807)

SCHWARTZANDREW (4811)

STONESARAH (4811)

MYCKCONSTANCE (4813)

SALOMONSSLANLEY (4819)

CASKS (4825)

SWARDENLS (4831)

ATTERBURYGB (4843)

DAVIS LINDA (4849)

MADORMOANLHONY J (4855)

X HUSTON (4855)

X PEACH GROVE (4855)

Source

Haines & Company, Inc.

2001 (continued)

HABERMANNEAL (4907)

ROSENBERGCHARLES (4913)

GORDONCRAIG (4919)

RIVERSIDE DR

XXXX (13918)

SULLIVAN PETER (13919)

XXXX (13920)

MCLEANEBEVERLY 00 C (13921)

KINOJOHN 00 C (13925)

XXXX (13926)

DUPOUY VICTORIA (13927)

WELDON CYNTHIA (13927)

WELDONLYLE (13927)

G 13931 DUPOUYVFICANA (13927)

P 13929 STONESOBERT (13927)

XXXX (13932)

XXXX (13934)

BERNSTEIN LEONARD M (13937)

XXXX (13939)

XXXX (13940)

GELLER IVING (13941)

XXXX (13942)

BENDERJUDITH (13943)

XXXX (13946)

XXXX (13950)

HUNTHOPE (13953)

RIVERSIDE DR 91423 CONT (13953)

XXXX (13954)

LING STEPHEN (13955)

LINGALICE T (13955)

OTHALER HOWARD (13955)

WECHTERMARY (13957)

XXXX (13958)

OHEYMANOONI (13959)

ODUROSEAMY (13961)

ODUROSEDOUG (13961)

XXXX (13963)

SANDERSJUDY (13965)

EPHRAIMHAMET (13967)

BAILEYVICTORIA (13969)

BYRDKELLY (13969)

XXXX (13970)

WEAVERLAUREE (13971)

Source 5 4 1

Haines & Company, Inc.

2001 (continued)

TYNDALLOONNA (13973)

XXXX (13974)

XXXX (13976)

0 HUNG FRANKLIN (13977)

XXXX (13978)

BRINKPAUL (13979)

X MURIETTAAV (13982)

XXXX (13982)

FASHION SOUARE (14000)

KENSINGTON (14000)

LOUIS VUL TON AT (14000)

LUGGAGE KIPLING (14000)

MACYS MACYS (14000)

SHERMANOAKS (14000)

XXXX (14001)

XXXX (14003)

XXXX (14004)

HARRISBARBARA (14007)

XXXX (14009)

XXXX (14013)

XXXX (14014)

MCINERNEYJANICE M N 6187A (14025)

XXXX (14027)

WHELANMATT (14031)

XXXX (14033)

XXXX (14034)

XXXX (14038)

ROBERTSFINEJWLRS (14040)

XXXX (14046)

XXXX (14050)

XXXX (14052)

BLOOMINGDALES (14060)

CHRISTMASTREES (14061)

SANTA 8 SONS 8 B (14061)

X HAZELTINEAV (14061)

PAULS PHARMACY 8184 S 04549 (14101)

TOTAL NUTRITION (14103)

DISTINCTIVE DRYCLN 0818 74 78 M (14105)

FARMBOY (14107)

2003 Address Not Listed in Research Source

Haines & Company

2004 Address Not Listed in Research Source

Source Year Uses 2006 Haines Company **HAZELTINE AVE** FORECASTLAND CORP (4741) FORECASTLAND INVST (4741) IVANOVIVAN (4824) RUVO MARIA (4824) SPURLIN NISSA (4824) PINES MARIA LENY (4832) O REILLYJON D (4833) CAMARA MARAMA (4838) LZARROBSA (4838) **ROBINSON EMELL (4838) GONZALEZGRENDA (4846) ROBBINS REBECCA (4846)** **HUSTON ST** Haines Company **HEIMIANNJURGEN (13958)** SCHARFF KEVIN (13959) NO CURRENT LISTING (13964) E DAVILA THEODORE (13965) **BANCROFT (13970)** CHRDSTOPHER A (13970) RHOADESGREGORY (13971) O HERKAL WAER H B (14000) E PINCHERLI HAZEL (14001) O SIRACUSASTEVE (14006) CASE JOHN (14007) **HOROWITZ JEFFREY (14012)** O BEHRENDTJACK Z (14013) NO CURRENT LISTING (14018) KHALIU FALBA (14019) KHALIU FARLBA (14019) O RAHNAMAAMASOUD (14019) FRIEDMAN LAZAR (14024) O CHOUTUK (14025)

LESSEM JEREMY (14030) BALDERRAMA FRANK (14031) TOUCHETTE LOIS (14031)

E AMIRIAN ROZLTA (13928) A DODGE NELSON (13929) LEVITT ADAM (13934) RACZYNSKI RAY (13935) O POTK IN BRIAN (13940) BRONSTEIN K (13941)

LA MAIDA ST

2006 (continued)

KING JUANITA (13946)

ESRA RITCH (13947)

JONES STEPHEN (13947)

NO CURRENT LISTING (13952)

ABC AUTHORIZED (13959)

APPLNC RPR (13959)

STERIN YEVGENY (13959)

NO CURRENT LISTING (13964)

E CAIOZZO CALLY (13965)

A WERNER KEITH 00 A (13966)

A SABUIC MARTA (13970)

0 COULTER JODY (13971)

A LAFFITTEDIANNE (13983)

WINEGARNERD (14000)

MANLAPAZJOSE (14006)

MILLER SYLVIA (14006)

NO CURRENT LISTING (14007)

NO CURRENT LISTING (14012)

A ROSS MICHELLE (14013)

A MOORER LEELA (14019)

JOHNS CEARMLLE (14024)

O ARNOLD JASON (14025)

E KARAZISSISNICOLAS (14030)

E BASSAR I (14031)

MURIETTA AVE

A JETER MARTENA (4801)

A LOESER SHAUN W (4807)

A COULTER BENEDICT (4813)

A SALOMONS STANLEY (4819)

A GIL BEN (4825)

NO CURRENT LISTING (4831)

KESSELP (4837)

A RUSSO ROBERT (4837)

A FELDMAN GARRET (4843)

NO CURRENT LISTING (4849)

A MADORMO ANTHONY J (4855)

HAMOVITZ LEE D (4901)

HABERMAN (4907)

MADELEINE (4907)

A ROSENBERG CHADES (4913)

GORDON GAYLE (4919)

PEACH GROVE ST

A HRONSKY YUD (13928)

Source

Haines Company

Source

Year Uses

2006 (continued)

MENENDEZVA IM (13929)

GREFSRUD BRIAN (13934)

WICKSTRAM CARI C (13935)

EASHOO ALBERT (13940)

NO CURRENT LISTING (13941)

A HADAWARLAB IBEH (13946)

PAPAZIAN GE (13947)

A METZNER DAVID (13952)

REMSONS (13953)

NO CURRENT LISTING (13958)

OCONNER (13959)

A OCONNOR STEPHEN 00 A (13959)

A LUCKL ETAM I 00 A (13964)

A KOSUGISHIGEO (13965)

RUJZ GEORGE (13970)

0 BOOTH WM (13971)

A HANANEL JEFFREY (14000)

AGSABIAN MICHAEL (14006)

A FOWNES BRANDON (14007)

HOY ROBEDT (14012)

UNELL RACHEL (14018)

NO CURRENT LISTING (14019)

A SHERMAN TINA (14024)

KATZ MICHAEL 00 A (14030)

A NEUCHARGAL I (14031)

RIVERSIDE DR

WILLSON MARION (13907)

MANKOVITZ ALAN (13909)

WINNICKJOLE (13911)

FALZONECRAIG (13913)

DOUSTAN LIII (13919)

LITVINOVOIEG (13921)

KING JOHN (13925)

MACDONALD (13925)

NORMAN C LAW OF (13925)

A RODEN MICHAEL (13927)

E DOBSON PETER (13929)

MARCUS (13933)

WOERSCHING (13933)

WESTMAN STEVE (13937)

GELLERPAUL (13941)

MEYERS SI (13947)

SPERBER SHARON (13949)

E ANDERSON JAMES (13949)

A LEE CHRIS HNA (13951)

O HUNT HOPE (13953)

O LINGAUICE T (13955)

SIDELL DAVID (13959)

O HEYMANDON I (13959)

BRAOBROOKE (13961)

NICHOLAS (13961)

A YOKOYAMA KRISTA (13963)

SANDERSJUDY (13965)

O KANTNER MILDRED (13969)

WEAVER LAUREE (13971)

CARSONI (13973)

CHIANG MICHAEL (13975)

HUNG FRANKLIN (13977)

ELIZABETH ARDEN (14000)

SALON (14000)

GREGORY BRUCE (14003)

BLOOMINGDALES (14060)

FARM BOY (14107)

DIGITAL RANCH (14110)

<u>Source</u>



The EDR Environmental Lien **Search Report**

WESTFIELD SHOPPING CENTER 14006 RIVERSIDE DRIVE SHERMAN OAKS, CALIFORNIA

Friday, December 14, 2007

Project Number: L07-10686

The Standard In Environmental Risk Management Information

440 Wheelers Farm Road Milford, Connecticut 06460

Nationwide Customer Service

Telephone: 1-800-352-0050 Fax: 1-800-231-6802

ENVIRONMENTAL LIEN REPORT

The EDR Environmental LienSearch Report provides results from a search of available current land title records for environmental cleanup liens and other activity and use limitations, such as engineering controls and institutional controls.

A network of professional, trained researchers, following established procedures, uses client supplied property information to:

- search for parcel information and/or legal description;
- · search for ownership information;
- research official land title documents recorded at jurisdictional agencies such as recorders' office, registries of deed, county clerks' offices, etc.;
- · access a copy of the deed;
- · search for environmental encumbering instrument(s) associated with the deed;
- provide a copy of any environmental encumbrance(s) based upon a review of key words in the instrument(s) (title, parties
 involved and description); and
- · provide a copy of the deed or cite documents reviewed;

Thank you for your business

Please contact EDR at 1-800-352-0050 with any questions or comments

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ENVIRONMENTAL LIEN REPORT

The EDR Environmental Lien Search Report is intended to assist in the search for environmental liens filed in land title records.

TARGET PROPERTY INFORMATION

ADDRESS

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, California

RESEARCH SOURCE

Source: Los Angeles County Assessor Los Angeles County Recorder

DEED INFORMATION

Type of Instrument: Grant Deed

Title is vested in: Sherman Oaks Fashion Associates, LP, a Delaware limited partnership

Title received from: City Freeholds (U.S.A.) Inc., a California corporation

Deed Dated: 03/18/1996 Deed Recorded: 03/19/1996 Instrument: 96-430956

LEGAL DESCRIPTION

Portion of Lot 1, in Tract 20915, according to the map or plat thereof, as filed of record in Book 663, Page 34 of Maps, Los Angeles County, State of California

Assessor's Parcel Number(s): 2269-025-032

ENVIRONMENTAL LIEN

Environmental Lien: Found ☐ Not Found ☒

OTHER ACTIVITY AND USE LIMITATIONS (AULs)

Other AULs: Found Not Found

96 430956

RECORDING REQUEST BY

WHEN RECORDED MAIL TO

MAMI

Allan G. Mutchnik, Esq. Skadden, Arps, Slate, Meagher & Flom 300 South Grand Avenue, Suite 3400 Los Angeles, California 90071 RECORDED/FILED IN OFFICIAL RECORDS
RECORDER'S OFFICE
LOS ANGELES COUNTY
CALIFORNIA

MAR 19 1996

AT 8 A.M.

SPACE ABOVE THIS LINE RESERVED FOR RECORDER'S USE

TITLE(S)

FEES 400°P

12

SURVEY MONUMENT FEE \$10. CODE \$9

GRANT DEED

THANK FFR TAX NOT A PUBLIC RECORD

R4**38** 6/94

RECORDING REQUESTED BY AND WHEN RECORDED RETURN TO: Allan G. Mutchnik, Esq. Skadden, Arps, Slate, Meagher & Flom 300 South Grand Avenue, Suite 3400 Los Angeles, California 90071

SURVEY MONUMENT FEE \$10. CODE 9

(Space Above This Line For Recorder's Use)

GRANT DEED

TRANSFER TAX
NOT A PUBLIC RECORD

A.P.N.:

2269-025-004, 2269-025-005. 2269-025-006, 2269-025-007, 2269-025-008, 2269-025-009, 2269-025-010, 2269-025-011, 2269-025-023, 2269-025-024, and 2269-025-026.

The undersigned Grantor declares:

Documentary transfer tax is: (Not Shown Pursuant to R&T Code Section 11932)

- (X) computed on full value of property conveyed, or
- computed on full value less value of liens and encumbrances remaining at time of sale.
- () Unincorporated area

(X) City of Los Angeles, and

FOR VALUABLE CONSIDERATION, the receipt and sufficiency of which is hereby acknowledged, City Freeholds (U.S.A.) Inc., a California corporation ("Grantor"), has granted, sold and conveyed, and by these presents does hereby grant, sell and convey, unto Sherman Oaks Fashion Associates, LP, a Delaware limited partnership ("Grantee"), all of Grantor's interest in and to that certain property located in the City of Los Angeles, County of Los Angeles, State of California, and more particularly described on Exhibit "A" attached hereto and incorporated herein by reference (the "Property").

TO HAVE AND TO HOLD the Property, together with all and singular the rights and appurtenances thereto in anywise belonging unto the said Property, subject to non-delinquent taxes and assessments and all other matters of record.

0064904.01-21**52**a

96 430956

9500694-09

....

IN WITNESS WHEREOF, Grantor has caused this Grant Deed to be executed this 18 day of March, 1996.

> CITY FREEHOLDS (U.S.A.), INC., a California corporation

By: Elacle Villewith
Name: Charles V. Thornton
Its: Assistant Secretary

0064904 01-2152a

(SEAL)



0064904.01-21SZa

2

STATE OF
COUNTY OF Los Hoyels)
On the H day of <u>HACCH</u> , 1996, before me,
PRIAL PICKERING NOTION , personally appeared
□ personally known to me or reproved to me on the basis of satisfactory evidence to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.
WITNESS my hand and official seal.
(SEAL)

96 430956

0061904 01-2152a

EXHIBIT "A"

LEGAL DESCRIPTION OF PROPERTY

PARCEL 1:

That portion of Lot 1 of Tract No. 20915, in the City of Los Angeles, County of Los Angeles, State of California, as per map recorded in Book 663 Page 34 of Maps, in the office of the County Recorder of said County, now shown as Parcel "B" of Parcel Map. Los Angeles No. 3284, as per Parcel Map filed on December 31, 1976 in Book 75 Pages 12, 13 and 14 of Parcel Maps, and Parcel "C" of Parcel Map Los Angeles No. 2013, as per Parcel Map filed in Book 27 Page 92 of Parcel Maps, all filed in the office of the County Recorder of said County.

Except from said Parcel "C" of Parcel Map No. 2013 the North 62 feet of the West 324 feet thereof.

Excepting all building and improvements now or hereafter situated thereon.

PARCEL 1A:

A 20% fee interest in all buildings, structures and improvements, including footings, foundation and supports therefore situated upon that portion of Parcel "B" of Parcel Map No. 3284, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 75, Pages 12, 13 and 14 of Parcel Maps, in the office of the County Recorder of said County described as follows:

Beginning at the most Southerly corner of said Parcel B; thence along the Southerly line of said Parcel North 88° 06' 28" West 442.81 feet; thence North 0° 00' 10" East 176.49 feet; thence North 88° 08' 25" West 4.95 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 224.26 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 186.82 feet; to the Westerly line of Parcel B, Parcel Map L.A. No. 2013 as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of said County Recorder; thence South a distance of 176.74 feet to the point of beginning.

Said portion is shown as "A Deck - Phase 1" on Sheet 4 of Grant Deed recorded April 30, 1979 as Instrument No. 79-460487 of Official Records.

PARCEL 1B:

All buildings, structures and improvements, including footings, foundations and supports therefore, situated upon the above described Parcel 1.

Excepting therefrom 80% fee interest in all buildings, structures and improvements, including footings, foundations and supports therefore situated upon that portion of Parcel 'B' of Parcel Map No. 3284, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 75 Pages 12, 13 and 14 of Parcel Maps, in the office of the County Recorder of said County described as follows:

Beginning at the most Southerly corner of said Parcel B: thence along the Southerly line of said Parcel North 88° 06' 28" West 442.81 feet; thence North 0° 00' 10" East 176.49 feet; thence North 88° 08' 25" West 4.95 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 224.26 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 186.82 feet; to the Westerly line of Parcel B, Parcel Map L.A. No. 2013 as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of said County Recorder; thence South a distance of 176.74 feet to the point of beginning.

Said portion is shown as "A Deck - Phase I" on Sheet 4 or Grant Deed recorded April 30, 1979 as Instrument No. "9-460487 or Official Records.

PARCEL 2:

An undivided 20% fee interest in all buildings, structures, and improvements, including footings, foundations and supports therefore, situated upon that portion of Parcel B. Parcel Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County, described as follows:

Beginning at the most Southwesteriy corner of said Parcel B: thence along the Westerly line of said Parcel North a distance of 176.74 feet; thence South 88° 08' 25" East 81.08 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 140.59 feet; thence North 1° 51' 35" East 12.33 feet; thence South 88° 08' 25" East 18.33 feet; thence South 1° 51' 35" West 12.33 feet; thence South 88° 08' 25" East 44.95 feet; thence South 1° 51' 35" West 176.91 feet to the Southerly line of said Parcel B; thence along said Southerly line North 88° 06' 28" West 466.28 feet to the point of beginning.

Said portion is shown as "A Deck - Phase II" on Exhibit "A" of Grant Deed recorded April 30, 1979 as Instrument No. 79-460488 of Official Records.

PARCEL 2A:

An undivided 20% fee interest in all buildings, structures and improvements, including footing, foundations and supports therefore, situated upon that portion of Parcei B. Parcei Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County, described as follows:

Beginning at the most Northeasteriy corner of said Parcel B; thence along the Southeriy line of Riverside Drive. 100.00 feet wide, as shown on said Parcel Map. West a distance of 281.00 feet; thence South a distance of 19.25 feet to the true point of beginning; thence continuing South a distance of 152.94 feet; thence West a distance of 197.00 feet; thence North a distance of 18.54 feet; thence West a distance of 13.40 feet; thence North a distance of 37.21 feet; thence East a distance of 13.40 feet; thence North a distance of 32.70 feet; thence East a distance of 38.65 feet; thence North a distance of 14.49 feet; thence East a distance of 158.35 feet to the true point of beginning.

Said portion is shown as 'B Deck - Phase I' on Page 3 or Deed recorded on April 30, 1979 as Instrument No. 79-460491 of Official Records.

PARCEL 2B:

An undivided 20% fee interest in all buildings, structures and improvements, including footings, foundations and supports therefore, situated upon that portion of Parcel B. Parcel Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County, described as follows:

Beginning at the most Northeasterly corner of said Parcel B; thence along the Southerly line of Riverside Drive. 100.00 feet wide, as shown on said Parcel Map. West a distance of 281.00 feet; thence South a distance of 172.19 feet to the true point of beginning; thence West a distance of 197.00 feet; thence South a distance of 163.44 feet; thence West a distance of 14.32 feet; thence South a distance of 14.93 feet; thence East a distance of 55.02 feet; thence North a distance of 54.40 feet; thence East a distance of 156.30 feet; thence North a distance of 123.97 feet to the true point of beginning.

Said portion is shown as "B Deck - Phase II" on Page 4 of Deed recorded on April 30, 1979 as Instrument No. 79-460491 of Official Records.

PARCEL 2C.

An undivided 20% fee interest in all buildings, structures and improvements, including footings, foundations and supports therefore, situated upon that portion of Parcel B. Parcel Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County, described as follows:

Beginning at the most Northeasterity corner of said Parcei B; thence along the Southerly line of Riverside Drive. 100.00 feet wide, as shown on said Parcel Map. West a distance of 231.00 feet; thence South a distance of 19.25 feet to the true point of beginning; thence continuing South a distance of 276.91 feet; thence East a distance of 226.16 feet; thence North a distance of 286.74 feet; thence West a distance of 15.17 feet; thence South a distance of 5.54 feet; thence West a distance of 4.79 feet; thence South a distance of 4.29 feet; thence West a distance of 206.20 feet to the true point of beginning.

PARCEL 3.

That portion of Parcel B. Parcel Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per map filed in Book 27 Page 92 of Parcel Maps, records of Los Angeles County, described as follows:

Beginning at the most Southwesteriy corner of Parcel B of said Parcel Map L.A. No. 2013; thence North along the most Westerly line of said Parcel B 227.27 feet to the Northerly terminus thereof and the true point of beginning; thence East 69.87 feet along the Northerly line of said Parcel B; thence South 40.00 feet; thence West 69.87 feet, parallel with said Northerly line of Parcel B to a point on said most Westerly line of Parcel B 40.00 feet South of the Northerly terminus thereof; thence North 40.00 feet to the true point of beginning.

Together with that portion of Parcel B of said Parcel Map L.A. No. 2013, described as follows:

Beginning at the most Southwesterly corner of Parcel "B" of said Parcel Map L.A. No. 2013; thence North along the most Westerly line of said Parcel B 227.27 feet to the Northerly terminus thereof; thence East 479.97 feet along the Northerly line of said Parcel B to the true point of beginning; thence West along said Northerly line 270.00 feet; thence South 39.96 feet; thence West 20.00 feet; thence South 16.95 feet to the Northerly face of a three-level parking structure; thence South 88° 04' 32" East 300.17 feet along the Northerly face of said parking structure; thence North 20.00 feet; thence West 10.00 feet; thence North 46.99 feet to the true point of beginning.

PARCEL +

That portion of Parcel B of Parcel Map L.A. No. 2013, in the City of Los Angeles, County of Los Angeles, State of California, as per map recorded in Book 27 Page 92 of Parcel Maps, records of said County, which lies above an elevation of 679,00 feet and below an elevation of 716,00 feet, in each case above mean sea level per Los Angeles City Engineer Vertical Datum, 1980 Adjustment, described as follows:

Beginning at the Southerly terminus of that certain course in the Westerly line of said Parcel B; shown on said Map as having a bearing and length of North 323.00 feet;

Thence North 76.00 feet along said certain course to the true point of beginning.

Thence continuing North along said certain course 196.00 feet: thence East 58.10 feet:

Thence North 10.17 feet: thence East 13.00 feet:

Thence South 221.34 feet: thence West 9.67 feet:

Thence North 15.17 feet: thence West 61.43 feet to the true point of beginning.

Said elevations are based on City Engineers Bench Mark No. 08 17508, wire spike 2.6 feet South of South curb line of Riverside Drive near B.C. Curb return East of Hazeltine Avenue at West end of Catch Basin: Elevation 645.241 feet, 1980 Adjustment.

PARCEL 5:

A non-exclusive easement(s) upon the common area(s) as defined in that certain document entitled "Amended and Restated Easement. Restriction and Operating Agreement", dated 12/30/88, by and between Bullock(s) Properties, Corp., a Delaware corporation, Carter Hawley Hale Stores, Inc., a Delaware corporation and Freeholds (U.S.A.), Inc., a California corporation, recorded June 30, 1989 as Instrument No. 89-1048092, Official Records, as said easement affects portions of the following described property:

(A) That portion of Lot 1 of Tract No. 20915, in the County of Los Angeles, State of California, as per map recorded in Book 663, Page 34, of Maps, in the office of the

County Recorder of said County, now shown as Parcei 'A" of Parcei Map, Los Angeles No. 3284, as per Parcei Map filed on December 31, 1976 in Book 75, Page 12, 13 and 14 of Parcel Maps, in the office of the County Recorder of said County.

PARCEL 6:

A non-exclusive easement(s) upon the common areas—as defined in that certain document entitled "Amended and Restated Easement, Restriction and Operating Agreement idated 12,30/88, by and between Bullock(s) Properties, Corp., a Delaware corporation, Carter. Hawley Hale Stores, Inc., a Delaware corporation and City Freewolds (U.S.A.), Inc., a California corporation, recorded June 30, 1989 as Instrument No. 39-1048092, Official Records, as said easement affects portions of the following described property:

(A) Parcel B of Parcel Map L.A. No. 2013, in the County of Los Angeles, State of California, as per Parcel Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County.

EXCEPT therefrom that portion of said Parcel B described as follows:

Beginning at the most Southwesterly corner of said Parce: B: thence North along the most Westerly line of said Parcel B 227.27 feet to the Northerly terminus thereof and the true point of beginning; thence East 69.87 feet along the Northerly line of said Parcel B: thence South 40.00 feet; thence West 69.87 feet, parallel with said Northerly line of Parcel B to a point on said most Westerly line of Parcel B 40.00 feet South of the Northerly terminus thereof; thence North 40.00 feet to the true point of beginning.

ALSO EXCEPT that portion of said Parcel B described as follows:

Beginning at the most Southwesterly corner of said Parce: B: thence North along the most Westerly line of said Parce! B 227.27 feet to the Northerly terminus thereof; thence East 479.97 feet along the Northerly line of said Parce! B to the true point of beginning; thence West along said Northerly line 270.00 feet: thence South 39.96 feet: thence West 20.00 feet: thence South 16.95 feet to the Northerly face of a three-level parking structure; thence South 88° 04' 32" East 300.17 feet along the Northerly face of said parking structure; thence North 20.00 feet: thence West 10.00 feet: thence North 46.99 feet to the true point of beginning.

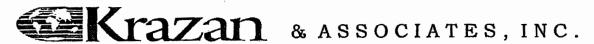
ALSO EXCEPT all buildings, structures and improvements, including footings, foundations and supports therefor, situated upon the land, as it existed on April 30, 1979, which buildings, structures and improvements are, and shall remain real property, as said buildings, structures and improvements, including footings, foundations and supports were granted to Fashion Square Associates, an Illinois limited partnership, as to an undivided 20% interest, by deed recorded April 30, 1979 as Instrument No. 79-460488 of Official Records, and to CHH Realty, Inc., a Delaware corporation, as to an undivided 80% interest, by deed recorded April 30, 1979 as Instrument No. 79-460489 of Official Records.

ALSO EXCEPT all buildings, structures and improvements, including footings, foundations and supports therefor, situated upon the land, as it existed on April 30, 1979, which buildings, structures and improvements are, and shall remain real property, as said buildings, structures and improvements, including footings, foundations and supports were granted to Fashion Square Associates, an Illinois limited partnership, as to an undivided 20% interest, by deed recorded April 30, 1979 as Instrument No. 79-460491 of Official Records, and to CHH Realty, Inc., a Delaware corporation, as to an undivided 80% interest, by deed recorded April 30, 1979 as Instrument No. 79-460492 of Official Records.

B) The North 62.00 feet of the West 324.00 feet of Parcel C of Parcel Map L.A. 2013, in the County of Los Angeles, State of California, as per Map filed in Book 27 Page 92 of Parcel Maps, in the office of the County Recorder of said County.

APPENDIX D

KRAZAN & ASSOCIATES, INC. GEOTECHNICAL ENGINEERING INVESTIGATION



GEOTECHNICAL ENGINEERING • ENVIRONMENTAL ENGINEERING CONSTRUCTION TESTING & INSPECTION

September 27, 2006

KA Project No. 112-06041

Mr. Chris Kitchen
Westfield Corporation, Inc.
11601 Wilshire Boulevard, 10th Floor
Los Angeles, CA 90025

RE:

Geotechnical Engineering Investigation Proposed Fashion Square Expansion Woodman Avenue and Riverside Drive Sherman Oaks, California

Dear Mr. Kitchen:

In accordance with your request and authorization, we have completed our Geotechnical Engineering Investigation for the above-referenced site. This report summarizes the results of our field investigation, laboratory testing and engineering analyses. Based on the data obtained, our understanding of the proposed project and our engineering analyses, it is our opinion that it is feasible to develop the site as planned.

As noted in our report, Krazan & Associates should be retained to review project plans and specifications prior to the start of construction, and to observe and test earthwork and foundation construction. Observation and testing services should also be performed by our field staff during construction activities will allow us to compare conditions exposed during construction with those encountered during our investigation and to present supplemental recommendations if warranted by different site conditions.

If you have any questions regarding the information or recommendations presented in our report, or if we may be of further assistance, please contact our Ontario, California office at (909) 974-4400.

Respectfully submitted, KRAZAN & ASSOCIATES, INC.

James Kellogg

James M. Kellogg, PE Regional Manager

cc: Addressee (4)

GEOTECHNICAL ENGINEERING INVESTIGATION PROPOSED FASHION SQUARE EXPANSION WOODMAN AVENUE AND RIVERSIDE DRIVE SHERMAN OAKS, CALIFORNIA

PROJECT No. 112-06041 SEPTEMBER 27, 2006

PREPARED FOR:

WESTFIELD CORPORATION, INC. 11601 WILSHIRE BOULEVARD, 10TH FLOOR LOS ANGELES, CALIFORNIA 90025

ATTENTION: Mr. CHRIS KITCHEN

PREPARED BY:

Krazan & Associates, Inc. 4221 Brickell Street Ontario, California 91761 (909) 974-4400 GEOTECHNICAL ENGINEERING • ENVIRONMENTAL ENGINEERING CONSTRUCTION TESTING & INSPECTION

GEOTECHNICAL ENGINEERING INVESTIGATION PROPOSED FASHION SQUARE EXPANSION WOODMAN AVENUE AND RIVERSIDE DRIVE SHERMAN OAKS, CALIFORNIA

TABLE OF CONTENTS

INTRODUCTION	1
PURPOSE AND SCOPE OF SERVICES	1
PROPOSED CONSTRUCTION	2
SITE LOCATION AND SITE DESCRIPTION	2
SITE INVESTIGATION	3
GEOLOGIC SETTING	3
FIELD AND LABORATORY INVESTIGATIONS	
SOIL PROFILE AND SUBSURFACE CONDITIONS	
GROUNDWATER	
SEISMICITY AND LIQUEFACTION POTENTIAL	4
SOIL CORROSIVITY	5
CONCLUSIONS AND RECOMMENDATIONS	6
Administrative Summary	6
GROUNDWATER INFLUENCE ON STRUCTURES/CONSTRUCTION	7
SITE PREPARATION	7
Engineered Fill	
TEMPORARY EXCAVATION STABILITY	
UTILITY TRENCH LOCATION, CONSTRUCTION AND BACKFILL	
COMPACTED MATERIAL ACCEPTANCE	
SURFACE DRAINAGE AND LANDSCAPING	
DEEP FOUNDATION	
FLOOR SLABS AND EXTERIOR FLATWORK	
RETAINING WALLS	
PAVEMENT DESIGN	
SITE COEFFICIENT	
SOIL CORROSIVITY TESTING AND INSPECTION	
LIMITATIONS	14
VICINITY MAP	Figure 1
SITE PLAN	Figure 2

KA No. 112-06041 Page No. 2

PILE CAPACITY CHART	Figure 3
FIELD AND LABORATORY INVESTIGATIONS	Appendix A
GENERAL EARTHWORK SPECIFICATIONS	Appendix B
GENERAL PAVEMENT SPECIFICATIONS	Appendix C

GEOTECHNICAL ENGINEERING • ENVIRONMENTAL ENGINEERING CONSTRUCTION TESTING & INSPECTION

September 27, 2006

KA Project No. 112-06041

GEOTECHNICAL ENGINEERING INVESTIGATION PROPOSED FASHION SQUARE EXPANSION WOODMAN AVENUE AND RIVERSIDE DRIVE SHERMAN OAKS, CALIFORNIA

INTRODUCTION

This report presents the results of our Geotechnical Engineering Investigation for the proposed mall expansion in Sherman Oaks, California. Discussions regarding site conditions are presented herein, together with conclusions and recommendations pertaining to site preparation, grading, utility trench backfill, drainage and landscaping, foundations, concrete floor slabs and exterior concrete flatwork, retaining walls, soil corrosivity, and pavement design.

A Vicinity Map showing the location of the site is presented on Figure 1. A Site Plan showing the approximate boring locations is presented on Figure 2. Descriptions of the field and laboratory investigations, boring log legend and boring logs are presented in Appendix A. Appendix A contains a description of the laboratory-testing phase of this study, along with the laboratory test results. Appendices B and C contain general guides for earthwork and flexible pavement specifications. If conflicts in the text of the report occur with the general specifications in the appendices, the recommendations in the text of the report have precedence.

Two Foundation Investigation reports prepared by R.T. Frankian and Associates for the parking structures "A" and "B" (Job No. 25074-F, dated November 24, 1975) and the department store expansion (Job No. 95-113-W, dated January 31, 1996) were provided to us. Applicable information included in these two reports has been utilized for our analyses and foundation recommendations.

<u>PURPOSE AND SCOPE OF SERVICES</u>

This geotechnical investigation was conducted to evaluate subsurface soil and groundwater conditions at the project site. Engineering analysis of the field and laboratory data was performed for the purpose of developing and providing geotechnical recommendations for use in the design and construction of the earthwork, foundation and payement aspects of the project.

Our scope of services was outlined in our proposal dated May 25, 2006 (KA Proposal No. PC299-06) and included the following:

• A site reconnaissance by a member of our engineering staff to evaluate the surface conditions at the project site.

- Review of selected published geologic maps, reports and literature pertinent to the site and surrounding area.
- A field investigation consisting of drilling ten (10) borings to depths ranging from 6 to 50 feet below the existing ground surface for evaluation of the subsurface conditions at the project site. The depth of investigation was limited due to auger refusal in 5 borings.
- Performing laboratory tests on representative soil samples obtained from the borings to evaluate
 the physical and index properties of the subsurface soils.
- Evaluation of the data obtained from the investigation and engineering analyses of the data with respect to the geotechnical aspects of structural design, and site grading and paving.
- Preparation of this report summarizing the results, conclusions, recommendations, and findings
 of our investigation.

PROPOSED CONSTRUCTION

Based on our review of the site plan and our discussions with the project representative, we understand that the proposed project will include demolition of two parking structures and construction of a new addition to the south and east of the existing mall buildings. The addition is planned to be of two to three -story, reinforced concrete construction with a slab-on-grade floor. Structural loads of the building were not available at the time of this investigation. Maximum column loads are assumed to be less than 350 kips.

Mass grading of the site is expected to entail minor cuts and fills from the existing grades to establish the building pad and to provide surface drainage of the site.

In the event these structural or grading details are inconsistent with the final design criteria, we should be notified so that we can evaluate the potential impacts of the changes on the recommendations presented in this report and provide an updated report as necessary.

SITE LOCATION AND SITE DESCRIPTION

The existing Fashion Square Mall is bounded by Woodman Avenue to the east, Freeway 101 to the south, Hazeltine Avenue to the west and Riverside Drive to the north in the City of Los Angeles (see Vicinity Map, Figure 1).

The mall consists of two major stores, Macy's and Bloomingdale's, a food court, a two-level enclosed mall between the two major stores, two parking structures, and a ground level parking lot. The site is relatively level with no major changes in grade. The average elevation of the site is approximately 640 feet above mean sea level.

SITE INVESTIGATION

GEOLOGIC SETTING

The site is underlain by Holocene and Pleistocene alluvium deposited in the San Fernando Valley, a structural basin surrounded by mountains on all four sides. The alluvium is estimated to be several hundred feet thick. These deposits are generally fine grained, consisting of mixtures of clay, silt and sand. Deposits encountered on the subject site during exploratory drilling are discussed in detail in this report.

Southern California is seismically active and will experience future earthquakes that will affect the project site. The earthquakes are predominately generated by periodic slip along the northwesterly trending faults associated with the San Andreas fault system and the east-west trending faults along the northern margin of the Los Angeles Basin. In addition to these probable earthquake sources, recent earthquakes in the region have occurred on previously unknown faults having no surface expression (1987 Whittier Narrows and the 1994 Northridge earthquakes). The Seismic hazard most likely to impact the site is groundshaking due to a large earthquake on one of the major active regional faults. The Hollywood Fault is the nearest active fault to the site, and is located approximately 4.8 kilometers away. The Santa Monica, Verdugo and Malibu Coast Fault Zones are located approximately 6.1, 9.8 and 14.3 kilometers from the site, respectively. Secondary hazards of earthquakes include rupture, seiche, landslides, liquefaction, and subsidence. Since there are no known faults within the immediate area, ground rupture from surface faulting should not be a potential problem. Seiche and landslides are not hazards in the area either. The area in consideration shows no mapped faults on-site according to maps prepared by the California Division of Mines and Geology (now known as the California Geologic Survey) and published by the International Conference of Building Officials (ICBO). No evidence of surface faulting was observed on the property during our reconnaissance. The site is located within a Seismic Zone 4.

FIELD AND LABORATORY INVESTIGATIONS

Subsurface soil conditions were explored by drilling ten (10) borings using a truck-mounted drill rig to depths ranging from 6 to 50 feet. The approximate boring locations are shown on the Site Plan, Figure 2. These approximate boring locations were estimated in the field based on pacing and measuring from the limits of existing site features. During drilling operations, penetration tests were performed at regular intervals to evaluate the soil consistency and to obtain information regarding the engineering properties of the subsurface soils. Soil samples were retained for laboratory testing. The soils encountered were continuously examined and visually classified in accordance with the Unified Soil Classification System. A more detailed description of the field investigation is presented in Appendix A.

Laboratory tests were performed on selected soil samples to evaluate their physical characteristics and engineering properties. The laboratory-testing program was formulated with emphasis on the evaluation of in-situ moisture and dry density, gradation, shear strength, consolidation and expansion potential, maximum dry density, R-value, pH value, minimum resistivity, sulfate and chloride contents of the materials encountered. Details of the laboratory-testing program are discussed in Appendix A. The results of the laboratory tests are presented on the borings logs or on the test reports, which are also

included in Appendix A. This information, along with the field observations, was used to prepare the final boring logs in Appendix A.

SOIL PROFILE AND SUBSURFACE CONDITIONS

Based on our findings, the subsurface conditions encountered appear typical of those found in the geologic region of the site. The soils within the depth of exploration consist of up to 5 feet of fill underlain by native alluvium. Deeper fill soils may be present onsite between our exploratory boring locations.

Below the fill soils, alternative layers of clayey silt, sandy silt, silty clay, silty sand and sand were encountered. Field and laboratory tests suggest that the native soils are moderately strong and slightly compressible. Penetration resistance, measured by the number of blows required to drive a Modified California sampler or a Standard Penetration Test (SPT) sampler, ranged from 6 to over 50 blows per foot. Dry densities ranged from 81.7 to 126.9 pounds per cubic feet (pcf). Representative soil samples had angles of internal friction of 19 to 38 degrees and cohesion of 0 to 500 pounds per square feet (psf). Representative soil samples consolidated approximately -0.4 to 2.2 percent under a 2-ksf load when saturated. Representative soil samples had Expansion Indexes (EI) of 17 to 56. A representative soil sample had a maximum dry density of 135 pcf. Representative subgrade soil samples had R-values of 14 to 27.

The above is a general description of soil conditions encountered at the site in the borings drilled for this investigation. For a more detailed description of the soil conditions encountered, please refer to the boring logs in Appendix A.

GROUNDWATER

Test boring locations were checked for the presence of groundwater during and immediately following the drilling operations. Free groundwater was encountered in 3 borings at depths of 34, 43.5 and 44.5 feet during this time of field exploration.

It should be recognized that water table elevation might fluctuate with time. The depth to groundwater can be expected to fluctuate both seasonally and from year to year. Fluctuations in the groundwater level may occur due to variations in precipitation, irrigation practices at the site and in the surrounding areas, climatic conditions, flow in adjacent or nearby canals, pumping from wells and possibly as the result of other factors that were not evident at the time of our investigation. Therefore, water level observations at the time of our field investigation may vary from those encountered during the construction phase of the project. The evaluation of such factors is beyond the scope of this report. Long-term monitoring in observation wells, sealed from the influence of surface water, is often required to more accurately define the potential range of groundwater conditions on a site.

SEISMICITY AND LIQUEFACTION POTENTIAL

Seismicity is a general term relating to the abrupt release of accumulated strain energy in the rock materials of the earth's crust in a given geographical area. The recurrence of accumulation and subsequent release of strain have resulted in faults and fault systems. Fault patterns and density reflect

relative degrees of regional stress through time, but do not necessarily indicate recent seismic activity; therefore, the degree of seismic risk must be determined or estimated by the seismic record in any given region.

Soil liquefaction is a state of soil particle suspension caused by a complete loss of strength when the effective stress drops to zero. Liquefaction normally occurs under saturated conditions in soils such as sand in which the strength is purely frictional. However, liquefaction has occurred in soils other than clean sand. Liquefaction usually occurs under vibratory conditions such as those induced by seismic events. To evaluate the liquefaction potential of the site, the following items were evaluated:

- 1) Soil type
- 2) Groundwater depth
- 3) Relative density
- 4) Initial confining pressure
- 5) Intensity and duration of ground shaking

The potential for soil liquefaction and densification (unsaturated sand) during a seismic event was evaluated using the LiquefyPro computer program (version 4.5b) developed by CivilTech. For the analysis, a maximum earthquake magnitude of 6.5 M_w and a peak horizontal ground surface acceleration of 0.49g (with a 10 percent probability of exceedance in 50 years) were considered appropriate for the liquefaction analysis. The maximum probable high groundwater depth of 30 feet was used in the analysis.

The analysis indicated that the loose to medium dense sandy soils have a low to moderate potential for liquefaction under seismic conditions. The total liquefaction-induced settlement was calculated to be on the order of 1 inch. The differential settlement is estimated to be on the order of ½ inch over a distance of 50 feet.

SOIL CORROSIVITY

Corrosion tests were performed to evaluate the soil corrosivity to the buried structures. The tests consisted of sulfate content, chloride content, and resistivity and the results of the tests are included as follows:

Parameter	Results	Test Method
Resistivity	6,670 ohms-cm	CALTRANS
Sulfate	303 ppm	EPA 9038
Chloride	17.1 ppm	EPA 9253
pH	7.77	EPA 9045C

CONCLUSIONS AND RECOMMENDATIONS

Based on the findings of our field and laboratory investigations, along with previous geotechnical experience in the project area, the following is a summary of our evaluations, conclusions, and recommendations.

ADMINISTRATIVE SUMMARY

Based on the data collected during this investigation, and from a geotechnical engineering standpoint, it is our opinion that the proposed development may be made as presently anticipated provided that the recommendations presented in this report are considered in the design and construction of the project.

In brief, the subject site and soil conditions, with the exception of the existing structures, undocumented fill, seismic-induced settlements and expansive clayer soils, appear to be conducive to the development of the project. Recommendations pertaining to the removal and recompaction of these loose soils are presented herein. After completion of the recommended site preparation, the site should be suitable for deep foundation support.

The estimated soil settlements for moderately loaded structures are anticipated to be excessive utilizing a shallow foundation system. In addition, all the current structures are supported on deep foundations. Therefore, it is recommended that the proposed structures be supported on similar deep foundations. Design values for drilled piles with various diameters are provided in the report.

Associated with the existing development are buried structures, such as footings, septic systems, backfilled excavations, and utility lines. These buried structures should be properly removed and the resulting excavations backfilled with Engineered Fill. Any other buried structures encountered during construction should be removed and backfilled in accordance with the recommendations of the Soils Engineer. The site should be inspected for possible buried fill material, using heavy excavating equipment. If loose fill material is encountered, excavations should extend to native ground. The exposed native subgrade should be scarified to a minimum of 6 inches, moisture-conditioned as necessary, and recompacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557. Limits of recompaction should extend 5 feet beyond structural elements. Prior to fill placement, Krazan & Associates, Inc. should inspect the bottom of the excavation to verify no additional excavation will be required.

It is recommended that any fill material encountered within proposed pavement areas be removed and/or recompacted. The fill material should be moisture-conditioned to near optimum moisture and compacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557. As an alternative, the owner may elect not to recompact the existing fill within paved areas. However, the owner should be aware that paved areas may settle which may require annual maintenance. At a minimum it is recommended that the upper 12 inches of subgrade soil be moisture-conditioned to at or above optimum moisture and recompacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557.

Expansion Index (EI) testing was performed on representative soil samples obtained from the borings. The test results indicate that the clayey soils have an expansion potential of moderately high. The estimated swell pressure of the clayey material may cause movement affecting slabs and brittle exterior finishes. To minimize the potential soil movement, it is recommended that the upper 24 inches of soil within the building slab and exterior flatwork areas be replaced with "non-expansive" soils (with EI≤20).

With the anticipated seismic-induced settlements, the foundation shallower than 30 feet should be designed to tolerate seismic settlements of 1 inch total and ½ inch differential over a distance of 50 feet. The static settlements are anticipated to be less than ½ inch total and ¼ inch differential over a distance of 50 feet.

Sandy soil conditions were also encountered at the site. These cohesionless soils have a tendency to cave in trench wall excavations. Shoring or sloping back trench sidewalls may be required within these loose cohesionless soils.

The shrinkage on recompacted soil and fill placement is estimated at 10 to 15 percent. This value is an estimate and may vary significantly depending on several items including soil conditions, compaction effort, weather, etc.

All grading and earthwork should be performed in accordance with the Grading Ordinances of the City of Los Angeles and the applicable portions of the General Earthwork Specifications in Appendix B, except as modified herein.

GROUNDWATER INFLUENCE ON STRUCTURES/CONSTRUCTION

Based on our findings and historical records, it is not anticipated that groundwater will rise within the zone of structural influence or affect the construction of foundations and pavements for the project. However, if earthwork is performed during or soon after periods of precipitation, the subgrade soils may become saturated, "pump," or not respond to densification techniques. Typical remedial measures include: discing and aerating the soil during dry weather; mixing the soil with dryer materials; removing and replacing the soil with an approved fill material; or mixing the soil with an approved lime or cement product. Our firm should be consulted prior to implementing remedial measures to observe the unstable subgrade conditions and provide appropriate recommendations.

SITE PREPARATION

General site clearing should include removal of vegetation and existing utilities; structures; including foundations basement walls and floors; existing stockpiled soil; trees and associated root systems; rubble; rubbish; and any loose and/or saturated materials. Site stripping should extend to a minimum depth of 2 to 4 inches, or until all organics in excess of 3 percent by volume are removed. Deeper stripping may be required in localized areas. These materials will not be suitable for reuse as Engineered Fill. However, stripped topsoil may be stockpiled and reused in landscape or non-structural areas.

It is recommended that the upper 24 inches of soil within proposed building and exterior flatwork areas consist of non-expansive Engineered Fill. The intent is to support the proposed slab-on-grade and

exterior flatwork areas with 24 inches of non-expansive fill. The non-expansive fill material should be a well-graded silty sand or sandy silt soil. A clean sand or very sandy soil is not acceptable for this purpose. A sandy soil will allow the surface water to drain into the expansive clayey soils below, which may result in soil swelling. Imported Fill should be approved by the Soils Engineer prior to placement. The fill should be placed as specified as Engineered Fill.

Within the proposed pavement areas, it is recommended that the upper 12 inches of subgrade soil be moisture-conditioned to near optimum moisture and recompacted to a minimum of 90 percent of maximum density based on ASTM D1557 Test Method.

The upper soils, during wet winter months, become very moist due to the absorptive characteristics of the soil. Earthwork operations performed during winter months may encounter very moist unstable soils, which may require removal to grade a stable building foundation. Project site winterization consisting of placement of aggregate base and protecting exposed soils during the construction phase should be performed.

Any buried structures or loosely backfilled excavations encountered during construction should be properly removed and the resulting excavations backfilled with Engineered Fill. Excavations, depressions, or soft and pliant areas extending below planned finished subgrade levels should be cleaned to firm, undisturbed soil and backfilled with Engineered Fill. In general, any septic tanks, debris pits, cesspools, or similar structures should be entirely removed. Concrete footings should be removed to an equivalent depth of at least 3 feet below proposed footing elevations or as recommended by the Soils Engineer. Any other buried structures should be removed in accordance with the recommendations of the Soils Engineer. The resulting excavations should be backfilled with Engineered Fill.

A representative of our firm should be present during all site clearing and grading operations to test and observe earthwork construction. This testing and observation is an integral part of our service, as acceptance of earthwork construction is dependent upon compaction and stability of the material. The Soils Engineer may reject any material that does not meet compaction and stability requirements. Further recommendations of this report are predicated upon the assumption that earthwork construction will conform to recommendations set forth in this section and the Engineered Fill section.

ENGINEERED FILL

The organic-free, on-site, upper soils are predominately silty sand and sandy silt with various amount of clay. Some of these soils may be suitable for reuse as non-expansive Engineered Fill, provided they are cleansed of excessive organics and debris. The soils with Expansion Index greater than 20 should not be used within the upper 24 inches of the building pad and exterior flatwork areas.

The preferred materials specified for Engineered Fill are suitable for most applications with the exception of exposure to erosion. Project site winterization and protection of exposed soils during the construction phase should be the sole responsibility of the contractor, since he has complete control of the project site at that time. Imported non-expansive Fill should consist of a well-graded, slightly cohesive, fine silty sand or sandy silt soil, with relatively impervious characteristics when compacted.

This material should be approved by the Soils Engineer prior to use and should typically possess the following characteristics:

Percent Passing No. 200 Sieve	20 to 50
Plasticity Index	10 maximum
UBC Standard 29-2 Expansion Index	20 maximum

Fill soils should be placed in lifts approximately 6 inches thick, moisture-conditioned as necessary, and compacted to achieve at least 90 percent of maximum density as determined by ASTM D1577 Test Method. Additional lifts should not be placed if the previous lift did not meet the required dry density or if soil conditions are not stable.

TEMPORARY EXCAVATION STABILITY

All excavations should comply with the current OSHA requirements. All cuts greater than 3 feet in depth should be sloped or shored. Temporary excavations should be sloped at 1:1 (horizontal to vertical) or flatter, up to a maximum depth of 10 feet. Heavy construction equipment, building materials, excavated soil, and vehicular traffic should not be allowed within five feet of the top (edge) of the excavation.

Where sloped excavations are not feasible due to site constraints, the excavations may require shoring. The design of the shoring system is normally the responsibility of the contractor or shoring designer, and therefore, is outside the scope of this report. The design of the temporary shoring should take into account lateral pressures exerted by the adjacent soil, and, where anticipated, surcharge loads due to adjacent buildings and any construction equipment or traffic expected to operate alongside the excavation.

The excavation recommendations provided herein are based on soil characteristics derived from test borings within the area. Variations in soil conditions will likely be encountered during the excavations. Krazan & Associates, Inc. should be afforded the opportunity to provide field review to evaluate the actual conditions and account for field condition variations, not otherwise anticipated in the preparation of this recommendation.

UTILITY TRENCH LOCATION, CONSTRUCTION AND BACKFILL

To maintain the desired support for existing or new foundations, new utility trenches should be located such that the base of the trench excavation is located above an imaginary plane having an inclination of 1.0 horizontal to 1.0 vertical, extending downward from the bottom edge of the adjacent footing.

Utility trenches should be excavated according to accepted engineering practices following OSHA (Occupational Safety and Health Administration) standards by a contractor experienced in such work. The responsibility for the safety of open trenches should be borne by the contractor. Traffic and vibration adjacent to trench walls should be kept to a minimum; cyclic wetting and drying of excavation side slopes should be avoided. Depending upon the location and depth of some utility trenches,

groundwater flow into open excavations could be experienced, especially during or shortly following periods of precipitation.

For purposes of this section of the report, backfill is defined as material placed in a trench starting one foot above the pipe; bedding and shading (also referred to as initial backfill) is all material placed in a trench below the backfill. With the exception of specific requirements of the local utility companies or building department, pipe bedding and shading should consist of clean medium-grained sand. The sand should be placed in a damp state and should be compacted by mechanical means prior to the placement of backfill soils. Above the pipe zone, underground utility trenches may be backfilled with either free-draining sand, on-site soil or approved imported soil. The trench backfill should be compacted to at least 90 percent relative compaction.

COMPACTED MATERIAL ACCEPTANCE

Compaction specifications are not the only criteria for acceptance of the site grading or other such activities. However, the compaction test is the most universally recognized test method for assessing the performance of the Grading Contractor. The numerical test results from the compaction test cannot be solely used to predict the engineering performance of the compacted material. Therefore, the acceptance of compacted materials will also be dependent on the moisture content and the stability of that material. The Geotechnical Engineer has the option of rejecting any compacted material regardless of the degree of compaction if that material is considered to be too dry or excessively wet, unstable or if future instability is suspected. A specific example of rejection of fill material passing the required percent compaction is a fill which has been compacted with in-situ moisture content significantly less than optimum moisture. Where expansive soils are present, heaving of the soils may occur with the introduction of water. Where the material is a lean clay or silt, this type of dry fill (brittle fill) is susceptible to future settlement if it becomes saturated or flooded.

SURFACE DRAINAGE AND LANDSCAPING

The ground surface should slope away from building and pavement areas toward appropriate drop inlets or other surface drainage devices. We recommended that adjacent paved exterior grades be sloped at a minimum of 2 percent for a minimum distance of 5 feet away from structures. Ideally, asphalt concrete pavement areas should be sloped at a minimum of 2 percent, with Portland cement concrete sloped at a minimum of one percent toward drainage structures. These grades should be maintained for the life of the project.

Roof drains should be designed to avoid discharging into landscape areas adjacent to the buildings. Downspouts should be directed to discharge directly onto paved surfaces to allow for surface drainage into the storm systems or should be connected directly to the on-site storm drain.

DEEP FOUNDATION

The bearing capacities for the structures can be provided by means of a deep foundation system. Drilled piles with diameters of 18, 24, 30 and 36 inches are recommended. The allowable capacities for compression are illustrated as Figure 3. These values are similar to the recommended chart presented in

the reports prepared by R. T. Frankian and Associates. A one-third increase may be used when considering temporary seismic or wind loads. The uplift capacity of the piles may be assumed to be one-half of the download (compression) capacity.

The existing piles for the parking structures may be left-in-place and reused for the new addition structures provided they are evaluated and approved by the structural engineer.

If groundwater is encountered during pile excavation or installation, casing and/or drilling mud may be required to prevent caving. The City of Los Angeles Building Code also has special provisions for installing drilled piles below groundwater level.

The drilling of piles should be continuously inspected by a representative of the Geotechnical Engineer of Record. The same representative should approve each pile excavation prior to placement of reinforcing steel and prior to casting concrete. These provisions are also required by the City of Los Angeles Building Code. Concrete should be placed the same day of pile excavation.

The total soil movement is not expected to exceed ½ inch. Differential soil movement should be less than ¼ inch. Piles spaced on centers at 2½ times of the diameter or greater will not require any reduction in load capacity due to group effect.

The bearing capacities of the drilled piers should be verified by load test. Lateral bearing may be taken as 300 pounds per cubic feet. A 1/3 increase in the allowable bearing loads may be used for short duration, wind, or seismic loads. The lateral load criteria for the piles are given below. A lateral deflection of ¼ inch has been considered at the pile head.

Pile Diameter (Inches)	Maximum Moment (kip-ft)	Maximum Reverse Moment (kip-ft)	Depth to Reverse Moment (ft)	Depth to Point of Inflection (ft)	Depth to Zero Moment (ft)	Allowable Load (kips)
18	5.1P*	1.3P	10	6.1	20	16.5
24	6.5P	1.6P	-13	7.8	26 .	- 26
30	. 7.7P	1.9P	15	9.2	31	37
36	8.9P	2.2P	18	10.7	36	50

^{*}P is applied lateral load in kips

FLOOR SLABS AND EXTERIOR FLATWORK

Concrete slab-on-grade floors should be underlain by a water vapor retarder. The water vapor retarder should be installed in accordance with ASTM Specification E 1643-98. According to ASTM Guidelines, the water vapor retarder should consist of a vapor retarder sheeting underlain by a minimum of 3 inches of compacted, clean, gravel of ¾-inch maximum size. To aide in concrete curing an optional 2 to 4 inches of granular fill may be placed on top of the vapor retarder. The granular fill should consist of

damp clean sand with at least 10 to 30 percent of the sand passing the 100 sieve. The sand should be free of clay, silt or organic material. Rock dust which is manufactured sand from rock crushing operations is typically suitable for the granular fill. This granular fill material should be compacted.

The exterior floors should be poured separately in order to act independently of the walls and foundation system. All fills required to bring the building pads to grade should be Engineered Fills.

Moisture within the structures may be derived from water vapors, which were transformed from the moisture within the soils. This moisture vapor can travel through the vapor membrane and penetrate the slab-on-grade. This moisture vapor penetration can affect floor coverings and produce mold and mildew in the structure. To minimize moisture vapor intrusion, it is recommended that a vapor retarder be installed in accordance with ASTM guidelines. It is recommended that the utility trenches within the structure be compacted, as specified in our report, to minimize the transmission of moisture through the utility trench backfill. Special attention to the immediate drainage and irrigation around the building is recommended. Positive drainage should be established away from the structure and should be maintained throughout the life of the structure. Ponding of water should not be allowed adjacent to the structure. Over-irrigation within landscaped areas adjacent to the structure should not be performed. In addition, ventilation of the structure (i.e. ventilation fans) is recommended to reduce the accumulation of interior moisture.

RETAINING WALLS

Walls retaining horizontal backfill and capable of deflecting a minimum of 0.1 percent of its height at the top may be designed using an equivalent fluid active pressure of 35 pounds per square foot per foot of depth. Walls that are incapable of this deflection or walls that are fully constrained against deflection may be designed for an equivalent fluid at-rest pressure of 55 pounds per square foot per foot per depth. Expansive soils should not be used for backfill against walls. The wedge of non-expansive backfill material should extend from the bottom of each retaining wall outward and upward at a slope of 2:1 (horizontal to vertical) or flatter. The wall backfill should be compacted to at least 90 percent of maximum density based on ASTM D1557-00 Test Method.

The active and at-rest earth pressures do not include hydrostatic pressures. To reduce the build-up of hydrostatic pressures, drainage should be provided behind the retaining walls. Wall drain should consist of a minimum 12-inch wide zone of drainage material, such as 3/4-inch or 1/2-inch drain rock wrapped in a non-woven polypropylene geotextile filter fabric such as Mirafi 140N or equivalent. Alternatively, drainage may be provided by the placement of a commercially produced composite drainage blanket, such as Miradrain, extending continuously up from the base of the wall. The drainage material should extend from the base of the wall to finished subgrade in paved areas and to within about 12 inches below the top of the wall in landscape areas. In landscape areas the top 12 inches should be backfilled with compacted native soil. A 4-inch minimum diameter, perforated, Schedule 40 PVC drain pipe should be placed with holes facing down in the lower portion of the wall drainage material, surrounded with drain rock wrapped in filter fabric. A solid drainpipe leading to a suitable discharge point should provide drainage outlet. As an alternative, weep holes may be used to provide drainage. If weep holes are used the weep holes should be 3 inches in diameter and spaced about 8 feet on centers. The backside of the

weep holes should be covered with a corrosion-resistant mesh to prevent loss of backfill and/or drainage material.

PAVEMENT DESIGN

Based on the laboratory R-value testing of the near-surface materials, an R-value of 20 was used for the preliminary flexible asphaltic concrete pavement design. The R-value should be verified during grading of the pavement areas. The following table shows the recommended pavement sections for various traffic indices.

Traffic Index	Asphaltic Concrete (inches)	Class 2 Aggregate Base* (inches)	Compacted Subgrade** (inches)
4.5	3.0	5.5	12.0
6.0	4.0	8.5	12.0
7.0	4.0	12.0	12.0

^{* 95%} compaction based on ASTM D1557 Test Method or CAL 216

If traffic indices are not available, an estimated (typical value) index of 4.5 may be used for automobile parking and an index of 7.0 may be used for light truck traffic.

Pavement areas should be sloped and drainage gradients maintained to carry all surface water off the site. A cross slope of 2 percent is recommended in asphalt concrete pavement areas to provide good surface drainage and to reduce the potential for water to penetrate into the pavement structure.

SITE COEFFICIENT

The site coefficient, per Table 16-J, California Building Code (CBC), is based upon the site soil conditions. It is our opinion that a site coefficient of soil type S_D is appropriate for building design at this site. For seismic design of the structures, in accordance with the seismic provisions of the Building Code, we recommend the following parameters:

Seismic Item	Value	CBC Reference
Zone Factor	0.4	Table 16-I
Source Type	В	Table 16-U
Coefficient Na	1.0	Table 16-S
Coefficient N _v	1.2	Table 16-T
Coefficient Ca	0.45	Table 16-Q
Coefficient C _v	0.78	Table 16-R

^{** 90%} compaction based on ASTM D1557 Test Method or CAL 216

SOIL CORROSIVITY

Excessive sulfate or chloride in either the soil or native water may result in an adverse reaction between the cement in concrete and the soil. California Building Code has developed criteria for evaluation of sulfate and chloride levels and how they relate to cement reactivity with soil and/or water. The soil samples from the subject site were tested to have negligible sulfate and chloride concentrations. Therefore, normal concrete mixes may be used for concentrations such as found in these soils.

Electrical resistivity testing of the soil indicates that the onsite soils may have a low potential for metal loss from electrochemical corrosion process. A qualified corrosion engineer should be consulted regarding the corrosion effects of the onsite soils on underground metal utilities.

TESTING AND INSPECTION

A representative of Krazan & Associates, Inc. should be present at the site during the earthwork activities to confirm that actual subsurface conditions are consistent with the exploratory fieldwork. This activity is an integral part of our service, as acceptance of earthwork construction is dependent upon compaction testing and stability of the material. This representative can also verify that the intent of these recommendations is incorporated into the project design and construction. Krazan & Associates, Inc. will not be responsible for grades or staking, since this is the responsibility of the Prime Contractor.

LIMITATIONS

Geotechnical Engineering is one of the newest divisions of Civil Engineering. This branch of Civil Engineering is constantly improving as new technologies and understanding of earth sciences advance. Although your site was analyzed using appropriate and current techniques and methods, undoubtedly there will be substantial future improvements in this branch of engineering. In addition to advancements in the field of Geotechnical Engineering, physical changes in the site due to site clearing or grading activities, new agency regulations, or possible changes in the proposed structure or development after issuance of this report will result in the need for professional review of this report. Updating or revisions to the recommendations report, and possibly additional study of the site may be required at that time. In light of this, the Owner should be aware that there is a practical limit to the usefulness of this report without critical review. Although the time limit for this review is strictly arbitrary, it is suggested that two years be considered a reasonable time for the usefulness of this report.

Foundation and earthwork construction is characterized by the presence of a calculated risk that soil and groundwater conditions have been fully revealed by the original foundation investigation. This risk is derived from the practical necessity of basing interpretations and design conclusions on limited sampling of the earth. The recommendations made in this report are based on the assumption that soil conditions do not vary significantly from those disclosed during our field investigation. The logs of the exploratory borings do not provide a warranty as to the conditions that may exist beneath the entire site. The extent and nature of subsurface soil and groundwater variations may not become evident until construction begins. It is possible that variations in soil conditions and depth to groundwater could exist beyond the points of exploration that may require additional studies, consultation, and possible design revisions. If conditions are encountered in the field during construction, which differ from those described in this

report, our firm should be contacted immediately to provide any necessary revisions to these recommendations.

This report presents the results of our Geotechnical Engineering Investigation, which was conducted for the purpose of evaluating the soil conditions in terms of foundation and retaining wall design, and grading and paving of the site. This report does not include reporting of any services related to environmental studies conducted to assessment the presence or absence of hazardous and/or toxic materials in the soil, groundwater, or atmosphere, or the presence of wetlands. Any statements in this report or on any boring log regarding odors, unusual or suspicious items, or conditions observed, are strictly for descriptive purposes and are not intended to convey professional judgment regarding the presence of potential hazardous or toxics substances. Conversely, the absence of statements in this report or on any boring log regarding odors, unusual or suspicious items, or conditions observed, does not constitute our rendering professional judgment regarding the absence of potentially hazardous or toxics substances.

The conclusions of this report are based on the information provided regarding the proposed construction. We emphasize that this report is valid for the project as described in the text of this report and it should not be used for any other sites or projects. The geotechnical engineering information presented herein is based upon our understanding of the proposed project and professional interpretation of the data obtained in our studies of the site. It is not warranted that such information and interpretation cannot be superseded by future geotechnical engineering developments. The Geotechnical Engineer should be notified of any changes to the proposed project so the recommendations may be reviewed and re-evaluated. The work conducted through the course of this investigation, including the preparation of this report, has been performed in accordance with the generally accepted standards of geotechnical engineering practice, which existed in geographic area of the project at the time the report was written. No other warranty, express or implied, is made. This report is issued with the understanding that the owner chooses the risk they wish to bear by the expenditures involved with the construction alternatives and scheduling that are chosen.

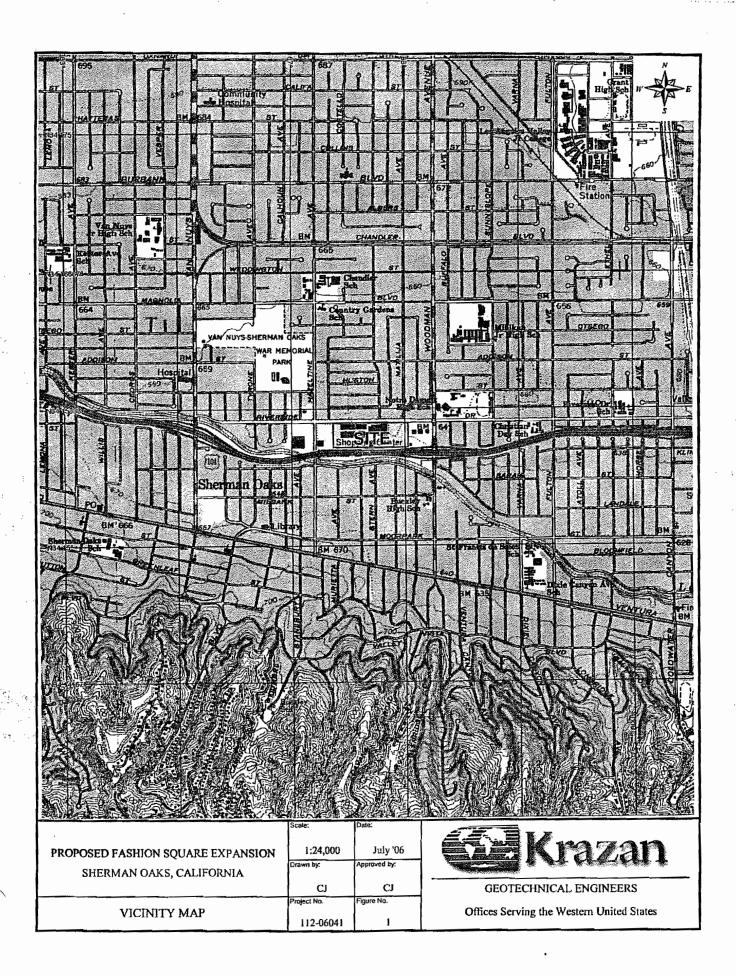
If you have any questions, or if we may be of further assistance, please do not hesitate to contact our office at (909) 974-4400.

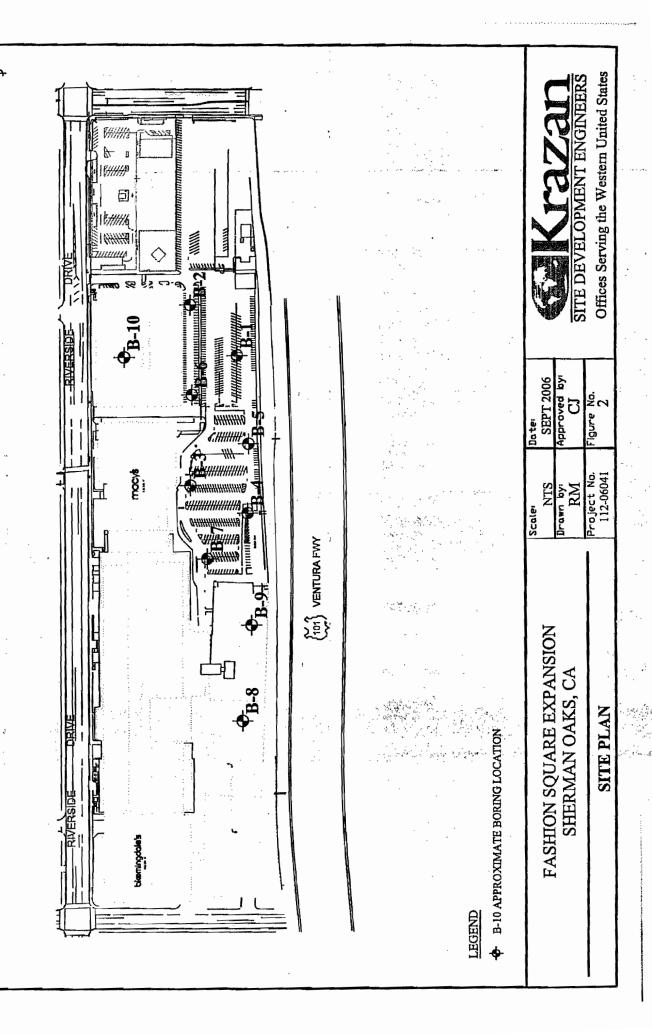
Respectfully submitted, KRAZAN & ASSOCIATES, INC.

James Kellogg

Clarence Jiang, GE Project Engineer R.G.E. No. 2477 James M. Kellogg, PE Regional Manager R.C.E. No. 65092

CJ/JMK:rm





20 30 **Depth (ft)** 8 0.0 Bearing Capacity (kips) 300.0 - 0.0 550.0 500.0 450.0 150.0 100.0 50.0

FIGURE 3, PILE CAPACITY

APPENDIX A

FIELD AND LABORATORY INVESTIGATIONS

Field Investigation

Our field investigation consisted of a surface reconnaissance and a subsurface exploration program consisted of drilling, logging and sampling a total of 10 borings. The depths of exploration ranged from about 6 feet to 50 feet below the existing site surface.

Members of our staff visually classified the soils in the field as the drilling and excavating progressed and recorded a continuous log of each boring. Visual classification of the soils encountered in our exploratory borings was made in general accordance with the Unified Soil Classification System (ASTM D2487). A key for the classification of the soil and the boring logs are presented in this Appendix.

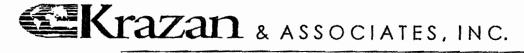
During drilling operations, penetration tests were performed at regular intervals to evaluate the soil consistency and to obtain information regarding the engineering properties of the subsoils. Samples were obtained from the borings by driving either a 2.5-inch inside diameter Modified California tube sampler fitted with brass sleeves or a 2-inch outside diameter, 1-3/8-inch inside diameter Standard Penetration ("split-spoon") test (SPT) sampler without sleeves. Soil samples were retained for possible laboratory testing. The samplers were driven up to a depth of 18 inches into the underlying soil using a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler was recorded for each 6-inch penetration interval and the number of blows required to drive the sampler the last 12 inches are shown as blows per foot on the boring logs.

The approximate locations of our borings are shown on the Site Plan, Figure 2. These approximate locations were estimated in the field based on pacing and measuring from the limits of existing site features.

Laboratory Investigation

The laboratory investigation was programmed to determine the physical and mechanical properties of the soil underlying the site. The laboratory-testing program was formulated with emphasis on the evaluation of in-situ moisture and dry density, gradation, shear strength, expansion potential, and R-value of the materials encountered. In addition, chemical tests were performed to evaluate the soil/cement reactivity and corrosivity. Test results were used in our engineering analysis with respect to site and building pad preparation through mass grading activities, foundation and retaining wall design recommendations, pavement section design, evaluation of the materials as possible fill materials and for possible exclusion of some soils from use at the structures as fill or backfill.

Select laboratory test results are presented on the boring logs, with graphic or tabulated results of selected tests included in this Appendix. The laboratory test data, along with the field observations, was used to prepare the final boring logs presented in the Appendix.



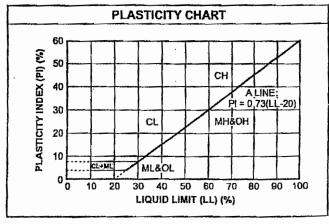
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UNIFIED SOIL CLASSIFICATION SYSTEM

UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART							
COARSE-GRAINED SOILS (more than 50% of material is larger than No. 200 sleve size.)							
Clean Gravels (Less than 5% fines)							
Well graded gravels gravel send							
GRAVELS	GV	Well-graded gravels, gravel-sand mixtures, little or no fines					
More than 50% of coarse	GF	Poorly-graded gravels, gravel-sand mixtures, little or no fines					
fraction larger	fraction larger Gravels with fines (More than 12% fines						
sieve size	G/	Slity gravels, gravel-sand-slit mixtures					
	GC	Clayey gravels, gravel-sand-clay mixtures					
	Clea	n Sands (Less than 5% fines)					
SANDS	sv	Well-graded sands, gravelly sands, little or no fines					
50% or more of coarse	SP	Poorly graded sands, gravelly sands, little or no fines					
fraction smaller	San	ds with fines (More than 12% fines)					
than No. 4 sieve size	SM	Silty sands, sand-silt mixtures					
	so	Clayey sands, sand-clay mixtures					
(50% or m		E-GRAINED SOILS terial is smaller than No. 200 sleve size.)					
SILTS AND	ML	inorganic slits and very fine sands, rock flour, slity of clayey fine sands or clayey slits with slight plasticity					
CLAYS Liquid limit less than	CL.	inorganic clays of low to medium plasticity, gravelly clays, sandy clays, slity clays, lean clays					
50%	OL.	Organic silts and organic silty clays of low plasticity					
SILTS	MF	inorganic slits, micaceous or diatomaceous fine sandy or silty solis, elastic slits					
AND CLAYS Liquid limit 50%	CH	Inorganic clays of high plasticity, fat clays					
or greater	o⊦	Organic clays of medium to high plasticity, organic slits					
HIGHLY ORGANIC SOILS	소 의 PT 소 의 PT	Peat and other highly organic soils					

CONSISTENCY CLASSIFICATION					
Description	Blows per Foot				
Granula	ır Soils				
Very Loose	< 5				
Loose	5 – 15				
Medium Dense	16 – 40				
Dense	41 – 65				
Very Dense	> 65				
. Cohesiv	e Soils				
Very Soft	< 3				
Soft	3-5				
Firm	6-10				
Stiff	11 – 20				
Very Stiff	21 – 40				
Hard	> 40				

GRAIN SIZE CLASSIFICATION						
Grain Type	Standard Sieve Size	Grain Size in Millimeters				
Boulders	Above 12 inches	Above 305				
Cobbles	12 to 13 inches	305 to 76.2				
Gravel	3 inches to No. 4	76.2 to 4.76				
Coarse-grained	3 to ¼ inches	76.2 to 19.1				
Fine-grained	1/4 inches to No. 4	19.1 to 4.76				
Sand	No. 4 to No. 200	4.76 to 0.074				
Coarse-grained	No. 4 to No. 10	4.76 to 2.00				
Medium-grained	No. 10 to No. 40	2.00 to 0.042				
Fine-grained	No. 40 to No. 200	0.042 to 0.074				
Silt and Clay	Below No. 200	Below 0.074				



Log of Drill Hole B1
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-1

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

	SUBSURFACE PROFILE		SAM	(PLE		
Depth (ft)	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
0	Ground Surface					
2-111	3" AC ON 8" AB FILL: SANDY SILT W/SOME CLAY (ML), fine grained, brown, slightly moist, firm FILL: SILTY SAND (SM), fine to medium grained, brown, slightly moist, loose	113.2	5.2	X	10	
##	4" fiber mesh pipe at 4' (abandoned)	81.7	38.1	Y	13	
10-1 12-1 14-1 16-1 20-1	CLAYEY SILT (ML), fine grained, brown/tan, wet, medium stiff TERMINATED @ 6' / GRAVEL Total Depth = 6' No groundwater was encountered during drilling Hole backfilled with soll cuttings and patched 06/27/06	81.7	38.1		13	
22- 						

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Drill Date: 06/27/06

Hole Size: 8"

Elevation: See Site Plan

Sheet: 1 of 1

Driller: TS

Log of Drill Hole B2
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-2

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

		SUBSURFACE PROFILE SAMPLE					
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
0	9111770	Ground Surface					
1 1		FILL: SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML),					
2-		fine to medium grained, brown/black/tan, moist, medium dense/stiff	98.6	20.8		25.	=
4-							
1		CLAYEY SILT (ML), tan/brown, wet, soft, (tan material slightly competant)		42.3	-	8	=
6-				42.0			
8-		SANDY SILT W/TRACE CLAY (ML), fine to medium grained, dark brown, moist, medium	1				
		stiff					
10-			ļ	14.4	-	7	=
12-							
=							
14-		SILTY SAND (SM),	-				_
16-		fine to medium grained, brown, moist, medium dense		11.2		12	=
		Same as above, very firm drilling					
18-		·					
20-							
		SILTY CLAY (CL),	Ī	8.7		27	
22-		olive, slightly moist, very stiff					' }
24-		Same as above, increasingly difficult drilling					
27		Same as above, moist, medium stiff w/some fine to		15.0			=
26-		medium grained sand		15.3		10	
28-		SILTY CLAY (CL),	-				
20-		olive, moist, stiff					
30-						15	=

Krazan and Associates

Drill Method: Hollow Stem Auger

Drill Date: 06/27/06

Drill Rig: CME 55

Hole Size: 8"

Driller: TS

Elevation: See Site Plan

Log of Drill Hole B2
Project: Westfield Fashion Square Expansion

Client: Westfield Corporation, Inc.

Location: Sherman Oaks, CA

Depth to Water>

Initial:

Project No: 112-06041

Figure No.: A-2

Logged By: AK

At Completion:

		SUBSURFACE PROFILE		SAN	1PLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ff.	Water Content (%)
32-				23.4		15	
34-		Same as above, w/fine grained sand					
36-		SILTY SAND W/TRACE CLAY (SM),		19.0	7	12	
38		fine to medium grained, brown, moist, medium dense SILTY CLAY (CL), olive, moist, stiff					
40		CLAYEY SILT W/SOME SAND (ML), fine grained, olive, moist, medium stiff		24.8		11	= ·
42-		Same as above, decrease in sand/blueish gray		21.0		••	
44-		Same as above, stiff		27.2	100	15	=
48-							
50-		End of Borehole					
52-		Total Depth = 50' No groundwater was encountered during drilling Hole backfilled with soil cuttings and patched	:				
54-		06/27/06					
56-							
60-							

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Hole Size: 8"

Elevation: See Site Plan

Drill Date: 06/27/06

Sheet: 2 of 2

Driller: TS

Log of Drill Hole B4
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-4

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

		SUBSURFACE PROFILE		SAM	1PLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
		Ground Surface					
2-		2" AC ON 6" AB FILL: SILTY SAND (SM), fine to medium grained, brown, damp, loose	113.4	10.1	X	30	
4-		SILTY SAND W/SOME CLAY (SM), fine to medium grained, brown, molst, medium dense SANDY SILT W/TRACE CLAY (ML), fine grained, brown, moist, very stiff	106.5	20.3		48	u u
8-		SILTY SAND/SANDY SILT (SM/ML), fine to medium grained, brown, moist, medium dense					
10-		es de	-	10.9		22	=
12-		SILTY SAND W/TRACE CLAY (SM), fine to medium grained, brown, medium dense				·	·
16-		Same as above, decrease in grain size		11.6		17	= ·
18-		Same as above, very tight					
20-		Same as above, increase in density		8.8		37	=
22-						2	
24-		SILTY SAND/SANDY SILT W/TRACE CLAY				1.	
26-		(SM/ML) fine grained, brown, moist, dense		12.7		44	-
28-		REFUSAL AT 27' / MATERIAL TOO TIGHT Total Depth = 27' No groundwater was encountered during drilling Hole backfilled with soil cuttings and patched					

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Driller: TS

Krazan and Associates

Drill Date: 06/28/06

Hole Size: 8"

Elevation: See Site Plan

Log of Drill Hole B3
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-3

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water> 43.5'

Initial:

At Completion:

		SUBSURFACE PROFILE		SAN	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%) 10 20 30 40
0		Ground Surface					
2-		3" AC ON 4" AB FILL: SANDY SILT WITRACE CLAY (ML), fine grained, brown, damp, stiff SANDY SILT WITRACE CLAY (ML),	126.9	12.1	X	30	=
4-		fine grained, brown, damp, stiff SANDY CLAY (CL),					
6-		fine grained, brown, moist, very stiff	1 1 5.6	18.1	X	48	•
8- 10- 12-		Same as above, increase in coarse grained material		17.6		22	=
14-		SANDY SILT (ML), fine grained, brown, moist, stiff		19.2		17	
18-		SILTY SAND (SM), fine to medium grained, brown, moist, medium dense					· · · · · · · · · · · · · · · · · · ·
22-				11.7		37	
24		SILTY SAND/SAND (SM/SP), fine to coarse grained, brown, damp, dense		3.1		44	: ·
26 - 28 - 30 -		SAND (SP), fine to coarse grained, brown/light brown, damp, dense				47	B

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Driller: TS

Krazan and Associates

Drill Date: 06/28/06

Hole Size: 8"

Elevation: See Site Plan

Initial:

Log of Drill Hole B3
Project: Westfield Fashion Square Expansion

Client: Westfield Corporation, Inc.

Location: Sherman Oaks, CA

Depth to Water> 43.5'

Project No: 112-06041

Figure No.: A-3

Logged By: AK

At Completion:

		SUBSURFACE PROFILE		SAN	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
				3.4		47	
32-		Same as above, dark brown					
36-		Same as assist, dark brown		3.3	7	37	_
38-	m						
40-		SILTY CLAY W/SOME SAND (CL), fine grained, brown/plive, moist, firm			less:		. .
			-	23.6		6	2.5%
42-							
46-		Same as above, stiff/grey/decrease in coarse material		23.2		14	. · · · <u>=</u> .
48-							
52 54 54 56 58		End of Borehole Total Depth = 50' Groundwater was encountered during drilling at 43,5' Hole backfilled with soil cuttings and patched 06/28/05				· * (
60-							

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Drill Date: 06/28/06

Hole Size: 8"

Elevation: See Site Plan

Sheet: 2 of 2

Driller: TS

Log of Drill Hole B5
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-5

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

initial:

At Completion:

		SUBSURFACE PROFILE	****	SAM	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
		Ground Surface					
2-		2" AC ON 5" AB FILL: SILTY SAND W/LITTLE CLAY (SM), fine to medium grained, brown, damp, medium dense SANDY SILT W/SOME CLAY (ML),	105.5	14.5	X	37	: . B
6-		fine grained, brown, moist, stiff SANDY CLAY (CL), fine grained, brown, moist, stiff	94.4	26.5	×	27	. Е
10-		SILTY SAND W/TRACE CLAY (SM), fine to medium grained, brown, moist, medium dense		10.3		22	
12-		SANDY CLAY (CL), fine grained, brown, moist, stiff SANDY SILT W/LITTLE CLAY (ML), fine grained, brown, moist, very stiff		10.0		0.7	E
16-		SILTY SAND (SM), fine to medium grained, brown, molst, medium dense SILTY SAND/SANDY SILT (SM/ML), fine grained, brown, slightly moist, dense		13.9		37	
20-		mio granico, biomi, digitaj molet, cente		6.7		42	.
22-		Same as above, increasingly difficult					
26-		Same as above, w/trace clay		9.5		47	
30-		REFUSAL AT 29' / MATERIAL TOO TIGHT					

Krazan and Associates

Drill Method: Hollow Stem Auger

Drill Date: 06/28/06

Drill Rig: CME 55

Hole Size: 8"

Driller: TS

Elevation: See Site Plan

Log of Drill Hole B6
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-6

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water> 34'

Initial:

At Completion:

Description Description Description Description			SUBSURFACE PROFILE		SAM	IPLE		
2 10 10 10 10 10 10 10 10 10 1	Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
FILL: SANDY SILT (ML), fine grained, brown, damp, loose SANDY SILT (ML), fine grained, brown, damp, medium dense SILTY CLAY WITRACE SAND (CL), fine grained, brown, moist, very stiff Same as above, stiff w/increase in plasticity 28.9 12 SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 22 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense 15.2 37								
fine grained, brown, damp, medium dense SILTY CLAY WITRACE SAND (CL), fine grained, brown, moist, very stiff 8 10 Same as above, stiff w/increase in plasticity 28.9 12 14 SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff 16- CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 21 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense 15.2 37	2		FILL: SANDY SILT (ML), fine grained, brown, damp, loose	106.8	22.5	X	37	=
SILTY CLAY WITRACE SAND (CL), fine grained, brown, moist, very stiff Same as above, stiff w/increase in plasticity 28.9 12 SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 21 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense	4-		fine grained, brown, damp, medium dense	.				
Same as above, stiff w/increase in plasticity 12 14 SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff 16- 18 CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 20- SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense	6-		SILTY CLAY W/TRACE SAND (CL),	92.7	30.6	X	26	=
12— 14— SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff 18— CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 22— SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense 15.2 37	8-			,				
SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, moist, very stiff 16. CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 11.8 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense			Same as above, stiff w/increase in plasticity		28.9		12	
fine to medium grained, brown, moist, very stiff 16- 18- CLAY (CL), brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 22- SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense 15.2 34								
brown, moist, stiff SILTY SAND/CLAYEY SILT (SM/ML), fine to medium grained, brown, moist, dense/hard 22 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense 15.2 37	16-		fine to medium grained, brown, moist, very stiff		16.6		34	e
fine to medium grained, brown, moist, dense/hard 22 SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense	18-		brown, moist, stiff					`
SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, very firm, medium dense	20-		fine to medium grained, brown, moist, dense/hard		11.8		43	: = .:
medium dense	"		(SM/ML),				.*	
			medium dense		15.2	-	37	=
30								· · · · · · · · · · · · · · · · · · ·

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Hole Size: 8"

Drill Date: 06/28/06

Driller: TS

Elevation: See Site Plan

Log of Drill Hole B6
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-6

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water> 34'

Initial:

At Completion:

		SUBSURFACE PROFILE		SAN	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
-		SILTY SAND W/CLAY (SM), moist, very dense		13.5		50 @ 6"	
32-		SILTY CLAY W/TRACE SAND (CL),					· · · · · · · · · · · · · · · · · · ·
36-		fine grained, olive, moist, very stiff		24.5		21	
38						٠,	
40-		Same as above, stiff w/increase in sand		25.1		18 .	=
42-							_
46-		Same as above, very stiff		25.7	1	29	
48-		SILTY SAND W/TRACE CLAY (SM), fine to medium grained, brown, slightly moist, medium dense					
		End of Borehole		,		·	
52- 54-		Total Depth = 50' Groundwater was encountered during drilling at 34' Hole backfilled with soil cuttings and patched 06/28/06		٠.			
56-							
58-							
60-							

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Drill Date: 06/28/06

Hole Size: 8"

Elevation: See Site Plan

Sheet: 2 of 2

Driller: TS

Log of Drill Hole B7

Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-7

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

	SUBSURFACE PROFILE						
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
0		Ground Surface					
2-		2" AC ON 5" AB FILL: SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML), fine to medium grained, brown, moist, medium dense	112.9	19.9	X	37	n
4-		SANDY SILT W/SOME CLAY (ML), fine grained, brown, moist, stiff Same as above, decrease in clay	106.2	22.0		36	
8-		SILTY SAND/SAND (SM/SP), fine to medium grained, brown, slightly moist, medium dense	100.2	22.0		30	
10-			117.5	5.3	X	33_	
14-		SILTY SAND (SM), fine to medium grained, brown, moist, dense Same as above, w/lenses of silt and trace of clay		7.8		43	=
18-		SAND (SP), fine to medium grained, light brown/brown, damp, dense					<u>.</u>
22-		Same as above, fine to coarse grained		1.4		46	
24-		SANDY SILT WITRACE CLAY (ML), fine grained, brown, moist, stiff	:	15.3		19	. =
28-		SILTY SAND W/TRACE CLAY (SM), fine grained, brown, moist, medium dense					

Drill Method: Hollow Stem Auger

Drill Date: 06/28/06

Drill Rig: CME 55

Krazan and Associates Hole Size: 8"

Driller: TS

Elevation: See Site Plan

Log of Drill Hole B7
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-7

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

		SUBSURFACE PROFILE		SAM	IPLE					
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Wa 10	ater Co	30	40 40
				17.9	7	32	,	=		
32-		SILTY SAND/CLAYEY SAND (SM/SC), fine to medium grained, brown, moist, medium dense		17.8						
34-	1111222	REFUSAL AT 34' / MATERIAL TOO TIGHT								
36-		Total Depth = 34' No groundwater was encountered during drilling Hole backfilled with soil cuttings and patched								
38-		Hole backfilled with soil cuttings and patched 06/28/06	•							
40-			·							
42-										
44-									/	
46-		•								
48-				l.						
50-										
52-										
54-				ŀ						
56-			•							
58-										
60-	1-1					`				

Drill Method: Hollow Stem Auger

Drill Rig: CME 55

Krazan and Associates

Drill Date: 06/28/06

Hole Size: 8"

Elevation: See Site Plan

Sheet: 2 of 2

Driller: TS

Log of Drill Hole B8
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-8

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water> 44.5

Initial: 45.5

At Completion: 44.5

		SUBSURFACE PROFILE		SAM	1PLE		. "
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
		Ground Surface					, · · · · · · · · · · · · · · · · · · ·
2-		3" AC ON 3" AB FILL: SANDY SILT W/SOME CLAY (ML), fine to medium grained, brown, moist, very stiff Same as above, decrease in clay	122.7	11.9	×	29	=
6-	A secondary	SILTY SAND W/TRACE CLAY/GRAVEL (SM), fine to coarse grained, brown, slightly moist, dense	118.0	4.6	X	54	=
10-		SILTY SAND/SAND W/LITTLE GRAVEL (SM/SP), fine to coarse grained, brown, slightly moist, very dense	121.0	3.4	×	50 @ 6"	
14-		SILTY SAND/SANDY SILT (SM/ML), fine to medium grained, brown, very moist, medium dense	107.0	00.0			
16-		CLAY (CL), brown, very moist, very stiff	107.6	22.2		29	:
20-			107.6	20.1	×	27	
22-		CANDY SILT IN/COME OF AV (ML)					
26-		SANDY SILT W/SOME CLAY (ML), fine grained, brown, very molst, very stiff	106.2	22.1	X	28	=
30-		SILTY CLAY W/TRACE SAND (CL), fine grained, brown, moist, very stiff			<u> </u>	34	 =

Drill Method: Hollow Stem Auger

Drill Date: 08/14/06

Drill Rig: LAR (custom)

Krazan and Associates Hole Size: 8"

Driller: Pacific Drilling

Elevation: See Site Plan

Log of Drill Hole B8
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-8

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water> 44.5

Initial: 45.5

At Completion: 44.5

		SUBSURFACE PROFILE		SAM	APLE.		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Type	Blows/ft.	Water Content (%)
			106.0	23.2	X	34	_
32-		Same as above, hard		:			
36-		Came as above, naro	106.5	21.4	X	53	[
38-		SANDY SILT W/TRACE CLAY (ML),					
40		fine grained, brown, wet, hard					
42-		Same as above, increase in clay	99.9	25.1	*	48	,.
46-		SILTY SAND/SAND (SM/SP),	104.5	22.2	X	50	•
48-		fine to medium grained, brown, very moist, dense SILTY SAND (SM), fine to medium grained, brown, damp, dense					
52		End of Borehole Total Depth = 50' Groundwater was encountered at 45.5' during drilling Hole backfilled with soil cuttings and patched 08/14/06				٠,	
58- 60-							

Drill Method: Hollow Stem Auger

Krazan and Associates

Drill Date: 08/14/06

Drill Rig: LAR (custom) **Driller:** Pacific Drilling

Hole Size: 8"

Elevation: See Site Plan

Log of Drill Hole B9
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-9

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

SUBSURFACE PROFILE					1PLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Type	Blows/ft.	Water Content (%)
0	Ш	Ground Surface					
=		2" OVERLAY AC ON 3" CONCRETE ON 2" AB	1				· ·
2		FILL: SANDY SILT W/LITTLE CLAY (ML), fine to medium grained, brown, slightly moist, loose	113.8	6.3		16	₽
4		SANDY SILT W/TRACE CLAY/GRAVEL (ML),	110.0	0.5		10	
"=		fine to medium grained, brown, slightly moist, medium					
6-		dense	116.0	6.2	X	28	-
]							
8-		SILTY SAND W/TRACE CLAY/GRAVEL (SM), fine to medium grained, brown, damp, medium dense					
10-			124.6	2.7		30	=
-	TÜTÜ	SAND W/SOME GRAVEL (SP),	124.6	2.7		30	
12-		fine to coarse grained, brown, damp, medium dense					
14-	нінні	SILTY SAND W/TRACE CLAY (SM),	!				
'		fine to medium grained, brown, moist, medium dense		10.0			=
16-		SILTY SAND/SANDY SILT W/TRACE CLAY	118.1	13.8		69	}
1 40 =	1111111	(SM/ML),					
18-		Same as above, fine to medium grained					
20-							
1 3		CLAY (CL), brown/olive, wet, very stiff	98.9	25.9	_	32	
22-		blowipolive, wet, very suil					,
24-		SANDY SILT W/SOME CLAY (ML),	1 1				
24		fine grained, brown, moist, hard					
26-		CLAY (CL),	110.9	17.1	X	63	
]	HIHHI	brown, very moist, hard SILTY SAND (SM),					
28-		fine grained, brown, moist, dense					
30	шШ	CLAYEY SILT W/TRACE SAND (ML),					_
"		fine grained, brown, very moist, hard	109.5	21.4	X	42	
32-		REFUSAL AT 29.5' / MATERIAL TOO Total Depth = 29.5'					
] _ [No groundwater was encountered during drilling					
34-		Hole backfilled with soil cuttings and patched					
		08/15/06					

Drill Method: Hollow Stem Auger

Drill Rig: LAR (custom)

Krazan and Associates

Hole Size: 8"

Drill Date: 08/15/06

Elevation: See Site Plan

Sheet: 1 of 1

Driller: Pacific Drilling

Log of Drill Hole B10 Project: Westfield Fashion Square Expansion

Project No: 112-06041 Figure No.: A-10

Client: Westfield Corporation, Inc.

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

		SUBSURFACE PROFILE		SAN	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
0		Ground Surface					
2-		2" AC ON 3" AB FILL: SILTY SAND (SM), fine to medium grained, brown, very moist, loose	106.0	21.4		23	. =
4-		FILL: SILTY CLAY W/SOME SAND (CL), fine to medium grained, brown, very moist, very stiff SILTY CLAY W/TRACE SAND (CL),					
6-		\fine grained, brown, moist, very stiff SILTY SAND (SM), fine to medium grained, brown, moist, medium dense	111.5	12.1	×	39	
8-		CLAYEY SILT (ML), brown, very moist, very stiff					
10-		Same as above, w/some sand	99.2	22.9	X	21	x =
14-		SILTY SAND/SANDY SILT W/TRACE CLAY					
16-		(SM/ML), fine to medium grained, brown, moist, medium dense	111.9	15.9	X	37	
18-		SILTY CLAY (CL), brown, wet, very stiff					
20-		Same as above, decrease in silt	90.9	31.7	\mathbf{X}	3 3.	
22							
24-		SANDY SILT W/TRACE CLAY (ML), fine to medium grained, brown, very moist, medium dense					,
26-	1		112.9	18.9		37	
28-		CLAY (CL), brown, moist, very stiff					
30-					\mathbf{x}	40	=

Drill Method: Hollow Stem Auger

Drill Rig: LAR (custom)

Driller: Pacific Drilling

Krazan and Associates

Drill Date: 08/16/06

Hole Size: 8"

Elevation: See Site Plan

Sheet: 1 of 2

Log of Drill Hole B10
Project: Westfield Fashion Square Expansion

Project No: 112-06041

Client: Westfield Corporation, Inc.

Figure No.: A-10

Location: Sherman Oaks, CA

Logged By: AK

Depth to Water>

Initial:

At Completion:

		SUBSURFACE PROFILE		SAM	IPLE		
Depth (ft)	Symbol	Description	Dry Density (pcf)	Moisture (%)	Туре	Blows/ft.	Water Content (%)
		SANDY SILT (ML),	107.1	22.0	X	40	
32		fine grained, brown, very moist, medium dense		ŀ			
34		SILTY CLAY (CL), brown, very moist, very stiff					_
36-			97.7	29.1	X	24	_
38		CLAYEY SILT W/LITTLE SAND (ML), fine grained, brown, wet, medium dense					
40-			106.2	24.0		35	=
-			100.2	24.0		35	·
42-		SANDY SILT W/TRACE CLAY (ML), fine grained, brown, very moist, very dense					
] _ =		SILTY SAND/SANDY SILT W/TRACE CLAY (SM/ML),	112.8	17.6		74	. =
46-	111111	fine to medium grained, brown, very moist, very stiff	11	11.0			
48		SANDY SILT W/SOME CLAY (ML), fine grained, brown, molst, very stiff					
-		End of Borehole					,
52		Total Depth = 50'					
54		No groundwater was encountered during drilling Hole backfilled with soil cuttings and patched 08/16/06					·
56	:						
58							
60-							

Drill Method: Hollow Stem Auger

Drill Rig: LAR (custom)

Driller: Pacific Drilling

Krazan and Associates

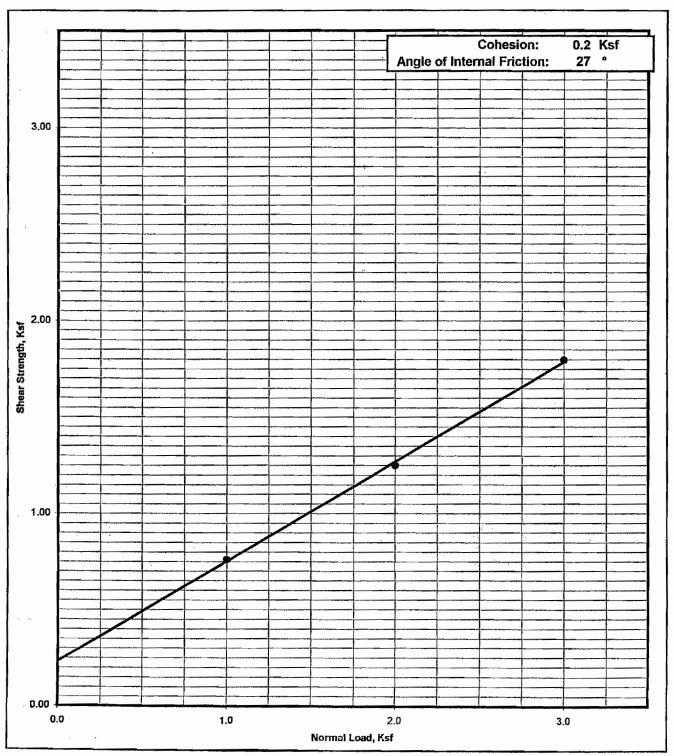
Drill Date: 08/16/06

Hole Size: 8"

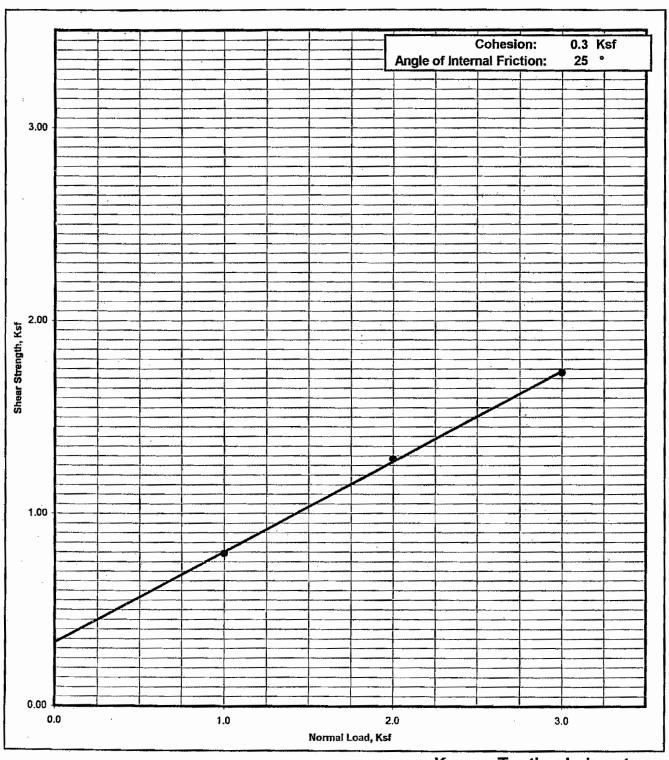
Elevation: See Site Plan

Sheet: 2 of 2

Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-2 @ 2'	(ML), Sandy Silt w/ Clay	7/6/06

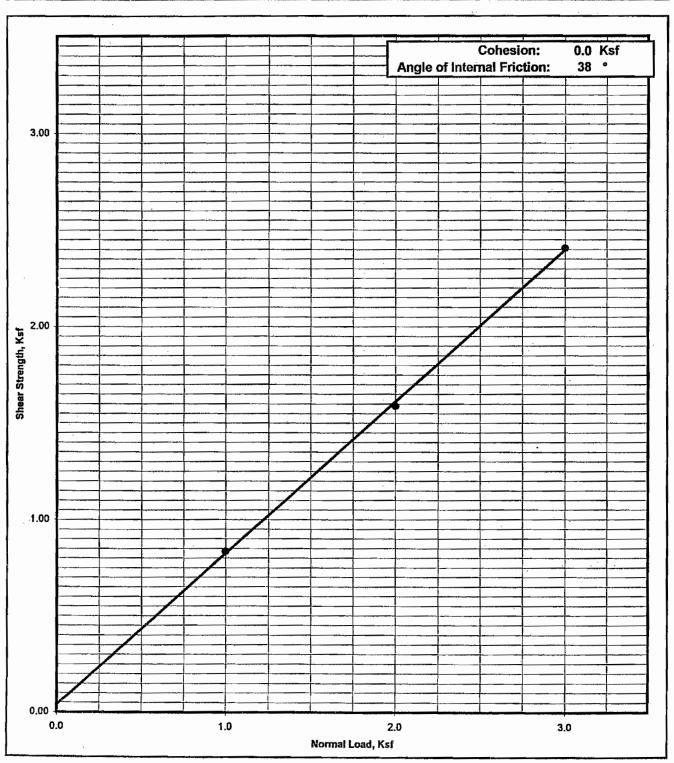


Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-7 @ 2'	(ML), Sandy Silt w/ Clay	7/6/06



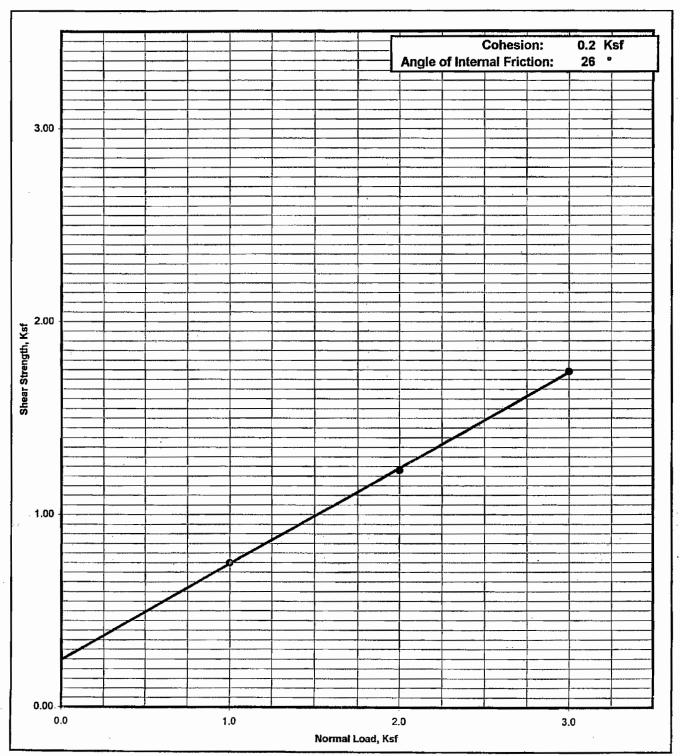
Krazan Testing Laboratory

Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-8 @ 10' - 11'	(SM-SP), Silty Sand - Sand w/ Trace Gravel	8/21/06

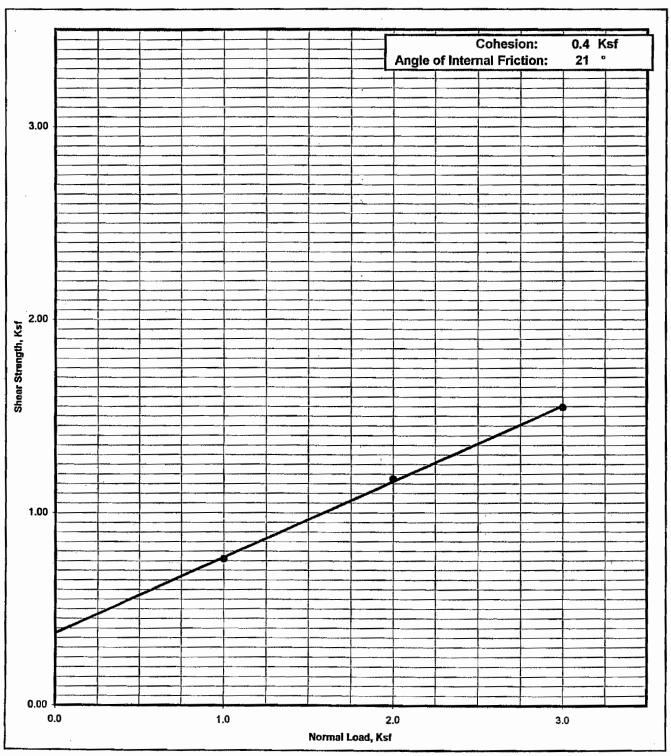


Krazan Testing Laboratory

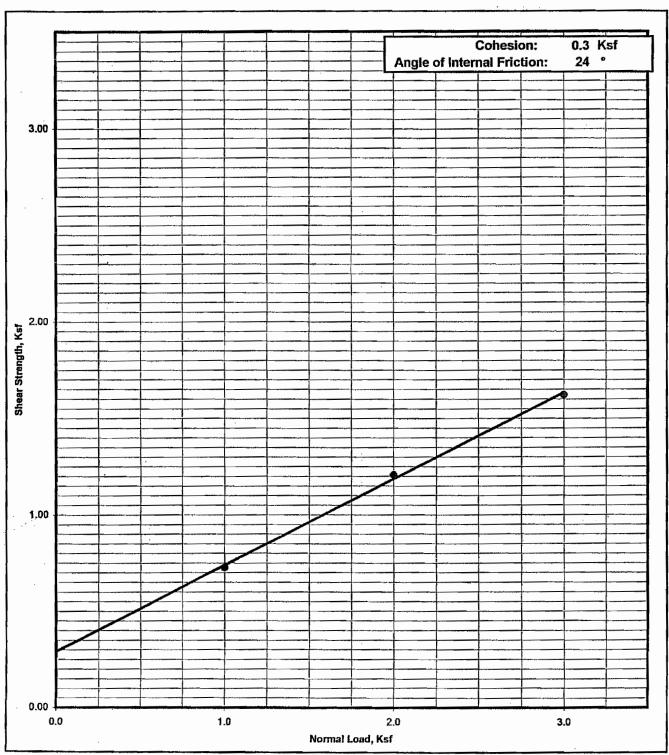
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-8 @ 20' - 21'	(ML), Sandy Silt w/ Clay	8/21/06



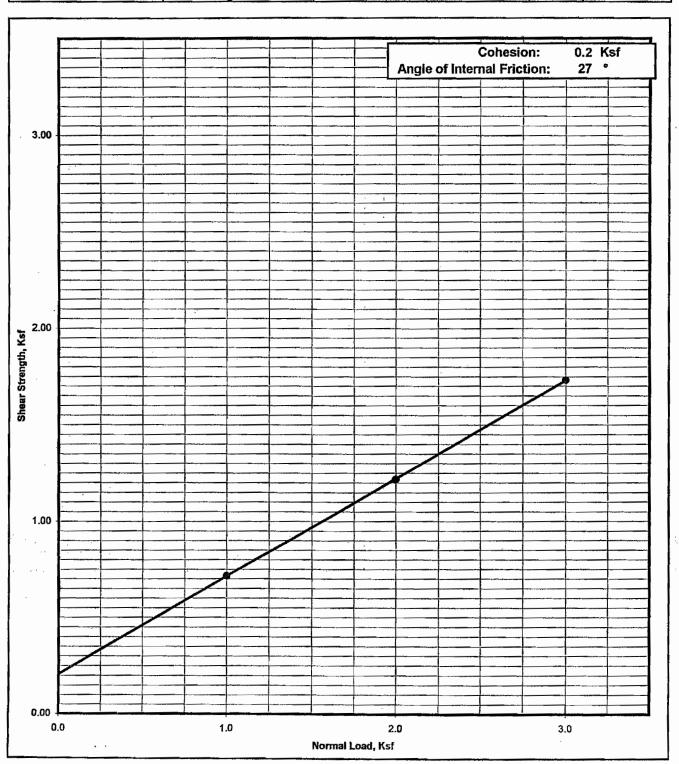
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-8 @ 30' - 31'	(ML), Clayey Silt	8/21/06



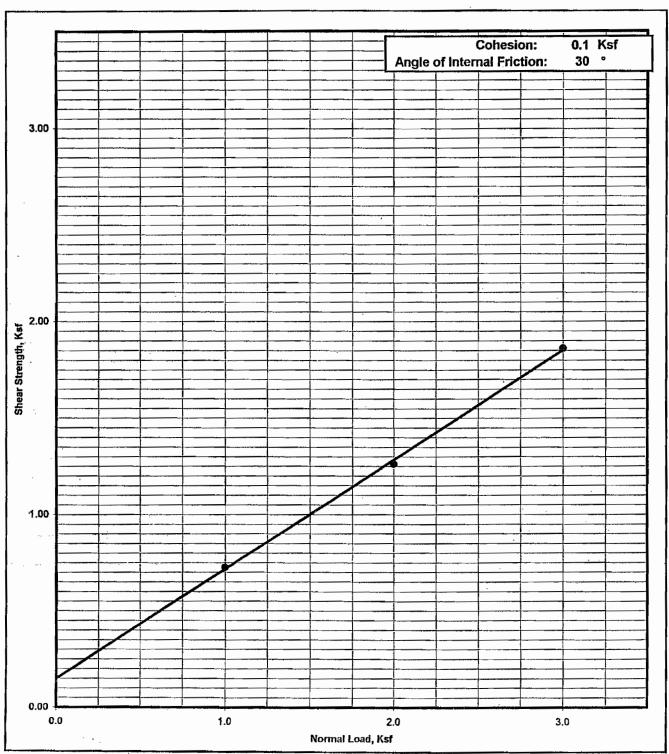
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-8 @ 40' - 41'	(ML), Sandy Silt w/ Clay	8/21/06



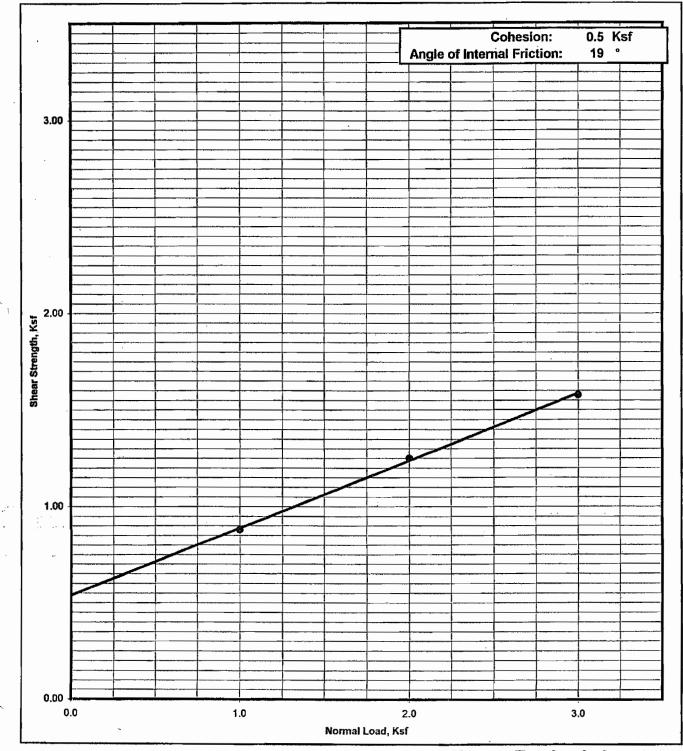
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-10 @ 15' - 16'	(ML), Sandy Silt w/ Trace Clay	8/21/06



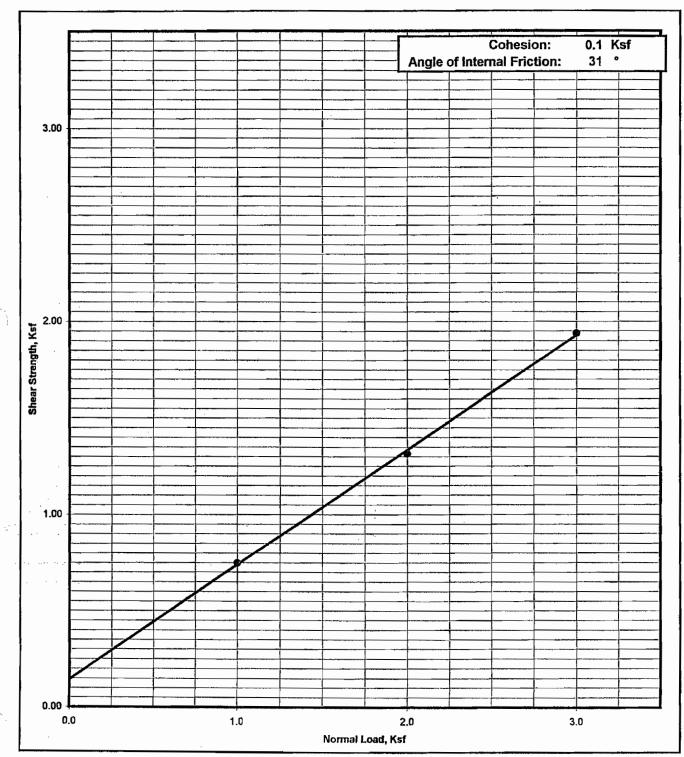
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-10 @ 25' - 26'	(SM-ML), Silty Sand-Sandy Silt w/ Trace Clay	8/21/06



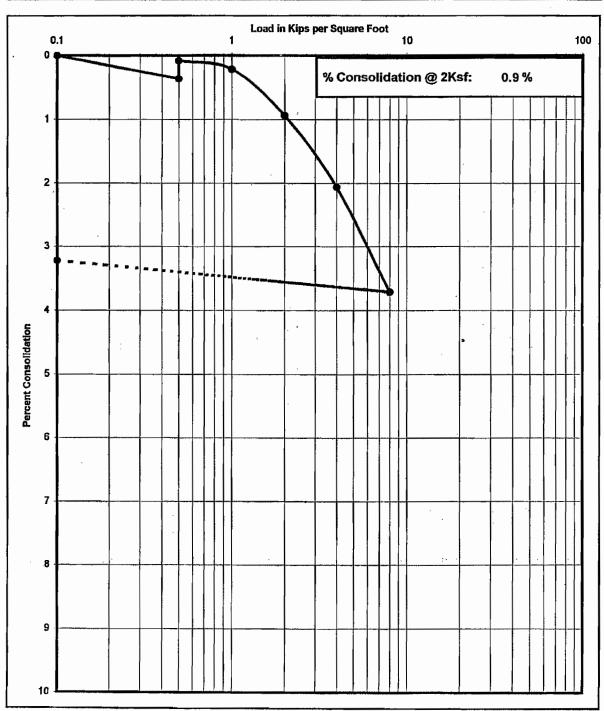
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-10 @ 35' - 36'	(CL), Silty Clay	8/21/06



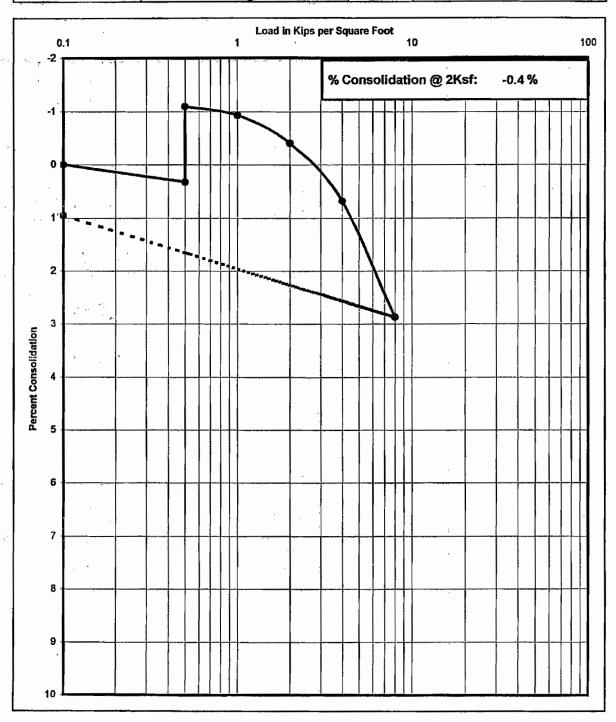
Project Number	Boring No. & Depth	Soil Type	Date
112-06041	B-10 @ 45' - 46'	(SM-ML), Silty Sand-Sandy Silt w/ Trace Clay	8/21/06



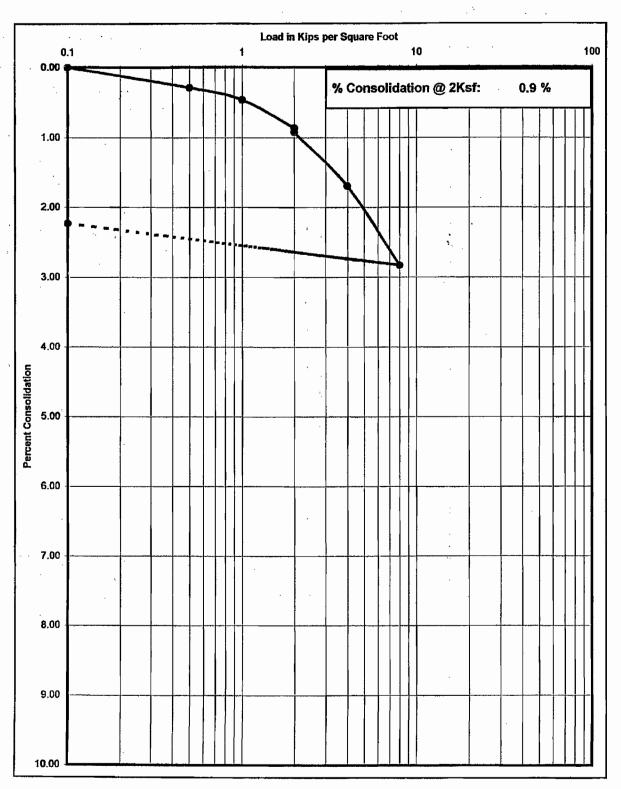
Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-1 @ 5'	7/6/06	(ML), Clayey Silt



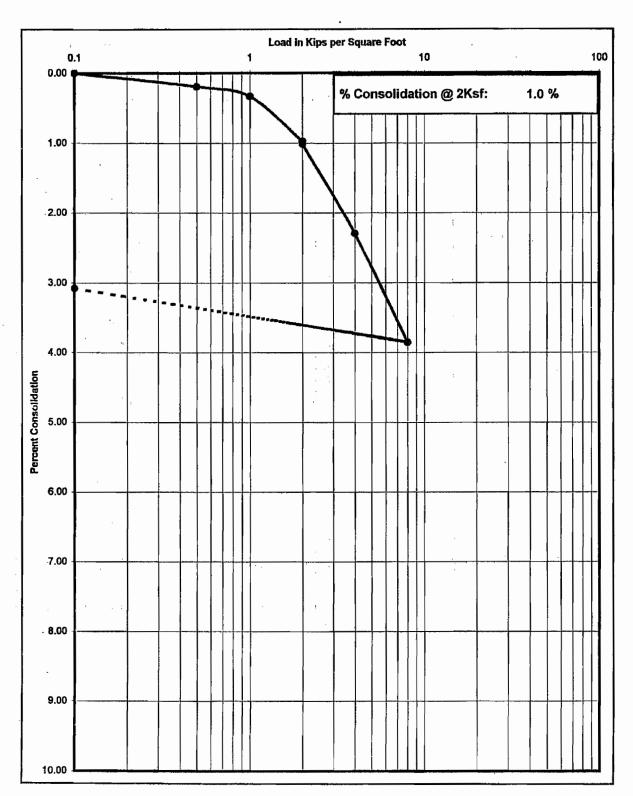
Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-5 @ 5'	7/6/06	(CL), Silty Clay



Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-7 @ 5'	7 <i>1</i> 6/06	(ML), Sandy Silt w/ Clay

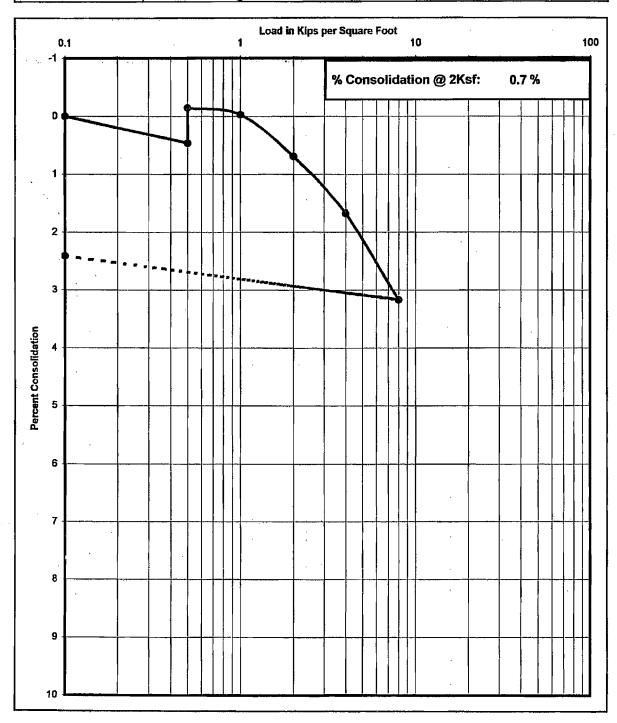


Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-10 @ 10' - 11'	8/21/06	(ML), Sandy Silt w/ Clay

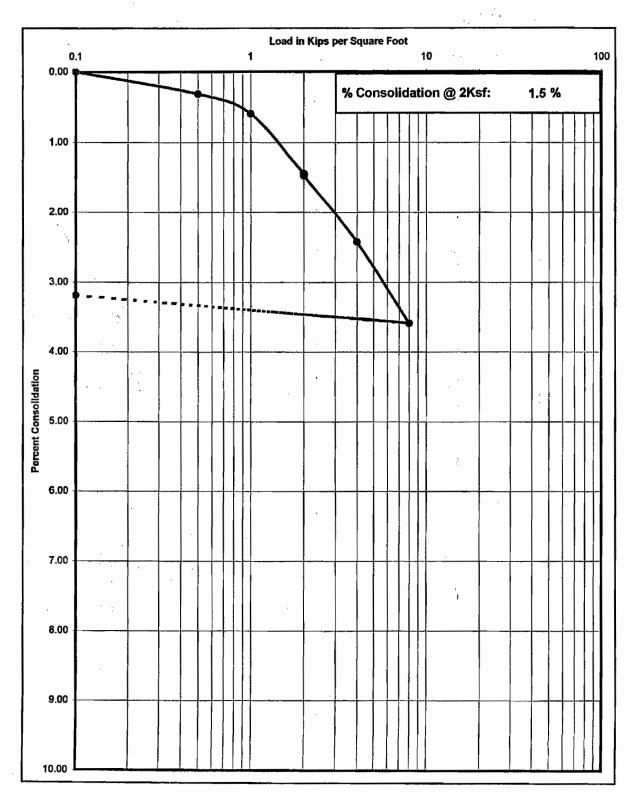


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Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-10 @ 20' - 21'	8/21/06	(CL), Silty Clay

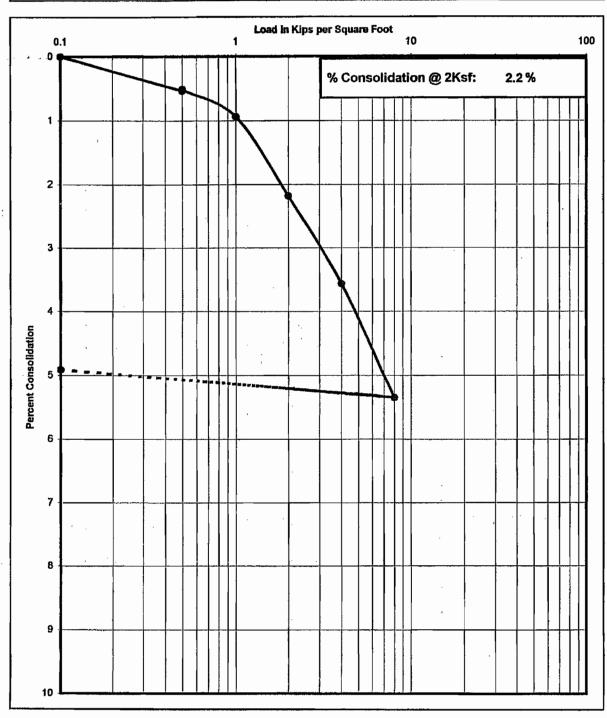


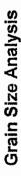
Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-10 @ 30' - 31'	8/21/06	(ML), Sandy Silt w/ Trace Clay

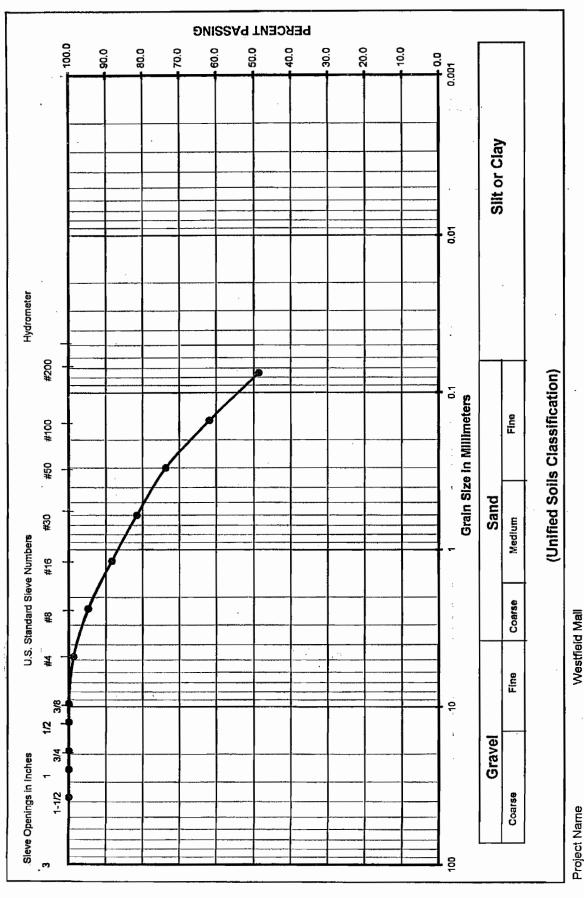


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Project No	Boring No. & Depth	Date	Soil Classification
112-06041	B-10 @ 40' - 41'	8/21/06	(ML), Clayey Silt





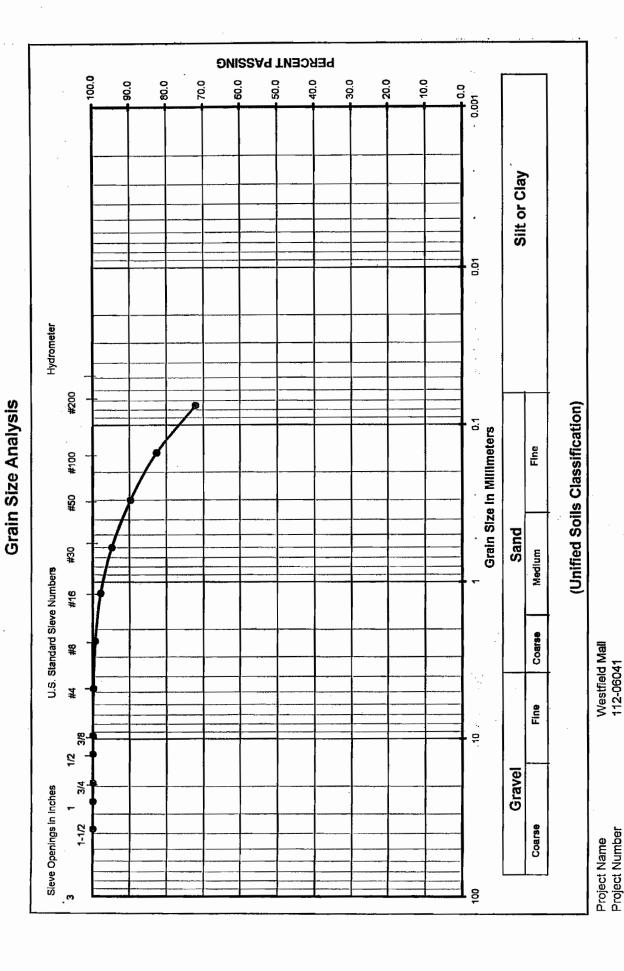


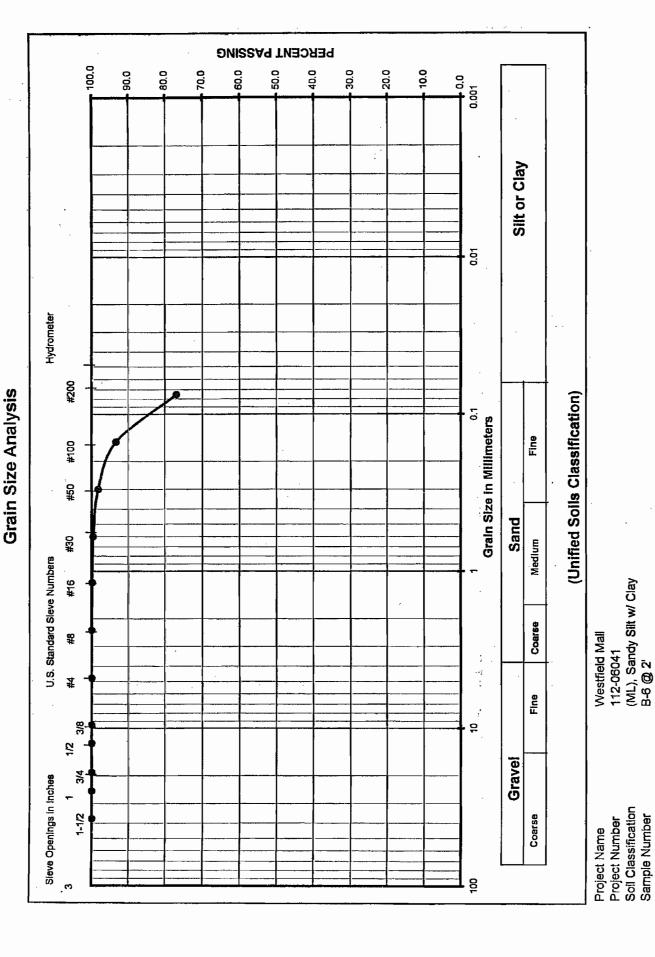
Soil Classification Sample Number Project Number

112-06041 (SM-ML), Silty Sand-Sandy Silt w/ Clay B-2 @ 0-3'

(ML), Sandy Silt w/ Clay B-3 @ 2'

Soil Classification Sample Number





0.7

Grain Size in Millimeters

9

9

Sand

Fine

Medium

Coarse

Fine

Coarse

Gravel

PERCENT PASSING

50.0

40.0

30.0

20.0

10.0

0.0 0.001

60.0

70.0

100.0

١

Grain Size Analysis

Hydrometer

#200

#100

#20

#30

#18

點

#

42

Sleve Openings in Inches

3/8

34

1-1/2

U.S. Standard Sleve Numbers

90.0

80.0



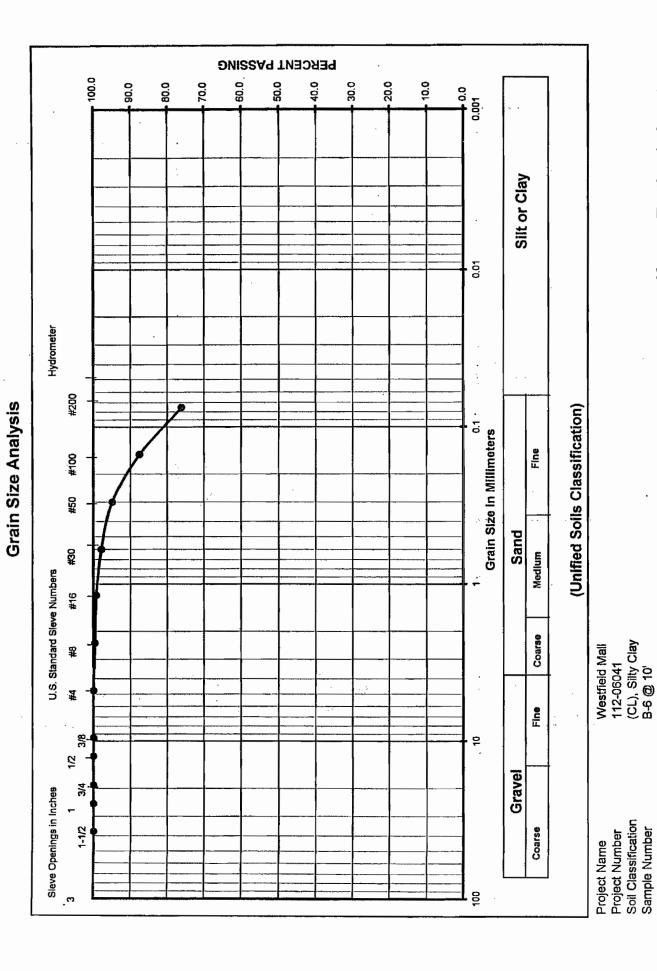
Soil Classification Sample Number Project Number

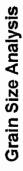
Project Name

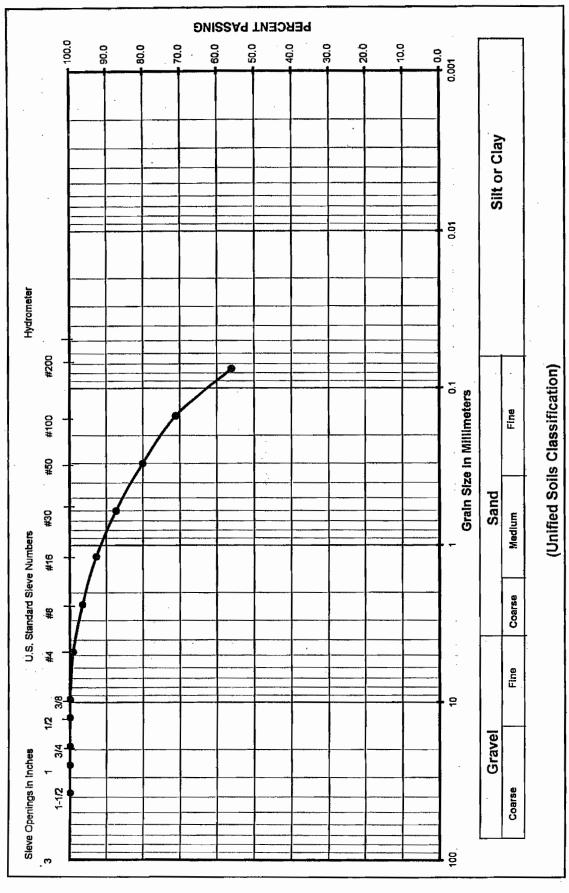
112-06041 (ML), Clayey Silt B-6 @ 5'

(Unified Soils Classification)

Westfield Mall



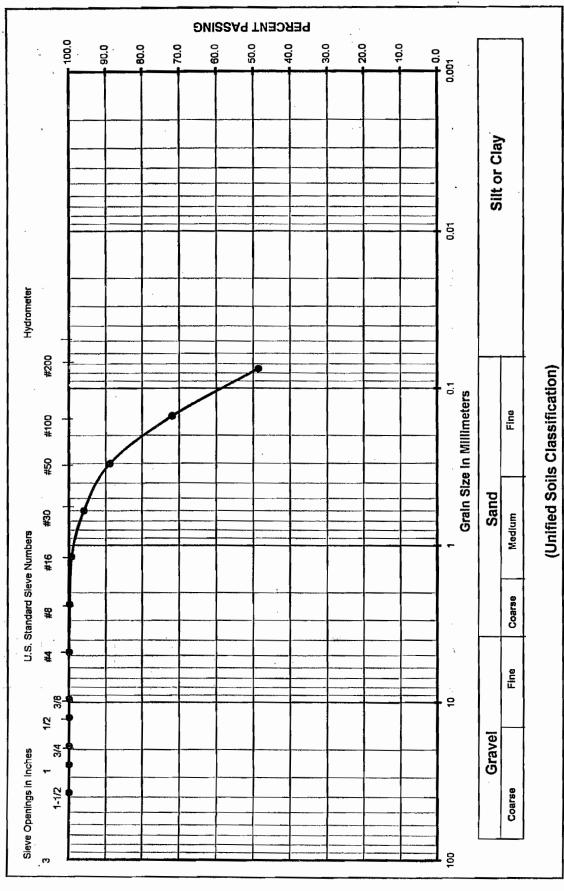




Project Number Soil Classification Sample Number Project Name

(ML), Sandy Silt w/ Clay B-6 @ 15' Westfield Mall 112-06041



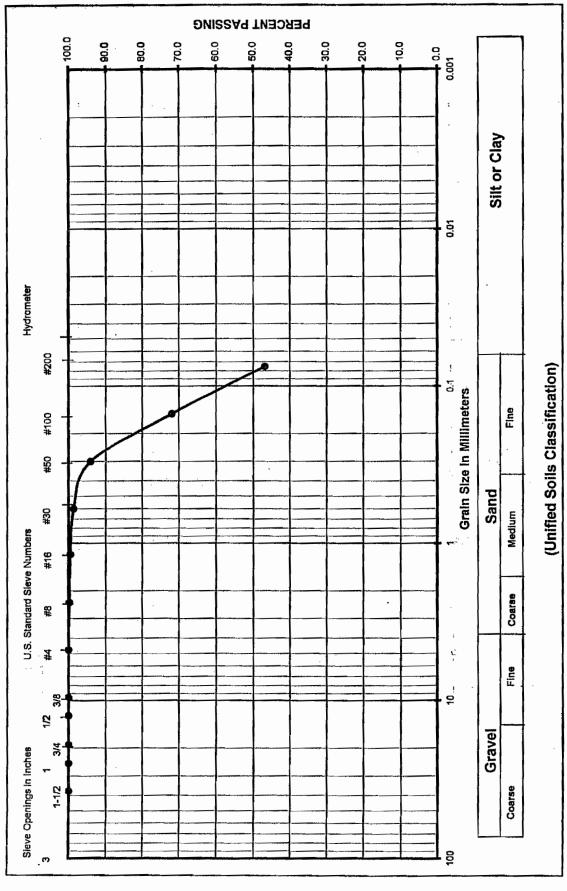


Project Name Project Number Soil Classification Sample Number

112-06041 (SM-ML), Silty Sand-Sandy Silt w/ Clay B-6 @ 20'

Westfield Mall





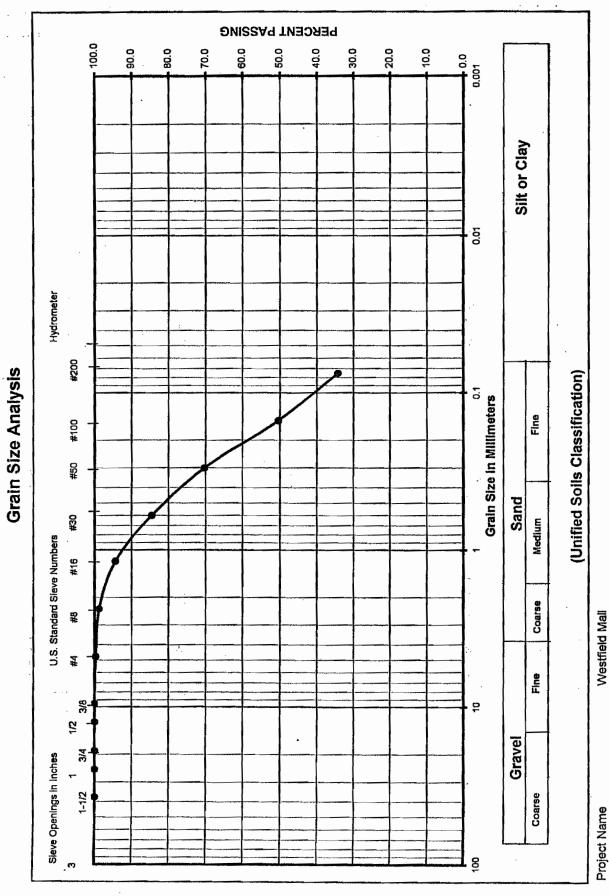
Project Name Project Number Soil Classification Sample Number

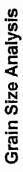
Westfield Mail 112-06041

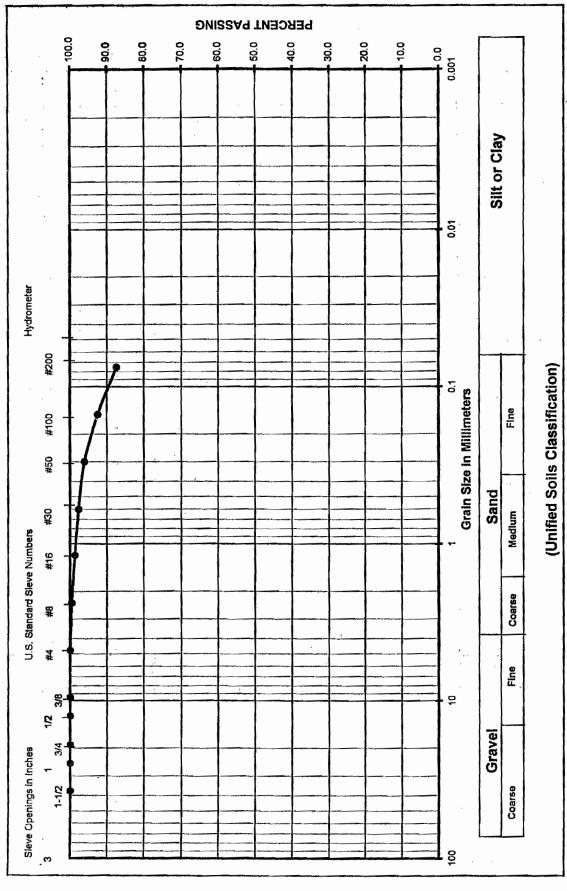
112-06041 (SM-ML), Silty Sand-Sandy Silt w/ Clay B-6 @ 25'

112-06041 (SM), Silty Sand w/ Clay B-6 @ 30'

Project Number Soil Classification Sample Number

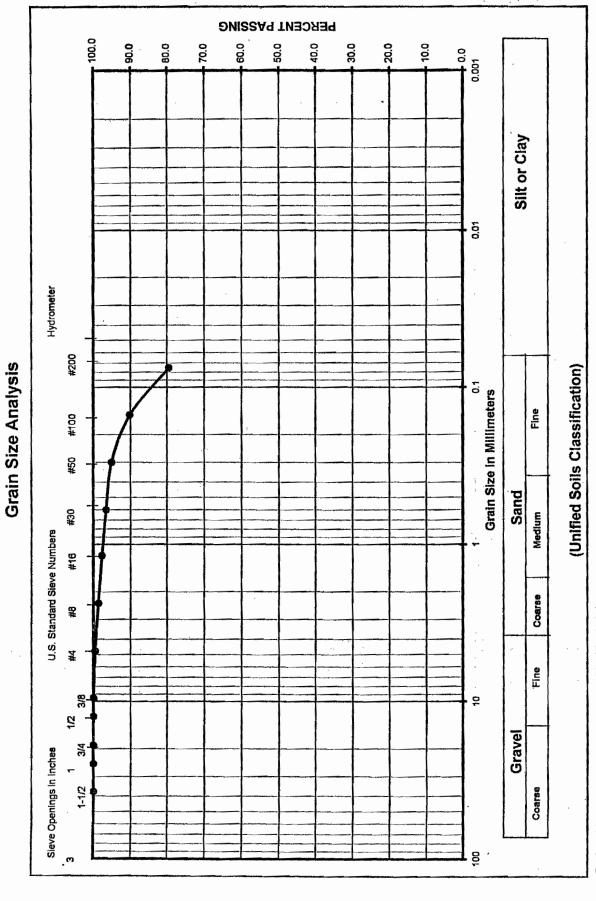






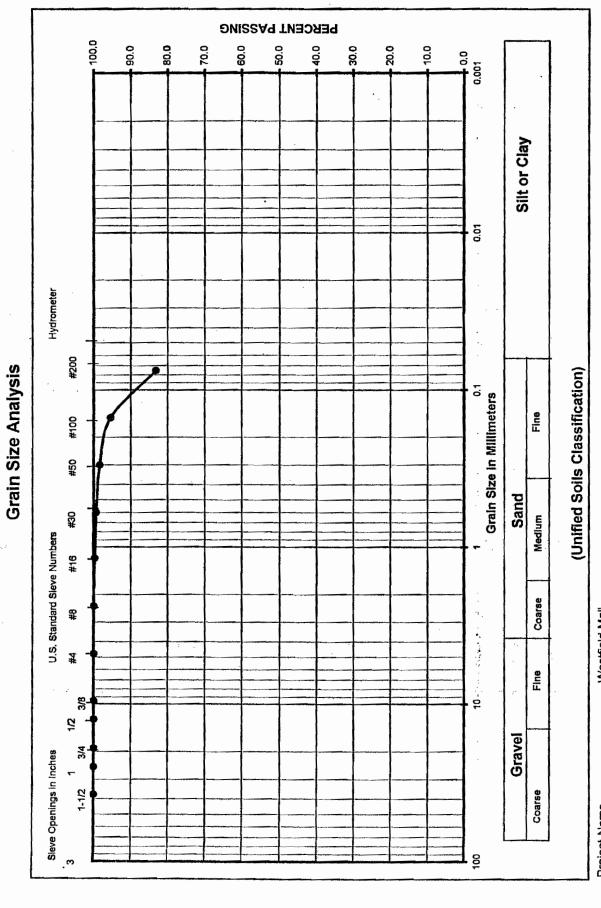
Project Name Project Number Soil Classification Sample Number

112-06041 (CL), Silty Clay B-6 @ 35' Westfield Mail



Soil Classification Sample Number Project Number Project Name

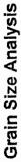
112-06041 (CL), Slity Clay B-6 @ 40' Westfield Mali

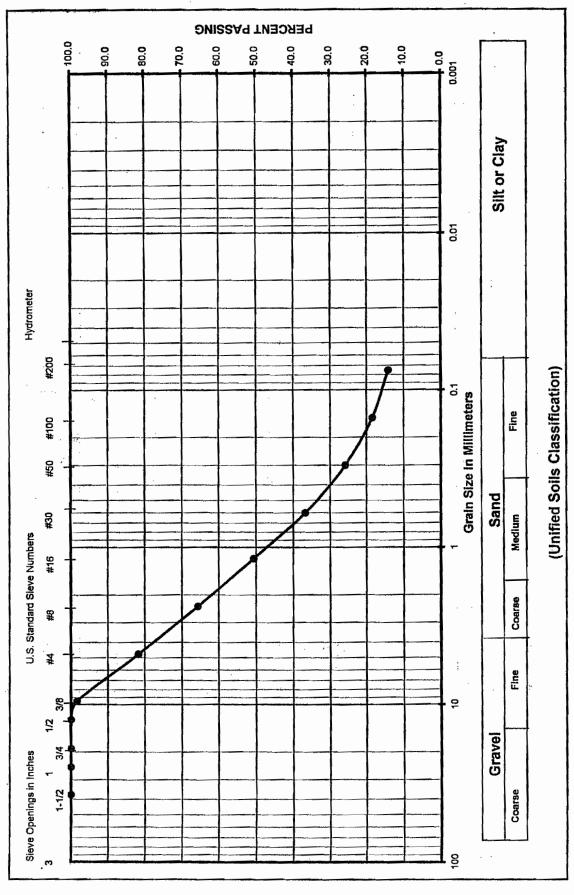


Project Name Project Number Soil Classification Sample Number

Westfield Mall

112-06041 (CL), Silty Clay B-6 @ 45'

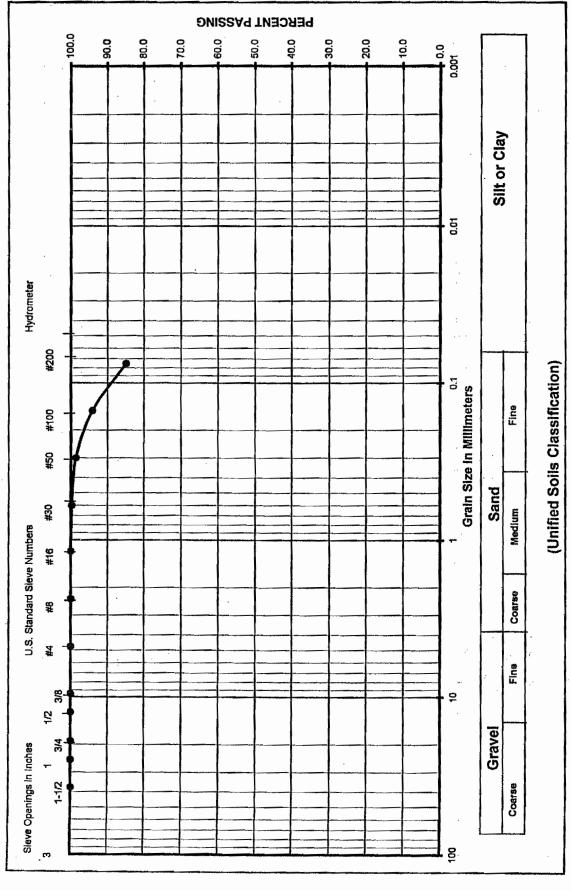




Project Name Project Number Soil Classification Sample Number

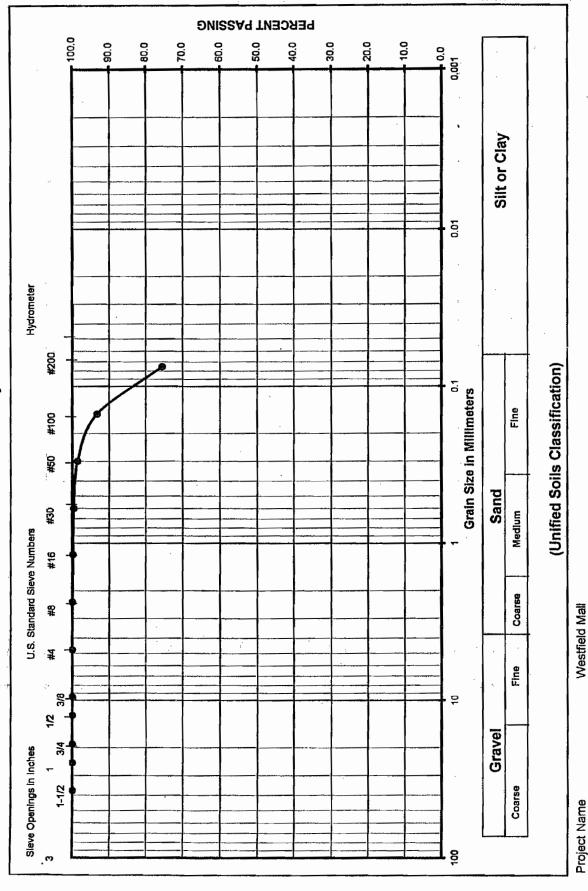
Westfield Mall 112-06041 (SM), Silty Sand w/ Little Gravel B-8 @ 5' - 6'





Soil Classification Sample Number Project Name Project Number

Westfield Mall 112-06041 (CL), Silty Clay B-8 @ 15' - 16'

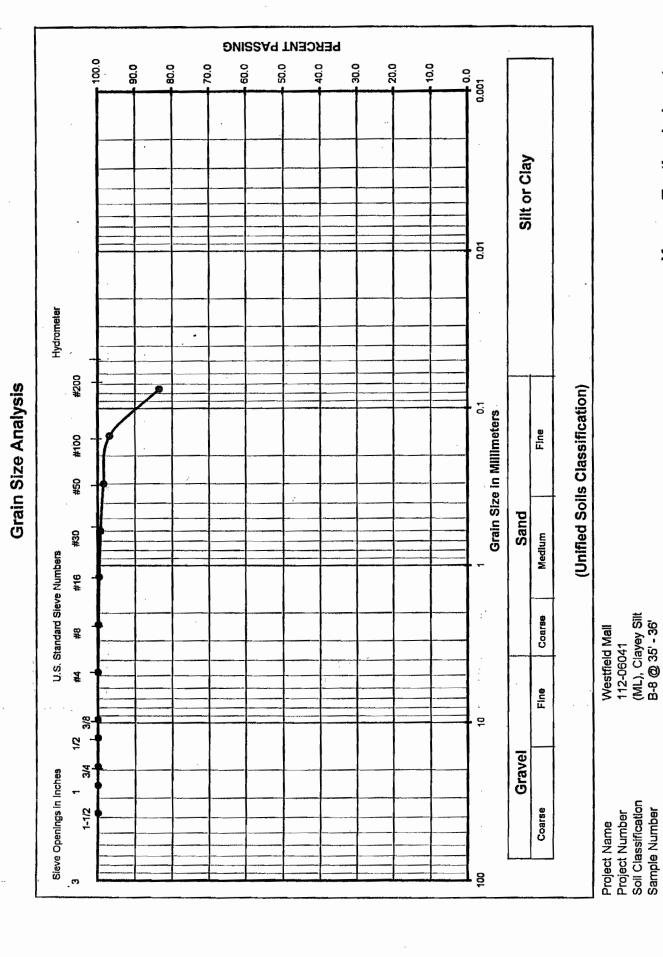


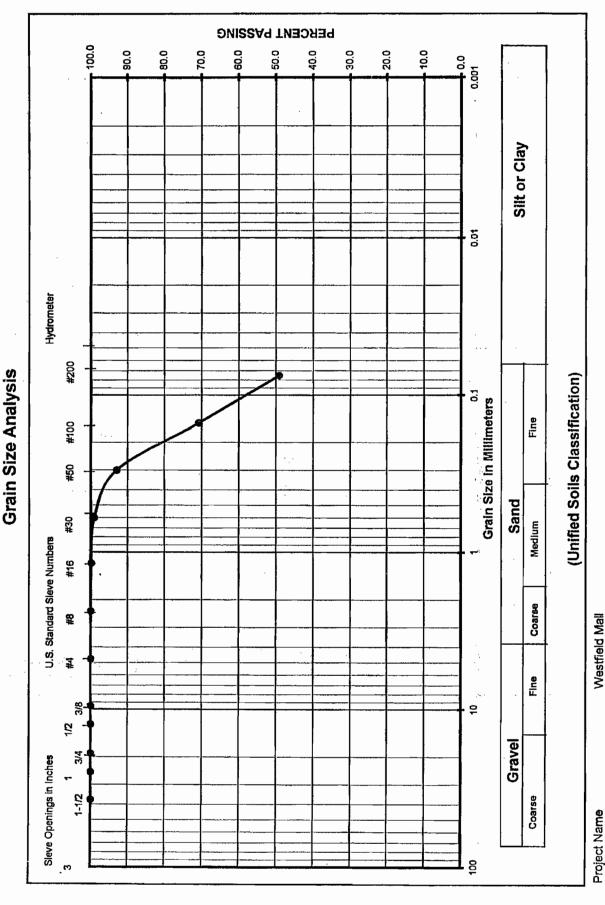
Krazan Testing Laboratory

(ML), Sandy Silt w/ Clay B-8 @ 25' - 26'

Project Number Soil Classification Sample Number

112-06041





Soil Classification Sample Number Project Number

(SM-ML), Silty Sand-Sandy Silt w/ Trace Clay B-8 @ 45' - 46' 112-06041

Expansion Index Test

ASTM D - 4829/ UBC Std. 18-2

Project Number

: 112-06041

Project Name

: Westfield Mall

Date

: 7/6/06

Sample location/ Depth

: B-1 @ 0-3'

Sample Number

: 1

Soil Classification

: (SM), Silty Sand w/ Trace Clay

Trial #	1	2	3
Weight of Soil & Mold, gms	596.7		
Weight of Mold, gms	170.8		
Weight of Soil, gms	425.9		
Wet Density, Lbs/cu.ft.	128.4		
Weight of Moisture Sample (Wet), gms	300.0		
Weight of Moisture Sample (Dry), gms	277.8		
Moisture Content, %	8.0		
Dry Density, Lbs/cu.ft.	118.9		
Specific Gravity of Soil	2.7		
Degree of Saturation, %	51.8		

Time	Inital	30 min	1 hr	6hrs	12 hrs	24 hrs
Dial Reading				:		0.016

Expansion Index _{measured} = 16

Expansion Index $_{50}$ = 16.9

Expansion Index = 17

Expansion Potential Table				
Exp. Index	Potential Exp.			
0 - 20	Very Low			
21 - 50	Low			
51 - 90	Medium			
91 - 130	High			
>130	Very High			

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Expansion Index Test

ASTM D - 4829/ UBC Std. 18-2

Project Number

: 112-06041

Project Name

: Westfield Mall

Date

: 7/6/06

Sample location/ Depth

: B-4 @ 0-3'

Sample Number

: 2

Soil Classification

: (SM), Silty Sand w/ Clay

Trial #	1	2	3
Weight of Soil & Mold, gms	615.8		
Weight of Mold, gms	185.0		
Weight of Soil, gms	430.8		
Wet Density, Lbs/cu.ft.	129.9		
Weight of Moisture Sample (Wet), gms	300.0		
Weight of Moisture Sample (Dry), gms	277.0	***	
Moisture Content, %	8.3		
Dry Density, Lbs/cu.ft.	120.0		
Specific Gravity of Soil	2.7		- 1-11:
Degree of Saturation, %	55.4	·	

Time	Inital	30 min	1 hr	6hrs_	12 hrs	24 hrs
Dial Reading			·		w/	0.042

Expansion Index $_{\text{measured}}$ = 42

Expansion Index $_{50}$ = 45.5

Expansion Index = 46

Expansion Potential Table				
Exp. Index	Potential Exp.			
0 - 20	Very Low			
21 - 50	Low			
51 - 90	Medium			
91 - 130	High			
>130	Very High			

Krazan Testing Laboratory

Expansion Index Test

ASTM D - 4829/ UBC Std. 18-2

Project Number

: 112-06041

Project Name

: Westfield Mall

Date

: 7/6/06

Sample location/ Depth

: B-7 @ 0-3'

Sample Number

: 3

Soil Classification

: (ML), Sandy Silt w/ Clay

Trial #	1	2	3
Weight of Soil & Mold, gms	599.7		······································
Weight of Mold, gms	185.0		
Weight of Soil, gms	414.7		
Wet Density, Lbs/cu.ft.	125.1		
Weight of Moisture Sample (Wet), gms	300.0		
Weight of Moisture Sample (Dry), gms	274.5	- <u> </u>	
Moisture Content, %	9.3		
Dry Density, Lbs/cu.ft.	114.4		
Specific Gravity of Soil	2.7		
Degree of Saturation, %	53.1		

Time	Inital	30 min	1 hr	6hrs	12 hrs	24 hrs
Dial Reading		1		- 1,500		0.054

Expansion Index measured

= 54

Expansion Index 50

= 56.2

Expansion Index =

56

Expansion Potential Table					
Exp. Index	Potential Exp.				
0 - 20	Very Low				
21 - 50	Low				
51 - 90	Medium				
91 - 130	High				
>130	Very High				

Krazan Testing Laboratory

R - VALUE TEST ASTM D - 2844 / CAL 301

Project Number

112-06041

Project Name

Westfield Mall

Date

9/21/06

Sample Location/Curve Number

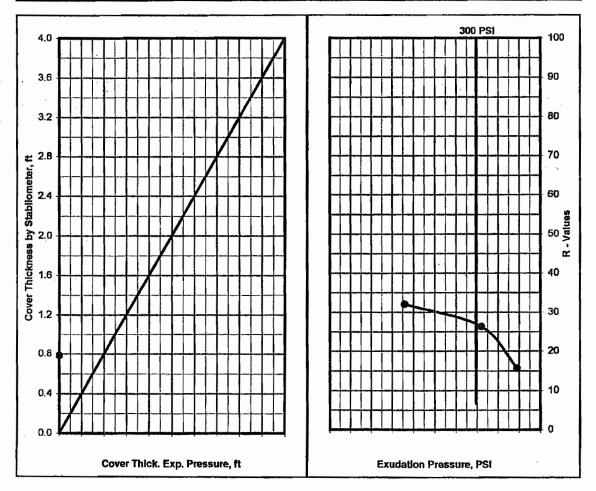
RV#1 (B-1 @ 0-3')

Soil Classification

(SM), Silty Sand w/ Clay

TEST	Α	В	C
Percent Moisture @ Compaction, %	14.5	13.5	15.5
Dry Density, Ibm/cu.ft.	120.5	121.1	122.2
Exudation Pressure, psi	280	640	110
Expansion Pressure, (Dial Reading)	0	0	0
Expansion Pressure, psf	0	0	0
Resistance Value R	26	32	16

R Value at 300 PSI Exudation Pressure	(27)
R Value by Expansion Pressure (TI =): 5	Expansion Pressure nil



R - VALUE TEST ASTM D - 2844 / CAL 301

· Project Number

112-06041

Project Name

Westfield Mall

Date

9/21/06

Sample Location/Curve Number

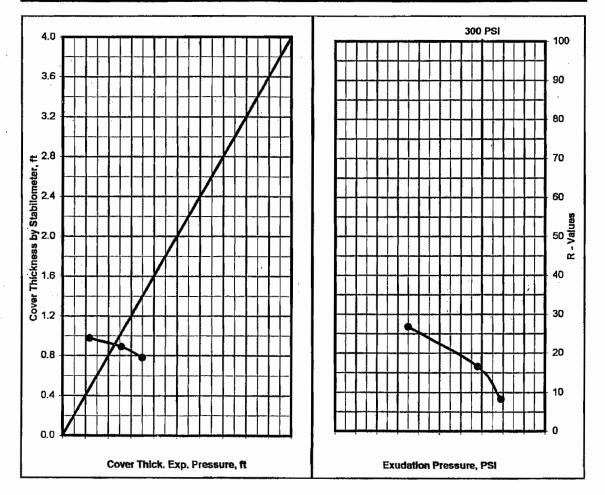
RV#7 (B-2 @ 0-3')

Soil Classification

(ML), Sandy Silt w/ Clay

TEST	A	В	С
Percent Moisture @ Compaction, %	18.8	19.9	17.7
Dry Density, Ibm/cu.ft.	111.2	112.3	111.6
Exudation Pressure, psi	320	210	650
Expansion Pressure, (Dial Reading)	31	14	42
Expansion Pressure, psf	134	61	182
Resistance Value R	17	8	27

51-12-1 Hard	
R Value by Expansion Pressure (TI =): 5	(14)
R Value at 300 PSI Exudation Pressure	16



Laboratory Compaction Curve ASTM - D1557, D698

Project Number Project Name : 112-06041 : Westfield Mall

Date

: 07/06/06

Sample location

: B-7 @ 0-3'

Sample/Curve Number

: 3

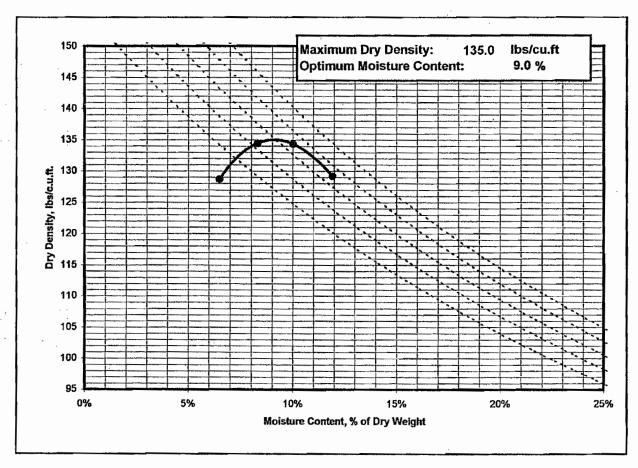
Soil Classification

: (SM), Silty Sand w/ Clay

Test Method

: 1557A

	1	2	3	4
Weight of Moist Specimen & Mold, gm	4229.3	4195.9	4067.8	4181.6
Weight of Compaction Mold, gm	2003.7	2003.7	2003.7	2003.7
Weight of Moist Specimen, gm	2225.6	2192.2	2064.1	2177.9
Volume of mold, cu. ft.	0.0332	0.0332	0.0332	0.0332
Wet Density, lbs/cu.ft.	147.8	145.6	137.1	144.6
Weight of Wet (Moisture) Sample, gm	200.0	200.0	200.0	200.0
Weight of Dry (Moisture) Sample, gm	181.8	184.7	187.8	178.7
Moisture Content, %	10.0%	8.3%	6.5%	11.9%
Dry Density, Ibs/cu.ft.	134.3	134.4	128.7	129.2



Enviro - Chem, Inc.

1214 E. Lexington Avenue, Pomona, CA 91766 Tel (909) 590-5905 Fax (909) 590-5907

LABORATORY REPORT

CUSTOMER: Krazan & Associates, Inc.

4221 Brickell St. Ontario, CA 91761

Tel(909)974-4400 Fax(909)974-4022

PROJECT: Sherman Oaks

MATRIX: SOIL

DATE RECEIVED: 07/11/06

SAMPLING DATE: 06/29/06 DATE ANALYZED: 07/11-12/06

REPORT TO: MR. CLARENCE JIANG DATE REPORTED: 07/14/06

SAMPLE I.D.: 112-06041 / B-1@0-3' LAB I.D.: 060711-9

	. 		- 		
PARAMETER	SAMPLE RESULT	UNIT	PQL	DF	METHOD
RESISTIVITY	6670	OHMS-CM	100000*		CALTRANS
SULFATE	303	MG/KG	10	2	EPA 9038
CHLORIDE	17.1	MG/KG	10	1	EPA 9253
рН	7.77	pH/Unit	~ ~		EPA 9045C

COMMENTS

DF = DILUTION FACTOR PQL = PRACTICAL QUANTITATION LIMIT ACTUAL DETECTION LIMIT = DF X PQL MG/KG = MILLIGRAM PER KILOGRAM = PPM OHMS-CM = OHMS-CENTIMETER RESISTIVITY = 1/CONDUCTIVITY * = HIGH LIMIT

DATA REVIEWED AND APPROVED BY: CAL-DHS ELAP CERTIFICATE No.: 1555

APPENDIX B

GENERAL EARTHWORK SPECIFICATIONS

GENERAL

When the text of the report conflicts with the general specifications in this appendix, the recommendations in the report have precedence.

SCOPE OF WORK: These specifications and applicable plans pertain to and include all earthwork associated with the site rough grading, including, but not limited to, the furnishing of all labor, tools and equipment necessary for site clearing and grubbing, stripping, preparation of foundation materials for receiving fill, excavation, processing, placement and compaction of fill and backfill materials to the lines and grades shown on the project grading plans and disposal of excess materials.

PERFORMANCE: The Contractor shall be responsible for the satisfactory completion of all earthworks in accordance with the project plans and specifications. This work shall be inspected and tested by a representative of Krazan and Associates, Incorporated, hereinafter referred to as the Soils Engineer and/or Testing Agency. Attainment of design grades, when achieved shall be certified by the project Civil Engineer. Both the Soils Engineer and the Civil Engineer are the Owner's representatives. If the Contractor should fail to meet the technical or design requirements embodied in this document and on the applicable plans, he shall make the necessary adjustments until all work is deemed satisfactory as determined by both the Soils Engineer and the Civil Engineer. No deviation from these specifications shall be made except upon written approval of the Soils Engineer, Civil Engineer, or project Architect.

No earthwork shall be performed without the physical presence or approval of the Soils Engineer. The Contractor shall notify the Soils Engineer at least 2 working days prior to the commencement of any aspect of the site earthwork.

The Contractor agrees that he shall assume sole and complete responsibility for job site conditions during the course of construction of this project, including safety of all persons and property; that this requirement shall apply continuously and not be limited to normal working hours; and that the Contractor shall defend, indemnify and hold the Owner and the Engineers harmless from any and all liability, real or alleged, in connection with the performance of work on this project, except for liability arising from the sole negligence of the Owner or the Engineers.

TECHNICAL REQUIREMENTS: All compacted materials shall be densified to no less that 90 percent of relative compaction based on ASTM D1557-00 Test Method, UBC or CAL-216, as specified in the technical portion of the Soil Engineer's report. The location and frequency of field density tests shall be as determined by the Soils Engineer. The results of these tests and compliance with these specifications shall be the basis upon which satisfactory completion of work will be judged by the Soils Engineer.

SOILS AND FOUNDATION CONDITIONS: The Contractor is presumed to have visited the site and to have familiarized himself with existing site conditions and the contents of the data presented in the Geotechnical Engineering Report.

The Contractor shall make his own interpretation of the data contained in the Geotechnical Engineering Report and the Contractor shall not be relieved of liability under the Contractor for any loss sustained as a result of any variance between conditions indicated by or deduced from said report and the actual conditions encountered during the progress of the work.

DUST CONTROL: The work includes dust control as required for the alleviation or prevention of any dust nuisance on or about the site or the borrow area, or off-site if caused by the Contractor's operation either during the performance of the earthwork or resulting from the conditions in which the Contractor leaves the site. The Contractor shall assume all liability, including court costs of codefendants, for all claims related to dust or wind-blown materials attributable to his work.

SITE PREPARATION

Site preparation shall consist of site clearing and grubbing and preparation of foundation materials for receiving fill.

CLEARING AND GRUBBING: The Contractor shall accept the site in this present condition and shall demolish and/or remove from the area of designated project earthwork all structures, both surface and subsurface, trees, brush, roots, debris, organic matter and all other matter determined by the Soils Engineer to be deleterious. Such materials shall become the property of the Contractor and shall be removed from the site.

Tree root systems in proposed building areas should be removed to a minimum depth of 3 feet and to such an extent, which would permit removal of all roots greater than 1 inch in diameter. Tree roots removed in parking areas may be limited to the upper 1½ feet of the ground surface. Backfill or tree root excavation should not be permitted until all exposed surfaces have been inspected and the Soils Engineer is present for the proper control of backfill placement and compaction. Burning in areas, which are to receive fill materials, shall not be permitted.

SUBGRADE PREPARATION: Surfaces to receive Engineered Fill, shall be prepared as outlined above, excavated/scarified to a minimum depth of 8 inches, moisture-conditioned as necessary, and recompacted to at least 90 percent relative compaction.

Loose soil areas and/or areas of disturbed soil shall be moisture-conditioned as necessary and recompacted to 90 percent relative compaction. All ruts, hummocks, or other uneven surface features shall be removed by surface grading prior to placement of any fill materials. All areas, which are to receive fill materials, shall be approved by the Soils Engineer prior to the placement of any of the fill material.

EXCAVATION: All excavation shall be accomplished to the tolerance normally defined by the Civil Engineer as shown on the project grading plans. All over-excavation below the grades specified shall be backfilled at the Contractor's expense and shall be compacted in accordance with the applicable technical requirements.

FILL AND BACKFILL MATERIAL: No material shall be moved or compacted without the presence of the Soils Engineer. Material from the required site excavation may be utilized for construction site fills, provided prior approval is given by the Soils Engineer. All materials utilized for constructing site fills shall be free from vegetation or other deleterious matter as determined by the Soils Engineer.

PLACEMENT, SPREADING AND COMPACTION: The placement and spreading of approved fill materials and the processing and compaction of approved fill and native materials shall be the responsibility of the Contractor. However, compaction of fill materials by flooding, ponding, or jetting shall not be permitted unless specifically approved by local code, as well as the Soils Engineer.

Both cut and fill shall be surface-compacted to the satisfaction of the Soils Engineer prior to final acceptance.

SEASONAL LIMITS: No fill material shall be placed, spread, or rolled while it is frozen or thawing, or during unfavorable wet weather conditions. When the work is interrupted by heavy rains, fill operations shall not be resumed until the Soils Engineer indicates that the moisture content and density of previously placed fill is as specified.

<u>APPENDIX C</u>

GENERAL PAVEMENT SPECIFICATIONS

 DEFINITIONS - The term "pavement" shall include asphalt concrete surfacing, untreated aggregate base, and aggregate subbase. The term "subgrade" is that portion of the area on which surfacing, base, or subbase is to be placed.

The term "Standard Specifications": hereinafter referred to is the January 1999 Standard Specifications of the State of California, Department of Transportation, and the "Materials Manual" is the Materials Manual of Testing and Control Procedures, State of California, Department of Public Works, Division of Highways. The term "relative compaction" refers to the field density expressed as a percentage of the maximum laboratory density as defined in the ASTM D1557-00.

- 2. SCOPE OF WORK This portion of the work shall include all labor, materials, tools, and equipment necessary for, and reasonably incidental to the completion of the pavement shown on the plans and as herein specified, except work specifically notes as "Work Not Included."
- 3. PREPARATION OF THE SUBGRADE The Contractor shall prepare the surface of the various subgrades receiving subsequent pavement courses to the lines, grades, and dimensions given on the plans. The upper 12 inches of the soil subgrade beneath the pavement section shall be compacted to a minimum relative compaction of 90 percent. The finished subgrades shall be tested and approved by the Geotechnical Engineer prior to the placement of additional pavement courses.
- 4. UNTREATED AGGREGATE BASE The aggregate base material shall be spread and compacted on the prepared subgrade in conformity with the lines, grades, and dimensions shown on the plans. The aggregate base material shall conform to the requirements of Section 26 of the Standard Specifications for Class 2 material, ¾-inches maximum size. The aggregate base material shall be compacted to a minimum relative compaction of 95 percent. The aggregate base material shall be spread and compacted in accordance with Section 26 of the Standard Specifications. The aggregate base material shall be spread in layers not exceeding 6 inches and each layer of aggregate material course shall be tested and approved by the Geotechnical Engineer prior to the placement of successive layers.
- 5. AGGREGATE SUBBASE The aggregate subbase shall be spread and compacted on the prepared subgrade in conformity with the lines, grades, and dimensions shown on the plans. The aggregate subbase material shall conform to the requirements of Section 25 of the Standard Specifications for Class II material. The aggregate subbase material shall be compacted to a minimum relative compaction of 95 percent, and it shall be spread and compacted in accordance with Section 25 of the Standard Specifications. Each layer of aggregate subbase shall be tested and approved by the Geotechnical Engineer prior to the placement of successive layers.

6. ASPHALT CONCRETE SURFACING - Asphalt concrete surfacing shall consist of a mixture of mineral aggregate and paving grade asphalt, mixed at a central mixing plant and spread and compacted on a prepared base in conformity with the lines, grades, and dimensions shown on the plans. The viscosity grade of the asphalt shall be AR-4000. The mineral aggregate shall be Type B, ½-inch or ¾-inch maximum, medium grading, for the wearing course and ¾-inch maximum, medium grading for the base course, and shall conform to the requirements set forth in Section 39 of the Standard Specifications. The drying, proportioning, and mixing of the materials shall conform to Section 39.

The prime coat, spreading and compacting equipment, and spreading and compacting the mixture shall conform to the applicable chapters of Section 39, with the exception that no surface course shall be placed when the atmospheric temperature is below 50 degrees F. The surfacing shall be rolled with a combination steel-wheel and pneumatic rollers, as described in Section 39-6. The surface course shall be placed with an approved self-propelled mechanical spreading and finishing machine.

7. FOG SEAL COAT - The fog seal (mixing type asphalt emulsion) shall conform to and be applied in accordance with the requirements of Section 37.

APPENDIX E

EDR ENVIRONMENTAL DATABASE REPORT AND MULTI TENANT FACILITY REPORT



The EDR Radius Map with GeoCheck®

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number: 2096148.2s

December 10, 2007

The Standard in Environmental Risk Information

440 Wheelers Farms Road Milford, Connecticut 06461

Nationwide Customer Service

Telephone: 1-800-352-0050 Fax: 1-800-231-6802

Internet: www.edrnet.com

TABLE OF CONTENTS

SECTION	PAGE
Executive Summary	ES1
Overview Map.	_ 2
Detail Map	_ 3
Map Findings Summary	4
Map Findings	. 6
Orphan Summary	
Zip Scan Report.	ZIP-1
Government Records Searched/Data Currency Tracking	. GR-1
GEOCHECK ADDENDUM	
Physical Setting Source Addendum	. A-1
Physical Setting Source Summary	A-2
Physical Setting Source Map.	. A-7
Physical Setting Source Map Findings.	. A-8
Physical Setting Source Records Searched	A-9

Thank you for your business.
Please contact EDR at 1-800-352-0050
with any questions or comments.

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A search of available environmental records was conducted by Environmental Data Resources, Inc (EDR). The report was designed to assist parties seeking to meet the search requirements of EPA's Standards and Practices for All Appropriate Inquiries (40 CFR Part 312), the ASTM Standard Practice for Environmental Site Assessments (E 1527-05) or custom requirements developed for the evaluation of environmental risk associated with a parcel of real estate.

TARGET PROPERTY INFORMATION

ADDRESS

14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423

COORDINATES

Latitude (North):

34.156770 - 34° 9' 24.4"

Longitude (West):

118.435520 - 118° 26' 7.9"

Universal Tranverse Mercator: Zone 11 UTM X (Meters): UTM Y (Meters):

367668.7 3780274.5

Elevation:

659 ft. above sea level

USGS TOPOGRAPHIC MAP ASSOCIATED WITH TARGET PROPERTY

Target Property Map:

34118-B4 VAN NUYS, CA

Most Recent Revision:

1991

TARGET PROPERTY SEARCH RESULTS

The target property was identified in the following records. For more information on this property see page 6 of the attached EDR Radius Map report:

Site	Database(s)	EPA ID
SEPHORA STORES #22 14006 RIVERSIDE DR STE 75 SHERMAN OAKS, CA 91423	HAZNET	N/A
KITS CAMERA 1 HOUR NO 87 14006 RIVERSIDE DR STE 86 SHERMAN OAKS, CA 91423	RCRA-SQG FINDS	CAD983669599
SEPHORA STORE 22 FASHION SQUARE 14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423	RCRA-SQG HAZNET	CAR000147447
J P MECHANICAL 14006 RIVERSIDE SHERMAN OAKS, CA 91423	HAZNET	N/A
CITY FREEHOLDS (USA INC) 14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423	HAZNET	N/A

LENSCRAFTERS STORE #501 14006 RIVERSIDE DR SHERMAN OAKS, CA 91423 HAZNET EMI N/A

SEPHORA STORES #18 14006 RIVERSIDE DR STE 75 SHERMAN OAKS, CA 91423 HAZNET

N/A

SEPHORA STORE 22 FASHION SQUARE 14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423 FINDS

110015672039

DATABASES WITH NO MAPPED SITES

No mapped sites were found in EDR's search of available ("reasonably ascertainable") government records either on the target property or within the search radius around the target property for the following databases:

FEDERAL RECORDS

NPL National Priority List

Proposed NPL Proposed National Priority List Sites

Delisted NPL National Priority List Deletions

NPL LIENS..... Federal Superfund Liens

CERCLIS No Further Remedial Action Planned

CORRACTS..... Corrective Action Report

RCRA-TSDF........ Resource Conservation and Recovery Act Information RCRA-LQG.......... Resource Conservation and Recovery Act Information

ERNS..... Emergency Response Notification System

HMIRS..... Hazardous Materials Information Reporting System

CONSENT...... Superfund (CERCLA) Consent Decrees

TRIS...... Toxic Chemical Release Inventory System

TSCA..... Toxic Substances Control Act

FTTS...... FIFRA/ TSCA Tracking System - FIFRA (Federal Insecticide, Fungicide, & Rodenticide

Act)/TSCA (Toxic Substances Control Act)

DEBRIS REGION 9...... Torres Martinez Reservation Illegal Dump Site Locations

ICIS...... Integrated Compliance Information System

Land Use Control Information System

US CDL...... Clandestine Drug Labs

RADINFO...... Radiation Information Database

LIENS 2..... CERCLA Lien Information

HIST FTTS..... FIFRA/TSCA Tracking System Administrative Case Listing

PADS PCB Activity Database System MLTS..... Material Licensing Tracking System

MINES..... Mines Master Index File

RAATS..... RCRA Administrative Action Tracking System

STATE AND LOCAL RECORDS

HIST Cal-Sites Historical Calsites Database

CA BOND EXP. PLAN...... Bond Expenditure Plan

..... School Property Evaluation Program Toxic Pits Cleanup Act Sites SWF/LF...... Solid Waste Information System

CA WDS...... Waste Discharge System

WMUDS/SWAT...... Waste Management Unit Database

SWRCY..... Recycler Database Statewide SLIC Cases

AOCONCERN..... San Gabriel Valley Areas of Concern

UST..... Active UST Facilities

AST..... Aboveground Petroleum Storage Tank Facilities

Environmental Liens Listing SWEEPS UST Listing

CHMIRS...... California Hazardous Material Incident Report System

LA Co. Site Mitigation List

DEED Deed Restriction Listing

Voluntary Cleanup Program Properties

CLEANERS...... Cleaner Facilities

WIP. Well Investigation Program Case List

LOS ANGELES CO. HMS.... HMS: Street Number List Clandestine Drug Labs RESPONSE...... State Response Sites

HAULERS Registered Waste Tire Haulers Listing

TRIBAL RECORDS

INDIAN RESERV..... Indian Reservations

INDIAN LUST..... Leaking Underground Storage Tanks on Indian Land

INDIAN UST..... Underground Storage Tanks on Indian Land

EDR PROPRIETARY RECORDS

Manufactured Gas Plants... EDR Proprietary Manufactured Gas Plants EDR Historical Auto Stations EDR Proprietary Historic Gas Stations EDR Historical Cleaners EDR Proprietary Historic Dry Cleaners

SURROUNDING SITES: SEARCH RESULTS

Surrounding sites were identified in the following databases.

Elevations have been determined from the USGS Digital Elevation Model and should be evaluated on a relative (not an absolute) basis. Relative elevation information between sites of close proximity should be field verified. Sites with an elevation equal to or higher than the target property have been differentiated below from sites with an elevation lower than the target property.

Page numbers and map identification numbers refer to the EDR Radius Map report where detailed data on individual sites can be reviewed.

Sites listed in **bold italics** are in multiple databases.

Unmappable (orphan) sites are not considered in the foregoing analysis.

FEDERAL RECORDS

RCRAInfo: RCRAInfo is EPA's comprehensive information system, providing access to data supporting the Resource Conservation and Recovery Act (RCRA) of 1976 and the Hazardous and Solid Waste Amendments (HSWA) of 1984. RCRAInfo replaces the data recording and reporting abilities of the Resource Conservation and Recovery Information System(RCRIS). The database includes selective information on sites which generate, transport, store, treat and/or dispose of hazardous waste as defined by the Resource Conservation and Recovery Act (RCRA). Conditionally exempt small quantity generators (CESQGs) generate less than 100 kg of hazardous waste, or less than 1 kg of acutely hazardous waste per month. Small quantity generators (SQGs) generate between 100 kg and 1,000 kg of hazardous waste per month Large quantity generators generate over 1,000 kilograms (kg) of hazardous waste, or over 1 kg of acutely hazardous waste per month. Transporters are individuals or entities that move hazardous waste from the generator offsite to a facility that can recycle, treat, store, or dispose of the waste. TSDFs treat, store, or dispose of the waste.

A review of the RCRA-SQG list, as provided by EDR, and dated 06/13/2006 has revealed that there are 3 RCRA-SQG sites within approximately 0.25 miles of the target property.

Lower Elevation	Address	Dist / Dir Map ID	Page
BURBANK MEDICAL CLINIC INC	13739 RIVERSIDE DR		11
CHEVRON STATION 9 1683	14061 RIVERSIDE DR		16
HIGH TECH AUTO	4774 WOODMAN		17

STATE AND LOCAL RECORDS

CORTESE: This database identifies public drinking water wells with detectable levels of contamination, hazardous substance sites selected for remedial action, sites with known toxic material identified through the abandoned site assessment program, sites with USTs having a reportable release and all solid waste disposal facilities from which there is known migration. The source is the California Environmental Protection Agency/Office of Emergency Information.

A review of the Cortese list, as provided by EDR, and dated 04/01/2001 has revealed that there are 4 Cortese sites within approximately 0.5 miles of the target property.

Lower Elevation	Address	Dist / Dir Map ID	Page
CHEVRON #9-1683	14061 RIVERSIDE DR	1/8 - 1/4WNW B12	14
FASHION SQUARE CAR WASH	4625 WOODMAN AVE	1/4 - 1/2ESE 15	18
TOSCO S.S. #3176	13650 RIVERSIDE DR	1/4 - 1/2ENE C16	21
SUNKIST GROWERS INC	14130 RIVERSIDE DR	1/4 - 1/2W 18	25

LUST: The Leaking Underground Storage Tank Incident Reports contain an inventory of reported leaking underground storage tank incidents. The data come from the State Water Resources Control Board Leaking Underground Storage Tank Information System.

A review of the LUST list, as provided by EDR, and dated 10/10/2007 has revealed that there are 5 LUST sites within approximately 0.5 miles of the target property.

Lower Elevation	Address	Dist / Dir Map II	D Page
CHEVRON #9-1683 Facility Status: Case Closed	14061 RIVERSIDE DR	1/8 - 1/4WNW B12	14
FASHION SQUARE CAR WASH Facility Status: Remedial action (cleanup) Ur	4625 WOODMAN AVE	1/4 - 1/2ESE 15	18

Lower Elevation	Address	Dist / Dir	Map ID	Page
TOSCO S.S. #3176 Facility Status: Case Closed	13650 RIVERSIDE DR	1/4 - 1/2 ENE	C16	21
76 STATION 3176 Facility Status: Pollution Characterization	13650 RIVERSIDE DRIVE	1/4 - 1/2 ENE	C17	24
SUNKIST GROWERS INC Facility Status: Case Closed	14130 RIVERSIDE DR	1/4 - 1/2W	18	25

CA FID: The Facility Inventory Database contains active and inactive underground storage tank locations. The source is the State Water Resource Control Board.

A review of the CA FID UST list, as provided by EDR, and dated 10/31/1994 has revealed that there is 1 CA FID UST site within approximately 0.25 miles of the target property.

Lower Elevation	Address	Dist / Dir	Map ID	Page
CURRENT OCCUPANT	14061 RIVERSIDE DR	1/8 - 1/4WNV	V B11	14

HIST UST: Historical UST Registered Database.

A review of the HIST UST list, as provided by EDR, and dated 10/15/1990 has revealed that there is 1 HIST UST site within approximately 0.25 miles of the target property.

Lower Elevation	Address	Dist / Dir Map ID	Page
91683	14061 RIVERSIDE DR	1/8 - 1/4WNW B10	12

NOTIFY 65: Notify 65 records contain facility notifications about any release that could impact drinking water and thereby expose the public to a potential health risk. The data come from the State Water Resources Control Board's Proposition 65 database.

A review of the Notify 65 list, as provided by EDR, and dated 10/21/1993 has revealed that there is 1 Notify 65 site within approximately 1 mile of the target property.

Equal/Higher Elevation	Address	Dist / Dir	Map ID	Page
TOSCO - 76 STATION #2421	14478 VENTURA BLVD	1/2 - 1 WSW	/ 20	32

ENVIROSTOR: The Department of Toxic Substances Control's (DTSC's) Site Mitigation and Brownfields Reuse Program's (SMBRP's) EnviroStor database identifes sites that have known contamination or sites for which there may be reasons to investigate further. The database includes the following site types: Federal Superfund sites (National Priorities List (NPL)); State Response, including Military Facilities and State Superfund; Voluntary Cleanup; and School sites. EnviroStor provides similar information to the information that was available in CalSites, and provides additional site information, including, but not limited to, identification of formerly-contaminated properties that have been released for reuse, properties where environmental deed restrictions have been recorded to prevent inappropriate land uses, and risk characterization information that is used to assess potential impacts to public health and the environment at contaminated sites.

A review of the ENVIROSTOR list, as provided by EDR, and dated 08/28/2007 has revealed that there is

1 ENVIROSTOR site within approximately 1 mile of the target property.

Lower Elevation	Address	Dist / Dir	Map ID	Page	
JOY'S DRY CLEANERS	13313 MOORPARK STREET	1/2 - 1 ESE	21	38	
Facility Status: Refer: 1248 Local Agency					

Due to poor or inadequate address information, the following sites were not mapped:

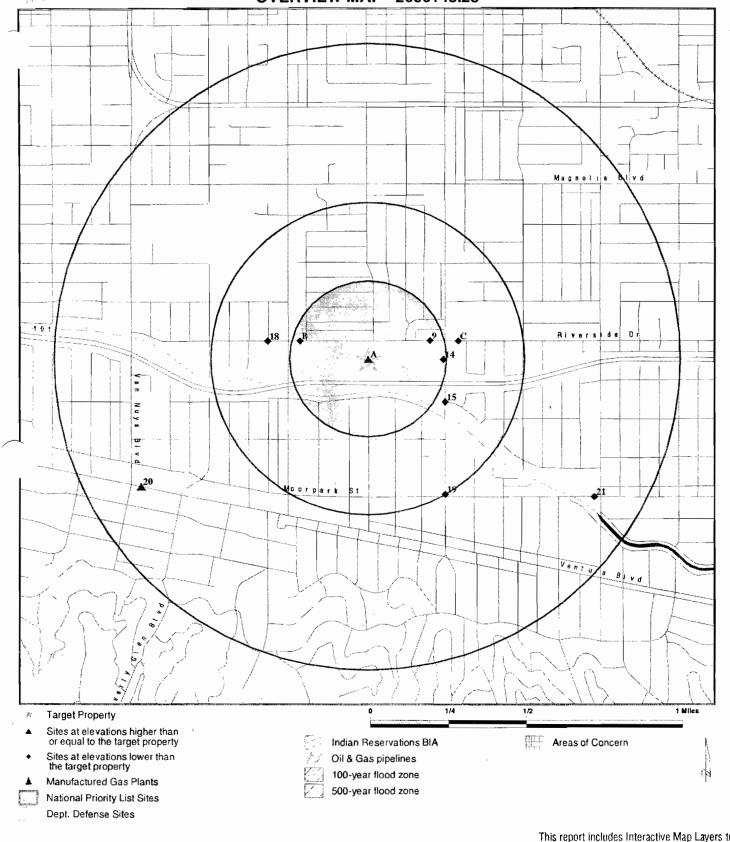
Site Name

Database(s)

1X MCKESSON DRUG CO

HAZNET, LUST, CHMIRS

OVERVIEW MAP - 2096148.2s



This report includes Interactive Map Layers to display and/or hide map information. The legend includes only those icons for the default map view.

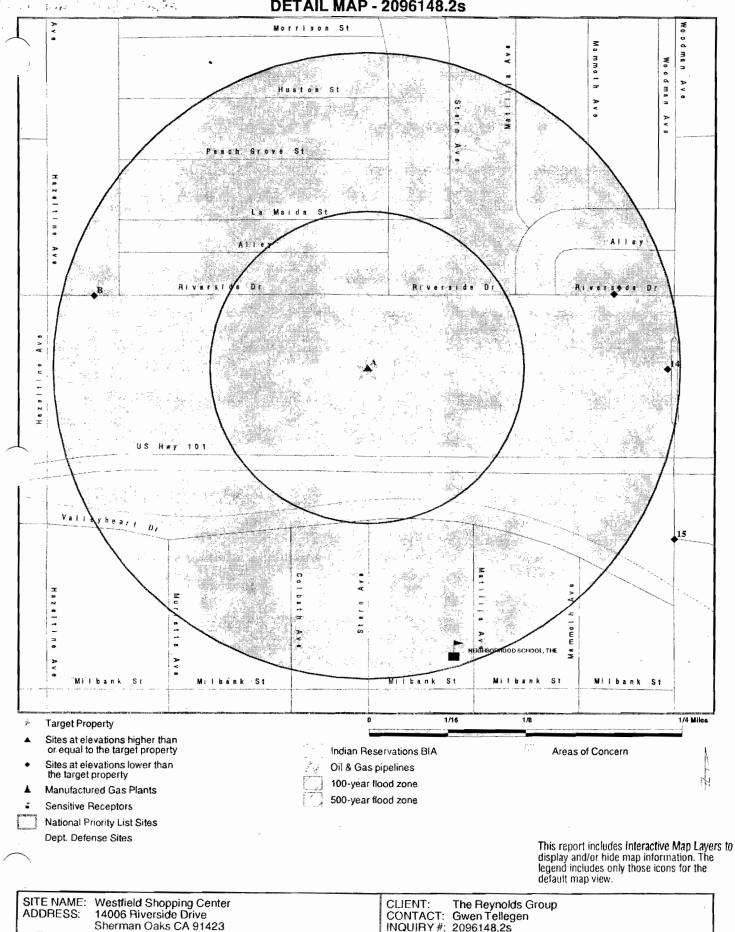
SITE NAME: Westfield Shopping Center ADDRESS: 14006 Riverside Drive

LAT/LONG:

Sherman Oaks CA 91423 34.1568 / 118.4355 CLIENT: The Reynolds Group CONTACT: Gwen Tellegen INQUIRY #: 2096148.2s

DATE: December 10, 2007 8:27 am

DETAIL MAP - 2096148.2s



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December 10, 2007 8:27 am

INQUIRY#: 2096148.2s

DATE:

LAT/LONG:

34.1568 / 118.4355

MAP FINDINGS SUMMARY

Database	Target Property	Search Distance (Miles)	< 1/8	1/8 - 1/4	1/4 - 1/2	1/2 - 1	>1	Total Plotted
FEDERAL RECORDS								
NPL Proposed NPL Delisted NPL NPL LIENS CERCLIS CERC-NFRAP CORRACTS RCRA TSD RCRA Lg. Quan. Gen. RCRA Sm. Quan. Gen. ERNS HMIRS US ENG CONTROLS US INST CONTROL DOD FUDS US BROWNFIELDS CONSENT ROD UMTRA ODI TRIS TSCA FTTS SSTS DOT OPS DEBRIS REGION 9 ICIS LUCIS CDL RADINFO LIENS 2 HIST FTTS PADS MLTS MINES FINDS RAATS	×	1.000 1.000 1.000 1.000 TP 0.500 0.500 1.000 0.250 0.250 TP TP 0.500 1.000 1.000 1.000 0.500 1.000 1.000 0.500 TP TP TP TP TP TP TP 0.500 TP	000K0000KK00000000000KKKKKK0K0KKKKKKKK	000K0000KKKKKKKKKKKKKKKKKKKKKKKKKKKKKK	000K000KKKKKKKK	0000XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX		000000000000000000000000000000000000000
STATE AND LOCAL RECO	ORDS			1411		NR	NR	J
Hist Cal-Sites CA Bond Exp. Plan SCH Toxic Pits State Landfill CA WDS WMUDS/SWAT	· · · · · · · · · · · · · · · · · · ·	1.000 1.000 0.250 1.000 0.500 TP 0.500	0 0 0 0 0 NR 0	0 0 0 0 0 NR 0	0 0 NR 0 0 NR 0	0 0 NR 0 NR NR NR	NR NR NR NR NR NR	0 0 0 0 0

MAP FINDINGS SUMMARY

Database	Target Property	Search Distance (Miles)	< 1/8	1/8 - 1/4	1/4 - 1/2	1/2 - 1	> 1	Total Plotted
Cortese		0.500	. 0	1	3	NR	NR	4
SWRCY		0.500	Ö	Ó	0	NR	NR	0
LUST		0.500	0	1	4	NR	NR	5
CA FID UST		0.250	0	1	NR	NR	NR	1
SLIC		0.500	0	0	0	·· NR	NR	0
AOCONCERN		1.000	0	0	0	0	NR	0
UST		0.250	0	0	NR	NR	NR	0
HIST UST		0.250	0	1	NR	NR	NR	1
AST		0.250	0	0	NR	NR	NR	0
LIENS		TP	NR	NR	NR	NR	NR	0
SWEEPS UST		0.250	0	0	NR	NR	NR	0
CHMIRS		ΤP	NR	NR	NR	NR	NR	0
Notify 65		1.000	0	0	0	1	NR	1
LA Co. Site Mitigation		TP	NR	NR	NR	NR	NR	0
DEED .		0.500	0	0	0	NR	NR	0
VCP		0.500	0	0	0	NR.	NR	0
DRYCLEANERS		0.250	0	0	NR	NR NR	NR	0
WIP		0.250	0	0	NR	NR	NR	0
Los Angeles Co. HMS		TP	NR	NR	NR NR	NR NR	NR NR	0 0
CDL		TP	NR	NR	0	0	NR NR	0
RESPONSE	v	1.000 TP	0 NR	0 NR	NR	NR	NR NR	0
HAZNET EMI	X X	TP	NR NR	NR NR	NR NR	NR	NR	0
ENVIROSTOR	^	1.000	0	0	0	1	NR	1
HAULERS :		TP	NR	NR	NR	NR	NR	Ö
HAULENS		11-	1417	1417	IVIX	1413	1414	U
TRIBAL RECORDS								
INDIAN RESERV		1.000	0	0	0 🕠	0	NR	0
INDIAN LUST		0.500	0	0	0 .	NR	NR	0
INDIAN UST		0.250	0	0	NR	NR	NR	0
EDR PROPRIETARY RECOI	RD\$							
Manufactured Gas Plants		1.000	0	0	0	0	NR	0
EDR Historical Auto Statio	ns	0.250	0	0	NR	NR	NR	0
EDR Historical Cleaners		0.250	0	0	NR	NR	NR	0

NOTES:

TP = Target Property

NR = Not Requested at this Search Distance

Sites may be listed in more than one database

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

EDR ID Number EPA ID Number

A1 **Target** **SEPHORA STORES #22** 14006 RIVERSIDE DR STE 75 SHERMAN OAKS, CA 91423

HAZNET

· S108220189

N/A

Site 1 of 8 in cluster A

Actual: 659 ft.

Property

HAZNET:

Gepaid: CAL000271061 Contact: SANDI BAKER Telephone: 4153483438 Facility Addr2: Not reported Mailing Name: Not reported

Mailing Address: 525 MARKET ST

Mailing City, St, Zip:

SAN FRANCISCO, CA 941050000

Gen County: TSD EPA ID: Los Angeles KYD053348108

TSD County:

Waste Category:

Oxygenated solvents (acetone, butanol, ethyl acetate, etc.)

Disposal Method:

Tons:

0.1

Facility County:

Not reported

A2 Target **Property** KITS CAMERA 1 HOUR NO 87 14006 RIVERSIDE DR STE 86 SHERMAN OAKS, CA 91423

RCRA-SQG 1000857522 CAD983669599 FINDS

Site 2 of 8 in cluster A

Actual: 659 ft.

RCRAInfo:

Owner:

RITS CAMERA CENTERS INC

(301) 419-0000

EPA ID:

CAD983669599

Contact:

Not reported

Classification:

Small Quantity Generator

TSDF Activities: Not reported

Violation Status: No violations found

FINDS:

Other Pertinent Environmental Activity Identified at Site

RCRAInfo is a national information system that supports the Resource Conservation and Recovery Act (RCRA) program through the tracking of events and activities related to facilities that generate, transport, and treat, store, or dispose of hazardous waste. RCRAInfo allows RCRA program staff to track the notification, permit, compliance, and corrective action activities required under RCRA.

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

EDR ID Number EPA ID Number

A3

SEPHORA STORE 22 FASHION SQUARE

Target Property

14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423 RCRA-SQG HAZNET 1006930253 CAR000147447

Site 3 of 8 in cluster A

Actual: 659 ft.

RCRAInfo:

Owner:

SHERMAN OAKS FASHION ASSOC

EPA ID:

CAR000147447

Contact:

SANDI BAKER

415-348-3488

Classification:

Small Quantity Generator

TSDF Activities: Not reported

Violation Status: No violations found

HAZNET:

Gepaid:

CAR000147447

Contact:

SANDI BAKER FACILITIES MGR.

Telephone:

4153483488

Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address:

525 MARKET ST

Mailing City, St, Zip: Gen County:

SAN FRANCISCO, CA 941050000

Los Angeles AZD081705402

TSD EPA ID: TSD County:

99

Waste Category:

Not reported

Disposal Method:

Recycler

Tons:

0.03

Facility County:

Not reported

Gepaid:

CAR000147447

Contact: Telephone: SANDI BAKER FACILITIES MGR. 4153483488

Facility Addr2:

Not reported

Mailing Name: Mailing Address: Not reported 525 MARKET ST

Mailing City, St, Zip:

SAN FRANCISCO, CA 941050000

Gen County:

Los Angeles

TSD EPA ID:

AZD081705402

TSD County:

99

Waste Category:

Not reported

Disposal Method:

Recycler 0.03

Tons: Facility County:

Not reported

Gepaid:

CAR000147447

Contact:

SANDI BAKER FACILITIES MGR.

Telephone: Facility Addr2: 4153483488 Not reported

Mailing Name:

Not reported

Mailing Address:

525 MARKET ST SAN FRANCISCO, CA 941050000

Mailing City, St, Zip: Gen County:

Los Angeles

TSD EPA ID:

CAD008364432

TSD County:

Los Angeles

Waste Category:

Unspecified solvent mixture Waste

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

EDR ID Number EPA ID Number

SEPHORA STORE 22 FASHION SQUARE (Continued)

1006930253

N/A

Disposal Method:

Tons:

0.1

Facility County:

Los Angeles

Transfer Station

A4

J P MECHANICAL

Target

14006 RIVERSIDE

Property

SHERMAN OAKS, CA 91423

HAZNET \$100937566

Site 4 of 8 in cluster A

Actual: 659 ft.

HAZNET:

Gepaid:

Contact:

CAL000111233 PAUL BECKER

Telephone:

8189861540

Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address:

14006 RIVERSIDE

Mailing City, St, Zip:

SHERMAN OAKS, CA 914230000

Gen County:

Los Angeles

TSD EPA ID: TSD County: CAT080031628 Kern

Waste Category:

Waste oil and mixed oil

Disposal Method:

Recycler

Tons:

.4587

Facility County:

Los Angeles

Α5

Target **Property** CITY FREEHOLDS (USA INC) 14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423

HAZNET \$102804950 N/A

Site 5 of 8 in cluster A

Actual: 659 ft.

HAZNET:

Gepaid:

CAC001104816

Contact:

CITY FREEHOLDS (USA INC)

Telephone: Facility Addr2: 8187893114 Not reported

Mailing Name:

Not reported

14006 RIVERSIDE DR

Mailing Address: Mailing City,St,Zip:

SHERMAN OAKS, CA 914230000

Gen County:

Los Angeles

TSD EPA ID:

TSD County:

CAD044429835

Los Angeles

Waste Category:

Other inorganic solid waste Treatment, Incineration

Disposal Method: Tons:

.0500

Facility County:

Los Angeles

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

EDR ID Number **EPA ID Number**

A6

LENSCRAFTERS STORE #501

Target Property 14006 RIVERSIDE DR SHERMAN OAKS, CA 91423 HAZNET S103629874 EMI N/A

Site 6 of 8 In cluster A

Actual: 659 ft.

HAZNET:

Gepaid:

CAD983669599

Contact:

RITZ CAMERA CENTERS INC

Telephone: Facility Addr2:

3014190000 Not reported

Mailing Name:

Not reported 6711 RITZ WAY

Mailing Address: Mailing City, St, Zip:

BELTSVILLE, MD 207051318

Gen County:

Los Angeles

TSD EPA ID:

CAT000613976

TSD County:

Orange

Waste Category:

Photochemicals/photoprocessing waste

Disposal Method:

Transfer Station

Tons:

3.1815

Facility County:

Los Angeles

Gepaid:

CAD983669599

Contact:

RITZ CAMERA CENTERS INC

Telephone:

3014190000

Facility Addr2:

Not reported Not reported

Mailing Name:

Mailing Address:

6711 RITZ WAY

Mailing City, St, Zip:

BELTSVILLE, MD 207051318

Gen County: TSD EPA ID: Los Angeles CAT000613976

TSD County:

Orange

Waste Category:

Not reported

Disposal Method:

Transfer Station

Tons:

.0000

Facility County:

Los Angeles

Gepaid:

CAD983669599

Contact:

RITZ CAMERA CENTERS INC

Telephone:

3014190000

Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address:

6711 RITZ WAY

Mailing City, St, Zip:

BELTSVILLE, MD 207051318

Gen County:

Los Angeles

TSD EPA ID:

CAD981402522

TSD County:

Kem

Waste Category:

Photochemicals/photoprocessing waste

Disposal Method:

Recycler 1.3551

Tons: Facility County:

Los Angeles

Gepaid:

CAL000172107

Contact:

GRACE HAGGARD

Telephone:

5137656278

Facility Addr2: Mailing Name: Not reported

Mailing Address:

Not reported 4000 LUXOTTICA PL

Mailing City, St, Zip:

MASON, OH 45040

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

EDR ID Number **EPA ID Number**

S103629874

LENSCRAFTERS STORE #501 (Continued)

Gen County:

Los Angeles

TSD EPA ID:

CAD008302903 Los Angeles

TSD County:

Other organic solids

Waste Category:

Disposal Method:

Transfer Station

Tons:

0.02

Facility County:

Not reported

Gepaid:

CAD983669599

Contact:

RITZ CAMERA CENTERS INC

Telephone: Facility Addr2:

3014190000 Not reported

Mailing Name:

Not reported

Mailing Address:

6711 RITZ WAY

Mailing City, St, Zip:

BELTSVILLE, MD 207051318

Gen County:

Los Angeles

TSD EPA ID:

CAD981402522

TSD County:

Kern

Waste Category:

Degreasing sludge

Disposal Method:

Recycler .0834

Tons: **Facility County:**

Los Angeles

Click this hyperlink while viewing on your computer to access 15 additional CA_HAZNET: record(s) in the EDR Site Report.

EMI:

Year:

1990

Carbon Monoxide Emissions Tons/Yr:

19

Air Basin: Facility ID: SC 76220

Air District Name:

SC

SIC Code: Air District Name:

6512 SOUTH COAST AQMD

Community Health Air Pollution Info System:

Not reported

Consolidated Emission Reporting Rule:

Not reported

Total Organic Hydrocarbon Gases Tons/Yr:

0

Reactive Organic Gases Tons/Yr:

0

Carbon Monoxide Emissions Tons/Yr:

0

NOX - Oxides of Nitrogen Tons/Yr:

0

SOX - Oxides of Sulphur Tons/Yr:

0

Particulate Matter Tons/Yr:

Part. Matter 10 Micrometers & Smilr Tons/Yr:

A7 Target **SEPHORA STORES #18**

14006 RIVERSIDE DR STE 75 SHERMAN OAKS, CA 91423 **Property**

Site 7 of 8 in cluster A

Actual: 659 ft.

HAZNET:

Gepaid:

CAL000273801

Contact:

SANDI BAKER

Telephone:

4153483438

Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address:

525 MARKET ST 11TH FLOOR

S107149329

N/A

HAZNET

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

EDR ID Number **EPA ID Number**

SEPHORA STORES #18 (Continued)

S107149329

FINDS 1007087658

110015672039

Mailing City, St, Zip:

SAN FRANCISCO, CA 941050000

Gen County: TSD EPA ID: Los Angeles KYD053348108

TSD County:

Los Angeles

Waste Category:

Oxygenated solvents (acetone, butanol, ethyl acetate, etc.)

Disposal Method:

Not reported

Tons:

0.15

Facility County:

Los Angeles

Gepaid:

CAL000273801

Contact: Telephone: SANDI BAKER 4153483438 Not reported

Facility Addr2: Mailing Name: Mailing Address:

Not reported 525 MARKET ST

Mailing City, St, Zip:

SAN FRANCISCO, CA 941050000

Gen County: TSD EPA ID: Los Angeles KYD053348108

TSD County:

Waste Category:

Oxygenated solvents (acetone, butanol, ethyl acetate, etc.)

Disposal Method:

Not reported

Tons:

Facility County:

0.75 Not reported

ΔR **Target** **SEPHORA STORE 22 FASHION SQUARE**

14006 RIVERSIDE DRIVE

Property

SHERMAN OAKS, CA 91423

Site 8 of 8 in cluster A

Actual:

659 ft.

FINDS:

Other Pertinent Environmental Activity Identified at Site

California - Hazardous Waste Tracking System - Datamart

RCRAInfo is a national information system that supports the Resource Conservation and Recovery Act (RCRA) program through the tracking of events and activities related to facilities that generate, transport, and treat, store, or dispose of hazardous waste. RCRAInfo allows RCRA

program staff to track the notification, permit, compliance, and

corrective action activities required under RCRA.

ENE

BURBANK MEDICAL CLINIC INC 13739 RIVERSIDE DR

1/8-1/4

1086 ft.

Relative: Lower

Actual: 652 ft.

SHERMAN OAKS, CA 91423

TC2096148.2s Page 11

1000597751

CAD983617770

RCRA-SQG

FINDS

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

HIST UST

U001568123

N/A

EDR ID Number EPA ID Number

BURBANK MEDICAL CLINIC INC (Continued)

1000597751

RCRAInfo:

Owner:

BURBANK MEDICAL CLINIC INC

(818) 842-4863

EPA ID:

CAD983617770

Contact:

RAEANN BLUE (818) 842-4863

Classification:

Small Quantity Generator

TSDF Activities: Not reported

Violation Status: No violations found

FINDS:

Other Pertinent Environmental Activity Identified at Site

RCRAInfo is a national information system that supports the Resource Conservation and Recovery Act (RCRA) program through the tracking of events and activities related to facilities that generate, transport, and treat, store, or dispose of hazardous waste. RCRAInfo allows RCRA program staff to track the notification, permit, compliance, and corrective action activities required under RCRA.

B10

WNW 1/8-1/4 1184 ft. 91683

14061 RIVERSIDE DR SHERMAN OAKS, CA 91403

Site 1 of 4 in cluster B

Relative: Lower

Actual: 652 ft.

HIST UST:

Region: Facility ID:

STATE 00000062110 Tank Num: 001

Container Num: Year Installed: Tank Capacity:

1958 00004000 Gas Station

Facility Type: Other Type: **Total Tanks:**

Not reported 0004 **PRODUCT**

Tank Used for: Type of Fuel: Tank Construction:

Not reported 0000170 unknown

Leak Detection: Contact Name:

Stock Inventor MAGUIRE, JOE J 8189959150

Telephone: Owner Name:

CHEVRON U.S.A. INC.

Owner Address:

575 MARKET

Owner City, St, Zip:

SAN FRANCISCO, CA 94105

Region: Facility ID:

STATE

Tank Num: Container Num: 00000062110 002

Year Installed: Tank Capacity:

1958 00005000 **Gas Station**

2

Facility Type: Other Type:

Not reported

Map ID Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number **EPA ID Number**

U001568123

91683 (Continued)

Total Tanks:

0004

Tank Used for: Type of Fuel:

PRODUCT Not reported

Tank Construction: Leak Detection: Contact Name:

0000250 unknown Stock Inventor MAGUIRE, JOE J

Telephone: Owner Name: 8189959150 CHEVRON U.S.A. INC.

Owner Address:

575 MARKET

Owner City, St, Zip:

SAN FRANCISCO, CA 94105

Region: Facility ID: STATE

00000062110

Tank Num: Container Num: Year Installed:

003

Tank Capacity: Facility Type:

1958 00002000 **Gas Station**

Other Type: Total Tanks: Tank Used for:

Not reported 0004 **PRODUCT**

Type of Fuel: Tank Construction: Leak Detection:

Not reported 0000170 unknown Stock Inventor MAGUIRE, JOE J

Contact Name: Telephone:

8189959150 CHEVRON U.S.A. INC.

Owner Name: Owner Address:

575 MARKET

Owner City,St,Zip:

SAN FRANCISCO, CA 94105

Region: Facility ID: STATE

Tank Num: Container Num:

00000062110 004

Year Installed: Tank Capacity: Facility Type: Other Type:

1958 00000550 **Gas Station** Not reported

Total Tanks: Tank Used for: Type of Fuel:

0004 WASTE Not reported 0000100 unknown Stock Inventor

Tank Construction: Leak Detection: Contact Name: Telephone:

MAGUIRE, JOE J 8189959150

Owner Name:

CHEVRON U.S.A. INC.

Owner Address:

575 MARKET

Owner City, St, Zip:

SAN FRANCISCO, CA 94105

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

CA FID UST

LUST

Cortese

S101298110

N/A

EDR ID Number **EPA ID Number**

S101582713

N/A

B11 WNW **CURRENT OCCUPANT** 14061 RIVERSIDE DR SHERMAN OAKS, CA 91403

1/8-1/4 1184 ft.

Site 2 of 4 in cluster B

Relative:

CA FID UST:

Lower Actual:

652 ft.

19001159 Facility ID: Regulated By: UTNKI Regulated ID: 00062110 Cortese Code: Not reported SIC Code: Not reported

Facility Phone: Mail To:

2130000000 Not reported

Mailing Address: Mailing Address 2:

14061 RIVERSIDE DR Not reported

Mailing City, St, Zip:

SHERMAN OAKS 914030000 Not reported

Contact: Contact Phone: DUNs Number: NPDES Number:

Not reported Not reported Not reported Not reported Not reported Inactive

EPA ID: Comments:

Status:

B12 WNW 1/8-1/4 1184 ft. **CHEVRON #9-1683** 14061 RIVERSIDE DR SHERMAN OAKS, CA 91423

Site 3 of 4 in cluster B

Relative: Lower

LUST:

Region:

STATE

Actual: 652 ft.

Case Type: Cross Street:

Other ground water affected HAZELTON

Enf Type: Funding: How Discovered: How Stopped:

Not reported Tank Closure Not reported Corrosion Tank

Not reported

Leak Source: Global Id: Stop Date:

Leak Cause:

T0603702477 1986-03-18 00:00:00

Confirm Leak: Workplan: Prelim Assess: Not reported Not reported Not reported

Pollution Char: Remed Plan: Remed Action: Not reported Not reported Not reported

Monitoring: Close Date: Discover Date: 1988-01-07 00:00:00 1994-10-17 00:00:00 1986-03-18 00:00:00

Enforcement Dt: Release Date: Review Date:

Not reported 1986-03-19 00:00:00

Enter Date:

1994-10-21 00:00:00 1986-12-31 00:00:00

MTBE Date:

Not reported Not reported

GW Qualifier: Soil Qualifier:

Not reported Max MTBE GW ppb: Not reported

Map ID Direction Distance Distance (ft.) Site Elevation

Database(s)

EDR ID Number EPA ID Number

S101298110

CHEVRON #9-1683 (Continued)

Max MTBE Soil ppb: Not reported

County:

19

Org Name: Reg Board:

Not reported Los Angeles Region

Status:

Case Closed

Chemical: Contact Person:

Gasoline Not reported

Responsible Party:

CHEVRON USA, INC

RP Address:

Interim:

Yes

Oversight Prgm: MTBE Class:

LUST

MTBE Conc:

0

MTBE Fuel:

MTBE Tested:

Site NOT Tested for MTBE.Includes Unknown and Not Analyzed.

Staff:

Staff Initials:

Lead Agency:

Regional Board

Local Agency:

19050

Hydr Basin #:

SAN FERNANDO VALLEY

Beneficial:

Not reported

Priority: Cleanup Fund Id:

Not reported Not reported

Work Suspended:

Not reported

Local Case #:

Not reported

Case Number:

914230289

Qtv Leaked:

Not reported

Abate Method:

Vent Soil - bore holes in soil to allow volatilization of contaminants

Operator:

Not reported

Water System Name: Not reported

Well Name:

Not reported

Distance To Lust:

Waste Discharge Global ID: Not reported

Waste Disch Assigned Name: Not reported

Summary:

ALL TANKS REMOVED. SAP NOT SUBMITTED PRIOR TO WORK.

ASSESSMENT IN PROGRESS.

LUST:

Region:

Staff:

UNK

County:

Los Angeles

Local Agency:

19050

Lead Agency:

Regional Board Groundwater

Case Type: Status:

Case Closed

Substance:

Gasoline

Cross Street: Global ID:

HAZELTON T0603702477 Not reported

Enforcement Type: Date Leak Discovered:

3/18/1986

Date Leak Record Entered: 12/31/1986 How Leak Discovered:

Tank Closure

How Leak Stopped: Cause of Leak:

Not reported

Leak Source:

Corrosion Tank

Date Leak Stopped:

3/18/1986 Date Confirmation Began: Not reported **ADDITIONAL**

Map ID Direction Distance Distance (ft.) Site Elevation

Database(s)

EDR ID Number EPA ID Number

CHEVRON #9-1683 (Continued)

S101298110

Operator:

Water System:

Not reported Not reported

Well Name:

Not reported

Approx. Dist To Production Well (ft): Abatement Method Used at the Site: 14222.015933717273347196822242 Vent Soil

Source of Cleanup Funding:

Vent Soil 3/19/1986

Date Leak First Reported: Preliminary Site Assessment Workplan Submitted: Not reported

Preliminary Site Assessment Began: Pollution Characterization Began:

Not reported Not reported

Remediation Plan Submitted: Remedial Action Underway:

Not reported Not reported 1/7/1988

Post Remedial Action Monitoring Began: Date the Case was Closed:

10/17/1994 10/21/1994

Date Case Last Changed on Database: **Enforcement Action Date:**

Not reported Not reported

Historical Max MTBE Date:

Not reported

Hist Max MTBE Conc in Groundwater:

Not reported

Hist Max MTBE Conc in Soil: Significant Interim Remedial Action Taken:

Yes

GW Qualifier:

Soil Qualifier:

Not reported Not reported Not reported

Organization: Regional Board:

04

Owner Contact:

Not reported CHEVRON USA, INC

Responsible Party: RP Address:

Program: Lat/Long:

LUST

Local Agency Staff:

34.1576981 / -1

Beneficial Use:

PEJ Not reported

Priority:

Not reported Not reported

Cleanup Fund Id: Suspended: Local Case No:

Not reported Not reported Not reported

Substance Quantity: Assigned Name:

Not reported Not reported

W Global ID: Summary:

ALL TANKS REMOVED. SAP NOT SUBMITTED PRIOR TO WORK.

ADDITIONAL

RCRA-SQG 1000921662

CAD983668385

FINDS

ASSESSMENT IN PROGRESS.

Cortese:

Region:

CORTESE

Facility Addr2:

14061 RIVERSIDE DR

B13 WNW **CHEVRON STATION 9 1683** 14061 RIVERSIDE DR SHERMAN OAKS, CA 91423

1/8-1/4 1184 ft.

Relative: Lower

Site 4 of 4 in cluster B

Actual: 652 ft.

Map ID
Direction
Distance
Distance (ft.)
Elevation Site

Database(s)

RCRA-SQG

FINDS

EDR ID Number EPA ID Number

CHEVRON STATION 9 1683 (Continued)

1000921662

1000238651

CAD981966617

RCRAInfo:

Owner:

CHEVRON USA PRODUCTS CO

(310) 694-7452

EPA ID:

CAD983668385

Contact:

Not reported

Classification:

Small Quantity Generator

TSDF Activities: Not reported

Violation Status: No violations found

FINDS:

Other Pertinent Environmental Activity Identified at Site

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14 East 1/8-1/4 HIGH TECH AUTO 4774 WOODMAN

SHERMAN OAKS, CA 91403

1268 ft.

Relative: Lower RCRAInfo:

Owner:

LOUIE DIAZ & MARTY RUTHMAN

(415) 555-1212

Actual: 645 ft. EPA ID:

CAD981966617

Contact:

ENVIRONMENTAL MANAGER

(818) 986-9771

Classification:

Small Quantity Generator

TSDF Activities: Not reported

Violation Status: No violations found

FINDS

Other Pertinent Environmental Activity Identified at Site

RCRAInfo is a national information system that supports the Resource Conservation and Recovery Act (RCRA) program through the tracking of events and activities related to facilities that generate, transport, and treat, store, or dispose of hazardous waste. RCRAInfo allows RCRA program staff to track the notification, permit, compliance, and corrective action activities required under RCRA.

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

HAZNET

Cortese

LUST

EDR ID Number **EPA ID Number**

S102059830

N/A

15 **ESE** **FASHION SQUARE CAR WASH 4625 WOODMAN AVE**

1/4-1/2

SHERMAN OAKS, CA 91423

1486 ft.

Relative: Lower

HAZNET:

Gepaid: Contact: CAL000174184

Actual: 630 ft.

Telephone: Facility Addr2: **FASHION SQUARE ENTERPRISES** 000000000

Mailing Name:

Not reported Not reported

Mailing Address:

4625 WOODMAN AVE

Mailing City, St, Zip:

SHERMAN OAKS, CA 914233131

Gen County:

Los Angeles CAD028409019

TSD EPA ID: TSD County:

Los Angeles

Other empty containers 30 gallons or more

Waste Category:

Disposal Method:

Transfer Station

Tons:

.1250

Facility County:

Los Angeles

Gepaid:

CAL000174184 **FASHION SQUARE ENTERPRISES**

Contact:

000000000

Telephone: Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address:

4625 WOODMAN AVE

Mailing City, St, Zip: Gen County:

SHERMAN OAKS, CA 914233131

TSD EPA ID:

Los Angeles CAD044429835

Los Angeles

TSD County: Waste Category:

Other inorganic solid waste

Disposal Method:

Disposal, Other

Tons: **Facility County:** .7000

Los Angeles

Gepaid: Contact: CAC002246529

Telephone:

SOCKET FAMILY TRUST

Facility Addr2:

8189812333

Not reported

Mailing Name:

Not reported

Mailing Address:

4625 WOODMAN AVE

Mailing City, St, Zip:

SHERMAN OAKS, CA 914230000

Gen County: TSD EPA ID: Los Angeles CAD028409019

TSD County:

Los Angeles

Waste Category: Disposal Method:

Tank bottom waste Treatment, Tank

Tons:

10.8420

Facility County:

Los Angeles

Gepaid:

CAL000174184

Contact:

Telephone:

8189812333

Facility Addr2:

Not reported

Mailing Name:

Not reported

Mailing Address: Mailing City, St, Zip: 4625 WOODMAN AVE

Gen County:

SHERMAN OAKS, CA 914233131 Los Angeles

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

S102059830

FASHION SQUARE CAR WASH (Continued)

TSD EPA ID:

Not reported

TSD County:

Los Angeles

Waste Category:

Aqueous solution with less than 10% total organic residues

Disposal Method:

Recycler 2.71

Tons:

Facility County:

Not reported

Gepaid:

CAL000174184

Contact:

FASHION SQUARE ENTERPRISES

Telephone: Facility Addr2: 000000000 Not reported

Mailing Name:

Not reported

Mailing Address:

4625 WOODMAN AVE

Mailing City, St, Zip:

SHERMAN OAKS, CA 914233131

Gen County:

Los Angeles

TSD EPA ID:

CAT080013352

TSD County:

Los Angeles Aqueous solution with less than 10% total organic residues

Waste Category: Disposal Method:

Recycler

Tons:

1.6680 Los Angeles

Facility County:

Click this hyperlink while viewing on your computer to access 6 additional CA_HAZNET: record(s) in the EDR Site Report.

LUST:

Region:

STATE

Case Type:

Other ground water affected

Cross Street:

101 FWY

Enf Type:

Not reported

Funding:

SEL Not reported

How Discovered: How Stopped:

Not reported

Leak Cause:

UNK

Leak Source: Global Id:

UNK T0603702480

Stop Date:

Not reported

Confirm Leak:

Not reported

Workplan:

1998-03-10 00:00:00

Prelim Assess:

1991-02-13 00:00:00

1995-05-05 00:00:00

Pollution Char: Remed Plan:

1998-04-15 00:00:00

Remed Action:

2006-01-17 00:00:00

Monitoring:

Not reported

Close Date:

Not reported

Discover Date:

1991-01-07 00:00:00 Not reported

Enforcement Dt:

1991-02-13 00:00:00

Release Date: Review Date:

Not reported

Enter Date:

1991-02-19 00:00:00

MTBE Date:

1965-01-01 00:00:00

GW Qualifier:

Not reported

Soil Qualifier:

Not reported

Max MTBE GW ppb: 4,100,000

Max MTBE Soil ppb: 17,000

County:

Org Name:

Not reported

Map ID Direction Distance Distance (ft.) Site Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

S102059830

FASHION SQUARE CAR WASH (Continued)

Reg Board:

Los Angeles Region

Status:

Remedial action (cleanup) Underway

Chemical: Contact Person:

Gasoline Not reported **DINESH RAO**

Responsible Party: RP Address:

4625 WOODMAN AVE. Not reported

Interim:

LUST

Oversight Prgm: MTBE Class:

MTBE Conc: 2 MTBE Fuel:

MTBE Tested:

MTBE Detected. Site tested for MTBE and MTBE detected

Staff:

CEC HRQ

Staff Initials:

Regional Board

Lead Agency:

Local Agency:

19050 SAN FERNANDO VALLEY

Hydr Basin #: Beneficial:

Not reported

Priority:

Not reported

Cleanup Fund Id:

Not reported

Work Suspended:

Not reported Not reported

Local Case #: Case Number:

914230316

Qty Leaked:

Not reported

Abate Method:

Not reported

Operator:

Not reported

Water System Name: Not reported

Well Name:

Not reported

Distance To Lust:

Waste Discharge Global ID: Not reported Waste Disch Assigned Name: Not reported

6/19/00 WELL INSTALL. RPT; 7/1/00 2ND QTR GW MON RPT 2000; 10/11/00 3RD QTR GW

MON RPT 2000; 4/11/01 1ST QTR GW MON RPT 2001

LUST:

Region:

Staff:

CEC

County:

Los Angeles

Local Agency:

19050

Lead Agency:

Regional Board

Case Type:

Groundwater

Status:

Remedial action (cleanup) Underway

Substance:

Gasoline 101 FWY

Cross Street: Global ID:

T0603702480 LET

Enforcement Type: Date Leak Discovered:

1/7/1991

Date Leak Record Entered: 2/19/1991

How Leak Discovered:

Not reported

How Leak Stopped:

Not reported

Cause of Leak: Leak Source:

UNK

UNK

Date Leak Stopped:

Not reported

Date Confirmation Began:

Not reported

Operator:

Not reported

Water System:

Not reported

Well Name:

Not reported

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Not reported Not reported

2/13/1991

2/13/1991

5/5/1995

4/15/1998

3/9/2000

2/13/1991

Not reported 7/15/2002

Not reported

Not reported

1/1/1965

4100000

17000

14721.309795986669480587094339

6/19/00 WELL INSTALL. RPT; 7/1/00 2ND QTR GW MON RPT 2000; 10/11/00

3RD QTR GW MON RPT 2000; 4/11/01 1ST QTR GW MON RPT 2001

Database(s)

EDR ID Number EPA ID Number

FASHION SQUARE CAR WASH (Continued)

S102059830

Approx. Dist To Production Well (ft):

Abatement Method Used at the Site:

Source of Cleanup Funding:

Date Leak First Reported:

Preliminary Site Assessment Workplan Submitted: 3/10/1998

Preliminary Site Assessment Began: Pollution Characterization Began:

Remediation Plan Submitted: Remedial Action Underway:

Post Remedial Action Monitoring Began: Date the Case was Closed:

Date Case Last Changed on Database:

Enforcement Action Date: Historical Max MTBE Date:

Hist Max MTBE Conc in Groundwater: Hist Max MTBE Conc in Soil: Significant Interim Remedial Action Taken:

GW Qualifier: Soil Qualifier: Organization:

Regional Board:

Owner Contact: Responsible Party:

RP Address:

Program: Lat/Long:

Local Agency Staff: Beneficial Use: Priority: Cleanup Fund Id:

Suspended: Local Case No: Substance Quantity: Assigned Name:

W Global ID: Summary:

Cortese: Region: CORTESE

Facility Addr2:

4625 WOODMAN AVE

Not reported

Not reported

Not reported

Not reported

34.1544072 / -1

Not reported

CHUCK SOCKETT/JEFF PAUL

4625 WOODMAN AVE.

04

LUST

PEJ

C16 ENE

1/4-1/2 1551 ft.

Site 1 of 2 in cluster C

13650 RIVERSIDE DR

SHERMAN OAKS, CA 91423

TOSCO S.S. #3176

Relative: Lower Actual:

652 ft.

LUST:

Region: Case Type:

Cross Street:

WOODMAN AVE Enf Type: Not reported Funding: Not reported OM

How Discovered: How Stopped:

Leak Cause:

Not reported Not reported

STATE

Soil only

LUST

Cortese

S103282041

N/A

Map ID
Direction
Distance
Distance (ft.)
Elevation Site

Database(s)

EDR ID Number EPA ID Number

TOSCO S.S. #3176 (Continued)

Summary:

Not reported

S103282041

Leak Source: Not reported T0603702486 Global Id: Stop Date: Not reported Not reported Confirm Leak: Not reported Workplan: Prelim Assess: Not reported Not reported Pollution Char: Remed Plan: Not reported Remed Action: Not reported Monitoring: Not reported 1999-07-15 00:00:00 Close Date: 1997-09-16 00:00:00 Discover Date: Enforcement Dt: Not reported 1997-09-16 00:00:00 Release Date: Review Date: 1999-07-15 00:00:00 Enter Date: 1998-03-10 00:00:00 MTBE Date: Not reported Not reported GW Qualifier: Soil Qualifier: Not reported Max MTBE GW ppb: Not reported Max MTBE Soil ppb: Not reported County: Org Name: Not reported Los Angeles Region Reg Board: Status: Case Closed Chemical: Benzene Contact Person: Not reported TOSCO MARKETING CO Responsible Party: RP Address: P.O. BOX 25376, SANTA ANA, CA 92799 Interim: Not reported Oversight Prgm: LUST MTBE Class: MTBE Conc: 0 MTBE Fuel: MTBE Tested: Not Required to be Tested. Staff: YR Staff Initials: EL Local Agency Lead Agency: Local Agency: 19050 SAN FERNANDO VALLEY Hydr Basin #: Beneficial: Not reported Priority: Not reported Cleanup Fund Id: Not reported Work Suspended: Not reported Local Case #: Not reported 914230370 Case Number: Qty Leaked: Not reported Abate Method: Not reported Operator: Not reported Water System Name: Not reported Well Name: Not reported Distance To Lust: Waste Discharge Global ID: Not reported Waste Disch Assigned Name: Not reported

Map ID
Direction
Distance
Distance (ft.)
Elevation Site

Database(s)

EDR ID Number EPA ID Number

TOSCO S.S. #3176 (Continued)

Suspended:

Local Case No:

Assigned Name:

Substance Quantity:

S103282041

```
LUST:
  Region:
                             UNK
  Staff:
  County:
                             Los Angeles
                             19050
  Local Agency:
  Lead Agency:
                             Local Agency
  Case Type:
                             Soil
  Status:
                             Case Closed
  Substance:
                             Benzene
                             WOODMAN AVE
  Cross Street:
  Global ID:
                             T0603702486
  Enforcement Type:
                             Not reported
  Date Leak Discovered:
                             9/16/1997
  Date Leak Record Entered: 3/10/1998
  How Leak Discovered:
                             OM
  How Leak Stopped:
                             Not reported
  Cause of Leak:
                             Not reported
  Leak Source:
                             Not reported
                             Not reported
  Date Leak Stopped:
  Date Confirmation Began:
                             Not reported
  Operator:
                             Not reported
  Water System:
                             Not reported
  Well Name:
                             Not reported
  Approx. Dist To Production Well (ft):
                                                  13932.030913639274210326536029
  Abatement Method Used at the Site:
                                                  Not reported
  Source of Cleanup Funding:
                                                  Not reported
  Date Leak First Reported:
                                                  9/16/1997
  Preliminary Site Assessment Workplan Submitted: Not reported
  Preliminary Site Assessment Began:
                                                  Not reported
  Pollution Characterization Began:
                                                  Not reported
  Remediation Plan Submitted:
                                                  Not reported
  Remedial Action Underway:
                                                  Not reported
  Post Remedial Action Monitoring Began:
                                                  Not reported
  Date the Case was Closed:
                                                  7/15/1999
  Date Case Last Changed on Database:
                                                  7/15/1999
  Enforcement Action Date:
                                                  Not reported
  Historical Max MTBE Date:
                                                  Not reported
  Hist Max MTBE Conc in Groundwater:
                                                  Not reported
  Hist Max MTBE Conc in Soil:
                                                  Not reported
  Significant Interim Remedial Action Taken:
                                                  Not reported
  GW Qualifier:
                             Not reported
  Soil Qualifier:
                             Not reported
  Organization:
                             Not reported
  Regional Board:
                             04
  Owner Contact:
                             Not reported
  Responsible Party:
                             TOSCO MARKETING CO
  RP Address:
                             P.O. BOX 25376, SANTA ANA, CA 92799
  Program:
                             LUST
  Lat/Long:
                             34.1574601 / -1
  Local Agency Staff:
                             PEJ
  Beneficial Use:
                             Not reported
  Priority:
                             Not reported
  Cleanup Fund Id:
                             Not reported
```

Not reported

Not reported

Not reported

Not reported

Map ID Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

TOSCO S.S. #3176 (Continued)

S103282041

W Global ID:

Summary:

Not reported Not reported

Cortese:

Region:

CORTESE

Facility Addr2:

13650 RIVERSIDE DR

C17 ENE 1/4-1/2 1551 ft. **76 STATION 3176** 13650 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423 LUST S108418237

N/A

Relative:

Site 2 of 2 in cluster C

Lower

LUST:

Region: Case Type: STATE

Actual: 652 ft.

Soil only

Cross Street:

WOODMAN AVENUE

Enf Type:

LOC

Funding:

SEL

How Discovered:

OM

How Stopped:

Other Means

Leak Cause:

UNK

Leak Source:

UNK

Global Id:

T0603797414

Stop Date:

Not reported

Confirm Leak:

Not reported

Workplan:

Not reported

Prelim Assess: Pollution Char: Not reported 2007-06-07 00:00:00

Remed Plan:

Not reported

Remed Action:

Not reported

Monitoring:

Not reported

Close Date:

Not reported

Discover Date: **Enforcement Dt:** 2003-10-15 00:00:00 Not reported

Release Date:

2005-05-19 00:00:00

Review Date:

Not reported

Enter Date:

Not reported

MTBE Date:

Not reported

GW Qualifier:

Not reported

Soil Qualifier:

Not reported

Max MTBE GW ppb: Not reported

Max MTBE Soil ppb: Not reported

County:

Org Name: Reg Board: Not reported Los Angeles Region

Status:

Pollution Characterization

Chemical:

Gasoline

Contact Person:

Not reported

Responsible Party: RP Address:

SHARI LONDON 911 S. PRIMROSE AVE., STE. K

Interim:

Not reported

Oversight Prgm:

LUST

MTBE Class:

MTBE Conc:

0

MTBE Fuel:

MTBE Tested:

Site NOT Tested for MTBE.Includes Unknown and Not Analyzed.

Map ID Direction Distance Distance (ft.) Elevation

Database(s)

HAZNET

Cortese

LUST

EMI

1000271015

N/A

EDR ID Number EPA ID Number

S108418237

76 STATION 3176 (Continued)

Staff:

CEC

Staff Initials:

Lead Agency:

Regional Board

Local Agency:

19050

Hydr Basin #: Beneficial:

Not reported Not reported

Priority:

Not reported

Cleanup Fund Id:

Not reported

Work Suspended: Local Case #:

Not reported 16813

Case Number:

914230370A

Qty Leaked:

Not reported

Abate Method:

Operator:

Not reported

Not reported

Water System Name: Not reported

Well Name:

Not reported

Distance To Lust:

Waste Discharge Global ID: Not reported

Waste Disch Assigned Name: Not reported

Summary:

Not reported

18 West SUNKIST GROWERS INC 14130 RIVERSIDE DR

1/4-1/2

SHERMAN OAKS, CA 91423

1718 ft.

Relative:

HAZNET:

Gepaid:

CAD982328403

Lower Actual:

655 ft.

Contact:

SUNKIST GROWERS

Telephone: Facility Addr2: 8183797353

Mailing Name:

Not reported Not reported

Mailing Address: Mailing City, St, Zip:

PO BOX 7888 VAN NUYS, CA 914097888

Gen County: TSD EPA ID: Los Angeles CAD003963592

TSD County:

Santa Clara

Waste Category:

Photochemicals/photoprocessing waste

Disposal Method: Tons:

Recycler .7589

Facility County:

Los Angeles

Gepaid: Contact:

CAD982328403 SUNKIST GROWERS

Telephone:

8183797353 Not reported

Facility Addr2: Mailing Name:

Not reported PO BOX 7888

Mailing Address: Mailing City, St, Zip:

VAN NUYS, CA 914097888

Gen County:

Los Angeles CAD099452708

TSD EPA ID: TSD County:

Los Angeles

Waste Category: Disposal Method: Waste oil and mixed oil

Tons: Facility County: Recycler 2.0640 Los Angeles

Gepaid:

CAD982328403

Map ID Direction . Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

1000271015

SUNKIST GROWERS INC (Continued)

Contact:

SUNKIST GROWERS

Telephone:

8183797353 Not reported

Facility Addr2:

Not reported

Mailing Name: Mailing Address:

PO BOX 7888

Mailing City, St, Zip:

VAN NUYS, CA 914097888 Los Angeles

Gen County: TSD EPA ID:

CAD099452708

TSD County:

Los Angeles

Waste Category:

Unspecified aqueous solution

Disposal Method:

Recycler

Tons: Facility County: 3.4402

Los Angeles

Gepaid: Contact: CAD982328403

Telephone:

SUNKIST GROWERS

8183797353

Facility Addr2:

Not reported

Mailing Name:

Not reported PO BOX 7888

Mailing Address:

VAN NUYS, CA 914097888

Mailing City, St, Zip: Gen County:

Los Angeles

TSD EPA ID:

CAT080013352

TSD County: Waste Category:

Los Angeles Aqueous solution with 10% or more total organic residues

Disposal Method:

Recycler

Tons: Facility County:

.4587 Los Angeles

CAD982328403

Gepaid: Contact:

SUNKIST GROWERS 8183797353

Telephone: Facility Addr2:

Not reported

Mailing Name:

Mailing Address:

Not reported PO BOX 7888

Mailing City, St, Zip:

VAN NUYS, CA 914097888

Gen County:

Los Angeles

TSD EPA ID:

CAD009007626

TSD County:

Los Angeles

Waste Category:

Asbestos-containing waste

Disposal Method:

Not reported

Tons: Facility County:

.0000 Los Angeles

Click this hyperlink while viewing on your computer to access 55 additional CA_HAZNET: record(s) in the EDR Site Report.

LUST:

Region:

STATE

Case Type:

Other ground water affected

Cross Street:

VAN NUYS

Enf Type:

Not reported

Funding:

Not reported

How Discovered:

Subsurface Monitoring

How Stopped: Leak Cause:

Not reported

Leak Source:

UNK Piping Map ID
Direction
Distance
Distance (ft.)
Elevation Site

Database(s)

EDR ID Number EPA ID Number

SUNKIST GROWERS INC (Continued)

1000271015

T0603702482 Global Id: 1995-12-26 00:00:00 Stop Date: Confirm Leak: Not reported Workplan: Not reported Prelim Assess: Not reported Pollution Char: Not reported Remed Plan: Not reported Remed Action: Not reported Monitoring: Not reported Close Date: 1996-11-07 00:00:00

Discover Date: 1996-01-03 00:00:00
Enforcement Dt: Not reported
Release Date: 1996-01-04 00:00:00
Review Date: 1996-10-08 00:00:00
Enter Date: 1996-02-16 00:00:00

MTBE Date: 1965-01-01 00:00:00
GW Qualifier: Not reported
Soil Qualifier: Not reported
Max MTBE GW ppb: 100
Max MTBE Soil ppb: Not reported

County: 19
Org Name: Not reported
Reg Board: Los Angeles Region

Status: Case Closed Chemical: Gasoline Contact Person: Not reported

Responsible Party: SUNKIST GROWERS INC

RP Address: 14130 RIVERSIDE DR, SHERMAN OAKS, 91423

Interim: Not reported
Oversight Prgm: LUST
MTBE Class: Not reported
MTBE Conc: 1

MTBE Fuel:

MTBE Tested: MTBE Detected. Site tested for MTBE and MTBE detected

Staff: YR
Staff Initials: HRQ
Lead Agency: Regional Board

Local Agency: 19050 Hydr Basin #: SAN FERNANDO VALLEY

Beneficial: Not reported
Priority: Not reported
Cleanup Fund Id: Not reported
Work Suspended: Not reported
Local Case #: Not reported
Case Number: 914230334
Qty Leaked: Not reported

Abate Method: Excavate and Dispose - remove contaminated soil and dispose in

approved site
Operator: OLD CASE #011603

Water System Name:Not reported Well Name: Not reported

Distance To Lust: 0

Waste Discharge Global ID: Not reported Waste Disch Assigned Name: Not reported

Summary: O-RING WAS NOT PLACED IN SUB-UNIT CHECK VALVE PROPERLY. 10/08/96 3RD

QUARTERLY GW MONITORING REPORT

Site

Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

SUNKIST GROWERS INC (Continued)

1000271015

LUST:

Region: Staff:

County: Local Agency:

Lead Agency: Case Type: Status: Substance:

Cross Street: Global ID: Enforcement Type:

Date Leak Discovered: Date Leak Record Entered: 2/16/1996 How Leak Discovered:

How Leak Stopped:

Cause of Leak:

Leak Source: Date Leak Stopped:

Date Confirmation Began: Operator: Water System:

Well Name:

Approx. Dist To Production Well (ft): Abatement Method Used at the Site:

Source of Cleanup Funding: Date Leak First Reported:

Preliminary Site Assessment Workplan Submitted: Not reported

Preliminary Site Assessment Began: Pollution Characterization Began: Remediation Plan Submitted: Remedial Action Underway:

Post Remedial Action Monitoring Began: Date the Case was Closed:

Date Case Last Changed on Database: **Enforcement Action Date:**

Historical Max MTBE Date: Hist Max MTBE Conc in Groundwater:

Hist Max MTBE Conc in Soil: Significant Interim Remedial Action Taken:

GW Qualifier: Not reported Soil Qualifier: Not reported Not reported

Organization: Regional Board:

Owner Contact:

Responsible Party: RP Address:

Program: LUST Lat/Long: 34.1574791 / -1

Local Agency Staff: Beneficial Use: Priority:

Cleanup Fund Id: Suspended: Local Case No: Substance Quantity:

Assigned Name:

Not reported Not reported Not reported

04

Not reported Not reported

UNK

Los Angeles 19050 Regional Board

Groundwater Case Closed Gasoline **VAN NUYS** T0603702482 Not reported

1/3/1996

Subsurface Monitoring

Not reported UNK

Piping 12/26/1995 Not reported

Not reported

OLD CASE #011603 Not reported

> 14499.892736142759254080938682 Excavate and Dispose

Excavate and Dispose 1/4/1996

Not reported Not reported

Not reported Not reported Not reported 11/7/1996

10/8/1996 Not reported 1/1/1965 100

Not reported Not reported

Not reported SUNKIST GROWERS INC

14130 RIVERSIDE DR, SHERMAN OAKS, 91423

PEJ Not reported Not reported

Map ID '

Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number **EPA ID Number**

SUNKIST GROWERS INC (Continued)

1000271015

W Global ID:

Not reported

Summary:

O-RING WAS NOT PLACED IN SUB-UNIT CHECK VALVE PROPERLY.

3RD QUARTERLY GW MONITORING REPORT

Cortese:

Region:

CORTESE

Facility Addr2:

14130 RIVERSIDE DR

EMI:

Year: Carbon Monoxide Emissions Tons/Yr: 1990

19

Air Basin:

SC

Facility ID:

28081

Air District Name:

SC

SIC Code:

2033 SOUTH COAST AQMD

Air District Name:

Not reported

Community Health Air Pollution Info System: Consolidated Emission Reporting Rule:

Not reported

Total Organic Hydrocarbon Gases Tons/Yr:

0

Reactive Organic Gases Tons/Yr: Carbon Monoxide Emissions Tons/Yr:

0

NOX - Oxides of Nitrogen Tons/Yr:

0

SOX - Oxides of Sulphur Tons/Yr:

0

Particulate Matter Tons/Yr: Part. Matter 10 Micrometers & Smllr Tons/Yr: 0

19

SSE 1/2-1 2640 ft. SHELL #204-7199-0307 4404 WOODMAN AVE SHERMAN OAKS, CA 91423

LUST S102437059 Cortese N/A

Relative: Lower

Actual:

637 ft.

LUST: Region:

Case Type:

Cross Street:

Other ground water affected MOORPARK ST

Enf Type:

Funding:

Not reported LET

How Discovered:

Tank Closure

How Stopped:

Not reported

Leak Cause:

UNK Piping

Leak Source: Global Id:

T0603702474

Stop Date:

1988-06-20 00:00:00 1988-06-20 00:00:00

Confirm Leak: Workplan:

2002-01-23 00:00:00 2002-01-23 00:00:00

Prelim Assess: Pollution Char:

2002-06-12 00:00:00

Remed Plan:

Not reported

Remed Action: Monitoring:

2002-06-13 00:00:00 Not reported

Close Date: Discover Date: 1998-07-16 00:00:00 2001-05-10 00:00:00

Enforcement Dt:

Not reported

Release Date: Review Date:

2001-11-19 00:00:00 2001-12-12 00:00:00

Enter Date: MTBE Date: 1986-12-31 00:00:00 1998-09-11 00:00:00

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

EDR ID Number EPA ID Number

S102437059

SHELL #204-7199-0307 (Continued)

GW Qualifier:

Soil Qualifier: Not reported Max MTBE GW ppb: 28,600 Max MTBE Soil ppb: Not reported

County:

Org Name: Reg Board: Not reported Los Angeles Region

Status:

Remedial action (cleanup) Underway

Chemical:

Gasoline

Contact Person:

Not reported

Responsible Party:

SHELL OIL PRODUCTS CO

RP Address:

P.O. BOX 7869

Interim: Oversight Prgm:

MTBE Class:

MTBE Conc: 1

MTBE Fuel:

Yes

LUST

MTBE Tested:

MTBE Detected. Site tested for MTBE and MTBE detected CEC

Staff: Staff Initials:

HRQ

Lead Agency:

Regional Board

Local Agency:

19050

Hydr Basin #:

SAN FERNANDO VALLEY

Not reported Beneficial: Priority: Cleanup Fund Id: Work Suspended:

Not reported Not reported Not reported Not reported

Local Case #: Case Number: Qty Leaked:

914230170A Not reported

Abate Method:

Remove Free Product - remove floating product from water table

Operator: MR. ED PADEN Water System Name: Not reported Well Name: Not reported

Distance To Lust:

Waste Discharge Global ID: Not reported

Waste Disch Assigned Name: Not reported

Summary:

03/15/98 - TRANSM, OF QTR RPT (4TH QTR 97)

04/30/98 - TRANSM. 07/14/98 - EXTENSION REQUEST FOR QTRLY

OF QTR RPT (1ST QTR 98)

RPT

LUST:

Region:

Staff: County:

CEC Los Angeles

Local Agency:

19050

Lead Agency: Case Type:

Regional Board Groundwater

Status:

Pollution Characterization

Substance: Cross Street: Global ID:

Gasoline MOORPARK ST

Enforcement Type:

T0603702474 LET

Date Leak Discovered: Date Leak Record Entered: 12/31/1986

5/10/2001

How Leak Discovered:

Tank Closure

How Leak Stopped:

Not reported

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

SHELL #204-7199-0307 (Continued)

S102437059

Cause of Leak:

Leak Source:

Piping

Date Leak Stopped:

6/20/1988 6/20/1988

Date Confirmation Began: Operator:

MR. ED PADEN

Water System:

Not reported

Well Name:

Not reported

Approx. Dist To Production Well (ft): Abatement Method Used at the Site: 16129.317889535663379150858187 Remove Free Product

Source of Cleanup Funding:

Remove Free Product

Date Leak First Reported:

11/19/2001

Preliminary Site Assessment Workplan Submitted: 1/23/2002 Preliminary Site Assessment Began:

1/23/2002

Pollution Characterization Began:

8/8/2002

Remediation Plan Submitted:

Not reported

Remedial Action Underway: Post Remedial Action Monitoring Began: 6/13/2002

Date the Case was Closed:

11/19/2001 7/16/1998

Date Case Last Changed on Database:

8/8/2002

Enforcement Action Date:

Not reported

Historical Max MTBE Date:

5/1/2003

Hist Max MTBE Conc in Groundwater:

51000

24000

Hist Max MTBE Conc in Soil: Significant Interim Remedial Action Taken:

Yes

GW Qualifier:

Soil Qualifier: Organization:

Not reported

Regional Board: Owner Contact:

04 Not reported

SHELL OIL PRODUCTS CO

Responsible Party:

P.O. BOX 7869

RP Address: Program:

LUST

Lat/Long:

Local Agency Staff:

34.1504484 / -1

Beneficial Use:

PEJ

Priority:

Not reported Not reported

Cleanup Fund Id:

Not reported

Suspended: Local Case No: Not reported Not reported

Substance Quantity: Assigned Name:

Not reported Not reported

W Global ID:

Not reported

Summary:

03/15/98 - TRANSM, OF QTR RPT (4TH QTR 97)

04/30/98 -

TRANSM. OF QTR RPT (1ST QTR 98)

07/14/98 - EXTENSION

REQUEST FOR QTRLY RPT

Cortese:

Region:

CORTESE

Facility Addr2:

4404 WOODMAN AVE

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

EDR ID Number EPA ID Number

1000166679

N/A

20 **WSW** 1/2-1 4388 ft. **TOSCO - 76 STATION #2421** 14478 VENTURA BLVD SHERMAN OAKS, CA 91423

HAZNET

LUST Cortese **CA FID UST** HIST UST **SWEEPS UST**

Notify 65

Relative: Higher

Actual: 662 ft.

Notify 65:

Date Reported: Not reported Staff Initials: Not reported Board File Number: Not reported Facility Type: Not reported Discharge Date: Not reported Incident Description: 91423-2607

HAZNET:

Gepaid:

CAD981645377 7144286560

Contact: Telephone: UNION OIL COMPANY OF CALIFORNI

Facility Addr2:

Not reported

Mailing Name:

Not reported PO BOX 25376

Mailing Address: Mailing City, St, Zip:

SANTA ANA, CA 927995376

Gen County:

Los Angeles CAD099452708

TSD EPA ID: TSD County:

Los Angeles

Waste Category:

Waste oil and mixed oil

Disposal Method:

Recycler

Tons: Facility County: 3.0232 Los Angeles

Gepaid:

Contact:

CAD981645377 UNION OIL COMPANY OF CALIFORNI

Telephone: Facility Addr2: 7144286560

Mailing Name: Mailing Address: Not reported Not reported PO BOX 25376

Mailing City, St, Zip:

SANTA ANA, CA 927995376

Gen County: TSD EPA ID: Los Angeles CAD099452708

TSD County:

Los Angeles

Waste Category:

Oil/water separation sludge

Disposal Method: Tons:

Recycler .5004

Facility County:

Los Angeles

Gepaid: Contact: CAD981645377 UNION OIL COMPANY OF CALIFORNI

Telephone: Facility Addr2: 7144286560 Not reported Not reported

Mailing Name: Mailing Address:

PO BOX 25376 SANTA ANA, CA 927995376

Mailing City, St, Zip: Gen County:

Los Angeles CAT080011059

TSD EPA ID: TSD County:

Los Angeles

Waste Category:

Aqueous solution with 10% or more total organic residues

Disposal Method:

Not reported

Map ID Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

1000166679

TOSCO - 76 STATION #2421 (Continued)

Tons:

.4378

Facility County:

Los Angeles

Gepaid:

CAD981645377

Contact:

UNION OIL COMPANY OF CALIFORNI

Telephone: Facility Addr2: Mailing Name: 7144286560 Not reported Not reported

Mailing Address: Mailing City, St, Zip: PO BOX 25376 SANTA ANA, CA 927995376

Gen County: TSD EPA ID: Los Angeles CAT080013352 Los Angeles

TSD County: Waste Category:

Unspecified aqueous solution

Disposal Method:

Recycler

Tons: Facility County: .3127

Los Angeles

Gepaid:

CAD981645377

Contact:

UNION OIL COMPANY OF CALIFORNI

Telephone: Facility Addr2: 7144286560 Not reported

Mailing Name:

Not reported PO BOX 25376

Mailing Address:

SANTA ANA, CA 927995376

Mailing City, St, Zip: Gen County:

Los Angeles

TSD EPA ID:

CAD028409109

TSD County:

Waste Category:

Aqueous solution with less than 10% total organic residues

Disposal Method:

Treatment, Tank

Tons:

Facility County: Los Angeles

> Click this hyperlink while viewing on your computer to access 7 additional CA_HAZNET: record(s) in the EDR Site Report.

LUST:

Region:

STATE

1.0008

Case Type:

Other ground water affected

Cross Street: Enf Type:

VAN NUYS BLVD Not reported

Funding:

How Discovered:

DLSEL

How Stopped:

ОМ Not reported

Leak Cause:

Other Cause

Leak Source:

Piping

Global Id:

T0603702483

Stop Date:

1988-04-12 00:00:00

Confirm Leak:

Not reported

Workplan:

1999-03-01 00:00:00

Prelim Assess: Pollution Char: Not reported

Remed Plan:

2007-01-09 00:00:00

Not reported

Remed Action:

Not reported

Monitoring: Close Date: Not reported Not reported

Discover Date:

1988-04-12 00:00:00

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Database(s)

EDR ID Number **EPA ID Number**

TOSCO - 76 STATION #2421 (Continued)

1000166679

Enforcement Dt:

Not reported

Release Date:

1988-04-14 00:00:00

Review Date:

2002-07-15 00:00:00

Enter Date:

1988-04-21 00:00:00

MTBE Date:

2000-04-06 00:00:00

GW Qualifier: Soil Qualifier: Not reported

Not reported

Max MTBE GW ppb: 50,000

Max MTBE Soil ppb: Not reported

County:

Org Name: Reg Board: Not reported Los Angeles Region

Status: Chemical: Pollution Characterization

Contact Person:

Gasoline Not reported

Responsible Party:

SHARI LONDON

RP Address:

3611 HARBOR BLVD., SUITE #200

Interim:

Not reported

Oversight Prgm:

LUST

MTBE Class: MTBE Conc:

MTBE Fuel:

MTBE Tested:

MTBE Detected. Site tested for MTBE and MTBE detected

Staff:

ΑT HRQ

Staff Initials: Lead Agency:

Regional Board

19050

Local Agency: Hydr Basin #:

SAN FERNANDO VALLEY

Beneficial:

Not reported Not reported

Priority: Cleanup Fund Id: Work Suspended:

Not reported Not reported Not reported

Local Case #: Case Number:

914230343 Not reported

Qtv Leaked:

Not reported

Abate Method:

Operator:

NAMSON, JOSEPH N.

Water System Name: Not reported

Well Name:

Not reported

Distance To Lust:

Waste Discharge Global ID: Not reported Waste Disch Assigned Name: Not reported

Summary:

ABANDONED PIPE CONNECTED TO OLD PUMP ISLAND WAS NOT CAPPED. CONTRACTOR DISCOVERED LEAKING UG PIPE.; 7/14/00 2ND QTR GW MON RPT; 10/30/00 SUPPLE. SITE

ASESSM. RPT AND REMED. TESTING PROPOSAL; 12/31/00 4TH QTR GW MON RPT 2000

LUST:

Region:

Staff:

AT

County:

Los Angeles

Local Agency:

19050

Lead Agency:

Regional Board

Case Type:

Groundwater

Status:

Pollution Characterization

Substance:

Gasoline

Cross Street:

VAN NUYS BLVD

Global ID:

T0603702483

Map ID Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

TOSCO - 76 STATION #2421 (Continued)

1000166679

Enforcement Type: DLSEL Date Leak Discovered: 4/12/1988 Date Leak Record Entered: 4/21/1988 How Leak Discovered: OM

How Leak Stopped: Not reported Cause of Leak: Other Cause Leak Source: **Piping** Date Leak Stopped: 4/12/1988 Date Confirmation Began: Not reported

NAMSON, JOSEPH N. Operator: Not reported Water System:

Well Name: Not reported

Approx. Dist To Production Well (ft): 17639.953186757741589545141318

Abatement Method Used at the Site: Not reported Source of Cleanup Funding: Not reported Date Leak First Reported: 4/14/1988 Preliminary Site Assessment Workplan Submitted: 3/1/1999

Preliminary Site Assessment Began: Not reported Pollution Characterization Began: 7/25/2000 Remediation Plan Submitted: Not reported Remedial Action Underway: Not reported Post Remedial Action Monitoring Began: Not reported

Date the Case was Closed: Not reported Date Case Last Changed on Database: 7/15/2002 **Enforcement Action Date:** Not reported Historical Max MTBE Date: 4/6/2000

Hist Max MTBE Conc in Groundwater: 50000 Hist Max MTBE Conc in Soil: Not reported Significant Interim Remedial Action Taken: Not reported

Not reported GW Qualifier: Soil Qualifier: Not reported Organization: Not reported Regional Board: 04

Owner Contact: Not reported Responsible Party: SHARI LONDON

RP Address: 3611 HARBOR BLVD., SUITE #200 LUST

Program: Lat/Long: 34.1506913 / -1

Local Agency Staff: PEJ Beneficial Use: Not reported Priority: Not reported Cleanup Fund Id: Not reported Suspended: Not reported Local Case No: Not reported

Substance Quantity: Not reported Assigned Name: Not reported W Global ID: Not reported

Summary: ABANDONED PIPE CONNECTED TO OLD PUMP ISLAND WAS NOT CAPPED. CONTRACTOR

DISCOVERED LEAKING UG PIPE.; 7/14/00 2ND QTR GW MON RPT; 10/30/00 SUPPLE. SITE ASESSM. RPT AND REMED. TESTING PROPOSAL; 12/31/00 4TH QTR

GW MON RPT 2000

Cortese:

Region: CORTESE

14478 VENTURA BLVD Facility Addr2:

19001622

00029443

Not reported

Not reported 8187842275

Not reported

Not reported

3701 WILSHIRE BLVD

UTNKA

Map ID Direction Distance Distance (ft.) Elevation Site

Database(s)

EDR ID Number EPA ID Number

TOSCO - 76 STATION #2421 (Continued)

1000166679

CA FID UST:

Facility ID:

Regulated By:

Regulated ID: Cortese Code:

SIC Code: Facility Phone: Mail To:

Mailing Address:

Mailing Address 2:

Not reported SHERMAN OAKS 914030000

Mailing City, St, Zip:

Contact:

Contact Phone: Not reported DUNs Number: Not reported NPDES Number: Not reported Not reported

EPA ID: Comments: Status:

Not reported Active

HIST UST:

Region: Facility ID:

STATE 00000029443 001

2421-1

Tank Num: Container Num: Year installed: Tank Capacity:

1967 00009950 Facility Type: Gas Station Not reported Other Type: Total Tanks: 0003 **PRODUCT** Tank Used for: UNLEADED

Type of Fuel: Tank Construction:

Not reported

Leak Detection: Contact Name:

Stock Inventor, Pressure Test JOSEPH N NAMSON 8187842275

Telephone: Owner Name: Owner Address:

UNION OIL COMPANY OF CALIFORNI 3701 WILSHIRE BOULEVARD-SUITE

Owner City,St,Zip: LOS ANGELES, CA 90010

2421-2

Region: Facility ID: STATE 00000029443 002

Tank Num: Container Num: Year Installed: Tank Capacity:

1967 00009950 Gas Station Not reported 0003

Total Tanks: Tank Used for: Type of Fuel:

Facility Type:

Other Type:

PRODUCT PREMIUM Not reported

Tank Construction: Leak Detection: Contact Name:

Stock Inventor, Pressure Test

JOSEPH N NAMSON 8187842275

Telephone: Owner Name: Owner Address:

UNION OIL COMPANY OF CALIFORNI 3701 WILSHIRE BOULEVARD-SUITE

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

TOSCO - 76 STATION #2421 (Continued)

1000166679

Owner City, St, Zip: LOS ANGELES, CA 90010

Region:

STATE

Facility ID:

00000029443

Tank Num:

003

Container Num: Year Installed:

2421-4 1967

Tank Capacity:

00000550

Facility Type:

Gas Station

Other Type:

Not reported

Total Tanks:

0003

Tank Used for:

PRODUCT

Type of Fuel:

WASTE OIL

Tank Construction:

Not reported

Leak Detection:

Stock Inventor, Pressure Test

Contact Name:

JOSEPH N NAMSON

Telephone:

8187842275

Owner Name: Owner Address: UNION OIL COMPANY OF CALIFORNI 3701 WILSHIRE BOULEVARD-SUITE

Owner City, St, Zip:

LOS ANGELES, CA 90010

SWEEPS UST:

Status:

Comp Number:

1787

Number: Board Of Equalization: 44-000051

Ref Date:

09-30-92

Act Date:

10-07-93

Created Date:

02-29-88

Tank Status:

Owner Tank Id: Swrcb Tank Id:

000001787 19-050-001787-000004

Actv Date:

09-30-92

Capacity:

12000

Tank Use:

M. V. FUEL

Stg:

Content:

REG UNLEADED

Number Of Tanks:

Status:

Comp Number: Number:

1787

Board Of Equalization:

44-000051

Ref Date:

09-30-92

Act Date:

10-07-93

Created Date:

02-29-88

Tank Status:

Owner Tank Id:

000001787

Swrcb Tank Id:

19-050-001787-000005

Actv Date:

09-30-92

Capacity:

12000

Tank Use:

Stg:

M. V. FUEL

Content:

REG UNLEADED

Number Of Tanks:

Not reported

Status:

Α

Map ID Direction Distance Distance (ft.) Elevation Site

MAP FINDINGS

Database(s)

ENVIROSTOR \$106665626

N/A

EDR ID Number EPA iD Number

TOSCO - 76 STATION #2421 (Continued)

1000166679

Comp Number:

Number:

Board Of Equalization:

44-000051 09-30-92

Ref Date: Act Date:

10-07-93

1787

Created Date:

02-29-88

Tank Status:

Α

Owner Tank Id: Swrcb Tank ld:

000001787

Actv Date:

19-050-001787-000006

09-30-92

Capacity:

550

Tank Use:

OIL

Stg:

w

Content:

WASTE OIL

Number Of Tanks:

Not reported

21 ESE 1/2-1 **JOY'S DRY CLEANERS** 13313 MOORPARK STREET SHERMAN OAKS, CA 91423

4486 ft.

Relative:

Lower Actual:

631 ft.

ENVIROSTOR:

Site Type:

Site Type Detailed:

Evaluation Not reported

Evaluation

Acres: NPL:

Regulatory Agencies:

NONE SPECIFIED NONE SPECIFIED

Lead Agency: Program Manager: Supervisor:

Not reported Referred - Not Assigned

Division Branch: Facility ID:

So Cal - Cypress 19720026

Site Code:

Not reported

Assembly: Senate:

42

Special Program:

23 Not reported

Status:

Refer: 1248 Local Agency

Status Date:

2004-04-16 00:00:00

Restricted Use:

NO

Funding: Latitude: Not Applicable

Longitude:

0 0

Alias Name:

19720026

Alias Type:

Envirostor ID Number

APN:

NONE SPECIFIED

APN Description:

Not reported

Comments:

Not reported Not reported

Completed Area Name: Completed Sub Area Name:

Not reported

Completed Document Type: Completed Date:

Not reported Not reported

Confirmed:

NONE SPECIFIED

Confirmed Description:

Not reported

Future Area Name:

Not reported

Future Sub Area Name: Future Document Type:

Not reported Not reported

Future Due Date: Media Affected:

Not reported NONE SPECIFIED Map ID Direction Distance Distance (ft.) Elevation

MAP FINDINGS

Database(s)

EDR ID Number EPA ID Number

S106665626

JOY'S DRY CLEANERS (Continued)

Media Affected Desc:

Management Required:

Not reported NONE SPECIFIED

Management Required Desc: Not reported

Potential:

NONE SPECIFIED

Potenital Description: Schedule Area Name: Not reported Not reported

Schedule Sub Area Name:

Not reported

Schedule Document Type:

Not reported Not reported

Schedule Due Date: Schedule Revised Date:

Not reported

PastUse:

NONE SPECIFIED

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Database(s)	HAZNET, LUST, CHMIRS
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Site Address	2
Site Name	58 1X MCKESSON DRUG CO
EDR 1D	S105642458
City	LOS ANGELES COUNTY

Address

Facility ID Name

ZIP EDR-ID

State Databases

City

Map/Dir/Dist

Databases		SLIC REG 2 SLIC REG 2	ERNS FINDS NOTIEY 65	CHMIRS HAZNET HAZNET	FINDS RCRAInfo-SQG HAZNET	WIP, CORTESE, LUST CA FID UST, SWEEPS UST LA Co. Site Mitigation	HAZNET EMI, CHMIRS, HIST UST RCRAInfo-SQG, FINDS,	HIST UST CA FID UST, SWEEPS UST RCRAINFO-SQG, FINDS	CA FID UST, SWEEPS UST HIST UST	CA FID USI, SWEEPS USI LA Co. Site Mitigation HIST UST	ERNS HAZNET CA FID UST, SWEEPS UST HAZNET	HAZNET HAZNET FINDS	EMI, HAZNET RCRAInfo-SQG, FINDS	CA FID UST, SWEEPS UST HIST UST HAZNET	HAZNET CA FID UST, SWEEPS UST SWRCY, CA FID UST, SWEEPS	CA FID UST, SWEEPS UST HAZNET	EMI, FINDS HAZNET	CA FID UST RCRAINFO-SQG, FINDS, ORANGE CO HET HAZMET	ο.	HAZINET HAZINET RCRAINF-SOG, FINDS RCRAINF-SOG, FINDS.	HAZNET	ERNS CA FID UST, SWEEPS UST HAZNET	CA FID UST, SWEEPS UST RCRAINFO-SOG, FINDS HAZNET CLEANERS, EMI, HAZNET	December 10, 2007 Page ZIP-1
State		8888	5885	5888	కకక	বহুত	გ გგ	8888	5888	3558	5555	88 8	888	5888	8888	88	გ გგ	555	58	§ ১১১১			5888	
City	đđ.	VAN NUYS VAN NUYS LOS ANGELES	VAN NUYS VAN NUYS	LOS ANGELES VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS	W W W W W W W W W W W W W W W W W W W	W W W W W W W W W W W W W W W W W W W	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	ENCINO VAN NUYS SHERMAN OAKS	VAN NUYS VAN NUYS	SAN NCYS SAN NCYS SAN NCYS	VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS	VAN NUYS SHERMAN OAKS	VAN NUYS VAN NUYS	VAN NUYS	SHERMAN OAKS VAN NUYS VAN NUYS	VAN NUYS	NORTH HOLLYWOOD	NORTH HOLLYWOOD VAN NUYS VAN NUYS VAN NUYS	Report# 2096148.2s
wap/Un/Ust	Further review recommended.	::::	::	:::	:::								:											
Address	assigned a latitude/longitude coordinate. Furthe		#5201 - 15711 VICTORY BLVD 1543 W 16 24TH / PACIFIC	200 FT NC / 6036 HA OSS THE S	14117 AETHNA ST 14117 AETHNA ST 14117 AETHNA ST UNIT S	7 AETNA ST 7 AETNA ST 7 AETNA ST	14117 AETNA ST 14117 AETNA ST 14117 AETNA ST	14152 AETNA ST 14152 AETNA ST 14232 AETNA ST 14232 AETNA ST	14257 AETNA 14257 AETNA 14257 AETNA ST	14301 AETNA ST 14301 AETNA ST 14301 AETNA ST	14320 AE INA ST 14421 AE THA ST 14425 AETNA ST 15822 ARMINTA	5001 BALBOA BLVD 13921 BESSEMER ST 14111-14165 BESSEMER	14129 BESSEMER ST 14129 BESSEMER STREET	14200 BESSEMER ST 14200 BESSEMER ST 14200 BESSEMER ST 14201 RESSEMER ST	14201 BESSEMER ST 14243 BESSEMER ST 14300 BESSEMER ST	14346 BESSEMER ST 14741 BESSEMER	14747 BESSEMER STREET 13100-13120 BURBANK BLVD	13200 BURBANK BLVD 13200 BURBANK BLVD	13208 BURBANK BLVD	13218 BURBANK BLVD 13218 BURBANK BLVD 13244 BURBANK BLVD	13244 BURBANK BLVD.	13244 BUKBANK BLVD. 13250 BURBANK BLVD 13300 BURBANK BLVD	13321 BURBANK BLVD 13326 BURBANK BLVD 13660 BURBANK BLVD 13634 BURBANK BLVD	
Name	Indicates location may or may not be in requested radius. Site has not been assign	COUNTY OF LOS ANGELES - TRANSPORT COUNTY OF LOS ANGELES - TRANSPORT	#5201 - 15711 VICTORY BLVD SAWYER PETROLEUM	ROGER LOVEGREN 1X LA CO TRANSPORTATION COMMISSIO	SHELL SERVICE STATION 170563 SHELL SERVICE STATION 170563 SHELL SERVICE STATION 170563		AMBER RESOURCES LLC DBA SAWYER F BULK PLANT L T "LEE" SAYWER, INC	KSO UNK LANDM STRIPPING 1 9 M STRIPPING	MC KAY LUMBER CAL-WAL GYPSUM SUPPLY	AETNA LUMBER CO INC. AETNA LUMBER CO. INC.	14320 AE INA SI YIL TOWING INC KAPE PROPERTY MANAGEMENT COMPAI	LA FIRE STATION 83 HOME SAVINGS OF AMERICA ADELPHIA COMMUNICATIONS CORPORATIONS	TORTOISE POWDER COATING COAT COLOR INC	WALL DISNET STORIOS/HERBIE HEETLAND ROOFING CO., INC. HEETLAND ROOFING CO., INC. INIVERSAL PICTIBES	SONY PICTURES ENTERTAINMENT UNION ICE CO ACTIVE RECICLING	MATT WEST CO 1X MR ELTON	RICH & FAMOUS AUTO BODY & UPHO THE HELP GROUP FIRE STATION 102	LOS ANGELES FIRE STATION 102 LOS ANGELES FIRE STATION 102	JAGUAR CAR SVC	LANKERSHIM AUTO CORP. LANDERSHIME AUTO CORP. ALCALAS BODY & PAINT	ALCALAS BODY & PAINT SHOP	13244 BORBANK BLVD. KARSTECK IMPORTS DAD AND MF AUTO REPAIR	HENRY SERLIN TOBAR COLOR LAB M G S LLC BURWOOD VILLAGE CLEANERS	
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Databases	CLEANERS HAZNET CORTESE, LUST HAZNET HAZNET HAZNET HAZNET HOST ORANGE CO. UST CLEANERS, EMI, HAZNET	CLEANERS HAZNET HAZNET HAZNET SWRCY HAZNET	HAZNET RCRAInfo-SQG, FINDS CORTESE, LUST HIST UST	HAZNET HAZNET EMI HAZNET, CA FID UST HIST UST	SWEEPS UST SWEEPS UST CA FID UST, SWEEPS UST HIST UST RCRAInfo-SQG, FINDS	EMI HAZNET RCRAINIO-SQG, FINDS HAZNET HAZNET	CHMIKS HAZNET EMI, HAZNET EMI CA FID UST, SWEEPS UST	HAZNET HAZNET RCRAInfo-SQG, FINDS RCRAInfo-SQG, FINDS,	RCRAING-SQG, FINDS HAZNET EMI CHMIRS HAZNET	HAZNET HAZNET RCCRAIRO-SQG, FINDS,	RCRAING-SQG, FINDS LUST LUST, CA FID UST, SWEEPS	HAZNET HAZNET RCRAInfo-SQG, FINDS HAZNET	CA FID UST, SWEEPS UST ORANGE CO. UST HAZNET RCRAINDS	FINDS RCRAINFO-SQG, FINDS ORANGE CO. UST HAZNET, HIST UST, CA	HZNET, SWEEPS UST RCRAINfo-SQG, FINDS RCRAINfo-SQG, FINDS
State	5555555		ধ	<u> </u>	4444 6	35555	588888	\$ \$\$\$	88888	8888	888	555 5	444 6	5555	555
City	VAN NUYS VAN NUYS VAN NUYS SHERMAN OAKS VAN NUYS VAN NUYS VAN NUYS	VAN NUYS SHERMAN OAKS VAN NUYS SHERMAN OAKS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	VAN NUYSS VAN NUYSS VAN NUYSS VAN NUYSS	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	S S S S S S S S S S S S S S S S S S S	VAN NUYS VAN NUYS VAN NUYS VAN NUYS VAN NUYS	SHERMAN OAKS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS VAN NEYS	VAN NUYS VAN NUYS VAN NUYS VAN NUYS	VAN NUYS VAN NUYS VAN NUYS
Map/Dir/Dist					: :	:			:	:		:			
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Address	13634 BURBANK BLVD. 14001 BURBANK BLVD. 14106 BURBANK BLVD. 14106 BURBANK BLVD. 14106 BURBANK BLVD. 14106 BURBANK BLVD. 14107 BURBANK BLVD.	14107 BURBANK BLV 14242 BURBANK BLV 14399 BURBANK BL. 14401 BURBANK BLV	1441 BURBANK BLV 14850 BURBANK BLV 14850 BURBANK BLV 14850 BURBANK BLV	14850 BURBANK BLV 14850 BURBANK BLV 14857 BURBANK BLV 14903 BURBANK BLV 14903 BURBANK BLV	14106 BURNBANK BI 14903 BURNBANK BI 14911 CALIFA ST 14911 CALIFA ST 14941 CALIFA STREE	15043 CALIFA ST 15131 CALIFA ST 14911 CALIFER ST 14911 CALIFER ST 14128 CALVERT ST	14318 W CALVERT S 14345 CALVERT ST 14348 CALVERT ST 14348 CALVERT ST 14401 CALVERT	14402 CALVERT ST 14428 CALVERT ST 14428 CALVERT ST 14737 CALVERT ST	14804 CALVERT ST CALVERT / VAN NESS 6103 CEDROS AV 6803 CEDROS AVENUE 13141 CHANINI ED ELVIN	13359 CHANDLER BI 7350 CHESTER AVE 14310 DELANO ST	14540 DELANO ST 14433 EMELIA AVE 14433 EMELITA ST	14433 EMELITA STR 13400 ERWIN ST 13400 ERWIN ST 14,400 ERWIN ST	14400 ERWIN ST 14400 ERWIN STREE 14437 ERWIN ST 14437 FRWIN STREE	14437 ERWIN STREET MA 15232 ERWIN ST 5701 ETHEL AV 5701 ETHEL AVE	5701 ETHEL AVE 14410 FRAIR 14401 FRIAR ST
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Name	DANIEL ROTHBART AND ASSOICATES DANIEL ROTHBART AND ASSOICATES 15357 MAGNOLIA EMPTY LOT	WALTER SEERY #14-125 (MOBIL) MICHAFI PANTFI FO PAINTING	MANDEVILLE PUMP STATION DEPARTMENT OF WATER & POWER/MAN	LOS ANGELES DEPT OF WATER AND 4540 NATICK PARTNERSHIP AMCORP/CEDAR COVE APTS HOME SAVINGS OF AMERICA	VICKERS INC. ULTRA CLEARNERS CURRENT OCCUPANT COURENT OCCUPANT COURT.	BETA PHI ASSOCIATES INC. MR DRYCLEAN	MR. DRY CLEAN MR. DRY CLEAN INC MR. DRYCLEAN INC EL POLLO LOCO PARKING LOT	MICHAEL BATES CONSTRUCTION MICHAEL BATES AND COMBANY	MCI COMMUNICATIONS SHERMIN OAKS PARTNERS VALLEY OAKS CLEANERS	VALLEYS OAKS CLEANERS CALTRANS DIST 7/ ROW PAI MIEPI CI EANER		BIN ENTERPRISES INC. MOBIL OIL CORPORATION 18-FXV JUDITH NIELSEN 14-165 MOBIL #11-FXV	B L N ENTERPRISES INC JUDY'S MOBIL SERVICE	MOBIL SERVICE STATION FXV	FOUR SEANSONS/M DAVID PAUL & ASSO PACIFIC BELL LOS ANGELES FIRE STA 88	LOS ANGELES FIRE STATION 88	L A CITY FIRE STATION #88 EXXON SERVICE STATION 7-0 CITY OF LA GENERAL SERVICES	FIRE STATION 88 LOS ANGELES FIRE STATION 88 VAN NUYS SPORTS CAR SERVICE CT	SHERMAN OAKS ARMY RESERVE CENT DANIELS HALL - US ARMY RESERVE CEN	DANIELS HALL - US ARMY RESERVE CEN F.D.I.C. RECEIVER FOR HOME FEDERAL SHERMAN OAKS CENTRIUM	SEPULVEDA BLVD PINECREST SCHOOL	PINECREST SCHOOL SHERMAN WAY DENTAL ASSOCIATES	WINALL OIL COMPANY/RESEDA MR STANLEY PETIT JORGE A. ALVREZ, D.D.S. SHERMAN CAKS MEDICAL GROLID	RICK PALLACK-MENS WEAR EXPRESSLY PORTRAITS INC THE BUCKLEY SCHOOL
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Databases	HIST UST SWEEPS UST, ORANGE CO.	EMI, CA FID UST HAZNET HAZNET	CA FID UST, SWEEPS UST HAZNET	CA FID UST, SWEEPS UST HAZNET	ORANGE CO. UST HAZNET	CORTESE, LUST HAZNET	CLEANERS, HAZNET RCRAInfo-SQG, FINDS CLEANERS	EMI HAZNET	HAZNET RCRAInfo-SQG, FINDS,	RCRAINTO-SQG, FINDS HAZNET	RCRAInfo-SQG, FINDS, HAZNET	HAZNET HAZNET PCPAINFOLOG HAZNET	CA FID UST RCRAInfo-LQG, HAZNET	HAZNET, CORTESE, SWEEPS UST, LUST	HAZNET HAZNET	HIST UST ORANGE CO. UST LUST	FINDS	HIST UST CA FID UST, SWEEPS UST	CORTESE, LUST HIST UST	ORANGE CO. UST FRNS	LUST	HAZNET HAZNET	HAZNET EMI, HAZNET	HAZNEI RCRAInfo-SQG, FINDS	HIST UST	HAZNET	HAZNET	HAZNET HAZNET	HAZNET HAZNET	HAZNET RCRAInfo-SQG, FINDS,	HAZNET
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Clty	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	TUJUNGA SHERMAN OAKS	VAN NUYS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS	SHERMAN CARS SHERMAN OAKS VAN NIKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN CARS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS
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Name	BEVERLY GLEN PUMPING STATION BEVERLY GLEN PUMP STATION							щ	WRAM DEVELOPMENT SHERMAN OAK CENTER CLEANERS	SHERMAN OAKS EXCLUSIVE AUTO PUBLIC STORAGE INC	AUTOMOTIVE PITSTOP	AUTOMOTIVE PIT STOP AUTOMOTIVE M & M EXXONIMODIL OIL CORDODATION 17885	ABRAM ABRAHEMIAN #14-712 EXXONMOBIL OIL CORP.	MOBIL #11-LLD	MOBIL OIL CORPORATION 18-LLD 1X MOBIL OIL CO #11-LLD	ABRAM ABRAHEMIAN #14-712 MOBIL SERVICE STATION LLD EXXONMOBIL STATION #18-110	EXXONMOBIL CICRPORATION 4822 VAN NITS BLVD	SERVICE STATION 6183 DALLA INC	UNOCAL #6183 (FORMER) UNION OIL SERVICE STATION LEAS	UNOCAL SERVICE STATION #6183 SHERMAN OAKS UNOCAL AUTO &SERVIC 4822 VAN NITYS BLVD	76 STATION #6183 RONALD KAPI AN AMEDICAL GROUP	PENG THIM FAN MD SANFORD J WEITZBUCH DPM	VALLEY PODIATRISTS GROUP SHERMAN OAKS MEDICAL BLDG	DAVID HESKIAOFF MD ALLIED X RAY IMAGING	LEONARD WAGNER MD PIONEER PAINT CORPORATION	CONTECT TAIN CONTION CONTECT TAIN CONTION CONTECT TO PERMENDING	DR GARY SCHNEIDER DO DR MARK GREENSPAN MD	GROSS & SILVERMAN HAND SURGERY GREENSPAN MARK MD	DIVIVSION ALLIED MANAGEMENT SHARON NEMEC DDS	SUSUMU FOGUCHI DUS PAUL R BILOVSKY DDS APC CLIFFORD FRASER M D	OMAR T. SOSA DDS INC
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e Databases	HAZNET HAZNET HAZNET CDL	CHMIRS HAZNET, CORTESE LUST EMI, HAZNET, CA FID	UST, SWEEPS UST CHMIRS ORANGE CO. UST, HAZNET,	HIST UST RCRAINFO-SQG, FINDS HAZNET HAZNET	HAZNET CLEANERS, EMI, HAZNET HAZNET	HAZNET RCRAINTO-SQG, FINDS HAZNET HAZNET	HAZNET HAZNET HAZNET HAZNET RCRAINTO-SOG, FINDS,	HAZNET HAZNET HAZNET HAZNET	CA FID UST, SWEEPS UST RCRAInfo-SQG HAZNET	RCRAInfo-SQG, FINDS CA FID UST, SWEEPS UST CA FID UST	FINDS RCRAINfo-SQG, FINDS SWEEPS UST, ORANGE CO.	CASID UST RCRAINO-SQG, FINDS CORTESE, LUST	HAZNET, CORTESE ORANGE CO. UST RECRING-SQG, FINDS,	LUST CA FID UST, SWEEPS UST HAZNET	HAZNET RCRAInfo-SQG, FINDS CA FID UST	EMI RCRAInfo-SQG, FINDS ERNS	ERNS HAZNET, CORTESE, SWEEPS UST 1 UST	ORANGE CO. UST CA FID UST HIST UST LA TYET	HAZNET HAZNET HAZNET	HAZNET, CLEANERS
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City	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS VAN NUYS	VAN NUYS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	VAN NUYS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	VAN NUYS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	LOS ANGELES SHERMAN OAKS VAN NUYS	SHERMAN OAKS LOS ANGELES SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS VAN NUYS VAN NUYS	VAN NUYS SHERMAN OAKS VAN NUYS	SHERMAN OAKS VAN NUYS SHERMAN OAKS	VAN NUYS VAN NUYS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS
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Address	4910 VAN NUYS BLVD STE 210 4910 VAN NUYS BLVD STE100 4911 VAN NUYS BL #102 4920 VAN NUYS BLVD, #118	4929 N VAN NUYS BLVD 4929 VAN NUYS 4929 VAN NUYS BLVD 4929 VAN NUYS BLVD	4929 VAN NUYS BLVD 4929 VAN NUYS BLVD		UVS BLVD		4955 VAN NUYS BLVD,#514 4955 VAN NUYS BLVD,#615 4955 VAN NUYS #1VD. #611 4955 VAN NUYS, #110 5000 VAN NUYS BLVD	5000 VAN NUYS BLVD #200 5000 VAN NUYS BLVD STE 320 5000 VAN NUYS BLVD, #210	S \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			5151 VAN NUYS BLVD 5160 VAN NUYS BLVD 5160 VAN NUYS BLVD	5161 VAN NUYS 5161 VAN NUYS BLVD 5161 VAN NUYS BLVD	5161 VAN NUYS BLVD 5161 VAN NUYS BLVD 5161 VAN NUYS BLVD	7300 VAN NUYS BLVD 7300 VAN NUYS BLVD 13646 VENTURA BLVD	14058 VENTURA BLVD 14210 VENTURA BLVD 14478 VENTURA BLVD	14478 VENTURA BLVD. 14505 VENTURA BLVD	14505 VENTURA BLVD 14505 VENTURA BLVD 14505 VENTURA BLVD 14505 VENTURA BLVD	14535 VENTURA BLVD 14622 VENTURA BLVD 14629 VENTURA BLVD	14645 VENTURA BLVD
Name	OLIVIA S CAJULIS DDS INC ELMER SOUSA DDS WKWS MDS	SHERMAN OAKS COMUNITY HOS SHERMAN OAKS COMUNITY HOSPITAL SHERMAN OAKS COMMUNITY HOSPITAL		PACIFIC OAKS MEDICAL GROUP SO OAKS DENTAL CENTER VIEW POINT HEALTHGARE	MIKE KIM CLASSIC 1 HR CLEANERS BECHTELL INC	RCHARD SWATT DDS INC GOTTLIEB MEDICAL GRP SHERMAN OAKS RADIOLOGY PAUL ENDLER, MD	HARRY S KAHN MD INC LOS ANGELES ORTHOPAEDIC STUART R. MARKMAN, D.D.S. G & L REALTY CORP ARETE ASSOCIATES	VIEWPORT HEALTH CARE MED GRP BARRY ROSENTHAL DDS ADVANCED ORTHOPAEDICS	UNK CLAUDIO'S ITALIA SPORTS CARS INC ABE'S MERCEDES SERVICE	ITALIA SPORTS CARS TETRAD INC. LA FOGOTA	AUTOZONE #5485 MILLER HONDA FOREMAN HONDA MOTORS	FOREMAN HONDA MOTORS CHEVRON STATION 96745 CHEVRON #9-6745	SHELL JERRY'S SHELL SERVICE CENTER SHELL SERVICE STATION	SHELL JERRY'S SHELL SERVICE CENTER JERRY'S SHELL SVC CENTER INC	MID VALLEY COLLISION CENTER TOP AUTO JEFFREY N STEPHAN	ILW INC. SHERMAN OAKS ALLEY G GOODYEAR 9347 14478 VENTURA BLVD	144/8 VENTURA BLVD. CHEVRON #9-1333	CHEVRON STATION #9-1333 HARJINDER MAC 91333 CHEVRON 04333	ONE HOUR PHOTO CENTER LA REINA PLACE SHERMAN OAKS CHIROPRACTORS	PRISTINE CLEANERS
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Databases	RCRAINE CLEANE HAZNET	HAZNET HAZNET HAZNET	HIST US HAZNET HAZNET RCRAIN	HAZNET CA FID L	NAZNET SWRCY EMI	HAZNET	EMI HAZNET	HIST UST CA FID UST CA FID UST	CA FID C	HAZNET HAZNET HAZNET	HAZNET HAZNET HAZNET	CA FID (HAZNET	RCRAIN FINDS, F	<u>.</u> ММ М М М М М	HAZNET CA FID L	RCRAIN CA FID	CA FID ORANGI	HAZNET CA WDS	HAZNET CA WDS HAZNET	HAZNET HAZNET	HAZNET HAZNET RCRAIM	
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City	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN CAKS SHERMAN CAKS SHERMAN CAKS	SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	SHERMAN OAKS SHERMAN OAKS SHERMAN OAKS	
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To maintain currency of the following federal and state databases, EDR contacts the appropriate governmental agency on a monthly or quarterly basis, as required.

Number of Days to Update: Provides confirmation that EDR is reporting records that have been updated within 90 days from the date the government agency made the information available to the public.

FEDERAL RECORDS

NPL: National Priority List

National Priorities List (Superfund). The NPL is a subset of CERCLIS and identifies over 1,200 sites for priority cleanup under the Superfund Program. NPL sites may encompass relatively large areas. As such, EDR provides polygon coverage for over 1,000 NPL site boundaries produced by EPA's Environmental Photographic Interpretation Center (EPIC) and regional EPA offices.

Date of Government Version: 07/18/2007 Date Data Arrived at EDR: 08/03/2007

Date Made Active in Reports: 08/29/2007

Number of Days to Update: 26

Source: EPA Telephone: N/A

Last EDR Contact: 07/31/2007

Next Scheduled EDR Contact: 10/29/2007 Data Release Frequency: Quarterly

NPL Site Boundaries

EPA's Environmental Photographic Interpretation Center (EPIC)

Telephone: 202-564-7333

EPA Region 1

Telephone 617-918-1143

EPA Region 3 Telephone 215-814-5418

EPA Region 4

Telephone 404-562-8033

EPA Region 5

Telephone 312-886-6686

EPA Region 10 Telephone 206-553-8665 **EPA Region 6**

Telephone: 214-655-6659

EPA Region 7

Telephone: 913-551-7247

EPA Region 8

Telephone: 303-312-6774

EPA Region 9

Telephone: 415-947-4246

Proposed NPL: Proposed National Priority List Sites

A site that has been proposed for listing on the National Priorities List through the issuance of a proposed rule in the Federal Register. EPA then accepts public comments on the site, responds to the comments, and places on the NPL those sites that continue to meet the requirements for listing.

Date of Government Version: 08/09/2007 Date Data Arrived at EDR: 09/05/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 36

Source: EPA Telephone: N/A

Last EDR Contact: 08/31/2007

Next Scheduled EDR Contact: 10/29/2007 Data Release Frequency: Quarterly

DELISTED NPL: National Priority List Deletions

The National Oil and Hazardous Substances Pollution Contingency Plan (NCP) establishes the criteria that the EPA uses to delete sites from the NPL. In accordance with 40 CFR 300.425.(e), sites may be deleted from the NPL where no further response is appropriate.

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 43

Source: EPA Telephone: N/A

Last EDR Contact: 08/29/2007

Next Scheduled EDR Contact: 10/29/2007 Data Release Frequency: Quarterly

NPL LIENS: Federal Superfund Liens

Federal Superfund Liens. Under the authority granted the USEPA by CERCLA of 1980, the USEPA has the authority to file liens against real property in order to recover remedial action expenditures or when the property owner received notification of potential liability. USEPA compiles a listing of filed notices of Superfund Liens.

Date of Government Version: 10/15/1991 Date Data Arrived at EDR: 02/02/1994 Date Made Active in Reports: 03/30/1994

Number of Days to Update: 56

Source: EPA

Telephone: 202-564-4267 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: No Update Planned

CERCLIS: Comprehensive Environmental Response, Compensation, and Liability Information System

CERCLIS contains data on potentially hazardous waste sites that have been reported to the USEPA by states, municipalities, private companies and private persons, pursuant to Section 103 of the Comprehensive Environmental Response, Compensation, and Liability Act (CERCLA). CERCLIS contains sites which are either proposed to or on the National Priorities List (NPL) and sites which are in the screening and assessment phase for possible inclusion on the NPL.

Date of Government Version: 04/23/2007 Date Data Arrived at EDR: 06/20/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 70

Source: EPA Telephone: 703-412-9810 Last EDR Contact: 12/06/2007

Next Scheduled EDR Contact: 03/17/2008 Data Release Frequency: Quarterly

CERCLIS-NFRAP: CERCLIS No Further Remedial Action Planned

Archived sites are sites that have been removed and archived from the inventory of CERCLIS sites. Archived status indicates that, to the best of EPA's knowledge, assessment at a site has been completed and that EPA has determined no further steps will be taken to list this site on the National Priorities List (NPL), unless information indicates this decision was not appropriate or other considerations require a recommendation for listing at a later time. This decision does not necessarily mean that there is no hazard associated with a given site; it only means that, based upon available information, the location is not judged to be a potential NPL site.

Date of Government Version: 06/21/2007 Date Data Arrived at EDR: 07/23/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 37

Source: FPA

Telephone: 703-412-9810 Last EDR Contact: 12/06/2007

Next Scheduled EDR Contact: 03/17/2008 Data Release Frequency: Quarterly

CORRACTS: Corrective Action Report

CORRACTS identifies hazardous waste handlers with RCRA corrective action activity.

Date of Government Version: 06/26/2007 Date Data Arrived at EDR: 08/08/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 21

Source: EPA

Telephone: 800-424-9346 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

RCRA: Resource Conservation and Recovery Act Information

RCRAInfo is EPA's comprehensive information system, providing access to data supporting the Resource Conservation and Recovery Act (RCRA) of 1976 and the Hazardous and Solid Waste Amendments (HSWA) of 1984. RCRAInfo replaces the data recording and reporting abilities of the Resource Conservation and Recovery Information System (RCRIS). The database includes selective information on sites which generate, transport, store, treat and/or dispose of hazardous waste as defined by the Resource Conservation and Recovery Act (RCRA). Conditionally exempt small quantity generators (CESQGs) generate less than 100 kg of hazardous waste, or less than 1 kg of acutely hazardous waste per month. Small quantity generators (SQGs) generate between 100 kg and 1,000 kg of hazardous waste per month. Large quantity generators (LQGs) generate over 1,000 kilograms (kg) of hazardous waste, or over 1 kg of acutely hazardous waste per month. Transporters are individuals or entities that move hazardous waste from the generator off-site to a facility that can recycle, treat, store, or dispose of the waste. TSDFs treat, store, or dispose of the waste.

Date of Government Version: 06/13/2006 Date Data Arrived at EDR: 06/28/2006 Date Made Active in Reports: 08/23/2006

Number of Days to Update: 56

Source: EPA

Telephone: (415) 495-8895 Last EDR Contact: 10/16/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Quarterly

ERNS: Emergency Response Notification System

Emergency Response Notification System. ERNS records and stores information on reported releases of oil and hazardous substances.

Date of Government Version: 12/31/2006 Date Data Arrived at EDR: 01/24/2007 Date Made Active in Reports: 03/12/2007

Number of Days to Update: 47

Source: National Response Center, United States Coast Guard

Telephone: 202-267-2180 Last EDR Contact: 10/19/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Annually

HMIRS: Hazardous Materials Information Reporting System

Hazardous Materials Incident Report System. HMIRS contains hazardous material spill incidents reported to DOT.

Date of Government Version: 07/02/2007 Date Data Arrived at EDR: 07/18/2007 Date Made Active in Reports: 09/18/2007

Number of Days to Update: 62

Source: U.S. Department of Transportation

Telephone: 202-366-4555 Last EDR Contact: 10/16/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Annually

US ENG CONTROLS: Engineering Controls Sites List

A listing of sites with engineering controls in place. Engineering controls include various forms of caps, building foundations, liners, and treatment methods to create pathway elimination for regulated substances to enter environmental media or effect human health.

Date of Government Version: 07/16/2007 Date Data Arrived at EDR: 08/03/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 69

Source: Environmental Protection Agency

Telephone: 703-603-8905 Last EDR Contact: 11/16/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Varies

US INST CONTROL: Sites with institutional Controls

A listing of sites with institutional controls in place. Institutional controls include administrative measures, such as groundwater use restrictions, construction restrictions, property use restrictions, and post remediation care requirements intended to prevent exposure to contaminants remaining on site. Deed restrictions are generally required as part of the institutional controls.

Date of Government Version: 07/16/2007 Date Data Arrived at EDR: 08/03/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 69

Source: Environmental Protection Agency

Telephone: 703-603-8905 Last EDR Contact: 11/16/2007

Next Scheduled EDR Contact: 12/31/2007

Data Release Frequency: Varies

DOD: Department of Defense Sites

This data set consists of federally owned or administered lands, administered by the Department of Defense, that have any area equal to or greater than 640 acres of the United States, Puerto Rico, and the U.S. Virgin Islands.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 11/10/2006 Date Made Active in Reports: 01/11/2007

Number of Days to Update: 62

Source: USGS

Telephone: 703-692-8801 Last EDR Contact: 11/09/2007

Next Scheduled EDR Contact: 02/04/2008 Data Release Frequency: Semi-Annually

FUDS: Formerty Used Defense Sites

The listing includes locations of Formerly Used Defense Sites properties where the US Army Corps of Engineers is actively working or will take necessary cleanup actions.

Date of Government Version: 12/31/2006 Date Data Arrived at EDR: 08/31/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 41

Source: U.S. Army Corps of Engineers

Telephone: 202-528-4285 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Varies

US BROWNFIELDS: A Listing of Brownfields Sites

Included in the listing are brownfields properties addresses by Cooperative Agreement Recipients and brownfields properties addressed by Targeted Brownfields Assessments. Targeted Brownfields Assessments-EPA's Targeted Brownfields Assessments (TBA) program is designed to help states, tribes, and municipalities—especially those without EPA Brownfields Assessment Demonstration Pilots-minimize the uncertainties of contamination often associated with brownfields. Under the TBA program, EPA provides funding and/or technical assistance for environmental assessments at brownfields sites throughout the country. Targeted Brownfields Assessments supplement and work with other efforts under EPA's Brownfields Initiative to promote cleanup and redevelopment of brownfields. Cooperative Agreement Recipients-States, political subdivisions, territories, and Indian tribes become Brownfields Cleanup Revolving Loan Fund (BCRLF) cooperative agreement recipients when they enter into BCRLF cooperative agreements with the U.S. EPA, EPA selects BCRLF cooperative agreement recipients based on a proposal and application process. BCRLF cooperative agreement recipients must use EPA funds provided through BCRLF cooperative agreement for specified brownfields-related cleanup activities.

Date of Government Version: 06/20/2007 Date Data Arrived at EDR: 07/09/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 51

Source: Environmental Protection Agency

Telephone: 202-566-2777 Last EDR Contact: 09/10/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Semi-Annually

CONSENT: Superfund (CERCLA) Consent Decrees

Major legal settlements that establish responsibility and standards for cleanup at NPL (Superfund) sites. Released periodically by United States District Courts after settlement by parties to litigation matters.

Date of Government Version: 04/13/2007 Date Data Arrived at EDR: 07/16/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 44

Source: Department of Justice, Consent Decree Library

Telephone: Varies

Last EDR Contact: 09/21/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Varies

ROD: Records Of Decision

Record of Decision. ROD documents mandate a permanent remedy at an NPL (Superfund) site containing technical and health information to aid in the cleanup.

Date of Government Version: 06/08/2007 Date Data Arrived at EDR: 07/03/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 57

Source: EPA

Telephone: 703-416-0223 Last EDR Contact: 11/08/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Annually

UMTRA: Uranium Mill Tailings Sites

Uranium ore was mined by private companies for federal government use in national defense programs. When the mills shut down, large piles of the sand-like material (mill tailings) remain after uranium has been extracted from the ore. Levels of human exposure to radioactive materials from the piles are low; however, in some cases tailings were used as construction materials before the potential health hazards of the tailings were recognized.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 11/08/2006 Date Made Active in Reports: 01/29/2007

Number of Days to Update: 82

Source: Department of Energy Telephone: 505-845-0011 Last EDR Contact: 09/19/2007

Next Scheduled EDR Contact: 12/17/2007

Data Release Frequency: Varies

ODI: Open Dump Inventory

An open dump is defined as a disposal facility that does not comply with one or more of the Part 257 or Part 258 Subtitle D Criteria.

Date of Government Version: 06/30/1985 Date Data Arrived at EDR: 08/09/2004 Date Made Active in Reports: 09/17/2004

Number of Days to Update: 39

Source: Environmental Protection Agency

Telephone: 800-424-9346 Last EDR Contact: 06/09/2004 Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

TRIS: Toxic Chemical Release Inventory System

Toxic Release Inventory System. TRIS identifies facilities which release toxic chemicals to the air, water and land in reportable quantities under SARA Title III Section 313.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 04/27/2007 Date Made Active in Reports: 07/05/2007

Number of Days to Update: 69

Source: EPA

Telephone: 202-566-0250 Last EDR Contact: 09/18/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: Annually

TSCA: Toxic Substances Control Act

Toxic Substances Control Act. TSCA identifies manufacturers and importers of chemical substances included on the TSCA Chemical Substance Inventory list. It includes data on the production volume of these substances by plant

Date of Government Version: 12/31/2002 Date Data Arrived at EDR: 04/14/2006 Date Made Active in Reports: 05/30/2006

Number of Days to Update: 46

Source: EPA

Telephone: 202-260-5521 Last EDR Contact: 11/14/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Every 4 Years

FTTS: FIFRA/ TSCA Tracking System - FIFRA (Federal Insecticide, Fungicide, & Rodenticide Act)/TSCA (Toxic Substances Control Act) FTTS tracks administrative cases and pesticide enforcement actions and compliance activities related to FIFRA, TSCA and EPCRA (Emergency Planning and Community Right-to-Know Act). To maintain currency, EDR contacts the Agency on a quarterly basis.

Date of Government Version: 07/06/2007 Date Data Arrived at EDR: 07/20/2007 Date Made Active in Reports: 09/18/2007

Number of Days to Update: 60

Source: EPA/Office of Prevention, Pesticides and Toxic Substances Telephone: 202-566-1667

Last EDR Contact: 09/17/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: Quarterly

FTTS INSP: FIFRA/ TSCA Tracking System - FIFRA (Federal Insecticide, Fungicide, & Rodenticide Act)/TSCA (Toxic Substances Control Act) A listing of FIFRA/TSCA Tracking System (FTTS) inspections and enforcements.

Date of Government Version: 07/06/2007 Date Data Arrived at EDR: 07/20/2007 Date Made Active in Reports: 09/18/2007

Number of Days to Update: 60

Source: EPA

Telephone: 202-566-1667 Last EDR Contact: 09/17/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: Quarterly

SSTS: Section 7 Tracking Systems

Section 7 of the Federal Insecticide, Fungicide and Rodenticide Act, as amended (92 Stat. 829) requires all registered pesticide-producing establishments to submit a report to the Environmental Protection Agency by March 1st each year. Each establishment must report the types and amounts of pesticides, active ingredients and devices being produced, and those having been produced and sold or distributed in the past year.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 03/13/2007 Date Made Active in Reports: 04/27/2007

Number of Days to Update: 45

Source: EPA

Telephone: 202-564-4203 Last EDR Contact: 10/15/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Annually

LUCIS: Land Use Control Information System

LUCIS contains records of land use control information pertaining to the former Navy Base Realignment and Closure properties.

Date of Government Version: 12/09/2005 Date Data Arrived at EDR: 12/11/2006 Date Made Active in Reports: 01/11/2007

Number of Days to Update: 31

Source: Department of the Navy Telephone: 843-820-7326 Last EDR Contact: 09/12/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Varies

DOT OPS: Incident and Accident Data

Department of Transporation, Office of Pipeline Safety Incident and Accident data.

Date of Government Version: 08/14/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 43

Source: Department of Transporation, Office of Pipeline Safety

Telephone: 202-366-4595 Last EDR Contact: 11/29/2007

Next Scheduled EDR Contact: 02/25/2008

Data Release Frequency: Varies

ICIS: Integrated Compliance Information System

The Integrated Compliance Information System (ICIS) supports the information needs of the national enforcement and compliance program as well as the unique needs of the National Pollutant Discharge Elimination System (NPDES) program.

Date of Government Version: 07/27/2007 Date Data Arrived at EDR: 08/13/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 59

Source: Environmental Protection Agency

Telephone: 202-564-5088 Last EDR Contact: 10/15/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Quarterly

DEBRIS REGION 9: Torres Martinez Reservation Illegal Dump Site Locations

A listing of illegal dump sites location on the Torres Martinez Indian Reservation located in eastern Riverside County and northern Imperial County, California.

Date of Government Version: 07/25/2007 Date Data Arrived at EDR: 07/31/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 72

Source: EPA, Region 9 Telephone: 415-972-3336 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007

Data Release Frequency: Varies

HIST FTTS: FIFRA/TSCA Tracking System Administrative Case Listing

A complete administrative case listing from the FIFRA/TSCA Tracking System (FTTS) for all ten EPA regions. The information was obtained from the National Compliance Database (NCDB). NCDB supports the implementation of FIFRA (Federal Insecticide, Fungicide, and Rodenticide Act) and TSCA (Toxic Substances Control Act). Some EPA regions are now closing out records. Because of that, and the fact that some EPA regions are not providing EPA Headquarters with updated records, it was decided to create a HIST FTTS database. It included records that may not be included in the newer FTTS database updates. This database is no longer updated.

Date of Government Version: 10/19/2006 Date Data Arrived at EDR: 03/01/2007 Date Made Active in Reports: 04/10/2007

Number of Days to Update: 40

Source: Environmental Protection Agency

Telephone: 202-564-2501 Last EDR Contact: 09/17/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: No Update Planned

CDL: Clandestine Drug Labs

A listing of clandestine drug lab locations. The U.S. Department of Justice ("the Department") provides this web site as a public service. It contains addresses of some locations where law enforcement agencies reported they found chemicals or other items that indicated the presence of either clandestine drug laboratories or dumpsites. In most cases, the source of the entries is not the Department, and the Department has not verified the entry and does not guarantee its accuracy. Members of the public must verify the accuracy of all entries by, for example, contacting local law enforcement and local health departments.

Date of Government Version: 12/01/2006 Date Data Arrived at EDR: 01/08/2007 Date Made Active in Reports: 01/11/2007

Number of Days to Update: 3

Source: Drug Enforcement Administration

Telephone: 202-307-1000 Last EDR Contact: 10/02/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Quarterly

RADINFO: Radiation Information Database

The Radiation Information Database (RADINFO) contains information about facilities that are regulated by U.S. Environmental Protection Agency (EPA) regulations for radiation and radioactivity.

Date of Government Version: 07/31/2007 Date Data Arrived at EDR: 08/01/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 28

Source: Environmental Protection Agency

Telephone: 202-343-9775 Last EDR Contact: 10/31/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: Quarterly

LIENS 2: CERCLA Lien Information

A Federal CERCLA ('Superfund') lien can exist by operation of law at any site or property at which EPA has spent Superfund monies. These monies are spent to investigate and address releases and threatened releases of contamination. CERCLIS provides information as to the identity of these sites and properties.

Date of Government Version: 03/08/2007 Date Data Arrived at EDR: 04/12/2007 Date Made Active in Reports: 05/14/2007

Number of Days to Update: 32

Source: Environmental Protection Agency

Telephone: 202-564-6023 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008

Data Release Frequency: Varies

PADS: PCB Activity Database System

PCB Activity Database. PADS Identifies generators, transporters, commercial storers and/or brokers and disposers of PCB's who are required to notify the EPA of such activities.

Date of Government Version: 04/12/2007 Date Data Arrived at EDR: 06/08/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 82

Source: EPA

Telephone: 202-566-0500 Last EDR Contact: 08/09/2007

Next Scheduled EDR Contact: 11/05/2007 Data Release Frequency: Annually

MLTS: Material Licensing Tracking System

MLTS is maintained by the Nuclear Regulatory Commission and contains a list of approximately 8,100 sites which possess or use radioactive materials and which are subject to NRC licensing requirements. To maintain currency, EDR contacts the Agency on a quarterly basis.

Date of Government Version: 07/09/2007 Date Data Arrived at EDR: 07/24/2007 Date Made Active in Reports: 09/18/2007 Number of Days to Update: 56

Source: Nuclear Regulatory Commission

Telephone: 301-415-7169 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Quarterly

MINES: Mines Master Index File

Contains all mine identification numbers issued for mines active or opened since 1971. The data also includes violation information.

Date of Government Version: 05/09/2007 Date Data Arrived at EDR: 06/28/2007 Date Made Active in Reports: 08/29/2007

Number of Days to Update: 62

Source: Department of Labor, Mine Safety and Health Administration

Telephone: 303-231-5959 Last EDR Contact: 09/26/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Semi-Annually

FINDS: Facility Index System/Facility Registry System

Facility Index System. FINDS contains both facility information and 'pointers' to other sources that contain more detail. EDR includes the following FINDS databases in this report; PCS (Permit Compliance System), AIRS (Aerometric Information Retrieval System), DOCKET (Enforcement Docket used to manage and track information on civil judicial enforcement cases for all environmental statutes), FURS (Federal Underground Injection Control), C-DOCKET (Criminal Docket System used to track criminal enforcement actions for all environmental statutes), FFIS (Federal Facilities Information System), STATE (State Environmental Laws and Statutes), and PADS (PCB Activity Data System).

Date of Government Version: 07/19/2007 Date Data Arrived at EDR: 07/25/2007 Date Made Active in Reports: 09/18/2007

Number of Days to Update: 55

Source: EPA

Telephone: (415) 947-8000 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Quarterly

RAATS: RCRA Administrative Action Tracking System

RCRA Administration Action Tracking System. RAATS contains records based on enforcement actions issued under RCRA pertaining to major violators and includes administrative and civil actions brought by the EPA. For administration actions after September 30, 1995, data entry in the RAATS database was discontinued. EPA will retain a copy of the database for historical records. It was necessary to terminate RAATS because a decrease in agency resources made it impossible to continue to update the information contained in the database.

Date of Government Version: 04/17/1995 Date Data Arrived at EDR: 07/03/1995 Date Made Active in Reports: 08/07/1995

Number of Days to Update: 35

Source: EPA

Telephone: 202-564-4104 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: No Update Planned

BRS: Biennial Reporting System

The Biennial Reporting System is a national system administered by the EPA that collects data on the generation and management of hazardous waste. BRS captures detailed data from two groups: Large Quantity Generators (LQG) and Treatment, Storage, and Disposal Facilities.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 03/06/2007 Date Made Active in Reports: 04/13/2007

Number of Days to Update: 38

Source: EPA/NTIS Telephone: 800-424-9346 Last EDR Contact: 09/12/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Biennially

USGS WATER WELLS: National Water Information System (NWIS)

This database consists of well records in the United States. Available site descriptive information includes well location information (latitude and longitude, well depth, site use, water use, and aquifer).

Date of Government Version: 03/25/2005 Date Data Arrived at EDR: 03/25/2005 Date Made Active in Reports: N/A Number of Days to Update: 0

Source: USGS Telephone: N/A

Last EDR Contact: 03/25/2005 Next Scheduled EDR Contact: N/A Data Release Frequency: N/A

PWS: Public Water System Data

This Safe Drinking Water Information System (SDWIS) file contains public water systems name and address, population served and the primary source of water

Date of Government Version: 02/24/2000 Date Data Arrived at EDR: 04/27/2005 Date Made Active in Reports: N/A Number of Days to Update: 0

Source: EPA Telephone: N/A Last EDR Contact: 11/15/2007 Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: N/A

STATE AND LOCAL RECORDS

HIST CAL-SITES: Calsites Database

The Calsites database contains potential or confirmed hazardous substance release properties. In 1996, California EPA reevaluated and significantly reduced the number of sites in the Calsites database. No longer updated by the state agency. It has been replaced by ENVIROSTOR.

Date of Government Version: 08/08/2005 Date Data Arrived at EDR: 08/03/2006

Date Made Active in Reports: 08/24/2006

Number of Days to Update: 21

Source: Department of Toxic Substance Control

Telephone: 916-323-3400 Last EDR Contact: 11/26/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: No Update Planned

CA BOND EXP. PLAN: Bond Expenditure Plan

Department of Health Services developed a site-specific expenditure plan as the basis for an appropriation of Hazardous Substance Cleanup Bond Act funds. It is not updated.

Date of Government Version: 01/01/1989 Date Data Arrived at EDR: 07/27/1994 Date Made Active in Reports: 08/02/1994

Number of Days to Update: 6

Source: Department of Health Services

Telephone: 916-255-2118 Last EDR Contact: 05/31/1994 Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

SCH: School Property Evaluation Program

This category contains proposed and existing school sites that are being evaluated by DTSC for possible hazardous materials contamination. In some cases, these properties may be listed in the CalSites category depending on the level of threat to public health and safety or the environment they pose.

Date of Government Version: 08/28/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 09/26/2007

Number of Days to Update: 28

Source: Department of Toxic Substances Control

Telephone: 916-323-3400 Last EDR Contact: 11/28/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: Quarterly

TOXIC PITS: Toxic Pits Cleanup Act Sites

Toxic PITS Cleanup Act Sites. TOXIC PITS identifies sites suspected of containing hazardous substances where cleanup has not yet been completed.

Date of Government Version: 07/01/1995 Date Data Arrived at EDR: 08/30/1995 Date Made Active in Reports: 09/26/1995

Number of Days to Update: 27

Source: State Water Resources Control Board

Telephone: 916-227-4364 Last EDR Contact: 10/26/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: No Update Planned

SWF/LF (SWIS): Solid Waste Information System

Active, Closed and Inactive Landfills. SWF/LF records typically contain an inventory of solid waste disposal facilities or landfills. These may be active or i nactive facilities or open dumps that failed to meet RCRA Section 4004 criteria for solid waste landfills or disposal sites.

Date of Government Version: 09/10/2007 Date Data Arrived at EDR: 09/12/2007 Date Made Active in Reports: 09/28/2007 Number of Days to Update: 16

Source: Integrated Waste Management Board Telephone: 916-341-6320

Last EDR Contact: 09/12/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Quarterly

WMUDS/SWAT: Waste Management Unit Database

Waste Management Unit Database System. WMUDS is used by the State Water Resources Control Board staff and the Regional Water Quality Control Boards for program tracking and inventory of waste management units. WMUDS is composed of the following databases: Facility Information, Scheduled Inspections Information, Waste Management Unit Information, SWAT Program Information, SWAT Report Summary Information, SWAT Report Summary Data, Chapter 15 (formerly Subchapter 15) Information, Chapter 15 Monitoring Parameters, TPCA Program Information, RCRA Program Information, Closure Information, and Interested Parties Information.

Date of Government Version: 04/01/2000 Date Data Arrived at EDR: 04/10/2000 Date Made Active in Reports: 05/10/2000

Number of Days to Update: 30

Source: State Water Resources Control Board

Telephone: 916-227-4448 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

CA WDS: Waste Discharge System

Sites which have been issued waste discharge requirements.

Date of Government Version: 06/19/2007 Date Data Arrived at EDR: 06/20/2007 Date Made Active in Reports: 06/29/2007

Number of Days to Update: 9

Source: State Water Resources Control Board

Telephone: 916-341-5227 Last EDR Contact: 09/17/2007

Next Scheduled EDR Contact: 12/17/2007
Data Release Frequency: Quarterly

CORTESE: "Cortese" Hazardous Waste & Substances Sites List

The sites for the list are designated by the State Water Resource Control Board (LUST), the Integrated Waste Board (SWF/LS), and the Department of Toxic Substances Control (Cal-Sites). This listing is no longer updated by the state agency.

Date of Government Version: 04/01/2001 Date Data Arrived at EDR: 05/29/2001 Date Made Active in Reports: 07/26/2001

Number of Days to Update: 58

Source: CAL EPA/Office of Emergency Information

Telephone: 916-323-3400 Last EDR Contact: 10/19/2007

Next Scheduled EDR Contact: 01/21/2008
Data Release Frequency: No Update Planned

SWRCY: Recycler Database

A listing of recycling facilities in California.

Date of Government Version: 10/09/2007 Date Data Arrived at EDR: 10/11/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 27

Source: Department of Conservation

Telephone: 916-323-3836 Last EDR Contact: 10/11/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Quarterly

LUST REG 9: Leaking Underground Storage Tank Report

Orange, Riverside, San Diego counties. For more current information, please refer to the State Water Resources Control Board's LUST database.

Date of Government Version: 03/01/2001 Date Data Arrived at EDR: 04/23/2001 Date Made Active in Reports: 05/21/2001

Number of Days to Update: 28

Last EDR Contact: 10/15/2007 Next Scheduled EDR Contact: 01/14/2008

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: No Update Planned

LUST REG 8: Leaking Underground Storage Tanks

California Regional Water Quality Control Board Santa Ana Region (8). For more current information, please refer to the State Water Resources Control Board's LUST database.

Date of Government Version: 02/14/2005 Date Data Arrived at EDR: 02/15/2005 Date Made Active in Reports: 03/28/2005

Number of Days to Update: 41

Source: California Regional Water Quality Control Board Santa Ana Region (8)

Source: California Regional Water Quality Control Board San Diego Region (9)

Telephone: 909-782-4496 Last EDR Contact: 11/05/2007

Telephone: 858-637-5595

Next Scheduled EDR Contact: 02/04/2008

Data Release Frequency: Varies

LUST REG 6V: Leaking Underground Storage Tank Case Listing

Leaking Underground Storage Tank locations. Inyo, Kern, Los Angeles, Mono, San Bernardino counties.

Date of Government Version: 06/07/2005 Date Data Arrived at EDR: 06/07/2005 Date Made Active in Reports: 06/29/2005

Number of Days to Update: 22

Source: California Regional Water Quality Control Board Victorville Branch Office (6)

Source: California Regional Water Quality Control Board Lahontan Region (6)

Telephone: 760-241-7365 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: No Update Planned

LUST REG 6L: Leaking Underground Storage Tank Case Listing

For more current information, please refer to the State Water Resources Control Board's LUST database.

Date of Government Version: 09/09/2003 Date Data Arrived at EDR: 09/10/2003 Date Made Active in Reports: 10/07/2003

Number of Days to Update: 27

Telephone: 530-542-5572 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: No Update Planned

LUST REG 5: Leaking Underground Storage Tank Database

Leaking Underground Storage Tank locations. Alameda, Alpine, Amador, Butte, Colusa, Contra Costa, Calveras, El Dorado, Fresno, Glenn, Kern, Kings, Lake, Lassen, Madera, Mariposa, Merced, Modoc, Napa, Nevada, Placer, Plumas, Sacramento, San Joaquin, Shasta, Solano, Stanislaus, Sutter, Tehama, Tulare, Tuolumne, Yolo, Yuba counties.

Date of Government Version: 07/01/2007 Date Data Arrived at EDR: 08/01/2007 Date Made Active in Reports: 08/09/2007

Number of Days to Update: 8

Source: California Regional Water Quality Control Board Central Valley Region (5)

Telephone: 916-464-4834 Last EDR Contact: 11/07/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Quarterly

LUST REG 4: Underground Storage Tank Leak List

Los Angeles, Ventura counties. For more current information, please refer to the State Water Resources Control Board's LUST database.

Date of Government Version: 09/07/2004 Date Data Arrived at EDR: 09/07/2004 Date Made Active in Reports: 10/12/2004

Number of Days to Update: 35

Source: California Regional Water Quality Control Board Los Angeles Region (4)

Telephone: 213-576-6710 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: No Update Planned

LUST REG 3: Leaking Underground Storage Tank Database

Leaking Underground Storage Tank locations. Monterey, San Benito, San Luis Obispo, Santa Barbara, Santa Cruz counties.

Date of Government Version: 05/19/2003 Date Data Arrived at EDR: 05/19/2003 Date Made Active in Reports: 06/02/2003

Number of Days to Update: 14

Source: California Regional Water Quality Control Board Central Coast Region (3)

Telephone: 805-542-4786 Last EDR Contact: 11/13/2007

Next Scheduled EDR Contact: 02/11/2008 Data Release Frequency: No Update Planned

LUST REG 2: Fuel Leak List

Leaking Underground Storage Tank locations. Alameda, Contra Costa, Marin, Napa, San Francisco, San Mateo, Santa Clara, Solano, Sonoma counties.

Date of Government Version: 09/30/2004 Date Data Arrived at EDR: 10/20/2004 Date Made Active in Reports: 11/19/2004 Number of Days to Update: 30

Source: California Regional Water Quality Control Board San Francisco Bay Region (2) Telephone: 510-622-2433

Last EDR Contact: 10/09/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Quarterly

LUST REG 1: Active Toxic Site Investigation

Del Norte, Humboldt, Lake, Mendocino, Modoc, Siskiyou, Sonoma, Trinity counties. For more current information, please refer to the State Water Resources Control Board's LUST database.

Date of Government Version: 02/01/2001 Date Data Arrived at EDR: 02/28/2001 Date Made Active in Reports: 03/29/2001 Number of Days to Update: 29 Source: California Regional Water Quality Control Board North Coast (1) Telephone: 707-570-3769 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008
Data Release Frequency: No Update Planned

LUST: Geotracker's Leaking Underground Fuel Tank Report

Leaking Underground Storage Tank Incident Reports. LUST records contain an inventory of reported leaking underground storage tank incidents. Not all states maintain these records, and the information stored varies by state. For more information on a particular leaking underground storage tank sites, please contact the appropriate regulatory agency.

Date of Government Version: 10/10/2007 Date Data Arrived at EDR: 10/11/2007 Date Made Active in Reports: 11/07/2007 Source: State Water Resources Control Board Telephone: see region list Last EDR Contact: 10/11/2007

Number of Days to Update: 27

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Quarterly

LUST REG 7: Leaking Underground Storage Tank Case Listing

Leaking Underground Storage Tank locations. Impenal, Riverside, San Diego, Santa Barbara counties.

Date of Government Version: 02/26/2004 Date Data Arrived at EDR: 02/26/2004 Date Made Active in Reports: 03/24/2004 Number of Days to Update: 27 Source: California Regional Water Quality Control Board Colorado River Basin Region (7) Telephone: 760-776-8943

Last EDR Contact: 11/15/2007 Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: No Update Planned

CA FID UST: Facility Inventory Database

The Facility Inventory Database (FID) contains a historical listing of active and inactive underground storage tank locations from the State Water Resource Control Board. Refer to local/county source for current data.

Date of Government Version: 10/31/1994 Date Data Arrived at EDR: 09/05/1995 Date Made Active in Reports: 09/29/1995 Number of Days to Update: 24 Source: California Environmental Protection Agency

Telephone: 916-341-5851 Last EDR Contact: 12/28/1998 Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

SLIC: Statewide SLIC Cases

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 10/10/2007 Date Data Arrived at EDR: 10/11/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 27

Source: State Water Resources Control Board

Telephone: 866-480-1028 Last EDR Contact: 10/11/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Varies

SLIC REG 1: Active Toxic Site Investigations

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 04/03/2003 Date Data Arrived at EDR: 04/07/2003 Date Made Active in Reports: 04/25/2003 Number of Days to Update: 18 Source: California Regional Water Quality Control Board, North Coast Region (1) Telephone: 707-576-2220

Last EDR Contact: 11/15/2007 Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: No Update Planned

SLIC REG 2: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 09/30/2004 Date Data Arrived at EDR: 10/20/2004 Date Made Active in Reports: 11/19/2004

Number of Days to Update: 30

Source: Regional Water Quality Control Board San Francisco Bay Region (2)

Telephone: 510-286-0457 Last EDR Contact: 10/09/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Quarterly

SLIC REG 3: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality

from spills, leaks, and similar discharges.

Date of Government Version: 05/18/2006 Date Data Arrived at EDR: 05/18/2006 Date Made Active in Reports: 06/15/2006

Number of Days to Update: 28

Source: California Regional Water Quality Control Board Central Coast Region (3)

Telephone: 805-549-3147 Last EDR Contact: 11/13/2007

Next Scheduled EDR Contact: 02/11/2008 Data Release Frequency: Semi-Annually

SLIC REG 4: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 11/17/2004 Date Data Arrived at EDR: 11/18/2004 Date Made Active in Reports: 01/04/2005

Number of Days to Update: 47

Source: Region Water Quality Control Board Los Angeles Region (4)

Telephone: 213-576-6600 Last EDR Contact: 10/19/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Varies

SLIC REG 5: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 04/01/2005 Date Data Arrived at EDR: 04/05/2005 Date Made Active in Reports: 04/21/2005

Number of Days to Update: 16

Source: Regional Water Quality Control Board Central Valley Region (5)

Telephone: 916-464-3291 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Semi-Annually

SLIC REG 6V: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 05/24/2005 Date Data Arrived at EDR: 05/25/2005 Date Made Active in Reports: 06/16/2005

Number of Days to Update: 22

Source: Regional Water Quality Control Board, Victorville Branch

Telephone: 619-241-6583 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Semi-Annually

SLIC REG 6L: SLIC Sites

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 09/07/2004 Date Data Arrived at EDR: 09/07/2004 Date Made Active in Reports: 10/12/2004

Number of Days to Update: 35

Source: California Regional Water Quality Control Board, Lahontan Region

Telephone: 530-542-5574 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: No Update Planned

SLIC REG 7: SLIC List

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality from spills, leaks, and similar discharges.

Date of Government Version: 11/24/2004 Date Data Arrived at EDR: 11/29/2004 Date Made Active in Reports: 01/04/2005

Number of Days to Update: 36

Source: California Regional Quality Control Board, Colorado River Basin Region

Telephone: 760-346-7491 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008

Data Release Frequency: No Update Planned

SLIC REG 8: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality

from spills, leaks, and similar discharges.

Date of Government Version: 10/02/2007 Date Data Arrived at EDR: 10/03/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 35

Source: California Region Water Quality Control Board Santa Ana Region (8)

Telephone: 951-782-3298 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Semi-Annually

SLIC REG 9: Spills, Leaks, Investigation & Cleanup Cost Recovery Listing

The SLIC (Spills, Leaks, Investigations and Cleanup) program is designed to protect and restore water quality

from spills, leaks, and similar discharges.

Date of Government Version: 09/10/2007 Date Data Arrived at EDR: 09/11/2007 Date Made Active in Reports: 09/28/2007

Number of Days to Update: 17

Source: California Regional Water Quality Control Board San Diego Region (9)

Telephone: 858-467-2980 Last EDR Contact: 11/26/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: Annually

UST: Active UST Facilities

Active UST facilities gathered from the local regulatory agencies

Date of Government Version: 10/10/2007 Date Data Arrived at EDR: 10/11/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 21

Source: SWRCB Telephone: 916-480-1028

Last EDR Contact: 10/11/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Semi-Annually

UST MENDOCINO: Mendocino County UST Database

A listing of underground storage tank locations in Mendocino County.

Date of Government Version: 09/25/2007 Date Data Arrived at EDR: 09/25/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 37

Source: Department of Public Health

Telephone: 707-463-4466 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Varies

HIST UST: Hazardous Substance Storage Container Database

The Hazardous Substance Storage Container Database is a historical listing of UST sites. Refer to local/county source for current data.

Date of Government Version: 10/15/1990 Date Data Arrived at EDR: 01/25/1991 Date Made Active in Reports: 02/12/1991

Number of Days to Update: 18

Source: State Water Resources Control Board

Telephone: 916-341-5851 Last EDR Contact: 07/26/2001 Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

LIENS: Environmental Liens Listing

A listing of property locations with environmental liens for California where DTSC is a lien holder.

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 08/28/2007 Date Made Active in Reports: 09/26/2007

Number of Days to Update: 29

Source: Department of Toxic Substances Control

Telephone: 916-323-3400 Last EDR Contact: 11/05/2007

Next Scheduled EDR Contact: 02/04/2008

Data Release Frequency: Varies

AST: Aboveground Petroleum Storage Tank Facilities

Registered Aboveground Storage Tanks.

Date of Government Version: 09/17/2007 Date Data Arrived at EDR: 09/18/2007 Date Made Active in Reports: 11/01/2007 Number of Days to Update: 44

Source: State Water Resources Control Board

Telephone: 916-341-5712 Last EDR Contact: 11/13/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: Quarterly

SWEEPS UST: SWEEPS UST Listing

Statewide Environmental Evaluation and Planning System. This underground storage tank listing was updated and maintained by a company contacted by the SWRCB in the early 1980's. The listing is no longer updated or maintained. The local agency is the contact for more information on a site on the SWEEPS list.

Date of Government Version: 06/01/1994 Date Data Arrived at EDR: 07/07/2005 Date Made Active in Reports: 08/11/2005 Number of Days to Update: 35

Source: State Water Resources Control Board

Telephone: N/A

Last EDR Contact: 06/03/2005 Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

CHMIRS: California Hazardous Material Incident Report System

California Hazardous Material Incident Reporting System. CHMIRS contains information on reported hazardous material incidents (accidental releases or spills).

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 02/23/2007 Date Made Active in Reports: 04/06/2007 Number of Days to Update: 42

Source: Office of Emergency Services Telephone: 916-845-8400 Last EDR Contact: 11/15/2007 Next Scheduled EDR Contact: 02/18/2008

Data Release Frequency: Varies

NOTIFY 65: Proposition 65 Records

Number of Days to Update: 18

Proposition 65 Notification Records. NOTIFY 65 contains facility notifications about any release which could impact drinking water and thereby expose the public to a potential health risk.

Date of Government Version: 10/21/1993 Date Data Arrived at EDR: 11/01/1993 Date Made Active in Reports: 11/19/1993

Telephone: 916-445-3846 Last EDR Contact: 10/15/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: No Update Planned

Source: State Water Resources Control Board

DEED: Deed Restriction Listing

Site Mitigation and Brownfields Reuse Program Facility Sites with Deed Restrictions & Hazardous Waste Management Program Facility Sites with Deed / Land Use Restriction. The DTSC Site Mitigation and Brownfields Reuse Program (SMBRP) list includes sites cleaned up under the program's oversight and generally does not include current or former hazardous waste facilities that required a hazardous waste facility permit. The list represents deed restrictions that are active. Some sites have multiple deed restrictions. The DTSC Hazardous Waste Management Program (HWMP) has developed a list of current or former hazardous waste facilities that have a recorded land use restriction at the local county recorder's office. The land use restrictions on this list were required by the DTSC HWMP as a result of the presence of hazardous substances that remain on site after the facility (or part of the facility) has been closed or cleaned up. The types of land use restriction include deed notice, deed restriction, or a land use restriction that binds current and future owners.

Date of Government Version: 10/02/2007 Date Data Arrived at EDR: 10/03/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 35

Source: Department of Toxic Substances Control

Telephone: 916-323-3400 Last EDR Contact: 10/03/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Semi-Annually

VCP: Voluntary Cleanup Program Properties

Contains low threat level properties with either confirmed or unconfirmed releases and the project proponents have request that DTSC oversee investigation and/or cleanup activities and have agreed to provide coverage for DTSC's costs.

Date of Government Version: 08/28/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 09/26/2007 Number of Days to Update: 28

Source: Department of Toxic Substances Control Telephone: 916-323-3400 Last EDR Contact: 11/28/2007 Next Scheduled EDR Contact: 02/25/2008

Data Release Frequency: Quarterly

DRYCLEANERS: Cleaner Facilities

A list of drycleaner related facilities that have EPA ID numbers. These are facilities with certain SIC codes: power laundries, family and commercial; garment pressing and cleaner's agents; linen supply; coin-operated laundries and cleaning; drycleaning plants, except rugs; carpet and upholster cleaning; industrial launderers; laundry and garment services.

Date of Government Version: 07/31/2007 Date Data Arrived at EDR: 07/31/2007 Date Made Active in Reports: 08/09/2007 Source: Department of Toxic Substance Control Telephone: 916-327-4498

Last EDR Contact: 10/15/2007

Number of Days to Update: 9

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Annually

WIP: Well Investigation Program Case List

Well Investigation Program case in the San Gabriel and San Fernando Valley area.

Date of Government Version: 09/30/2007 Date Data Arrived at EDR: 10/31/2007 Date Made Active in Reports: 11/07/2007 Source: Los Angeles Water Quality Control Board Telephone: 213-576-6726

Last EDR Contact: 10/23/2007

Number of Days to Update: 7

Next Scheduled EDR Contact: 01/21/2008

Data Release Frequency: Varies

CDL: Clandestine Drug Labs

A listing of drug lab locations. Listing of a location in this database does not indicate that any illegal drug lab materials were or were not present there, and does not constitute a determination that the location either requires or does not require additional cleanup work.

Date of Government Version: 09/30/2007 Date Data Arrived at EDR: 10/15/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 23

Source: Department of Toxic Substances Control

Telephone: 916-255-6504 Last EDR Contact: 10/15/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Varies

RESPONSE: State Response Sites

Identifies confirmed release sites where DTSC is involved in remediation, either in a lead or oversight capacity. These confirmed release sites are generally high-priority and high potential risk.

Date of Government Version: 08/28/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 09/26/2007 Number of Days to Update: 28

Source: Department of Toxic Substances Control

Telephone: 916-323-3400 Last EDR Contact: 11/28/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: Quarterly

HAZNET: Facility and Manifest Data

Facility and Manifest Data. The data is extracted from the copies of hazardous waste manifests received each year by the DTSC. The annual volume of manifests is typically 700,000 - 1,000,000 annually, representing approximately 350,000 - 500,000 shipments. Data are from the manifests submitted without correction, and therefore many contain some invalid values for data elements such as generator ID, TSD ID, waste category, and disposal method.

Date of Government Version: 12/31/2006 Date Data Arrived at EDR: 10/04/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 34

Source: California Environmental Protection Agency

Telephone: 916-255-1136 Last EDR Contact: 11/07/2007

Next Scheduled EDR Contact: 02/04/2008 Data Release Frequency: Annually

EMI: Emissions Inventory Data

Toxics and criteria pollutant emissions data collected by the ARB and local air pollution agencies.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 04/17/2007 Date Made Active in Reports: 05/10/2007

Number of Days to Update: 23

Source: California Air Resources Board

Telephone: 916-322-2990 Last EDR Contact: 10/18/2007

Next Scheduled EDR Contact: 01/14/2008

Data Release Frequency: Varies

HAULERS: Registered Waste Tire Haulers Listing A listing of registered waste tire haulers.

> Date of Government Version: 09/17/2007 Date Data Arrived at EDR: 09/18/2007 Date Made Active in Reports: 09/28/2007

Number of Days to Update: 10

ENVIROSTOR: EnviroStor Database

Source: Integrated Waste Management Board

Telephone: 916-341-6422

Last EDR Contact: 09/10/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Varies

The Department of Toxic Substances Control's (DTSC's) Site Mitigation and Brownfields Reuse Program's (SMBRP's) EnviroStor database identifies sites that have known contamination or sites for which there may be reasons to investigate further. The database includes the following site types: Federal Superfund sites (National Priorities List (NPL)); State Response, including Military Facilities and State Superfund; Voluntary Cleanup; and School sites. EnviroStor provides similar information to the information that was available in CalSites, and provides additional site information, including, but not limited to, identification of formerly-contaminated properties that have been released for reuse, properties where environmental deed restrictions have been recorded to prevent inappropriate land uses,

and risk characterization information that is used to assess potential impacts to public health and the environment at contaminated sites.

Date of Government Version: 08/28/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 09/26/2007

Number of Days to Update: 28

Source: Department of Toxic Substances Control

Telephone: 916-323-3400 Last EDR Contact: 11/28/2007

Next Scheduled EDR Contact: 02/25/2008

Data Release Frequency: Quarterly

TRIBAL RECORDS

INDIAN RESERV: Indian Reservations

This map layer portrays Indian administered lands of the United States that have any area equal to or greater than 640 acres.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 12/08/2006 Date Made Active in Reports: 01/11/2007

Number of Days to Update: 34

Source: USGS

Telephone: 202-208-3710 Last EDR Contact: 11/09/2007

Next Scheduled EDR Contact: 02/04/2008 Data Release Frequency: Semi-Annually

INDIAN LUST R7: Leaking Underground Storage Tanks on Indian Land

LUSTs on Indian land in Iowa, Kansas, and Nebraska

Date of Government Version: 06/01/2007 Date Data Arrived at EDR: 06/14/2007 Date Made Active in Reports: 07/05/2007 Number of Days to Update: 21

Source: EPA Region 7 Telephone: 913-551-7003 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Varies

INDIAN LUST R6: Leaking Underground Storage Tanks on Indian Land LUSTs on Indian land in New Mexico and Oklahoma.

Date of Government Version: 01/04/2005 Date Data Arrived at EDR: 01/21/2005 Date Made Active in Reports: 02/28/2005 Number of Days to Update: 38 Source: EPA Region 6 Telephone: 214-665-6597 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008

Data Release Frequency: Varies

INDIAN LUST R8: Leaking Underground Storage Tanks on Indian Land

LUSTs on Indian land in Colorado, Montana, North Dakota, South Dakota, Utah and Wyoming.

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 09/07/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 34

Source: EPA Region 8 Telephone: 303-312-6271 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN LUST R4: Leaking Underground Storage Tanks on Indian Land LUSTs on Indian land in Florida, Mississippi and North Carolina.

Date of Government Version: 09/05/2007 Date Data Arrived at EDR: 10/02/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 9

Source: EPA Region 4 Telephone: 404-562-8677 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Semi-Annually

INDIAN LUST R1: Leaking Underground Storage Tanks on Indian Land A listing of leaking underground storage tank locations on Indian Land.

Date of Government Version: 12/01/2006 Date Data Arrived at EDR: 12/01/2006 Date Made Active in Reports: 01/29/2007

Number of Days to Update: 59

Source: EPA Region 1 Telephone: 617-918-1313 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Varies

INDIAN LUST R10: Leaking Underground Storage Tanks on Indian Land LUSTs on Indian land in Alaska, Idaho, Oregon and Washington.

Date of Government Version: 09/12/2007 Date Data Arrived at EDR: 09/14/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 27

Source: EPA Region 10 Telephone: 206-553-2857 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN LUST R9: Leaking Underground Storage Tanks on Indian Land LUSTs on Indian land in Arizona, California, New Mexico and Nevada

Date of Government Version: 09/11/2007 Date Data Arrived at EDR: 09/14/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 27

Source: Environmental Protection Agency

Telephone: 415-972-3372 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN UST R7: Underground Storage Tanks on Indian Land

Date of Government Version: 06/01/2007 Date Data Arrived at EDR: 06/14/2007 Date Made Active in Reports: 07/05/2007

Number of Days to Update: 21

Source: EPA Region 7 Telephone: 913-551-7003 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Varies

INDIAN UST R6: Underground Storage Tanks on Indian Land

Date of Government Version: 08/31/2007 Date Data Arrived at EDR: 08/31/2007 Date Made Active in Reports: 10/11/2007 Number of Days to Update: 41

Source: EPA Region 6 Telephone: 214-665-7591 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Semi-Annually

INDIAN UST R9: Underground Storage Tanks on Indian Land

Date of Government Version: 09/11/2007 Date Data Arrived at EDR: 09/14/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 27

Source: EPA Region 9 Telephone: 415-972-3368 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN UST R1: Underground Storage Tanks on Indian Land

A listing of underground storage tank locations on Indian Land.

Date of Government Version: 12/01/2006 Date Data Arrived at EDR: 12/01/2006 Date Made Active in Reports: 01/29/2007

Number of Days to Update: 59

Source: EPA, Region 1 Telephone: 617-918-1313 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Varies

INDIAN UST R8: Underground Storage Tanks on Indian Land

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 09/07/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 34

Source: EPA Region 8 Telephone: 303-312-6137 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN UST R4: Underground Storage Tanks on Indian Land

Date of Government Version: 09/05/2007 Date Data Arrived at EDR: 10/02/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 9

Source: EPA Region 4 Telephone: 404-562-9424 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Semi-Annually

INDIAN UST R10: Underground Storage Tanks on Indian Land

Date of Government Version: 09/12/2007 Date Data Arrived at EDR: 09/14/2007 Date Made Active in Reports: 10/11/2007

Number of Days to Update: 27

Source: EPA Region 10 Telephone: 206-553-2857 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Quarterly

INDIAN UST R5: Underground Storage Tanks on Indian Land

Date of Government Version: 12/02/2004 Date Data Arrived at EDR: 12/29/2004 Date Made Active in Reports: 02/04/2005

Number of Days to Update: 37

Source: EPA Region 5 Telephone: 312-886-6136 Last EDR Contact: 11/15/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Varies

EDR PROPRIETARY RECORDS

Manufactured Gas Plants: EDR Proprietary Manufactured Gas Plants

The EDR Proprietary Manufactured Gas Plant Database includes records of coal gas plants (manufactured gas plants) compiled by EDR's researchers. Manufactured gas sites were used in the United States from the 1800's to 1950's to produce a gas that could be distributed and used as fuel. These plants used whale oil, rosin, coal, or a mixture of coal, oil, and water that also produced a significant amount of waste. Many of the byproducts of the gas production, such as coal tar (oily waste containing volatile and non-volatile chemicals), sludges, oils and other compounds are potentially hazardous to human health and the environment. The byproduct from this process was frequently disposed of directly at the plant site and can remain or spread slowly, serving as a continuous source of soil and groundwater contamination.

Date of Government Version: N/A
Date Data Arrived at EDR: N/A
Date Made Active in Reports: N/A
Number of Days to Update: N/A

Source: EDR, Inc. Telephone: N/A Last EDR Contact: N/A

Next Scheduled EDR Contact: N/A

Data Release Frequency: No Update Planned

EDR Historical Auto Stations: EDR Proprietary Historic Gas Stations

EDR has searched selected national collections of business directories and has collected listings of potential gas station/filling station/service station sites that were available to EDR researchers. EDR's review was limited to those categories of sources that might, in EDR's opinion, include gas station/filling station/service station establishments. The categories reviewed included, but were not limited to gas, gas station, gasoline station, filling station, auto, automobile repair, auto service station, service station, etc.

Date of Government Version: N/A Date Data Arrived at EDR: N/A Date Made Active in Reports: N/A Number of Days to Update: N/A

Source: EDR, Inc. Telephone: N/A Last EDR Contact: N/A

Next Scheduled EDR Contact: N/A Data Release Frequency: Varies

EDR Historical Cleaners: EDR Proprietary Historic Dry Cleaners

EDR has searched selected national collections of business directories and has collected listings of potential dry cleaner sites that were available to EDR researchers. EDR's review was limited to those categories of sources that might, in EDR's opinion, include dry cleaning establishments. The categories reviewed included, but were not limited to dry cleaners, cleaners, laundry, laundromat, cleaning/laundry, wash & dry etc.

Date of Government Version: N/A Date Data Arrived at EDR: N/A Date Made Active in Reports: N/A Number of Days to Update: N/A Source: EDR, Inc. Telephone: N/A Last EDR Contact: N/A

Next Scheduled EDR Contact: N/A Data Release Frequency: Varies

FEDERAL RECORDS

COLLEGES: Integrated Postsecondary Education Data

The National Center for Education Statistics' primary database on integrated postsecondary education in the United States.

Date of Government Version: N/A
Date Data Arrived at EDR: 10/12/2005
Date Made Active in Reports: N/A
Number of Days to Update: 0

Source: National Center for Education Statistics

Telephone: 202-502-7300 Last EDR Contact: 09/22/2006 Next Scheduled EDR Contact: N/A Data Release Frequency: N/A

PUBLIC SCHOOLS: Public Schools

The National Center for Education Statistics' primary database on elementary and secondary public education in the United States. It is a comprehensive, annual, national statistical database of all public elementary and secondary schools and school districts, which contains data that are comparable across all states.

Date of Government Version: N/A
Date Data Arrived at EDR: 07/13/2004
Date Made Active in Reports: N/A
Number of Days to Update: 0

Source: National Center for Education statistics

Telephone: 202-502-7300 Last EDR Contact: 10/10/2007

Next Scheduled EDR Contact: 01/07/2008

Data Release Frequency: N/A

PRIVATE SCHOOLS: Private Schools of the United States

The National Center for Education Statistics' primary database on private school locations in the United States.

Date of Government Version: N/A
Date Data Arrived at EDR: 10/07/2005
Date Made Active in Reports: N/A
Number of Days to Update: 0

Source: National Center for Education Statistics

Telephone: 202-502-7300 Last EDR Contact: 09/22/2006 Next Scheduled EDR Contact: N/A Data Release Frequency: N/A

NURSING HOMES: Directory of Nursing Homes

Information on Medicare and Medicaid certified nursing homes in the United States.

Date of Government Version: N/A Date Data Arrived at EDR: 10/11/2005

Date Made Active in Reports: N/A Number of Days to Update: 0

Source: N/A

Telephone: 800-568-3282 Last EDR Contact: 09/22/2006 Next Scheduled EDR Contact: N/A Data Release Frequency: N/A

MEDICAL CENTERS: Provider of Services Listing

A listing of hospitals with Medicare provider number, produced by Centers of Medicare & Medicaid Services, a federal agency within the U.S. Department of Health & Human Services.

Date of Government Version: 06/01/1998 Date Data Arrived at EDR: 11/10/2005 Date Made Active in Reports: N/A Number of Days to Update: 0

Source: Centers for Medicare & Medicaid Services Telephone: 410-786-3000

Last EDR Contact: 01/12/2007 Next Scheduled EDR Contact: N/A Data Release Frequency: N/A

HOSPITALS: AHA Hospital Guide

The database includes a listing of hospitals based on the American Hospital Association's annual survey of hospitals.

Date of Government Version: N/A Date Data Arrived at EDR: 10/19/1994 Date Made Active in Reports: N/A Number of Days to Update: 0

Source: American Hospital Association Telephone: 800-242-2626 Last EDR Contact: 09/22/2006 Next Scheduled EDR Contact: N/A

Data Release Frequency: N/A

COUNTY RECORDS

ALAMEDA COUNTY:

Contaminated Sites

A listing of contaminated sites overseen by the Toxic Release Program (oil and groundwater contamination from chemical releases and spills) and the Leaking Underground Storage Tank Program (soil and ground water contamination from leaking petroleum USTs).

Date of Government Version: 10/22/2007 Date Data Arrived at EDR: 10/23/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 15

Source: Alameda County Environmental Health Services

Telephone: 510-567-6700 Last EDR Contact: 10/22/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Semi-Annually

Underground Tanks

Underground storage tank sites located in Alameda county.

Date of Government Version: 08/03/2007 Date Data Arrived at EDR: 08/07/2007 Date Made Active in Reports: 09/24/2007

Number of Days to Update: 48

Source: Alameda County Environmental Health Services

Telephone: 510-567-6700 Last EDR Contact: 11/05/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Semi-Annually

CONTRA COSTA COUNTY:

List includes sites from the underground tank, hazardous waste generator and business plan/2185 programs.

Date of Government Version: 09/11/2007 Date Data Arrived at EDR: 09/14/2007 Date Made Active in Reports: 09/28/2007 Number of Days to Update: 14

Source: Contra Costa Health Services Department Telephone: 925-646-2286

Last EDR Contact: 11/26/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: Semi-Annually

FRESNO COUNTY:

CUPA Resources List

Certified Unified Program Agency. CUPA's are responsible for implementing a unified hazardous materials and hazardous waste management regulatory program. The agency provides oversight of businesses that deal with hazardous materials, operate underground storage tanks or aboveground storage tanks.

Date of Government Version: 10/09/2007 Date Data Arrived at EDR: 10/10/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 28

Source: Dept. of Community Health Telephone: 559-445-3271 Last EDR Contact: 11/05/2007

Next Scheduled EDR Contact: 02/04/2008 Data Release Frequency: Semi-Annually

KERN COUNTY:

Underground Storage Tank Sites & Tank Listing

Kem County Sites and Tanks Listing.

Date of Government Version: 10/03/2007 Date Data Arrived at EDR: 10/04/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 28

Source: Kern County Environment Health Services Department

Telephone: 661-862-8700 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008
Data Release Frequency: Quarterly

LOS ANGELES COUNTY:

San Gabriel Valley Areas of Concern

San Gabnel Valley areas where VOC contamination is at or above the MCL as designated by region 9 EPA office.

Date of Government Version: 12/31/1998 Date Data Arrived at EDR: 07/07/1999 Date Made Active in Reports: N/A

Number of Days to Update: 0

Source: EPA Region 9 Telephone: 415-972-3178 Last EDR Contact: 07/16/2007

Next Scheduled EDR Contact: 10/15/2007 Data Release Frequency: No Update Planned

HMS: Street Number List

Industrial Waste and Underground Storage Tank Sites.

Date of Government Version: 07/11/2007 Date Data Arrived at EDR: 10/23/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 15

Source: Department of Public Works Telephone: 626-458-3517 Last EDR Contact: 11/13/2007

Next Scheduled EDR Contact: 02/11/2008 Data Release Frequency: Semi-Annually

List of Solid Waste Facilities

Solid Waste Facilities in Los Angeles County.

Date of Government Version: 08/17/2007 Date Data Arrived at EDR: 09/24/2007 Date Made Active in Reports: 09/28/2007

Number of Days to Update: 4

Source: La County Department of Public Works

Telephone: 818-458-5185 Last EDR Contact: 11/14/2007

Next Scheduled EDR Contact: 02/11/2008

Data Release Frequency: Varies

City of Los Angeles Landfills

Landfills owned and maintained by the City of Los Angeles.

Date of Government Version: 03/01/2007 Date Data Arrived at EDR: 03/27/2007 Date Made Active in Reports: 04/27/2007

Number of Days to Update: 31

Source: Engineering & Construction Division

Telephone: 213-473-7869 Last EDR Contact: 09/10/2007

Next Scheduled EDR Contact: 12/10/2007

Data Release Frequency: Varies

Site Mitigation List

Industrial sites that have had some sort of spill or complaint.

Date of Government Version: 05/30/2007 Date Data Arrived at EDR: 07/11/2007 Date Made Active in Reports: 08/09/2007

Number of Days to Update: 29

Source: Community Health Services Telephone: 323-890-7806 Last EDR Contact; 11/13/2007 Next Scheduled EDR Contact: 02/11/2008

Data Release Frequency: Annually

City of El Segundo Underground Storage Tank

Underground storage tank sites located in El Segundo city.

Date of Government Version: 08/13/2007 Date Data Arrived at EDR: 09/24/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 38

Source: City of El Segundo Fire Department

Telephone: 310-524-2236 Last EDR Contact: 11/13/2007

Next Scheduled EDR Contact: 02/11/2008 Data Release Frequency: Semi-Annually

City of Long Beach Underground Storage Tank

Underground storage tank sites located in the city of Long Beach.

Date of Government Version: 03/28/2003 Date Data Arrived at EDR: 10/23/2003 Date Made Active in Reports: 11/26/2003

Number of Days to Update: 34

Source: City of Long Beach Fire Department

Telephone: 562-570-2563 Last EDR Contact: 11/16/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Annually

City of Torrance Underground Storage Tank

Underground storage tank sites located in the city of Torrance.

Date of Government Version: 09/24/2007 Date Data Arrived at EDR: 09/25/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 37

Source: City of Torrance Fire Department

Telephone: 310-618-2973 Last EDR Contact: 11/26/2007

Next Scheduled EDR Contact: 02/11/2008 Data Release Frequency: Semi-Annually

MARIN COUNTY:

Underground Storage Tank Sites

Currently permitted USTs in Marin County.

Date of Government Version: 08/06/2007 Date Data Arrived at EDR: 09/24/2007 Date Made Active in Reports: 11/01/2007 Number of Days to Update: 38

Source: Public Works Department Waste Management

Telephone: 415-499-6647 Last EDR Contact: 11/29/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: Semi-Annually

NAPA COUNTY:

Sites With Reported Contamination

A listing of leaking underground storage tank sites located in Napa county.

Date of Government Version: 09/24/2007 Date Data Arrived at EDR: 09/25/2007 Date Made Active in Reports: 09/28/2007 Number of Days to Update: 3

Source: Napa County Department of Environmental Management Telephone: 707-253-4269 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Semi-Annually

Closed and Operating Underground Storage Tank Sites

Underground storage tank sites located in Napa county.

Date of Government Version: 09/24/2007 Date Data Arrived at EDR: 09/25/2007 Date Made Active in Reports: 11/01/2007 Number of Days to Update: 37

Telephone: 707-253-4269 Last EDR Contact: 09/24/2007

Source: Napa County Department of Environmental Management

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Annually

ORANGE COUNTY:

List of Industrial Site Cleanups

Petroleum and non-petroleum spills.

Date of Government Version: 08/01/2007 Date Data Arrived at EDR: 09/28/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 40

Telephone: 714-834-3446 Last EDR Contact: 12/06/2007 Next Scheduled EDR Contact: 03/03/2008

Source: Health Care Agency

Data Release Frequency: Annually

List of Underground Storage Tank Cleanups

Orange County Underground Storage Tank Cleanups (LUST).

Date of Government Version: 08/01/2007 Date Data Arrived at EDR: 09/28/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 40

Source: Health Care Agency Telephone: 714-834-3446 Last EDR Contact: 12/06/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

List of Underground Storage Tank Facilities

Orange County Underground Storage Tank Facilities (UST).

Date of Government Version: 08/01/2007 Date Data Arrived at EDR: 09/25/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 37

Source: Health Care Agency Telephone: 714-834-3446 Last EDR Contact: 12/06/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

PLACER COUNTY:

Master List of Facilities

List includes aboveground tanks, underground tanks and cleanup sites.

Date of Government Version: 07/23/2007 Date Data Arrived at EDR: 07/23/2007 Date Made Active in Reports: 08/09/2007

Number of Days to Update: 17

Source: Placer County Health and Human Services

Telephone: 530-889-7312 Last EDR Contact: 09/17/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: Semi-Annually

RIVERSIDE COUNTY:

Listing of Underground Tank Cleanup Sites

Riverside County Underground Storage Tank Cleanup Sites (LUST).

Date of Government Version: 08/06/2007 Date Data Arrived at EDR: 08/07/2007 Date Made Active in Reports: 09/26/2007 Number of Days to Update: 50

Source: Department of Public Health Telephone: 951-358-5055 Last EDR Contact: 10/15/2007 Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Quarterly

Underground Storage Tank Tank List

Underground storage tank sites located in Riverside county.

Date of Government Version: 08/06/2007 Date Data Arrived at EDR: 08/07/2007 Date Made Active in Reports: 09/24/2007 Number of Days to Update: 48

Source: Health Services Agency Telephone: 951-358-5055 Last EDR Contact: 10/15/2007 Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Quarterly

SACRAMENTO COUNTY:

Contaminated Sites

List of sites where unauthorized releases of potentially hazardous materials have occurred.

Date of Government Version: 10/29/2007 Date Data Arrived at EDR: 10/30/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 8

Source: Sacramento County Environmental Management

Telephone: 916-875-8406 Last EDR Contact: 10/26/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: Quarterly

ML - Regulatory Compliance Master List

Any business that has hazardous materials on site - hazardous material storage sites, underground storage tanks, waste generators.

Date of Government Version: 10/29/2007 Date Data Arrived at EDR: 10/30/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 8

Source: Sacramento County Environmental Management

Telephone: 916-875-8406 Last EDR Contact: 10/26/2007

Next Scheduled EDR Contact: 01/28/2008 Data Release Frequency: Quarterly

SAN BERNARDINO COUNTY:

Hazardous Material Permits

This listing includes underground storage tanks, medical waste handlers/generators, hazardous materials handlers, hazardous waste generators, and waste oil generators/handlers.

Date of Government Version: 09/27/2007 Date Data Arrived at EDR: 09/28/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 40 Source: San Bernardino County Fire Department Hazardous Materials Division Telephone: 909-387-3041

Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 12/03/2007 Data Release Frequency: Quarterly

SAN DIEGO COUNTY:

Hazardous Materials Management Division Database

The database includes: HE58 - This report contains the business name, site address, business phone number, establishment 'H' permit number, type of permit, and the business status. HE17 - In addition to providing the same information provided in the HE58 listing, HE17 provides inspection dates, violations received by the establishment, hazardous waste generated, the quantity, method of storage, treatment/disposal of waste and the hauler, and information on underground storage tanks. Unauthorized Release List - Includes a summary of environmental contamination cases in San Diego County (underground tank cases, non-tank cases, groundwater contamination, and soil contamination are included.)

Date of Government Version: 05/16/2005 Date Data Arrived at EDR: 05/18/2005 Date Made Active in Reports: 06/16/2005 Number of Days to Update: 29

Source: Hazardous Materials Management Division

Telephone: 619-338-2268 Last EDR Contact: 10/05/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Quarterly

Solid Waste Facilities

San Diego County Solid Waste Facilities.

Date of Government Version: 11/01/2006 Date Data Arrived at EDR: 01/03/2007 Date Made Active in Reports: 01/24/2007

Number of Days to Update: 21

Source: Department of Health Services

Telephone: 619-338-2209 Last EDR Contact: 11/19/2007

Next Scheduled EDR Contact: 02/18/2008

Data Release Frequency: Varies

Environmental Case Listing

The listing contains all underground tank release cases and projects pertaining to properties contaminated with hazardous substances that are actively under review by the Site Assessment and Mitigation Program.

Date of Government Version: 08/22/2007 Date Data Arrived at EDR: 10/03/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 35

Source: San Diego County Department of Environmental Health

Telephone: 619-338-2371 Last EDR Contact: 10/03/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Varies

SAN FRANCISCO COUNTY:

Local Oversite Facilities

A listing of leaking underground storage tank sites located in San Francisco county.

Date of Government Version: 09/07/2007 Date Data Arrived at EDR: 09/07/2007 Date Made Active in Reports: 09/28/2007

Number of Days to Update: 21

Source: Department Of Public Health San Francisco County

Telephone: 415-252-3920 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

Underground Storage Tank Information

Underground storage tank sites located in San Francisco county.

Date of Government Version: 09/07/2007 Date Data Arrived at EDR: 09/07/2007 Date Made Active in Reports: 09/24/2007

Number of Days to Update: 17

Source: Department of Public Health

Telephone: 415-252-3920 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Quarterly

SAN JOAQUIN COUNTY:

San Joaquin Co. UST

A listing of underground storage tank locations in San Joaquin county.

Date of Government Version: 08/21/2007 Date Data Arrived at EDR: 08/22/2007 Date Made Active in Reports: 09/24/2007

Number of Days to Update: 33

Source: Environmental Health Department

Telephone: N/A

Last EDR Contact: 10/15/2007

Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Semi-Annually

SAN MATEO COUNTY:

Business Inventory

List includes Hazardous Materials Business Plan, hazardous waste generators, and underground storage tanks.

Date of Government Version: 10/24/2007 Date Data Arrived at EDR: 10/25/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 13

Source: San Mateo County Environmental Health Services Division

Telephone: 650-363-1921 Last EDR Contact: 10/09/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Annually

Fuel Leak List

A listing of leaking underground storage tank sites located in San Mateo county.

Date of Government Version: 10/09/2007 Date Data Arrived at EDR: 10/10/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 28

Source: San Mateo County Environmental Health Services Division

Telephone: 650-363-1921 Last EDR Contact: 10/09/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Semi-Annually

SANTA CLARA COUNTY:

HIST LUST - Fuel Leak Site Activity Report

A listing of open and closed leaking underground storage tanks. This listing is no longer updated by the county. Leaking underground storage tanks are now handled by the Department of Environmental Health.

Date of Government Version: 03/29/2005 Date Data Arrived at EDR: 03/30/2005 Date Made Active in Reports: 04/21/2005

Number of Days to Update: 22

Source: Santa Clara Valley Water District

Telephone: 408-265-2600 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: No Update Planned

LOP Listing

A listing of leaking underground storage tanks located in Santa Clara county.

Date of Government Version: 03/26/2007 Date Data Arrived at EDR: 03/27/2007 Date Made Active in Reports: 04/27/2007

Number of Days to Update: 31

Source: Department of Environmental Health

Telephone: 408-918-3417 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Varies

Hazardous Material Facilities

Hazardous material facilities, including underground storage tank sites.

Date of Government Version: 09/17/2007 Date Data Arrived at EDR: 09/17/2007 Date Made Active in Reports: 09/28/2007

Number of Days to Update: 11

Source: City of San Jose Fire Department

Telephone: 408-277-4659 Last EDR Contact: 12/03/2007

Next Scheduled EDR Contact: 03/03/2008 Data Release Frequency: Annually

SOLANO COUNTY:

Leaking Underground Storage Tanks

A listing of leaking underground storage tank sites located in Solano county.

Date of Government Version: 09/24/2007 Date Data Arrived at EDR: 10/23/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 15

Source: Solano County Department of Environmental Management

Telephone: 707-784-6770 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Quarterly

Underground Storage Tanks

Underground storage tank sites located in Solano county.

Date of Government Version: 09/24/2007 Date Data Arrived at EDR: 10/23/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 9

Source: Solano County Department of Environmental Management

Telephone: 707-784-6770 Last EDR Contact: 09/24/2007

Next Scheduled EDR Contact: 12/24/2007 Data Release Frequency: Quarterly

SONOMA COUNTY:

Leaking Underground Storage Tank Sites

A listing of leaking underground storage tank sites located in Sonoma county.

Date of Government Version: 10/22/2007 Date Data Arrived at EDR: 10/23/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 15

Source: Department of Health Services

Telephone: 707-565-6565 Last EDR Contact: 10/22/2007

Next Scheduled EDR Contact: 01/21/2008 Data Release Frequency: Quarterly

SUTTER COUNTY:

Underground Storage Tanks

Underground storage tank sites located in Sutter county.

Date of Government Version: 05/04/2007 Date Data Arrived at EDR: 05/04/2007 Date Made Active in Reports: 05/24/2007

Number of Days to Update: 20

Source: Sutter County Department of Agriculture

Telephone: 530-822-7500 Last EDR Contact: 10/01/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Semi-Annually

VENTURA COUNTY:

Business Plan, Hazardous Waste Producers, and Operating Underground Tanks

The BWT list indicates by site address whether the Environmental Health Division has Business Plan (B), Waste Producer (W), and/or Underground Tank (T) information.

Date of Government Version: 08/24/2007 Date Data Arrived at EDR: 10/04/2007 Date Made Active in Reports: 11/07/2007 Number of Days to Update: 34

Source: Ventura County Environmental Health Division Telephone: 805-654-2813

Last EDR Contact: 09/12/2007 Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Quarterly

Inventory of Illegal Abandoned and Inactive Sites

Ventura County Inventory of Closed, Illegal Abandoned, and Inactive Sites.

Date of Government Version: 08/01/2007 Date Data Arrived at EDR: 08/29/2007 Date Made Active in Reports: 09/26/2007

Number of Days to Update: 28

Source: Environmental Health Division

Telephone: 805-654-2813 Last EDR Contact: 11/19/2007

Next Scheduled EDR Contact: 02/18/2008 Data Release Frequency: Annually

Listing of Underground Tank Cleanup Sites

Ventura County Underground Storage Tank Cleanup Sites (LUST).

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 10/02/2007 Date Made Active in Reports: 11/07/2007

Number of Days to Update: 36

Source: Environmental Health Division

Telephone: 805-654-2813 Last EDR Contact: 09/12/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Quarterly

Underground Tank Closed Sites List

Ventura County Operating Underground Storage Tank Sites (UST)/Underground Tank Closed Sites List.

Date of Government Version: 09/26/2007 Date Data Arrived at EDR: 10/11/2007 Date Made Active in Reports: 11/01/2007

Number of Days to Update: 21

Source: Environmental Health Division

Telephone: 805-654-2813 Last EDR Contact: 10/11/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Quarterly

YOLO COUNTY:

Underground Storage Tank Comprehensive Facility Report

Underground storage tank sites located in Yolo county.

Date of Government Version: 07/30/2007 Date Data Arrived at EDR: 09/04/2007 Date Made Active in Reports: 09/24/2007 Number of Days to Update: 20 Source: Yolo County Department of Health Telephone: 530-666-8646 Last EDR Contact: 10/15/2007 Next Scheduled EDR Contact: 01/14/2008 Data Release Frequency: Annually

OTHER DATABASE(\$)

Depending on the geographic area covered by this report, the data provided in these specialty databases may or may not be complete. For example, the existence of wetlands information data in a specific report does not mean that all wetlands in the area covered by the report are included. Moreover, the absence of any reported wetlands information does not necessarily mean that wetlands do not exist in the area covered by the report.

CT MANIFEST: Hazardous Waste Manifest Data

Facility and manifest data. Manifest is a document that lists and tracks hazardous waste from the generator through transporters to a tsd facility.

Date of Government Version: 12/31/2005 Date Data Arrived at EDR: 06/15/2007 Date Made Active in Reports: 08/20/2007

Number of Days to Update: 66

Source: Department of Environmental Protection

Telephone: 860-424-3375 Last EDR Contact: 09/12/2007

Next Scheduled EDR Contact: 12/10/2007 Data Release Frequency: Annually

NJ MANIFEST: Manifest Information Hazardous waste manifest information.

Date of Government Version: 04/01/2007 Date Data Arrived at EDR: 04/05/2007 Date Made Active in Reports: 05/08/2007 Number of Days to Update: 33 Source: Department of Environmental Protection

Telephone: N/A

Last EDR Contact: 11/07/2007

Next Scheduled EDR Contact: 12/31/2007 Data Release Frequency: Annually

NY MANIFEST: Facility and Manifest Data

Manifest is a document that lists and tracks hazardous waste from the generator through transporters to a TSD facility.

Date of Government Version: 08/27/2007 Date Data Arrived at EDR: 08/30/2007 Date Made Active in Reports: 09/21/2007

Number of Days to Update: 22

Source: Department of Environmental Conservation

Telephone: 518-402-8651 Last EDR Contact: 11/29/2007

Next Scheduled EDR Contact: 02/25/2008 Data Release Frequency: Annually

PA MANIFEST: Manifest Information Hazardous waste manifest information.

Date of Government Version: 12/31/2006 Date Data Arrived at EDR: 08/23/2007 Date Made Active in Reports: 09/27/2007 Number of Days to Update: 35 Source: Department of Environmental Protection

Telephone: N/A

Last EDR Contact: 09/10/2007

Next Scheduled EDR Contact: 12/10/2007
Data Release Frequency: Annually

RI MANIFEST: Manifest information Hazardous waste manifest information

Date of Government Version: 04/09/2007 Date Data Arrived at EDR: 04/12/2007 Date Made Active in Reports: 04/27/2007 Number of Days to Update: 15 Source: Department of Environmental Management

Telephone: 401-222-2797 Last EDR Contact: 10/16/2007

Next Scheduled EDR Contact: 12/17/2007 Data Release Frequency: Annually

WI MANIFEST: Manifest Information

Hazardous waste manifest information.

Date of Government Version: 12/31/2006 Date Data Arrived at EDR: 04/27/2007

Date Made Active in Reports: 06/08/2007

Number of Days to Update: 42

Source: Department of Natural Resources

Telephone: N/A

Last EDR Contact: 10/09/2007

Next Scheduled EDR Contact: 01/07/2008 Data Release Frequency: Annually

Oil/Gas Pipelines: This data was obtained by EDR from the USGS in 1994. It is referred to by USGS as GeoData Digital Line Graphs from 1:100,000-Scale Maps. It was extracted from the transportation category including some oil, but primarily gas pipelines.

Electric Power Transmission Line Data

Source: PennWell Corporation Telephone: (800) 823-6277

This map includes information copyrighted by PennWell Corporation. This information is provided on a best effort basis and PennWell Corporation does not guarantee its accuracy nor warrant its fitness for any particular purpose. Such information has been reprinted with the permission of PennWell.

Sensitive Receptors: There are individuals deemed sensitive receptors due to their fragile immune systems and special sensitivity to environmental discharges. These sensitive receptors typically include the elderly, the sick, and children. While the location of all sensitive receptors cannot be determined, EDR indicates those buildings and facilities - schools, daycares, hospitals, medical centers, and nursing homes - where individuals who are sensitive receptors are likely to be located.

AHA Hospitals:

Source: American Hospital Association, Inc.

Telephone: 312-280-5991

The database includes a listing of hospitals based on the American Hospital Association's annual survey of hospitals.

Medical Centers: Provider of Services Listing

Source: Centers for Medicare & Medicaid Services

Telephone: 410-786-3000

A listing of hospitals with Medicare provider number, produced by Centers of Medicare & Medicaid Services,

a federal agency within the U.S. Department of Health and Hurnan Services.

Nursing Homes

Source: National Institutes of Health

Telephone: 301-594-6248

Information on Medicare and Medicaid certified nursing homes in the United States.

Public Schools

Source: National Center for Education Statistics

Telephone: 202-502-7300

The National Center for Education Statistics' primary database on elementary

and secondary public education in the United States. It is a comprehensive, annual, national statistical database of all public elementary and secondary schools and school districts, which contains data that are comparable across all states.

Private Schools

Source: National Center for Education Statistics

Telephone: 202-502-7300

The National Center for Education Statistics' primary database on private school locations in the United States.

Daycare Centers: Licensed Facilities
Source: Department of Social Services

Telephone: 916-657-4041

Flood Zone Data: This data, available in select counties across the country, was obtained by EDR in 1999 from the Federal Emergency Management Agency (FEMA). Data depicts 100-year and 500-year flood zones as defined by FEMA.

NWI: National Wetlands Inventory. This data, available in select counties across the country, was obtained by EDR in 2002 and 2005 from the U.S. Fish and Wildlife Service.

Scanned Digital USGS 7.5' Topographic Map (DRG)

Source: United States Geologic Survey
A digital raster graphic (DRG) is a scanned image of a U.S. Geological Survey topographic map. The map images are made by scanning published paper maps on high-resolution scanners. The raster image is georeferenced and fit to the Universal Transverse Mercator (UTM) projection.

STREET AND ADDRESS INFORMATION

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GEOCHECK®-PHYSICAL SETTING SOURCE ADDENDUM

TARGET PROPERTY ADDRESS

WESTFIELD SHOPPING CENTER 14006 RIVERSIDE DRIVE SHERMAN OAKS, CA 91423

TARGET PROPERTY COORDINATES

Latitude (North): Longitude (West): 34.15677 - 34° 9' 24.4"

Universal Tranverse Mercator:

118.43552 - 118° 26' 7.9"

UTM X (Meters):

Zone 11 367668.7

UTM Y (Meters):

3780274.5

Elevation:

659 ft. above sea level

USGS TOPOGRAPHIC MAP

Target Property Map:

34118-B4 VAN NUYS, CA

Most Recent Revision:

1991

EDR's GeoCheck Physical Setting Source Addendum is provided to assist the environmental professional in forming an opinion about the impact of potential contaminant migration.

Assessment of the impact of contaminant migration generally has two principle investigative components:

- 1. Groundwater flow direction, and
- 2. Groundwater flow velocity.

Groundwater flow direction may be impacted by surface topography, hydrology, hydrogeology, characteristics of the soil, and nearby wells. Groundwater flow velocity is generally impacted by the nature of the geologic strata.

GROUNDWATER FLOW DIRECTION INFORMATION

Groundwater flow direction for a particular site is best determined by a qualified environmental professional using site-specific well data. If such data is not reasonably ascertainable, it may be necessary to rely on other sources of information, such as surface topographic information, hydrologic information, hydrogeologic data collected on nearby properties, and regional groundwater flow information (from deep aquifers).

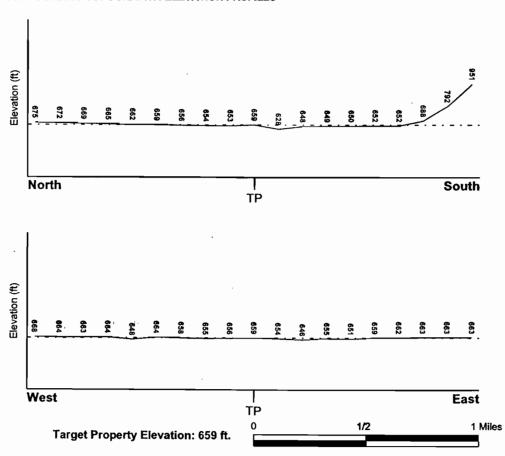
TOPOGRAPHIC INFORMATION

Surface topography may be indicative of the direction of surficial groundwater flow. This information can be used to assist the environmental professional in forming an opinion about the impact of nearby contaminated properties or, should contamination exist on the target property, what downgradient sites might be impacted.

TARGET PROPERTY TOPOGRAPHY

General Topographic Gradient: General SSE

SURROUNDING TOPOGRAPHY: ELEVATION PROFILES



Source: Topography has been determined from the USGS 7.5' Digital Elevation Model and should be evaluated on a relative (not an absolute) basis. Relative elevation information between sites of close proximity should be field verified.

HYDROLOGIC INFORMATION

Surface water can act as a hydrologic barrier to groundwater flow. Such hydrologic information can be used to assist the environmental professional in forming an opinion about the impact of nearby contaminated properties or, should contamination exist on the target property, what downgradient sites might be impacted.

Refer to the Physical Setting Source Map following this summary for hydrologic information (major waterways and bodies of water).

FEMA Flood

FEMA FLOOD ZONE

Target Property County

LOS ANGELES, CA

Electronic Data

YES - refer to the Overview Map and Detail Map

Flood Plain Panel at Target Property:

0601370045C

Additional Panels in search area:

0601370039C 0601370038C 0601370044C

NATIONAL WETLAND INVENTORY

NWI Quad at Target Property

NWI Electronic Data Coverage

VAN NUYS

YES - refer to the Overview Map and Detail Map

HYDROGEOLOGIC INFORMATION

Hydrogeologic information obtained by installation of wells on a specific site can often be an indicator of groundwater flow direction in the immediate area. Such hydrogeologic information can be used to assist the environmental professional in forming an opinion about the impact of nearby contaminated properties or, should contamination exist on the target property, what downgradient sites might be impacted.

Site-Specific Hydrogeological Data*:

Search Radius:

1.25 miles

Status:

Not found

AQUIFLOW®

Search Radius: 1.000 Mile.

Not Reported

EDR has developed the AQUIFLOW Information System to provide data on the general direction of groundwater flow at specific points. EDR has reviewed reports submitted by environmental professionals to regulatory authorities at select sites and has extracted the date of the report, groundwater flow direction as determined hydrogeologically, and the depth to water table.

LOCATION

MAP ID

FROM TP

GENERAL DIRECTION GROUNDWATER FLOW

GROUNDWATER FLOW VELOCITY INFORMATION

Groundwater flow velocity information for a particular site is best determined by a qualified environmental professional using site specific geologic and soil strata data. If such data are not reasonably ascertainable, it may be necessary to rely on other sources of information, including geologic age identification, rock stratigraphic unit and soil characteristics data collected on nearby properties and regional soil information. In general, contaminant plumes move more quickly through sandy-gravelly types of soils than silty-clayey types of soils.

GEOLOGIC INFORMATION IN GENERAL AREA OF TARGET PROPERTY

Geologic information can be used by the environmental professional in forming an opinion about the relative speed at which contaminant migration may be occurring.

ROCK STRATIGRAPHIC UNIT

GEOLOGIC AGE IDENTIFICATION

Era:

Cenozoic

Category: Stratified Sequence

System: Series:

Tertiary Miocene

Code:

Tm (decoded above as Era, System & Series)

Geologic Age and Rock Stratigraphic Unit Source: P.G. Schruben, R.E. Arndt and W.J. Bawiec, Geology of the Conterminous U.S. at 1:2,500,000 Scale - a digital representation of the 1974 P.B. King and H.M. Beikman Map, USGS Digital Data Series DDS - 11 (1994).

DOMINANT SOIL COMPOSITION IN GENERAL AREA OF TARGET PROPERTY

The U.S. Department of Agriculture's (USDA) Soil Conservation Service (SCS) leads the National Cooperative Soil Survey (NCSS) and is responsible for collecting, storing, maintaining and distributing soil survey information for privately owned lands in the United States. A soil map in a soil survey is a representation of soil patterns in a landscape. Soil maps for STATSGO are compiled by generalizing more detailed (SSURGO) soil survey maps. The following information is based on Soil Conservation Service STATSGO data.

Soil Component Name:

URBAN LAND

Soil Surface Texture:

variable

Hydrologic Group:

Not reported

Soil Drainage Class:

Not reported

Hydric Status: Soil does not meet the requirements for a hydric soil.

Corrosion Potential - Uncoated Steel: Not Reported

Depth to Bedrock Min:

> 10 inches

Depth to Bedrock Max:

> 10 inches

Soil Layer Information								
	Boundary			Classi	fication			
Layer	Upper	Lower	Soil Texture Class	AASHTO Group	Unified Soil	Permeability Rate (in/hr)	Soil Reaction (pH)	
1	0 inches	6 inches	variable	Not reported	Not reported	Max: 0.00 Min: 0.00	Max: 0.00 Min: 0.00	

OTHER SOIL TYPES IN AREA

Based on Soil Conservation Service STATSGO data, the following additional subordinant soil types may appear within the general area of target property.

Soil Surface Textures: loam

clay silt loam loamy sand sandy loam fine sand clay loam

gravelly - sandy loam

coarse sand gravelly - sand

sand

Surficial Soil Types: loam

clay silt loam loamy sand sandy loam fine sand clay loam

gravelly - sandy loam

coarse sand gravelly - sand

sand

Shallow Soil Types:

fine sandy loam gravelly - loam

gravelly - loar sand silty clay

Deeper Soil Types:

stratified clay loam

silty clay loam gravelly - sandy loam

coarse sand

sand

weathered bedrock very fine sandy loam

LOCAL / REGIONAL WATER AGENCY RECORDS

EDR Local/Regional Water Agency records provide water well information to assist the environmental professional in assessing sources that may impact ground water flow direction, and in forming an opinion about the impact of contaminant migration on nearby drinking water wells.

WELL SEARCH DISTANCE INFORMATION

DATABASE

SEARCH DISTANCE (miles)

Federal USGS

1.000

Federal FRDS PWS

Nearest PWS within 1 mile

State Database

1.000

FEDERAL USGS WELL INFORMATION

WELL ID

LOCATION

FROM TP

No Wells Found

FEDERAL FRDS PUBLIC WATER SUPPLY SYSTEM INFORMATION

MAP ID

MAP ID

WELL ID

LOCATION

FROM TP

No PWS System Found

Note: PWS System location is not always the same as well location.

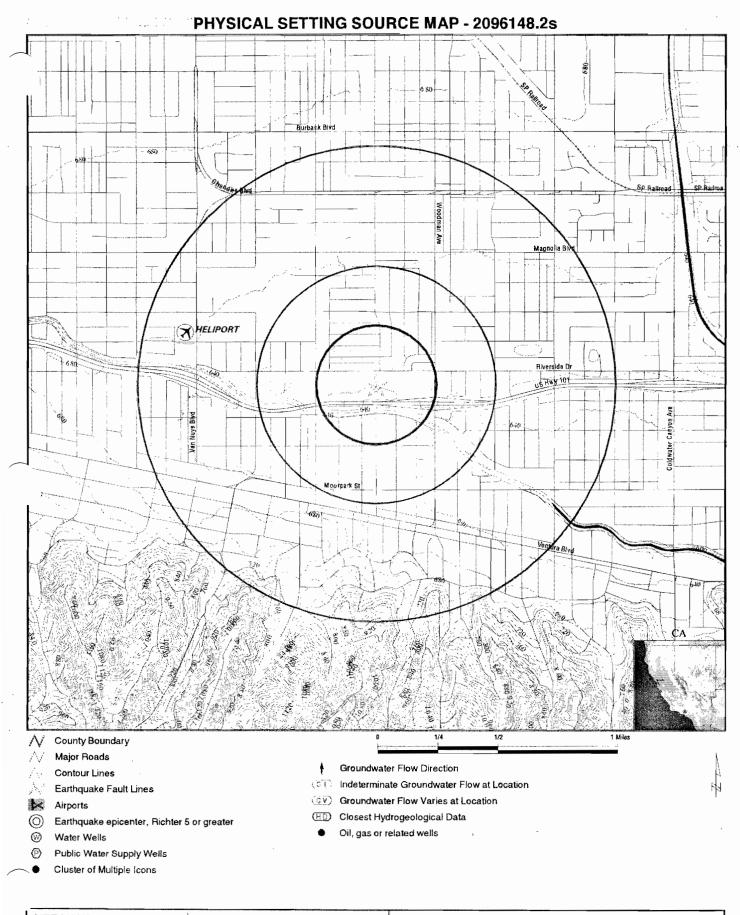
STATE DATABASE WELL INFORMATION

MAP ID

WELL ID

LOCATION FROM TP

No Wells Found



SITE NAME: Westfield Shopping Center ADDRESS: 14006 Riverside Drive Sherman Oaks CA 91423 LAT/LONG: 34.1568 / 118.4355

CLIENT: The Reynolds Group CONTACT: Gwen Tellegen INQUIRY#: 2096148.2s

DATE: December 10, 2007 8:27 am

GEOCHECK® - PHYSICAL SETTING SOURCE MAP FINDINGS RADON

AREA RADON INFORMATION

State Database: CA Radon

Radon Test Results

 Zip
 Total Sites
 > 4 Pci/L
 Pct. > 4 Pci/L

 —
 —
 —

 91423
 102
 14
 13.73

Federal EPA Radon Zone for LOS ANGELES County: 2

Note: Zone 1 indoor average level > 4 pCi/L.

: Zone 2 indoor average level >= 2 pCi/L and <= 4 pCi/L.

: Zone 3 indoor average level < 2 pCi/L.

Federal Area Radon Information for Zip Code: 91423

Number of sites tested: 1

Area Average Activity % <4 pCi/L % 4-20 pCi/L % >20 pCi/L Living Area - 1st Floor 0.500 pCi/L 100% 0% Living Area - 2nd Floor Not Reported Not Reported Not Reported Not Reported **Basement** Not Reported Not Reported Not Reported Not Reported

PHYSICAL SETTING SOURCE RECORDS SEARCHED

TOPOGRAPHIC INFORMATION

USGS 7.5' Digital Elevation Model (DEM)

Source: United States Geologic Survey

EDR acquired the USGS 7.5 Digital Elevation Model in 2002 and updated it in 2006. The 7.5 minute DEM corresponds to the USGS 1:24,000- and 1:25,000-scale topographic quadrangle maps. The DEM provides elevation data with consistent elevation units and projection.

Scanned Digital USGS 7.5' Topographic Map (DRG)

Source: United States Geologic Survey

A digital raster graphic (DRG) is a scanned image of a U.S. Geological Survey topographic map. The map images are made by scanning published paper maps on high-resolution scanners. The raster image is georeferenced and fit to the Universal Transverse Mercator (UTM) projection.

HYDROLOGIC INFORMATION

Flood Zone Data: This data, available in select counties across the country, was obtained by EDR in 1999 from the Federal Emergency Management Agency (FEMA). Data depicts 100-year and 500-year flood zones as defined by FEMA.

NWI: National Wetlands Inventory. This data, available in select counties across the country, was obtained by EDR in 2002 and 2005 from the U.S. Fish and Wildlife Service.

HYDROGEOLOGIC INFORMATION

AQUIFLOWR Information System

Source: EDR proprietary database of groundwater flow information

EDR has developed the AQUIFLOW Information System (AIS) to provide data on the general direction of groundwater flow at specific points. EDR has reviewed reports submitted to regulatory authorities at select sites and has extracted the date of the report, hydrogeologically determined groundwater flow direction and depth to water table information.

GEOLOGIC INFORMATION

Geologic Age and Rock Stratigraphic Unit

Source: P.G. Schruben, R.E. Arndt and W.J. Bawiec, Geology of the Conterminous U.S. at 1:2,500,000 Scale - A digital representation of the 1974 P.B. King and H.M. Beikman Map, USGS Digital Data Series DDS - 11 (1994).

STATSGO: State Soil Geographic Database

Source: Department of Agriculture, Natural Resources Conservation Services

The U.S. Department of Agriculture's (USDA) Natural Resources Conservation Service (NRCS) leads the national Conservation Soil Survey (NCSS) and is responsible for collecting, storing, maintaining and distributing soil survey information for privately owned lands in the United States. A soil map in a soil survey is a representation of soil patterns in a landscape. Soil maps for STATSGO are compiled by generalizing more detailed (SSURGO) soil survey maps.

SSURGO: Soil Survey Geographic Database

Source: Department of Agriculture, Natural Resources Conservation Services (NRCS)

Telephone: 800-672-5559

SSURGO is the most detailed level of mapping done by the Natural Resources Conservation Services, mapping scales generally range from 1:12,000 to 1:63,360. Field mapping methods using national standards are used to construct the soil maps in the Soil Survey Geographic (SSURGO) database. SSURGO digitizing duplicates the original soil survey maps. This level of mapping is designed for use by landowners, townships and county natural resource planning and management.

PHYSICAL SETTING SOURCE RECORDS SEARCHED

LOCAL / REGIONAL WATER AGENCY RECORDS

FEDERAL WATER WELLS

PWS: Public Water Systems

Source: EPA/Office of Drinking Water

Telephone: 202-564-3750

Public Water System data from the Federal Reporting Data System. A PWS is any water system which provides water to at

least 25 people for at least 60 days annually. PWSs provide water from wells, rivers and other sources.

PWS ENF: Public Water Systems Violation and Enforcement Data

Source: EPA/Office of Drinking Water

Telephone: 202-564-3750

Violation and Enforcement data for Public Water Systems from the Safe Drinking Water Information System (SDWIS) after

August 1995. Prior to August 1995, the data came from the Federal Reporting Data System (FRDS).

USGS Water Wells: USGS National Water Inventory System (NWIS)

This database contains descriptive information on sites where the USGS collects or has collected data on surface water and/or groundwater. The groundwater data includes information on wells, springs, and other sources of groundwater.

STATE RECORDS

Water Well Database

Source: Department of Water Resources

Telephone: 916-651-9648

California Drinking Water Quality Database

Source: Department of Health Services

Telephone: 916-324-2319

The database includes all drinking water compliance and special studies monitoring for the state of California since 1984. It consists of over 3,200,000 individual analyses along with well and water system information.

OTHER STATE DATABASE INFORMATION

California Oil and Gas Well Locations

Source: Department of Conservation

Telephone: 916-323-1779

RADON

State Database: CA Radon

Source: Department of Health Services

Telephone: 916-324-2208 Radon Database for California

Area Radon Information

Source: USGS

Telephone: 703-356-4020

The National Radon Database has been developed by the U.S. Environmental Protection Agency

(USEPA) and is a compilation of the EPA/State Residential Radon Survey and the National Residential Radon Survey. The study covers the years 1986 - 1992. Where necessary data has been supplemented by information collected at private sources such as universities and research institutions.

EPA Radon Zones Source: EPA

Telephone: 703-356-4020

Sections 307 & 309 of IRAA directed EPA to list and identify areas of U.S. with the potential for elevated indoor

radon levels.

PHYSICAL SETTING SOURCE RECORDS SEARCHED

OTHER

Airport Landing Facilities: Private and public use landing facilities

Source: Federal Aviation Administration, 800-457-6656

Epicenters: World earthquake epicenters, Richter 5 or greater

Source: Department of Commerce, National Oceanic and Atmospheric Administration

California Earthquake Fault Lines: The fault lines displayed on EDR's Topographic map are digitized quaternary fault lines, prepared in 1975 by the United State Geological Survey. Additional information (also from 1975) regarding activity at specific fault lines comes from California's Preliminary Fault Activity Map prepared by the California Division of Mines and Geology.

STREET AND ADDRESS INFORMATION

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The EDR Multi-Tenant **Retail Facility®** Report

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

Inquiry Number: 2096148.8

Friday, December 14, 2007

The Standard in **Environmental Risk** Information

440 Wheelers Farms Road Milford, Connecticut 06461

Nationwide Customer Service

Telephone: 1-800-352-0050 1-800-231-6802

Fax:

Internet:

www.edrnet.com

EDR Multi-Tenant Retail Facility ® Report

Environmental Data Resources, Inc.'s (EDR) Multi-Tenant Retail Facility Report is a screening report designed to assist environmental professionals in evaluating potential liability on a target property resulting from past activities. The EDR Multi-Tenant Retail Facility Report includes a search and abstract of multiple reasonably ascertainable standard historical sources. The Report provides information including tenants and other occupants.

Thank you for your business.

Please contact EDR at 1-800-352-0050 with any questions or comments.

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Environmental Data Resources, Inc. EDR Multi-Tenant Retail Facility ® Report

Target Property

Westfield Shopping Center 14006 Riverside Drive Sherman Oaks, CA 91423

<u>Year</u> 1957	Occupant(s) Address Not Listed
1958	Address Not Listed
1959	Address Not Listed
1960	Address Not Listed
1961	Address Not Listed
1962	Address Not Listed
1963	Address Not Listed
1964	Address Not Listed
1965	Address Not Listed
1966	Resource Not Available
1967	Address Not Listed
1968	Address Not Listed
1969	Resource Not Available
1970	Address Not Listed
1971	MAGIC MIRROR BEAUTY I SHOP BELLS GIFTS & CHINA BERGGRENS JEWELERS MANDELS SHOES MENS SHOES MONTE FACTOR MENS PHELPS-WILGER MENS PICKWICK WOMENS TOT ROBERTS TOYS ROYAL CAR WASH STOCKBROKER SEES CANDY SHOP LEEDS SHOE STORE MAGNIN WOMENS FASHIONS WOMENS FORGAN & CO ZZZZZ FREDS SANDWICH SHOP BULLOCKS DEPT ST COUNTRY CLUB C H BAKER SHOES

Occupant(s) <u>Year</u> RESTAURANT 1971 **DAVID ORGELL GIFTS & SILVER** DESMONDS DEPT STORE **DE LUCA** DRAPERS WOMENS JUDYS WOMENS LANZ WOMENS
PARTI-TIME PARTY SHOP **GARLANDS HUNTERS BOOKS** GUDES BARNETT SHOES
JOLLY ROGER
EXHIBIT I ART GALLERY
FIDU PONT GLORE
MORRIS BOOTMAKER **ORANGE JULIUS** TOGGERY WESTWARD HO MKT WETHERBY KAYSER SHOES CHARLOTTES WOMENS HOUSE OF NINE WOMENS CHILDRENS SHOE CORRAL SY DEVORE MENS 1972 **BULLOCKS BULLOCKS** 1973 1974 **BULLOCKS** 1975 **BULLOCKS DEPT ST** C H BAKER SHOES DRAPERS WOMENS DU PONT GLORE FORGAN DE LUCA JEWELERS DESMONDS DEPT STORE DAVID ORGELL GIFTS & SILVER GARLANDS WOMENS PARTI-TIME PARTY SHOP LANZ WOMENS JUDYS WOMENS MORRIS BOOTMAKER MENS SHOES JOLLY ROGER RESTAURANT GUDES BARNETT SHOES HUNTERS BOOKS I MAGNIN WOMENS FREDS SANDWICH SHOP LEEDS SHOE STORE SEES CANDY SHOP ROYAL CAR WASH SIMI VENTURA ROBERTS TOYS INC STOCKBROKER POTTERY BARN POTTERY BARN
PICKWICK WOMENS
PHELPS-MEAGER MENS
MONTE FACTOR MENS
MANDELS SHOES
BERGGRENS JEWELERS
BELLS GIFTS & CHINA
ART ATTIC ART GALLERY
MAGIC MIRROR BEAUTY SHOP
SY DEVORE MENS SY DEVORE MENS CHILDRENS SHOE CORRAL

CHARLOTTES WOMENS
COUNTRY CLUB FASHIONS WOMENS

HOUSE OF NINE WOMENS

<u>Year</u> 1975	Occupant(s) WETHERBY KAYSER SHOES ORANGE JULIUS	
1976	DRAPERS	
1977	BAKER SHOES	
1978	BROADWAY	
1979	BROADWAY	
1980	BROADWAY	
1981	Address Not Listed	
1982	Address Not Listed	
1983	Address Not Listed	
1984	AMERICAN EXPRESS	
1985	AMERICAN EXPRESS	
1986	AMERICAN EXPRESS	
1987	AMERICAN EXPRESS	
1988	AMERICAN EXPRESS	
1989	AMERICAN EXPRESS	
1990	AMERICAN EXPRESS	
1991	ANN TAYLOR	
1992	ANN TAYLOR AEROPOSTALE A J MORROW ACCESSORY LADY ADEPT RESEARCH FRENCH CAJUN GABYS JEWELERS FLORSHEIM SHOES FOOTLOCKER FINAL TOUCH FASHION CONSPIRACY FASHION SQUARE DELI FLOWERS SECURITY PACIFIC NATIONAL BANK SHOE DOCTOR SEBASTIANO MENS SEES CANDIES SACHA LONDON ROYAL PHOTO SATURDAYS LEEDS SHOES LERNER LEATHER LAND LECHTERS MACYS LILLIE RUBIN LIZ CLAIBORNE BACHRACHS AUDREY JONES BEBE	

<u>Year</u> Occupant(s) 1992 BEN BRIDGE JEWELERS **BERNINI MENS** MARTIN LAWRENCE GALLERIES **MONOGRAMS PLUS** PEA N POD PETITE SOPHISTICATES
PRINTS PLUS **ROCKS PIGEONS** PRESTIGIO SLAVICKS JEWELERS SILVERWOODS SILVANO SHOE **SNACKS** SOCKS DU JOUR
SOCKS DU JOUR
SPLENDIFEROUS
SUNSHINE BEAUTY SUPPLY
BROADWAY DEPARTMENT STORE
SUNGLASS PLACE
BODY SHOP HEALTH SPA BODY SHOP HEALTH SPA
BOWS N THINGS
BRENTANOS
BULLOCKS DEPARTMENT STORE **BROOKS KIDS SHOES DRAPERS** DR CROW OPT **EDDIE BAUER** RICHTERS DEJAUN JEWELERS DEVON BECK DIMATTIAS PIZZA & PASTA KARLS TOYS
ROBERTS & CO FINE JEWELERS
KAY JEWELERS
KITS CAMERAS LANE BRYANT **KNOW BODIES** LADY FOOTLOCKER PARTY TIME **GEMS GHQ FOR MEN GODIVA CHOCOLATIER** GREEK ISRAELI EXPRESS **GRINGOS** I MAGNIN & CO **I WISH GYMBOREE IMAGINARIUM** IMPOSTERS JACKS BURGERS JACQUES VERT JOHNSTON & MURPHY CACHE JUDYS CAL CRISP CARAMAR CARLTON HAIR CASTLEBY SHOES CASUAL CORNER MUSICLAND PARADISE BAKERY

CELIO PARIS PANDA EXPRESS NANCEE G

NATURAL WONDERS NATURALIZER

<u>Year</u> Occupant(s) **EXPANDING WALL** 1992 **ETCETERA EXPRESS EXTRAVAGNZA ERIKS SHOES ESPRIT** PACIFIC EYES & TS PACIFIC SUNWEAR TIE RACK WET SEAL WE WRAP VICTORIAS SECRET WALDENBOOKS WETHERBY-KAYSER WESTMINSTER LACE WIENERS LUGGAGE HALLMARK CARDS HOFFRITZ FOR CUTLERY HANNAH HELFTS HICKORY FARMS COUNTRY LINEN COUSINS CHILDRENS STORES COUSING CHILDRENS STORES COFFEE BEAN & TEA LEAF CONTEMPO CASUALS CHAYO HAIR SALON HOWARD & PHILS WQSTERN WEAR HUDSON GOODMAN JEWELERS **CIGNAL** CLAIRES BOUTIQUE TAMARAS SY DEVORE **TEEZ** TESTING 1-2-3 THE BOMBAY COMPANY THE GAP THE LIMITED THINGS REMEMBERED THIS END UP WIKIWIKI WINDSOR WLLIAMS-SONOMA WILLIAMS-SONOMA 1993 WLKL WIKI WINDSOR COUNITRY LINTEN THIS END UP THINGS REMEMBERED THE LIMITED THE GAP THE BOMBAY COMPANY TEEZ SY DEVORE TAMARAS **CLALRES BOUTIQUE** CIGNAL CHAYO HAIR SALON CONTEMPO CASUALS COFFEE BEAN & TEA LEAL **COPELANDS SPORTS** HANNIAH HFLICKORY FARRNS

HELFTS

HOFFRITZ FOR CUTLERY HOLD EVERTHING

<u>Year</u> Occupant(s) HOWARD & PHILS WESTERN WEAR 1993 HALLMARK CARDS H₂₀ PLUS WIENERS LUGGAGE WESTMINSTER LACE WETHERBY-KAYSER WALDENBOOKS **VICTORIAS SECRET** WEWRAP WET SEAL TIE RACK PACIFIC SUNWEAR ONE HOUR PHOTO ESPRIT ERIKS SHOES CATCH 21 **CELLO PARIS EXTRAVAGNZA EXPRESS ETCETERA EXPANDING WALL** NATURALIZER NATURAL WONDERS NANCEE G PANDA EXPRESS PACILIC EYES & TS PARADISE BAKERY MUSICLAND CASUAL CORNER CASTLEBY SHOES CARLTON HAIR CARAMAR CAL CRISP JUDYS CACHE JOHLNSTON & MURPHY JACQUES VERT JESSICA MCCUNTOCK **IMPOSTORS HUDSON GOODMAN JEWELERS** JACKS BUIRGERS IMAGINARIUM GYMBOREE **I WISH GRINGOS** GODIVA CHOCOLATLER GHQ FOR MEN GIFTWRAP GENERAL NUTRITION CENTER **GEMS PASTILE** PASTILE LADY FOOTLOCKER COUSINS CHLLDRENS STORES KITS CANMERAS KNOW BODIES LARNE BRYANT DIMNATTIAS PIZZA & PASTA **KAY JEWELERS** ROBERTS & CO FINE JEWELERS KARLS TOYS **DEVON BECKE**

DEJAUN JEWELERS

RICHTERS EDDIE BAUER DR CROW OPT DRAPERS **Year** Occupant(s) BROOKS KIDS SHOES BULLOCKS DEPARTMENT STORE 1993 **SPLENDITEROUS BRENTANOS** SUNGLASS PLACE BROADWAY DEPARTMENT STORE SUNSHINE BEAUTY SUPPLY STREAMERS STRUCTURES SOCKS DU JOUR **SNACKS** SILVANO SHOE PORT OCALL SLVERWOODS PRESTIGIO PIGEONS **ROCKS** PETITE SOPHISTICATES PEA N POD MONOGRAMS PLUS MARTIN LAWRENCE GALLERIES MAX STUDIO BENETTON MEDITERRANEAN DELIGHT BERNINI MENS **BEN BRIDGE JEWELERS** BEBE **AUDREY JONES** LIZ CLALBORNE BACHRACHS LEEDS SHIOES LILLIE RUBIN LEATHIER LAND LECHTER S FLORSHEIM SHLOES LERNER SATURDAYS ROYAL PHOTO SACHA LONDON SEES CANDIES SEBASTIANO MENS SHOE DOCTOR SECURITY PACIFIC NATIONAL BANK **FLOWERS** FASHION CONSPIRACY FASHION SQUARE DELI FINAL TOUCH FOOTLOCKER FREDERICKS GABYS JEWELERS 1 MAGNIN & CO 9 WEST ADEPT RESEARCH ACCESSORY LADY **AEROPOSTALE** ANN TAYLOR AT T 1994 1995 **BEBE BEN BRIDGE JEWELERS MEDITERRANEAN DELIGHT** BENETTON

MERLE NORMAN MAX STUDIO

2096148-8

Occupant(s) <u>Year</u> 1995 MARTIN LAWRENCE GALLERIES SILVANO SHOE **AUDREY JONES BANANA REPUBLIC** PEA N POD PERFUMAY **BATH N BODY WORKS ROCKS PIGEONS PRESTIGIO SNACKS SPLENDIFEROUS** SUNCOAST MOTION PICTURE CO STREAMERS SUNSHINE BEAUTY SUPPLY ANN TAYLOR AEROPOSTALE ACCESSORY LADY ADEPT RESEARCH 9 WEST GABYS JEWELERS **FREDERICKS FOOTLOCKER FLORSHEIM SHOES** FINAL TOUCH FASHION SQUARE DELI **BERNINI MENS FIRST ISSUE FLOWERS** SHOE DOCTOR SEBASTIANO MENS SEES CANDIES SACHA LONDON **ROYAL PHOTO SATURDAYS** SAM GOODY LERNER LEEDS SHOES LEATHER LAND **LECHTERS** LILLIE RUBIN GAP LIZ CLAIBORNE BROADWAY DEPARTMENT STORE SUNGLASS PLACE **BRENTANOS** BULLOCKS DEPARTMENT STORE BROOKS KIDS SHOES **DRAPERS** DR CROW OPT EDDIE BAUER **RICHTERS** STRUCTURE **DEJAUN JEWELERS DEVON BECKE** DIMATTIAS PIZZA & PASTA ROBERTS & CO FINE JEWELERS **KAY JEWELERS** KITS CAMERAS LANE BRYANT KNOW BODIES LADY FOOTLOCKER

PASTILE GEMS GAP KIDS GNC <u>Year</u> Occupant(s) 1995 **GIFTWRAP** GODIVA CHOCOLATIER GRINGOS **I WISH GYMBOREE** IMAGINARIUM HUDSON GOODMAN JEWELERS JESSICA MCCUNTOCK **JACKS BURGERS** CACHE JUDYS CACIQUE CAL CRISP CARAMAR CARLTON HAIR INTERNATIONAL CASTLEBY SHOES CASUAL CORNER PARADISE BAKERY PANDA EXPRESS NANCEE G NATURAL WONDERS NATURALIZER **NUT KETTLE EXPANDING WALL ETCETERA** EXPRESS **EXTRAVAGNZA CELLO PARIS** CATCH 21 **ERIKS SHOES ESPRIT** ONE HOUR PHOTO PACIFIC EYES & TS ORGELL PACIFIC SUNWEAR TIC TIME
TIE RACK
WET SEAL
VICTORIAS SECRET
WALDENBOOKS WIENERS LUGGAGE H₂₀ PLUS HALLMARK CARDS HOWARD & PHILS WESTERN WEAR HOLD EVERTHING HANNAH HICKORY FARMS COPELANDS SPORTS
COUSINS CHILDRENS STORES
CONTEMPO CASUALS
COFFEE BEAN & TEA LEAF CHAYO HAIR SALON HOME SHOP CIGNAL CLAIRES BOUTIQUE SY DEVORE
THE BOMBAY COMPANY
THE DISNEY STORE
THE LIMITED THINGS REMEMBERED THIS END UP FURNITURE WILLIAMS SONOMA WIKIWIKI

1996

WINDSOR WINDSOR

Occupant(s) <u>Year</u> 1996 **WIKIWIKI** WILLIAMS-SONOMA WIENERS LUGGAGE THIS END UP THINGS REMEMBERED THE LIMITED
THE DISNEY STORE
THE BOMBAY COMPANY SY DEVORE **CLAIRES BOUTIQUE** CIGNAL
HOME SHOP
CHAYO HAIR SALON
COFFEE BEAN & TEA LEAF
CONTEMPO CASUALS COUSINS CHILDRENS STORES COPELANDS SPORTS HICKORY FARMS HANNAH **HOLD EVERTHING** HOWARD & PHILS WESTERN WEAR HALLMARK CARDS H₂₀ PLUS **GYMBOREE** WALDENBOOKS WALDENBOOKS
VICTORIAS SECRET
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PAGE STATE STATE PACIFIC EYES & TS ONE HOUR PHOTO ESPRIT ERIKS SHOES CATCH 21 **EDDIE BAUER EXTRAVAGNZA EXPRESS** ETCETERA EXPANDING WALL **NUT KETTLE NATURALIZER** NINE WEST NATURAL WONDERS NATURAL WONDERS
NANCEE G
PANDA EXPRESS
CELIO PARIS
PARADISE BAKERY
CASUAL CORNER
CASTLEBY SHOES
CARLTON HAIR INTERNATIONAL
CARAMAR CARAMAR CAL CRISP CACIQUE CACITOE
CACHE
JACKS BURGERS
JESSICA MCCLINTOCK
HUDSON GOODMAN JEWELERS
LADY FOOT LOCKER **IMAGINARIUM I WISH GRINGOS** GODIVA CHOCOLATIER GIFTWRAP

GNC GAPKIDS

Occupant(s) <u>Year</u> 1996 **GEMS PASTILE** GAP KNOW BODIES LANE BRYANT KITS CAMERAS KAY JEWELERS JUDYS **DIMATTIAS PIZZA & PASTA DEVON BECKE DEJAUN JEWELERS RICHTERS** DR CROW OPT **DRAPERS** BROOKS KIDS SHOES
BULLOCKS DEPARTMENT STORE
BRENTANOS
SUNGLASS PLACE
BROADWAY DEPARTMENT STORE LIZ CLAIBORNE LILLIE RUBIN LECHTERS LEATHER LAND **BERNINI MENS LEEDS SHOES** LERNER NEW YORK SAM GOODY **ROYAL PHOTO** SACHA LONDON **SEES CANDIES** SEBASTIANO MENS SHOE DOCTOR FLOWERS FOOT LOCKER FIRST ISSUE **FASHION SQUARE DELI FINAL TOUCH** FINAL TOUCH FLORSHEIM SHOES FREDERICKS OF HOLLYWOOD GABYS JEWELERS ADEPT RESEARCH ACCESSORY LADY **AEROPOSTALE** SUNSHINE BEAUTY SUPPLY STREAMERS STRUCTURE SUNCOAST MOTION PICTURE CO SPLENDIFEROUS SNACKS SILVANO SHOE PRESTIGIO **PIGEONS ROCKS ROBERTS & CO FINE JEWELERS** PERFUMAY PEA N POD PACIFIC SUNWEAR BANANA REPUBLIC **AUDREY JONES ANN TAYLOR** SATURDAYS MARTIN LAWRENCE GALLERIES **MAX STUDIO** MERLE NORMAN COSMETIC STUDIOS BENETTON

MEDITERRANEAN DELIGHT

Occupant(s) <u>Year</u> 1996 BEN BRIDGE JEWELERS BEBE BATH & BODY WORKS 1997 ANN TAYLOR 1998 **ANN TAYLOR** 1999 A PEA IN THE POD 2000 A PEA IN THE POD 2001 A PEA IN THE POD 2002 A PEA IN THE POD 2003 **ANCHOR BLUE** 2004 **ANCHOR BLUE** A PEA IN THE POD ABERCROMBIE & FITCH ACCESS TO MONEY ADEPT RESEARCH 818 FREIGHT **FOOT LOCKER** SHOE DOCTOR SIANY **SEPHORA** SEES CANDIES SANRIO SURPRISES SANSEI JAPANESE RESTAURANT **SBARRO LENSCRAFTERS** LIMITED TOO MACYS MACYS SWIMWEAR MAISON DOPTIQUE PACIFIC SUNWEAR RADIO SHACK ANN TAYLOR ARDEN B **BANANA REPUBLIC** BANK OF AMERICA ATM **BATH & BODY WORKS** BEBE MISSION RENAISSANCE MAX STUDIO MASSIS KABOB POTTERY BARN PLANET FUNK POL ACHECKS JEWELERS **PRESTIGIO PRETZELMAKER** SILVANO FASHION FOOTWEAR SPEEDO AUTHENTIC FITNESS SUNGLASS HUT SUBWAY SURF CITY SQUEEZE BETSEY JOHNSON BISOU! BISOUL BLOOMINGDALES **BRENTANOS BRIGHTON COLLECTIBLES BROOKS SHOES FOR KIDS** BROOKSTONE RESTORATION HARDWARE

<u>Year</u> Occupant(s) 2004 **RAMPAGE** RENAISSANCE DESIGNS RITZ CAMERA DEJAUN JEWELERS DENISE CAROLYN DEAGUIAR DESIGNS **KENSINGTONS KIPLING** DISCOVERY CHANNEL STORE LANE BRYANT LAVENDERS HALLMARK LA SALSA GAP GAPKIDS GARYS TUX SHOP GNC LIVE WELL **GEORGIOU GREAT STEAK & POTATO CO GUESS** ITS A BLOOMING BUSINESS **J CREW** J JILL JACQUELINE JARROT JOURNEYS CALIFORNIA CRISP CARITON HAIR INTERNATIONAL CARLS JR CASUAL CORNER MRS FIELDS COOKIES
PAJAMA PARTY
PANDA EXPRESS PAPYRUS NINE WEST NORDIC TRACK OH BABY! I OLD PRO GALLERY EXPRESS MENS EDMUNDS ARTCARVED DIAMOND CENTER **ENZO** CATHY JEAN ERIKS SHOES OSIM GLOBAL HEALTH CARE TIC TIME WET SEAL TUMI VICTORIAS SECRET
VICTORIAS SECRET BEAUTY
WALKING COMPANY
WATCH STATION **GYMBOREE** HOLD EVERYTHING HEPBURNS COLDWATER CREEK SWEET SUE COFFEE BEAN & TEA LEAF CHAYO HAIR SALON HOME SHOP CLAIRES ACCESSORIES CLEO & CUCCI COACH TCBY TREATS THE BOMBAY COMPANY
THE BODY SHOP THE DISNEY STORE THE LIMITED

<u>Year</u>

Occupant(s)

2004

THINGS REMEMBERED WILLIAMS-SONOMA YANKEE CANDLE Z GALLERIE ZALES JEWELERS

APPENDIX G

STORMWATER QUALITY



Surface and Stormwater Quality - Introduction

The proposed Fashion Square shopping center located in the Sherman Oaks community of the City of Los Angeles will include project design features (PDFs) specifically designed to reduce urban runoff and associated pollutants. These PDFs include source controls, low impact development concepts, and treatment control best management practices (BMPs) that will be selected and sized in accordance with applicable regulations. The project is in the early stages of conceptual design and, therefore, the site-specific BMPs have not been selected or designed. However, in keeping with Westfield's forward-thinking approach to this and other projects, several stormwater quality improvement PDFs are being evaluated as part of best practice management (BPM) for project. Ultimately, site-specific constraints, such as needed paved surface area to meet parking requirements and traffic control and ADA requirements, and surface and underground utility clearance requirements for the project upgrades will dictate PDFs that will be evaluated as part of final design.

The purpose of this section of the EIR is to:

- 1) identify potential pollutants of concern that exist in the receiving waters adjacent to the project and/or that may be generated or transported from the proposed Project site and potentially impact receiving waters,
- 2) briefly summarize the applicable stormwater treatment and design requirements, and
- 3) provide a list of recommended project design features (PDFs) for implementation.

Potential Pollutants of Concern

Potential pollutants of concern consist of those pollutants that exhibit one or more of the following characteristics: current loadings or historic deposits of the pollutant are impacting the beneficial uses of a receiving water, elevated levels of the pollutant are found in sediments of a receiving water and/or have the potential to bioaccumulate in organisms therein, or the detectable inputs of the pollutant are at concentrations or loads considered potentially toxic to humans and/or flora and fauna. The potential pollutants of concern for the water quality analysis are those that are anticipated or potentially could be generated by the Project at concentrations, based on water quality data collected in Los Angeles County from land uses that are the same as those proposed by the Project, that exhibit these characteristics. Identification of the pollutants of concern for the Project considered proposed land uses, current 303(d) listings and Total Maximum Daily Loads (TMDLs) in the Los Angeles River, as well as pollutants that have the potential to cause toxicity or bioaccumulate in the Project's receiving waters.

The following pollutants were chosen as the potential pollutants of concern for purposes of evaluating water based upon the above considerations:

Sediments (**TSS** and **Turbidity**) – Excessive erosion, transport, and deposition of sediment in surface waters are a significant form of pollution resulting in water quality impairments. Sediment imbalances impair waters' designated uses. Excessive sediment can impair aquatic life by reducing beneficial habitat structure in stream channels affecting benthic infauna, by filling interstitial spaces of spawning gravels, impairing fish food sources, and filling rearing pools. In

LA0162/FSQ07-01 11/27/200711/12/200710/16/2007



addition, excessive sediment can cause taste and odor problems in drinking water supplies and block water intake structures or recharge systems.

Nutrients (Phosphorus and Nitrogen (Nitrate-N, Nitrite-N and Ammonia-N)) — Inorganic forms of nitrogen include nitrate, nitrite and ammonia. Organic forms of nitrogen are associated with vegetative matter such as particulates from sticks and leaves. Total Nitrogen (TN) is a measure of nitrogen present, including inorganic and particulate forms. There are several sources of nutrients in urban areas, mainly fertilizers in runoff from lawns, pet wastes, failing septic systems, and atmospheric deposition from industry and automobile emissions. Nutrient over-enrichment is especially prevalent in agricultural areas where manure and fertilizer inputs to crops significantly contribute to nitrogen and phosphorus levels in streams and other receiving waters. Eutrophication due to excessive nutrient input can lead to changes in algae, benthic, and fish communities; extreme eutrophication can cause hypoxia or anoxia, resulting in fish kills. Surface algal scum, water discoloration, and the release of toxins from sediment can also occur.

Various downstream reaches of the Los Angeles River are identified as impaired by nutrients in general and nitrogen compounds in particular. Evidence of impairment includes low diversity of benthic macroinvertebrates and observations of excessive algae growth. TMDLs have been developed and adopted into the Los Angeles Region Basin Plan for nitrogen compounds, including nitrate/nitrite and ammonia.

Trace Metals (Copper, Lead, and Zinc) – The primary sources of trace metals in stormwater are typically commercially available metals used in transportation (e.g., automobiles), buildings, and infrastructure. Metals are also found in fuels, adhesives, paints, and other coatings. Copper, lead, and zinc are the most prevalent metals typically found in urban runoff. Other trace metals, such as cadmium, chromium, and mercury, are typically not detected in urban runoff or are detected at very low levels. Metals are of concern because of the potential for toxic effects on aquatic life and the potential for ground water contamination resulting from surface water infiltration to underlying aquifer systems. High metal concentrations can lead to bioaccumulation in fish and shellfish and affect beneficial uses of receiving waters.

Various downstream reaches of the Los Angeles River are identified as impaired for metals including cadmium, copper, lead, and zinc and TMDLS have been developed and adopted into the Los Angeles Region Basin Plan.

Pathogens (Bacteria, Viruses, and Protozoa) – Elevated pathogens are typically caused by the transport of domestic animal, wildlife, or human fecal wastes from the watershed. Runoff that flows over land such as urban runoff can mobilize pathogens, including bacteria and viruses. Even runoff from natural areas can contain pathogens (e.g., from wildlife). Other sources of pathogens in urban areas include pets, leaky sanitary sewer pipes, and recreational vehicle waste discharges to the storm sewer system. The presence of pathogens in runoff can impair receiving waters and contaminate drinking water sources. Many of the downstream reaches of the Los Angeles River are identified as impaired by high fecal coliform counts. However, coliform TMDLs have not yet been developed.

Petroleum Hydrocarbons (Oil and Grease and PAHs) – The sources of oil, grease, and other petroleum hydrocarbons in urban areas include spillage fuels and lubricants, discharge of



domestic and industrial wastes, atmospheric deposition, and runoff. Runoff can be contaminated by leachate from road surfaces, wearing of tires, and deposition from automobile exhaust. Also, do-it-yourself auto mechanics may dump used oil and other automobile-related fluids directly into storm drains. Petroleum hydrocarbons, such as polycyclic aromatic hydrocarbons (PAHs), can bioaccumulate in aquatic organisms from contaminated water, sediments, and food and are toxic to aquatic life at low concentrations. Hydrocarbons can persist in sediments for long periods of time and result in adverse impacts on the diversity and abundance of benthic communities. Hydrocarbons can be measured as total petroleum hydrocarbons (TPH), oil and grease, or as individual groups of hydrocarbons, such as PAHs.

Pesticides – Pesticides (including herbicides, insecticides and fungicides) are chemical compounds commonly used to control insects, rodents, plant diseases, and weeds. Excessive application of a pesticide may result in runoff containing toxic levels of its active component. Pesticides may be classified as organochlorine pesticides or organophosphorus pesticides, the former being associated with persistent bioaccumulative pesticides (e.g., DDT and other legacy pesticides) which have been banned. The Los Angeles River estuary is listed as impaired for legacy pesticides. Organophosphorus pesticides include diazinon and chlorpyrifos whose uses also are being restricted by EPA.

Trash and Debris – Trash (such as paper, plastic, polystyrene packing foam, and aluminum materials) and biodegradable organic debris (such as leaves, grass cuttings, and food waste) are general waste products on the landscape that can be entrained in urban runoff. The presence of trash and debris may have a significant impact on the recreational value of a water body and aquatic habitat. Excess organic matter can create a high biochemical oxygen demand in a water body and thereby lower its water quality. Also, in areas where stagnant water exists, the presence of excess organic matter can promote septic conditions resulting in the growth of undesirable organisms and the release of odorous and hazardous compounds such as hydrogen sulfide. Trash TMDLs for the Los Angeles River Watershed are currently being scoped by the Los Angeles Regional Water Quality Control Board (RWQCB).

SUSMP Requirements

On March 8, 2000, the Los Angeles County Standard Urban Stormwater Mitigation Plan (SUSMP) requirements were approved by the RWQCB as part of the National Pollutant Discharge Elimination System (NPDES) MS4 program to address stormwater pollution from new construction and redevelopment projects in the County. The SUSMP contains a list of minimum site design, source control and treatment controls best management practices (BMPs) that must be employed to infiltrate or treat stormwater runoff, control peak flow discharge, and reduce the post-Project discharge of pollutants from stormwater conveyance systems. The SUSMP defines, based upon land use type, the types of practices that must be included and issues that must be addressed as appropriate to the development type and size.

The table below provides a summary of the SUSMP requirements and stormwater BMPs to be implemented on all significant new development and redevelopment projects in Los Angeles County. The Fashion Square Project fits the criteria of redevelopment projects requiring SUSMP mitigation for potential storm water quality impairments. The Project will adhere to SUSMP requirements to the maximum extent practicable.

LA0162/FSQ07-01 11/27/200711/12/200710/16/2007



SUSMP Requirement	Criteria/ Description
Peak Flow Controls	Control post-development peak discharge rates, velocities and duration in Natural Drainage Systems to prevent accelerated downstream erosion and to protect habitat related beneficial uses. Control post-development peak discharge rates, velocities and duration in
	All post-development runoff from a 2-year, 24-hour storm shall not exceed the predevelopment peak flow rate, burned, from a 2-year, 24-hour storm when the predevelopment peak flow rate equals or exceeds five cfs. Discharge flow rates shall be calculated using the County of Los Angeles Modified Rational Method.
	Post-development runoff from the 50-year capital storm shall not exceed the predevelopment peak flow rate, burned and bulked, from the 50-year capital storm.
	Control peak flow discharge to provide stream channel and over bank flood protection, based on flow design criteria selected by the local agency.
2. Conserve Natural Areas	Concentrate or cluster development on portions of a site while leaving the remaining land in a natural undisturbed condition.
	• Limit clearing and grading of native vegetation at a site to the minimum amount needed to build lots, allow access, and provide fire protection.
	Maximize trees and other vegetation at each site, planting additional vegetation, clustering tree areas, and promoting the use of native and/or drought tolerant plants.
	Promote natural vegetation by using parking lot islands and other landscaped areas.
	Preserve riparian areas and wetlands.
Minimize Stormwater Pollutants of Concern	Minimize, to the maximum extent practicable, the introduction of pollutants of concern that may result in significant impacts generated from site runoff of directly connected impervious areas (DCIA) to the stormwater conveyance system as approved by the building official.

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¹ This requirement is from Part 4, § D.1 of the MS4 Permit.



SUSMP Requirement		Criteria/ Description
4.	Protect Slopes and Channels	Project plans must include BMPs consistent with local codes and ordinances and the SUSMP requirements to decrease the potential of slopes and/or channels from eroding and impacting stormwater runoff:
		Convey runoff safely from the tops of slopes and stabilize disturbed slopes
		Utilize natural drainage systems to the maximum extent practicable
		Control or reduce or eliminate flow to natural drainage systems to the maximum extent practicable
		Stabilize permanent channel crossings
		Vegetate slopes with native or drought tolerant vegetation
		Install energy dissipaters, such as riprap, at the outlets of new storm drains, culverts, conduits, or channels that enter unlined channels in accordance with applicable specifications to minimize erosion with the approval of all agencies with jurisdiction, e.g., the U.S. Army Corps of Engineers and the California Department of Fish and Game.
5.	Provide Storm Drain System Stenciling and Signage	All storm drain inlets and catch basins within the Project area must be stenciled with prohibitive language and/or graphical icons to discourage illegal dumping.
		Signs and prohibitive language and/or graphical icons, which prohibit illegal dumping, must be posted at public access points along channels and creeks within the Project area.
		Legibility of stencils and signs must be maintained.
6.	Properly Design Outdoor Material Storage Areas	Where proposed Project plans include outdoor areas for storage of materials that may contribute pollutants to the stormwater conveyance system measures to mitigate impacts must be included.
7.	Properly Design Trash Storage Areas	All trash containers must meet the following structural or treatment control BMP requirements:
		Trash container areas must have drainage from adjoining roofs and pavement diverter around the areas.
		Trash container areas must be screened or walled to prevent offsite transport of trash.
8.	Provide Proof of Ongoing BMP Maintenance	Applicant required to provide verification of maintenance provisions through such means as may be appropriate, including, but not limited to legal agreements, covenants, and/or Conditional Use Permits.
9.	Design Standards for Structural or Treatment Control BMPs	Post-construction Structural or Treatment Control BMPs shall be designed to mitigate (infiltrate or treat) stormwater runoff using either volumetric treatment control BMPs or flow-based treatment control BMPs sized per listed criteria (see section 3.6.2 above).

LA0162/FSQ07-01 <u>11/27/200711/12/2007</u>10/16/2007



SUSMP Requirement	Criteria/ Description
10.B.1 Properly Design Loading/ Unloading Dock Areas (100,000 ft ² Commercial Developments)	 Cover loading dock areas or design drainage to minimize run-on and runoff of stormwater. Direct connections to storm drains from depressed loading docks (truck wells) are prohibited.
10B.2. Properly Design Repair/ Maintenance Bays (100,000 ft ² Commercial Developments)	 Repair/ maintenance bays must be indoors or designed in such a way that does not allow stormwater run-on or contact with stormwater runoff. Design a repair/maintenance bay drainage system to capture all wash water, leaks, and spills. Connect drains to a sump for collection and disposal. Direct connection of the repair/ maintenance bays to the storm drain system is prohibited. If required by local jurisdiction, obtain an Industrial Waste Discharge Permit.
10B.3. Properly Design Vehicle/ Equipment Wash Areas (100,000 ft ² Commercial Developments)	Self-contained and /or covered, equipped with a clarifier, or other pretreatment facility, and properly connected to a sanitary sewer.
10.D. Properly design fueling area (Retail Gasoline Outlets)	The fuel dispensing area must be covered with an overhanging roof structure or canopy. The cover's minimum dimensions must be equal to or greater than the area within the grade break. The cover must not drain onto the fuel dispensing area and the downspouts must be routed to prevent drainage across the fueling area.
	• The fuel dispensing area must be paved with Portland cement concrete (or equivalent smooth impervious surface). The use of asphalt concrete shall be prohibited.
	• The fuel dispensing areas must have a 2% to 4% slope to prevent ponding, and must be separated from the rest of the site by a grade break that prevents run-on of urban runoff.
	• At a minimum, the concrete fuel dispensing area must extend 6.5 feet (2.0 meters) from the corner of each fuel dispenser, or the length at which the hose and nozzle assembly may be operated plus 1 foot (0.3 meter), whichever is less.
10.E.1. Properly design fueling area (Automotive Repair Shops)	See requirement 10.D. above.
10.E.2. Properly design repair/ maintenance bays (Automotive Repair Shops)	See requirement 10.B.2 above.
10.E.3. Properly design vehicle/equipment wash areas (Automotive Repair Shops)	Self-contained and/or covered, equipped with a clarifier, or other pretreatment facility, and properly connected to a sanitary sewer or to a permitted disposal facility.
10.E.4. Properly design loading/ unloading dock areas (Automotive Repair Shops)	See requirement 10.B.1. above.



SUSMP Requirement	Criteria/ Description
10.F.1. Properly Design Parking Area (Parking Lots)	 Reduce impervious land coverage of parking areas. Infiltrate runoff before it reaches the storm drain system. Treat runoff before it reaches storm drain system.
10.F.2 Properly Design to Limit Oil Contamination and Perform Maintenance (Parking Lots)	 Treat to remove oil and petroleum hydrocarbons at parking lots that are heavily used. Ensure adequate operation and maintenance of treatment systems particularly sludge and oil removal.
13. Limitation of Use of Infiltration BMPs	 Infiltration is limited based on design of BMP, pollutant characteristics, land use, soil conditions, and traffic. Appropriate conditions (groundwater >10 ft from grade) must exist to utilize infiltration to treat and reduce stormwater runoff for the Project.

Recommended Project Design Features (PDFs)

The Project will likely utilize a mosaic of water quality improvement PDFs. Preferred over a "one size fits all" approach, the potential use of a few appropriately-placed PDFs will allow the Project meet the tight space constraints of the upgrade and to potentially divide flows for desired reduction in flow and water quality impacts to surrounding systems (both natural and engineered). Project Design Features (PDFs) for water quality and hydrologic impacts include site design, source control, and treatment control BMPs that will be incorporated into the Project and are considered a part of the Project for impact analysis. Effective management of wet and dry weather runoff water quality begins with limiting increases in runoff pollutants and flows at the source. Site design and source control BMPs are practices designed to minimize runoff peaks and volumes, as well as the initial introduction of pollutants in stormwater runoff. Treatment control BMPs are designed to remove pollutants once they have been mobilized by rainfall and runoff

In accordance with the SUSMP requirements, minimum site design and source control BMPs will be met or exceeded. The proposed Project will also incorporate, as PDFs, treatment control BMPs that will minimize urban runoff and associated impacts to receiving water quality and specifically address the identified pollutants of concern. Many BMP alternatives can be easily integrated into planned landscaping, right-of-ways, and infrastructure without requiring large areas of dedicated open space while still meeting the SUSMP sizing requirements.

The following paragraphs describe the types of BMP alternatives that recommended for implementation at the proposed Project. While these alternatives are described herein for planning purposes only (i.e., no site-specific designs have been finalized), they provide a listing of the water quality improvement BMPs specifically being evaluated for the Fashion Square Project. The alternatives have been grouped into 1) vegetated treatment BMPs, 2) onsite storage and reuse, 3) permeable paving, 4) roof top BMPs, and 5) media filters.

LA0162/FSQ07-01 11/27/2007<u>11/12/2007</u>10/16/2007



Vegetated Treatment BMPs

Vegetated treatment BMPs include swales, filter strips, bioretention and planter boxes. When properly designed and maintained, vegetated BMPs are among the most effective, cost efficient treatment approaches for dry and wet-weather runoff. While the Project is significantly space-constrained, areas such as the northern frontage of the Project adjacent to Riverside Drive will be evaluated for possible siting of such PDFs. Treatment occurs through sedimentation, filtration, adsorption to organic matter, and vegetative uptake. Additionally, vegetated treatment systems can help to reduce runoff volumes through soil soaking, infiltration, and evapotranspiration. A beneficial feature of vegetated treatment systems is that their design and implementation is highly flexible and adaptable. On-site implementation of these systems can be integrated into surface conveyances and on-site landscaping in innovative ways that provide site amenities, are functionally effective for runoff conveyance and water quality treatment, and in some cases are less costly to construct than traditional storm sewers.

Vegetated Swales – Vegetated swales are engineered vegetation-lined channels that provide water quality benefits in addition to stormwater conveyance. Swales provide pollutant removal through settling and filtration in the vegetation (often grasses) lining the channels and also provide the opportunity for volume reductions





through infiltration and evapotranspiration. Swales are most effective where longitudinal slopes are small (two to six percent), increasing the residence time for treatment, and where water depths are less than the vegetation height.

Bioretention – Bioretention is a structural BMP that makes use of soils and plants to remove pollutants from runoff. Runoff is typically directed over a grass buffer strip to a shallow vegetated depression that contains deep, porous soils. These depressions are designed to incorporate many of the pollutant removal mechanisms that operate in forested

ecosystems, including, filtration, sorption, plant uptake, microbial activity, decomposition, sedimentation and volatilization. Bioretention provides volume reduction through infiltration, soil soaking and evapotranspiration. Runoff is designed to pond in the bioretention area to allow









for adequate time for infiltration and pollutant adsorption and uptake. During large storms, a portion of runoff is diverted past the facility to the storm drain system. Depending on location and site constraints, bioretention systems can be designed with and without perforated



underdrains, which return filtered runoff to the storm drain system. Bioretention systems are flexible in their configuration and design, and can be readily integrated into site landscaping.

Stormwater Planter Boxes -

Stormwater planter boxes are structural landscaped reservoirs similar to bioretention that are used to collect and filter stormwater runoff, typically from rooftops, but sometimes from roadways. Infiltration planters allow water to





infiltrate through the planter soil matrix and into the ground. Flow-through planters include the use of a waterproof lining and underdrain. Both types of planter boxes require an overflow to an approved stormwater conveyance system and may be used to help fulfill site landscaping requirements.

Onsite Storage and Reuse

The goal of onsite storage and reuse is to temporarily detain stormwater and then use it to meet irrigation or other non-potable water demands. With the space and geotechnical constraints of the existing on-site (commercial buildings) and off-site (utility corridors and roadways) structures, large-scale retention is not feasible. Nevertheless, small-scale systems such as small storage tanks strategically located next to and upgradient from landscaped areas will be evaluated for feasibility at the Project.

Cisterns and Rain Barrels — Cisterns and rain barrels are low-cost water conservation devices that could be used to reduce runoff volume and, for smaller storm events, delay and reduce the peak runoff flow rates. They store and divert runoff from impervious roof areas. This stored runoff could provide a source of chemically untreated 'soft water' for landscaping, free of most sediment and dissolved salts





Individual cisterns and rain barrels can be located above-ground and beneath downspouts, or the desired storage volume could be provided in one common cistern that collects rainwater from several sources. Pre-manufactured cisterns are typically available in sizes ranging from 57 to 10,000 gallons, but cast-in-place tanks can also be used. Although the cistern option would not manage sufficient quantities of runoff to eliminate the need for other runoff management options, it can provide both a positive effect with regard to water conservation and can eliminate low flow runoff and associated loadings from very small storm events. The cisterns and rain barrels should have lids and screened inlets to minimize potential for breeding mosquitoes in the stored water.

LA0162/FSQ07-01 11/27/200714/12/200710/16/2007



Underground Storage – Underground storage involves capturing runoff from areas other than, or in addition to, rooftops and storing it for subsequent reuse on-site. These other areas may include driveways, parking lots, and sidewalks. Capturing and storing runoff from these areas would help to reduce runoff from common source areas of urban stormwater constituents of concern and would also help to reduce dry-weather runoff from common sources such as over-watering of landscape. Each system would be designed and sized to collect and treat runoff and would be stored underground in a system sized to supply an appropriate percentage of the water demand. This option could also include some treatment (such as onsite filtration and disinfection) and would require careful management and consideration of water distribution systems. These facilities would need to be installed underground in order to facilitate storage of large volumes of runoff. Without adequate treatment, landscape irrigation may require a controlled subsurface





distribution system (i.e., no sprinkler system) so that direct public contact would essentially be eliminated. Examples of these types systems include proprietary products by Evaporative Control Systems, Inc (http://www.ecsgreen.com) and Glenn Rehbein Companies (http://www.rehbein.com). As with the considerations described above related to the potential use of cisterns, the opportunities for these types of PDFs would have to be evaluated based on space and geotechnical constraints of the Project.

Permeable Paving

Areas such as roadways, driveways, parking areas, and walkways covered with impermeable (non-porous) pavement are one of the largest contributors to wet weather urban runoff. Permeable, or porous pavements are a special type of material that allows water to drain down to the underlying soil, yet are strong enough to structurally support vehicular or pedestrian traffic. Many types of porous pavements and configurations have been developed for a variety of applications. Most of the systems are supported by a stone base that has large pore spaces. This base acts both as payement support and as a reservoir to store water so that it can be infiltrated, if the soil conditions allow, or detained and slowly released to the storm drain system. In addition, the pavement roughness may be improved (i.e., increased with no significant effect on the driver) thereby providing greater control of runoff hydraulics (i.e., increasing the time required to reach discharge points). Supplemental storage facilities, such as underground vaults (described above) or drainage blankets, can be used in conjunction with these systems. Some of the available permeable pavements that may be further evaluated as PDFs for the Project, subject to geotechnical constraints, are described below. Similar to other PDF alternatives described above, these paving alternatives may be used in specific locations and in conjunction with other PDFs. It should also be noted that these systems are currently being evaluated for the concrete matrix ability to support beneficial bacterial growth that can provide treatment benefits to the water percolating through the pavement.



Pervious Concrete – Pervious concrete has stable air pockets that allow water to drain uniformly into the ground or engineered drainage structure below, where it can be naturally filtered. The material becomes stronger and more stable when it gets wet, so it does not deteriorate as fast as other paving materials. Its use should be restricted to parking lots and local roads since it may not support loads similar to those supported by standard concrete. Pervious concrete is cement based and therefore will



not release harmful chemicals into the environment. It has been in use throughout Europe for about fifty years. A domestic formula known as the Portland Cement Pervious Pavement has been used successfully since the 1970s in the U.S. The pavement is a special blend of Portland cement, sand-free coarse aggregate, and water.

Pervious Asphalt – Pervious asphalt mix pavements consist of a layer of pervious asphalt paving, underlain with a pervious base rock section. There may or may not be a layer of geotextile fabric that separates the base rock from underlying native soils. There also may be a perforated pipe underdrain system where native soils do not infiltrate well or where it is undesirable to infiltrate. The base rock section typically has very little fines to maximize the void ratio while providing for adequate compaction. The base rock section is typically



designed to temporarily store the volume of stormwater generated from a design storm and infiltrate it into underlying soils or into an underdrain system. Similar to pervious concrete designs, pervious asphalt mix pavements have been used in parking lots, private streets, driveways, and pedestrian access areas since the 1970s.

Media Filters

Media filtration is primarily intended to separate fine particulates and associated pollutants, but depending on the type of media, dissolved constituents, such as metals and nutrients, may be removed via sorption processes. Stormwater is captured and directed either under gravity or pressure through media such as sand, engineered media, compost, zeolite, or combinations of media. These PDFs can be either large installations (not described herein due to Project size constraints), or sized to address a portion of the Project runoff.

Cartridge Filters - Several proprietary filtration systems are now available that utilize disposable cartridge filters that are typically placed in an underground vault or manhole. These designs often include siphon-actuated filtration that provides backflushing of the filter media as the water level drops. A variety of media types engineered for the removal of specific stormwater constituents are available for these cartridges in several sizes depending on runoff area. Given the Project space constraints, further evaluation of the feasibility of media filtration is warranted.



LA0162/FSQ07-01 11/27/200711/16/2007

APPENDIX H

URBAN DECAY REPORT



HR&A ADVISORS, INC.

Economic Development, Real Estate Advisory & Public Policy Consultants

Final Draft

ANALYSIS OF POTENTIAL "URBAN DECAY" AS A CONSEQUENCE OF THE PROPOSED EXPANSION OF THE WESTFIELD FASHION SQUARE SHOPPING CENTER SHERMAN OAKS, CALIFORNIA

Prepared for:

Sherman Oaks Fashion Associates, L.P. 11601 Wilshire Boulevard Los Angeles, CA 90025

Prepared in association with Whitney & Whitney, Inc.

August 2007

Los Angeles New York

CONTENTS

		<u>Page</u>
Execu	utive Su	mmary1
I.	Introd	luction5
	A	Purpose of the Analysis5
	B.	Overview of Westfield Fashion Square and the Proposed Expansion Project5
	C.	The "Urban Decay" Concept in Environmental Impact Analysis11
II.	Metho	odological Approach and Impact Measurements12
	A.	Shoppers Goods Space Impact Analysis
	B.	Eating and Drinking Facilities Space Impact Analysis
III.	Concl	usions24
Appe	ndix A:	Summary Qualifications of HR&A Advisors, Inc. and Whitney & Whitney
Appe	ndix B:	Explanation of Population, Income and Retail Sales Allocation Factors Used in the Analysis

List of Figures

		<u>Page</u>
1.	Regional and Project Vicinity Map	6
2.	Conceptual Site Plan	8
3.	Shopper Goods Regional Market Area	13
4.	Eating and Drinking Facilities Market Area	18
	List of Tables	<u>Page</u>
1.	Proposed Expansion Project Use Categories	
2.	Expansion Project's Share of Supportable Retail Space in the Fashion Square Regional Market Area	3
3.	Expansion Project's Share of Supportable Eating & Drinking Facilities Space in a 3-Mile Market Radius Around Fashion Square	
4.	Proposed Expansion Project Tenant Profile	9
5.	Distribution of Space by Major Category, Westfield Fashion Square Expansion Project	10
6.	Distribution of Non-Department Store Shopper Goods Retail Sales in the Fashion Square Regional Market Area	
7.	Summary if Projected Increase in Shopper Goods Retail Sales Demand in the Fas Square Regional Market Area, 2007-2012	
8.	Projected Growth in Demand for Selected Shopper Goods in the Westfield Fashio Square Regional Market Area, 2007-2012	
9.	Summary of Projected Increase in Supportable GLA for Selected Shopper Goods in the Fashion Square Regional Market Area, 2007-2012	
10.	Projected Increase in Supportable Space for Selected Shopper Goods in the Westfield Fashion Square Regional Market Area, 2007-2012	16
11.	Comparison of Projected Increase in Market Demand with Projected Expansion Project Supply, Selected Shopper Goods Space, Westfield Fashion Square Region Market Area, 2007-2012	

		Page
12.	Summary of Projected Increase in Eating & Drinking Facilities Sales Demand in a 3-Mile Market Area Around Fashion Square, 2007-2012	18
13.	Projected Growth in Demand for Eating & Drinking Facilities, Westfield Fashion Square 3.0-Mile Market Area, 2007-2012	19
14.	Projected Increase in Supportable Space for Eating & Drinking Facilities Westfield Fashion Square 3.0-Mile Market Area, 2007-2012	20
15.	Comparison of Projected Market Demand with Projected Expansion Project Supply, Eating & Drinking Facilities Space, Westfield Fashion Square 3.0- Mile Market Area, 2007-2012	20
16.	Expansion Project's Share of Supportable Shopper Goods Retail Space in the Fashion Square Regional Market Area	
17.	Expansion Project's Share of Supportable Eating & Drinking Facilities Space in a 3-Mile Market Radius Around Fashion Square	25

Page iv August 2007 HR&A ADVISORS, INC.

EXECUTIVE SUMMARY

This Report analyzes the potential for the operation of the proposed expansion of the Westfield Fashion Square regional shopping center, located in the community of Sherman Oaks within the San Fernando Valley, City of Los Angeles, to directly or indirectly cause "urban decay," as that concept has been addressed in court decisions interpreting the California Environmental Quality Act (CEQA).

Analysis of the potential for new retail development to cause urban decay — which has been described as a chain reaction of store closures and long term vacancies, ultimately destroying existing neighborhoods and leaving decaying shells in their wake — suggests a twopart analysis. First, it must be determined whether the new retail development will attract retail sales away from existing and/or other planned future retail centers to any significant degree. Second, if sales will be attracted away, it must be determined whether the severity of this change in economic circumstances will cause disinvestment such that it is reasonably foreseeable that significant business closures, abandonment or other forms of physical deterioration or "decay" will result.

The proposed project consists of 280,000 square feet of Gross Leasable Area (GLA) to be distributed between retail stores and eating and drinking facilities as summarized in Table 1 below ("Expansion Project"):

Table 1 PROPOSED EXPANSION PROJECT USE CATEGORIES

Space Category	Square Feet Gross Leasable Area (GLA)
In-Line Retail Space	240,000
Eating and Drinking Facilities	40,000
Grand Total	280,000

Construction is planned for completion in 2010; stabilized conditions are projected to be reached in 2012, the second full year of Expansion Project operations.

Source: Sherman Oaks Fashion Associates, L.P.

The analysis presented here evaluates whether the retail space contained in the Expansion Project will result in a significant adverse economic impact on existing retail developments in the market area. Methodologically, the potential for such an impact can be determined in a given market area through a comparison of the relative growth in demand for retail goods, as measured by the change in supportable retail space for particular retail store categories, with the amount of proposed additions to the supply of retail space. In this particular context, the analysis focuses on whether the proposed amount of floor area in each major retail and dining use category planned for the Expansion Project exceeds the likely increase in demand for those uses within the relevant market area(s) around Fashion Square, as measured by the anticipated growth in population and per capita personal income that would be available for expenditure on specified retail goods and dining opportunities. If the proposed change in the supply of floor area for retail and eating and drinking activities exceeds anticipated growth in demand, the resulting

competitive conditions would challenge existing retailers and restaurateurs to such a degree that net sales could be attracted away from existing stores without their likely replacement by sales from the new sources of demand. Under such circumstances, further in-depth analyses would be required to assess whether it is foreseeable that this draining of sales from existing businesses would logically result in significant disinvestment, business closures, abandonment, other forms of physical deterioration, or other forms of "urban decay."

If, on the other hand, the amount of retail and eating and drinking facility space planned for the Expansion Project, together with space for such uses in other projects, *is less than* the increase in space that can be supported by projected increases in future demand, there are no significant adverse competitive pressures that could potentially lead to urban decay. This is because growth in customer demand will be large enough to comfortably support both the Expansion Project and other existing and planned projects offering comparable retail and restaurant uses. In this case, there is no need to evaluate the potential for urban decay as a consequence of the development of the Expansion Project.

Making these economic impact measurements requires: (1) establishing an appropriate market area for each retail and dining category in the Expansion Project for which future customer demand will be generated; (2) projecting the scale of customer demand based on population growth, income growth and spending growth for those relevant use categories over a relevant time period (i.e., 2007-2012); and (3) converting projected changes in future customer retail and eating and drinking facility spending into magnitudes of supportable square feet of GLA floor area, so that the projected increase in supportable space can be compared directly with the projected change in supply proposed for each use category in the Expansion Project development program.

Accordingly, separate market impact analyses were conducted for the types of commercial uses that are to be included in the Expansion Project: (1) three types of in-line regional retail space, including Apparel and Accessories, Furniture/Furnishings/Appliances and Specialty or "Other" retail; and (2) Eating and Drinking facility space.

The analysis concludes that, while the Expansion Project may add some new competitive retail and restaurant facilities to the regional market area, there is no reasonable likelihood that the operation of the Expansion Project would result in significant adverse economic competition within the regional market area to the degree that this competition would lead to urban decay. This conclusion is based on the finding that the amount of new retail and eating and drinking facility space that can be supported by future growth in customer demand exceeds the amounts of new retail and eating and drinking facility space that is planned for inclusion in the Expansion Project.

More specifically, the analysis includes the following impact findings and conclusions:

• Shopper Goods (Apparel, Furniture/Home Furnishings and Specialty Goods). The applicable regional market area (RMA) for analysis of the Expansion Project's shopper goods is the same as the regional market area for the existing shopping center. It consists of the land area represented by all or a portion of 26 ZIP codes, including all or portions of the

following cities and communities: Sherman Oaks; Toluca Lake; North Hollywood; Valley Village; Encino; Studio City; Van Nuys; Valley Glen; Tarzana; Bel Air Estates; Mount Olympus; Trousdale Estates; Beverly Glen; Brentwood; Hollywood; Hollywood Hills; City of Beverly Hills; and City of Burbank. Based on an analysis of this RMA, the net addition of 240,000 square feet GLA of Shopper Goods space in the Expansion Project is projected to capture the following market shares of the anticipated growth in demand for Apparel and Accessories space; Furniture, Furnishings and Appliances space; and Specialty or "Other" retail space over the period 2007 through 2012:

Table 2 EXPANSION PROJECT'S SHARE OF SUPPORTABLE SHOPPER GOODS SPACE IN THE FASHION SQUARE REGIONAL MARKET AREA

	Expansion Area	Percent of RMA
Retail Category	Square Feet GLA	Supportable Space
Apparel and Accessories	144,000	43%
Furniture, Furnishings and Appliances	24,000	9%
Specialty ("Other")	72,000	8%
Total	240,000	

Source: HR&A, Inc.: W & W, Inc.

This leaves substantial market share to be captured by other retailers in the RMA as well as allowing existing stores to expand their sales at rates above anticipated the inflationary growth rate. Thus, it may be concluded that the development of Shopper Goods uses in the Expansion Project will not be a cause of urban decay at any of the existing shopping centers and business districts found in the market area served by Westfield Fashion Square.

Eating and Drinking Facilities. The applicable market area for analysis of the Expansion Project's Eating and Drinking Facilities could also be defined as the RMA for shopper goods, but it is likely that patrons of the dinner restaurants will come from a more local area. Accordingly, the market area for all of the Eating and Drinking Facilities space has been conservatively defined as a more limited three-mile radius around the existing center. Analysis of the potential impact of the proposed Eating and Drinking Facility component of the Expansion Project indicates that there is ample market support generated by local resident population and purchasing power growth within a three-mile market radius to support the proposed net addition of 40,000 square feet GLA of Eating and Drinking Facility space. As summarized below, the market shares required to sustain the Expansion Project allow for significant future demand to be captured by existing and future competition.

Page 3 HR&A ADVISORS, INC.

Table 3 EXPANSION PROJECT'S SHARE OF SUPPORTABLE EATING & DRINKING FACILITIES SPACE IN A 3-MILE MARKET RADIUS AROUND FASHION SQUARE

		Expansion Area	Percent of Local N
Restaurant Category		Square Feet GLA	Supportable Sp
Fast Food Restaurants		10,000	8%
Restaurants with Alcohol		30,000	25%
	Total	40,000	

Source: HR&A, Inc.; W & W, Inc.

Because the addition of the proposed eating and drinking uses in the Expansion Project will not have a significant negative impact on the existing supply of competitive uses in the local market area, this component of the Expansion Project will not lead to urban decay at any of the existing shopping centers and business districts found in the market area served by Westfield Fashion Square.

Since we find that the scale of incremental growth in supportable shopper goods retail and eating and drinking facility space implied by future customer demand for these types of retail goods and services exceeds the floor area planned for the Expansion Project in each of the use categories that were evaluated, we conclude that no adverse economic impacts will result in the regional market area that will be served by the Expansion Project. As a result, there is no compelling economic reason to further evaluate potential changes in the physical environment (e.g., "urban decay") that could be associated with the economic interactions between the Expansion Project and its market context.

I. INTRODUCTION

A. Purpose of the Analysis

This Report analyzes the potential for the operation of a 280,000 square foot GLA addition ("Expansion Project") to the Westfield Fashion Square ("Westfield Fashion Square"), an existing regional shopping center located in the community of Sherman Oaks in the San Fernando Valley, City of Los Angeles, to directly or indirectly cause "urban decay," as that concept has been defined in court decisions interpreting the California Environmental Quality Act (CEQA).¹

Analysis of the potential for new retail development to cause urban decay — ". . . a chain reaction of store closures and long term vacancies, ultimately destroying existing neighborhoods and leaving decaying shells in their wake" — suggests a two-part analysis. First, it must be determined whether the new retail development will attract retail sales away from existing and/or other planned future retail centers to any significant degree. Second, if so, it must be determined whether the severity of this change in economic circumstances will cause significant disinvestment to such a degree such that it is reasonably foreseeable that business closures, abandonment or other forms of physical deterioration or "urban decay" will result.

This report was prepared for Sherman Oaks Fashion Associates, L.P., the owner of Westfield Fashion Square, by HR&A Advisors, Inc. (HR&A), in association with Whitney & Whitney, Inc. (W&W). The two firms provide independent professional urban and other economic analysis to a wide range of public and private clients. Summaries of the firms' respective qualifications are included in Appendix A to this Report.

B. Overview of Westfield Fashion Square and Expansion

The following is a description of the existing Westfield Fashion Square regional shopping center and the proposed Expansion Project.

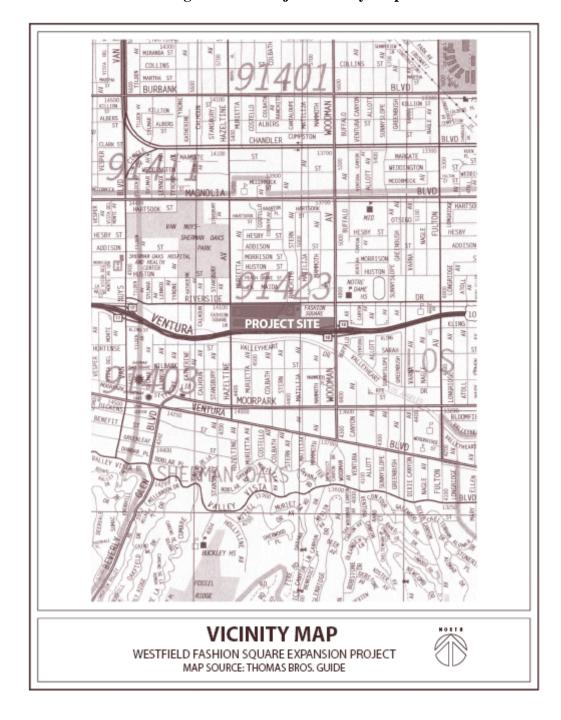
1. Project Location

The subject property is located along Riverside Drive between Woodman Avenue and Hazeltine Avenue at the existing Fashion Square shopping center. The entire shopping center is approximately 28.8 acres and is bordered by Riverside Drive to the north, Hazeltine Avenue to the west, the Ventura Freeway (101) to the south, and Woodman Avenue to the east within the Van Nuys–North Sherman Oaks Community Plan Area of the City of Los Angeles (see Figure 1).

² Collectively, Cal. Public Resources Code § 21000, *et seq.* and Calif. Admin. Code §15000 *et seq.*, commonly referred to as the "CEQA Guidelines."

² Bakersfield Citizens for Local Control v. City of Bakersfield (2004) 124 Cal.App.4th 1184 at 1204.

Figure 1 Regional and Project Vicinity Map



The project site is roughly rectangular covering almost the entire area bound by the roadways identified above. An approximately 3.0 acre parcel located at the southwest corner of the Riverside Drive/Woodman Avenue intersection that is currently developed with retail uses (Linens N' Things, a Ross store, a toy store, and a Bank of America) is not part of the project.

2. Project Background

The subject property is commonly known as the Fashion Square shopping center, which has been a vital commercial and retail portion of the Sherman Oaks community since the early 1960s. The entire shopping center is approximately 28.8 acres and is currently entirely developed with mall buildings or surface and structure parking. The shopping center features Macy's and Bloomingdale's department stores at the east and west ends of the center, respectively, as well as a collection of smaller retail stores and a food court. Under City of Los Angeles Department of City Planning case ZA-95-0899-CUZ, the shopping center was approved for a total of 975,000 gross leasable square feet, of which 867,000 square feet have been constructed to date, leaving an approved remainder of 108,000 gross leasable square feet.

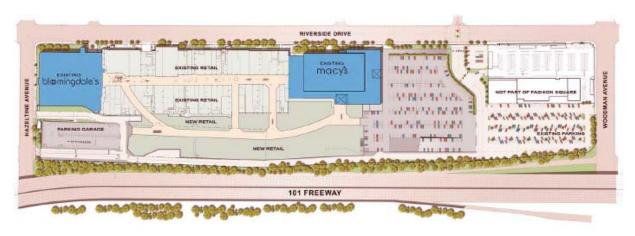
The proposed project includes construction of the remaining 108,000 gross leasable square feet of development previously permitted and the development of an additional 172,000 gross leasable square feet, for a total of approximately 280,000 gross leasable square feet of retail and restaurant uses. Accounting for mechanical/electrical equipment rooms, emergency access, tenant storage space, corridors and other City requirements, 280,000 gross leasable square feet is approximately 426,556 square feet and the building footprint is approximately, 482,740 square feet in size.

Land uses to the north, across Riverside Drive, include multi-and single-family residential properties. To the west, land uses include an office building west of Hazeltine Avenue, retail office, and City of Los Angeles Department of Water and Power uses at the intersection of Riverside Drive and Hazeltine Avenue. To the south, the site is bordered by the Ventura (101) Freeway. To the east, land uses include commercial along Woodman Avenue, south of Riverside Drive as well as the Notre Dame High School on the northeast corner of the intersection of Riverside Drive and Woodman Avenue.

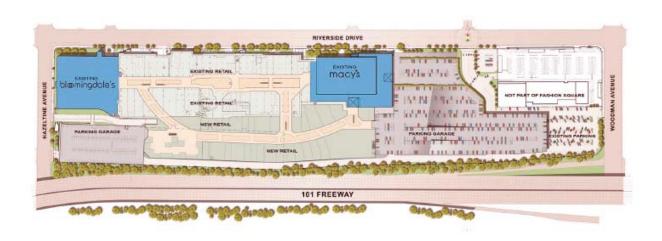
3. **Project Description**

The proposed Expansion Project will be located on the southerly portion of the site, primarily between the existing shopping center and the Ventura (101) Freeway. Due to the revised access scheme along Riverside Drive and construction on an enhanced parking structure for the site, a portion of the parking structure will extend toward Riverside Drive, between the existing Macy's building and the approximately 3.0 acre parcel at the southwesterly corner of the Riverside Drive/Woodman Avenue intersection that is not a part of this project (see Figure 2).

Figure 2. **Conceptual Site Plan**



Westfield Design 11601 Wilshow Blut, 11th Floor Law Angelos, CA 99025 **FASHION SQUARE LEVEL 1**



N For Str. 100 **FASHION SQUARE LEVEL 2**

The Expansion Project is proposed to the completed in one phase with two stages. The first stage would include the construction of a seven-level parking structure (one subterranean level, a grade level and five above-grade levels) south of the existing two-level parking structure serving the Macy's department store. The second stage would include demolition of the southern three-level parking structure serving the existing shopping center, and construction of two shopping mall levels, with one level of subterranean parking and rooftop parking. Construction of the Expansion Project is planned to be completed in 2010, making 2011 the first full year of operations. It is anticipated that stabilized operations in terms of retail and dining sales would be achieved by 2012.

The GLA in the Expansion Project will be distributed between two general tenant types commonly found in regional shopping centers. This distribution is detailed below in Table 4, together with projections of the expected sales volume per square foot of GLA for each type of space and the expected annual sales volume of the total addition expressed in 2007 dollars. The projected sales per square foot standards utilized in the table and at other places in this Report are based upon discussions held with market analysts at Westfield; published industry reports such as The Urban Land Institute's (ULI) Dollars & Cents of Shopping Centers biennial reports, discussions with other retail shopping center experts, and HR&A/W&W expert opinion of the market potential of the Westfield Fashion Square site.

Table 4 PROPOSED TENANT PROFILE, EXPANSION SPACE AT WESTFIELD FASHION SQUARE

Retail Space Category	Proposed <u>Tenant Type</u>	Square Feet GLA 1/	,	d Sales <u>2</u> / Ft GLA	Pro	jected Annual <u>Sales</u>
Shopper Goods 3/	Apparel/Homeware/ Other Specialty	240,000	\$	400	\$	96,000,000
Eating & Drinking	Food Court/Restaurants	40,000	\$	550	\$	22,000,000
	Total	180,000			\$	118,000,000

^{1/} GLA: Gross Leasable Area.

Source: Sherman Oaks Fashion Associates, L.P., Inc.; HR&A, Inc.; W&W, Inc.

A more detailed description of the proposed space in the Expansion Project floor area program is provided below:

Shopper Goods. Almost 86 percent of the proposed GLA in the Expansion Project, or 240,000 square feet, are to be allocated for "Shopper Goods." Also referred to as "Comparison Goods," this type of retail activity is the staple of regional shopping centers, as department stores and in-line retail stores selling Shopper Goods typically constitute at least 80 percent of the total occupied space. By definition, Shopper Goods encompass four types

^{2/} Measured in Constant 2007 Dollars.

^{3/} Shopper Goods, also referred to as Comparison Goods, refer to four categories of retail stores commonly found in regional shopping centers: Apparel and Accessories; General Merchandise except Drug Stores; Home Furnishings, Appliances and Related; and Specialty Retail items such as Books, Sporting Goods, Office Supplies and Jewelry.

of retail stores:³ apparel and accessories stores; general merchandise stores (most commonly, department stores); furniture, home furnishings, appliance and related stores; and specialty retail stores, encompassing a diverse array of retail shops selling such items as gifts, art goods, sporting goods, florists, photographic equipment, musical instruments, stationery, books, jewelry, and office and school supplies. Shopper or Comparison Goods derive their name from shopper behavior commonly related to their purchase. Characteristically, given the level of expenditure and the diversity of product choice involved, a shopper will travel a reasonable distance to compare prices and consider a range of alternative goods as part of the purchase decision.

Because department stores are not being considered for the Expansion Project, this analysis focuses on the three other major categories of Shopper Goods: Apparel and Accessories; Furniture, Home Furnishings and Appliances; and Specialty or "Other" retail stores.

Eating and Drinking Facilities. This use category will constitute a net addition of 40,000 square feet GLA, or slightly over 14 percent of the Expansion Project. Eating and drinking facilities will include both dinner restaurants offering full bar or wine and beer as well as fast-food units organized around a central food court. While a substantial amount of eating and drinking facility patronage will come from shoppers who are visiting other stores at Fashion Square, it is likely that there will also be local support for these facilities independent of shopping center customers that will be drawn from a local market that is best represented by a 3.0- mile radius around the existing center.

A preliminary distribution of the Expansion space by major retail space category is provided in Table 5 below:

Table 5 DISTRIBUTION OF SPACE BY MAJOR CATEGORY WESTFIELD FASHION SQUARE EXPANSION PROGRAM

Space Category	Square Feet Gross Leasable Area (GLA)
Retail	
Apparel	144,000
Furniture/Furnishings	24,000
Specialty/Other	72,000
Subtotal, Retail	240,000
Eating & Drinking	
Four Dinner Restaurants	30,000
13 Fast Food Units	10,000
Subtotal, Eating & Drinking	40,000
GRAND TOTAL	280,000

Source: Sherman Oaks Fashion Associates, L.P.

HR&A ADVISORS, INC. Page 10 August 2007

³ The definition of "Shopper Goods" generally follows the retail store classification system utilized by the State of California Board of Equalization.

C. The "Urban Decay" Concept in Environmental Impact Analysis

When a proposed development project is subject to CEQA, both direct and indirect (or "secondary") impacts of the project on the physical environment must be analyzed.⁴ Economic and social impacts of a project, though they may be included in a CEOA document, are not to be treated as "significant" impacts on the physical environment, 5 as defined. 6 To the extent that there is a direct or indirect causal connection between a change in economic or social circumstances and a change in the physical environment, the economic or social change may be used to establish whether the physical change is "significant."⁷

With this statutory and interpretive guidance in mind, the courts have recognized that there is a potential for a proposed new retail development to trigger economic competition with existing retailers in the project's host community. If existing retailers are adversely affected by this competition, declines in sales could directly result in and/or lead to disinvestment, business closures, abandonment and other forms of physical deterioration that are indicative of "urban decay." If the severity of this change in physical circumstances is so substantial that it adversely affects appropriate use of the area or otherwise threatens the public health, safety or general welfare, this situation may cross a threshold that defines a "significant impact" under CEQA, such that mitigation capable of reducing the impact on that physical environment must be considered.

Thus, for urban decay to be an issue within the meaning of CEQA, there must first be an adverse economic circumstance that is likely to be caused by a proposed project. If such an adverse effect is identified, then the severity of this economic impact must be evaluated for its potential to cause a significant change in the physical environment (i.e., "decay"). Accordingly, this Report presents an assessment of whether the proposed Expansion retail uses could reasonably be projected to cause adverse economic circumstances in the surrounding market areas applicable to the Expansion improvements. Only to the degree that such adverse circumstances can be predicted reasonably is there any need to evaluate the potential to cause "decay" or other significant physical changes in the environment.

Section II of this Report presents an analytic framework for assessing whether the Expansion development could cause adverse economic impacts on the surrounding retail market context, then applies this framework to the specific retail components of the Expansion improvements and their respective market areas. Appendix B includes further details on the data sources and projections used in this analysis.

Page 11 HR&A ADVISORS, INC. August 2007

⁴ CEQA Guidelines § 15358.

⁵ CEQA Guidelines §§ 15064 and 15382.

⁶ "A substantial or potentially substantial adverse change in the environment." (Public Resources Code § 21068). The focus on physical changes in the environment is further reinforced by §§ 21100 and 21151.

⁷ See, in general, CEQA Guidelines §§ 15131(a) and (b), and their associated discussion section.

II. METHODOLOGICAL APPROACH AND IMPACT MEASUREMENT

The analysis measures the degree to which the Expansion Project could result in a significant adverse economic impact on their respective market areas. Methodologically, any such impact is identified and measured by assessing the degree to which the amount of space planned for each Expansion Project retail and dining use category would exceed the anticipated increase in the supportable amount of retail and dining space that can be inferred from growth in future customer demand for comparable retail and dining in a defined market area. If proposed supply exceeds anticipated growth in demand, it could be argued that the Expansion Project could attract sales away from other existing or planned new retail and dining establishments of the same type. Such a finding, in turn, would require further investigation to assess whether it is foreseeable that this potential attraction of sales away from other retail and dining businesses could result in disinvestment, business closures, abandonment, other forms of physical deterioration that are effectively indicators of "urban decay." If, on the other hand, the amount of retail and dining space planned for Expansion Project is less than the amount of retail and dining space that can be supported by projected future demand, it can be concluded that the scale of potential customer demand is sufficiently large that it can support both the Expansion Project and all other existing and planned retail and dining space of the same general categories, and, as a result, there would be no need to evaluate the potential for urban decay.

Making these economic impact measurements typically requires: (1) establishing market area appropriate for each retail and dining category from which future customer demand will be generated; (2) projecting the likely increase in customer demand based on population growth, income growth and spending patterns for particular categories of retail goods and types of dining over a relevant time period (i.e., 2007-2012); and (3) converting the projected changes in future customer demand to amounts of supportable retail and dining GLA floor area, so that the level of change in demand can be compared directly to the projected change in GLA proposed for the Expansion Project.

Accordingly, separate market impact analyses were conducted for each of the three principal types of retail shopper goods space that are to be included in Expansion as well as the eating and drinking facilities space. These analyses are presented below.

A. Shopper Goods Space Impact Analysis

As noted above, the Expansion Project will provide a total of 240,000 square feet of Shopper Goods space that will encompass a variety of apparel, home furnishings and specialty goods retailers. In the existing Westfield Fashion Square, about 783,000 square feet (90% of total current floor area) are devoted to Shopper Goods space, per the ULI definition of shopper goods. The Expansion Project therefore represents a 31 percent expansion in the amount of shopper goods floor area at Westfield Fashion Square.

While this addition represents an important change in the center's composition of shopper goods merchandise, it does not alter the strong attraction of the exiting center, which is determined by its two Department Stores. Thus, the scale of the Regional Market Area (RMA) that applies to Westfield Fashion Square center today will be the same RMA that applies to the

Expansion Project. This RMA consists of the land area represented by all or a portion of 26 ZIP codes. The RMA, delineated in Figure 3 below, covers all or portions of the following cities and communities: Sherman Oaks; Toluca Lake; North Hollywood; Valley Village; Encino; Studio City; Van Nuys; Valley Glen; Tarzana; Bel Air Estates; Mount Olympus; Trousdale Estates; Beverly Glen; Brentwood; Hollywood; Hollywood Hills; City of Beverly Hills; and City of Burbank. These boundaries take into account customer patronage information provided by the center's on-site manager, the local and regional highway and road systems, locations of competitor centers, among other factors.

Figure 3: Shopper Goods Regional Market Area

(need better graphic to be prepared by environmental consultant)

Within this market area there are several shopping districts and community shopping centers which, to one degree or another, compete with Westfield Fashion Square. This competitive market supply principally includes the various retail offerings that are located along Ventura Boulevard from its origin at Lankershim Boulevard near Universal Studios on the east to Reseda Boulevard on the west. To a limited extent, the Beverly Hills Triangle, located within the RMA, is a source of competition for shoppers residing south of Mulholland Drive, along with a number of regional centers that are located on the periphery just outside the RMA, including:

Burbank Town Center (also known as Media Center); Westfield Topanga; Westfield Promenade; Westfield Century City; and Beverly Center.⁸

Estimates for the RMA prepared by Claritas, Inc., a well-accepted third party demographic data source, indicate that as of 2007 the population for the RMA is 728,332 persons. Existing per capita income⁹ is estimated at \$56,208, and the aggregate RMA personal income is estimated at \$40.9 billion. The demand for retail goods in the RMA is estimated at nearly \$13.6 billion in 2007, equivalent to 33.3 percent of the market area's aggregate personal income. As shown in Table 6, of this total retail demand, slightly under one-fifth (19.7%) can be expected to be captured by three categories of Shopper Goods stores: Apparel and Accessories Stores; Furniture, Furnishings and Appliance Stores (Household); and Specialty or "Other" Retail Stores.

Table 6
DISTRIBUTION OF NON-DEPARTMENT STORE SHOPPER GOODS
RETAIL SALES IN THE FASHION SQUARE RETAIL MARKET AREA

	Percent of
Retail Category	Retail Sales
Apparel and Accessories	4.41%
Furniture, Furnishings and Appliance	es 3.58%
Specialty ("Other")	11.71%
Total	19.70%

Source: California State Board of Equalization, 2005 Annual Report; HR&A, Inc.; W & W, Inc.

Together, these three shopper goods categories are anticipated to comprise nearly 86 percent of the Expansion space. As shown in a summary fashion in Table 7, and with greater detail in Table 8, between 2007 and 2012 the growth in demand in the RMA for these three retail store categories is projected to total over \$688.4 million, based upon anticipated population increases and personal income growth.

There is one older existing complex—the Laurel Plaza complex which is currently undergoing significant renovation—that is located within the RMA, but it serves a very different socioeconomic stratum and is not considered as a major competitive influence. Similarly, the Panorama Mall and Northridge regional shopping centers, located north and west of Westfield Fashion Square in the San Fernando Valley, though just outside the boundaries of the RMA, are not considered to be competitive, as their market orientations are very different from that of Westfield Fashion Square.

⁹ The per capita income measure utilized here is the personal income definition utilized by the Bureau of Economic Analysis as reported for residents of the State of California and each individual county. The percentage of personal income utilized for retail sales is based upon estimates of aggregate personal income for the state *vis a vis* total retail sales. For further detail on these relationships, please see the discussion of income concepts presented in Appendix B.

Table 7
SUMMARY OF PROJECTED INCREASES IN NON-DEPARTMENT STORE
SHOPPER GOODS RETAIL SALES DEMAND
IN THE FASHION SQUARE REGIONAL MARKET AREA, 2007-2012

	(in	millions)
Retail Category	200	07-2012
Apparel and Accessories	\$	154.1
Furniture, Furnishings and Appliances	\$	125.1
Specialty ("Other")	\$	409.2
Total	\$	688.4

Source: HR&A, Inc.; W & W, Inc.

Table 8
PROJECTED GROWTH IN DEMAND FOR SELECTED SHOPPER GOODS
WESTFIELD FASHION SQUARE REGIONAL MARKET AREA (RMA)
2007-2012

Net Change

		2	('000s) 007-2012	2007	2008	2009	<u>2010</u>	<u>2011</u>	<u>2012</u>
Regional Market Area (PMA) Population			42,102	728,332	736,564	744,889	753,309	761,823	770,434
Per Capita Personal Income		\$	10,549	\$ 56,208	\$ 58,175	\$ 60,211	\$ 62,319	\$ 64,500	\$ 66,757
Aggregate Regional Market Area Income ('000s))		\$	10,494,141	\$ 40,938,085	\$ 42,849,828	\$ 44,850,846	\$ 46,945,309	\$ 49,137,580	\$ 51,432,226
Percent of Personal Income Allocable for Retail Sales:				33.3%	33.3%	33.3%	33.3%	33.3%	33.3%
Potential Demand for Retail Sales ('000s))		\$	3,494,549	\$ 13,632,382	\$ 14,268,993	\$ 14,935,332	\$ 15,632,788	\$ 16,362,814	\$ 17,126,931
Calculation of Demand for Selected Shopper Goods by Majo	or Category	:							
	% of Total	Ne	et Change ('000s)						
	Demand	2	007-2012	2007	2008	<u>2009</u>	<u>2010</u>	<u>2011</u>	<u>2012</u>
Apparel	4.41%	\$	154,110	\$ 601,188	\$ 629,263	\$ 658,648	\$ 689,406	\$ 721,600	\$ 755,298
Incremental Growth in Demand by Year ('000s))					\$ 28,075	\$ 29,386	\$ 30,758	\$ 32,194	\$ 33,698
Cumulative Growth in Demand ('000s)					\$ 28,075	\$ 57,460	\$ 88,218	\$ 120,412	\$ 154,110
Household Furnishings, Appliances, et al	3.58%	\$	125,105	\$ 488,039	\$ 510,830	\$ 534,685	\$ 559,654	\$ 585,789	\$ 613,144
Incremental Growth in Demand by Year ('000s))					\$ 22,791	\$ 23,855	\$ 24,969	\$ 26,135	\$ 27,355
Cumulative Growth in Demand ('000s)					\$ 22,791	\$ 46,646	\$ 71,615	\$ 97,749	\$ 125,105
Specialty or "Other"	11.71%	\$	409,212	\$ 1,596,352	\$ 1,670,899	\$ 1,748,927	\$ 1,830,599	\$ 1,916,086	\$ 2,005,564
Incremental Growth in Demand by Year ('000s))					\$ 74,547	\$ 78,028	\$ 81,672	\$ 85,486	\$ 89,478
Cumulative Growth in Demand ('000s)					\$ 74,547	\$ 152,575	\$ 234,247	\$ 319,734	\$ 409,212

Source: California State Board of Equalization; Claritas, Inc.; HRA, Inc.; W & W, Inc.

Tables 9 (summary) and 10 (detailed presentation) translate the projected incremental change in RMA demand for Apparel/Household/Specialty goods into a measure of net supportable retail space, allowing for a threshold sales requirement of \$400 per square foot of GLA in 2007 to reflect the necessary basis for effective market support. This sales support requirement is expected to increase at the rate of three percent annually, reaching \$464 per square foot of GLA in 2012. Over the five-year analysis period, the projected increase in

supportable retail space for the combined Apparel/ Household/Specialty retail categories is nearly 1.5 million square feet.

Table 9 SUMMARY OF PROJECTED INCREASES IN SUPPORTABLE GLA FOR SELECTED SHOPPER GOODS IN THE FASHION SQUARE REGIONALMARKET AREA, 2007-2012

Retail Category		SF GLA
Apparel and Accessories		332,341
Furniture, Furnishings and App	oliances	269,791
Specialty ("Other")		882,474
T	otal	1,484,606

Source: HR&A, Inc.; W & W, Inc.

PROJECTED INCREASE IN SUPPORTABLE SPACE FOR SELECTED SHOPPER GOODS WESTFIELD FASHION SQUARE REGIONAL MARKET AREA (RMA) 2007-2012

	2007	2008	2009	<u>2010</u>	<u>2011</u>	2012
Projected Increase in Supportable Retail Space:						
Sales per Square Foot of GLA Requirement, Average: Base \$ 400 Annual Increase in Required Support 3.0%	\$ 40	\$ 412 \$	\$ 424 \$	\$ 437 \$	450	\$ 464
			In S	Square Feet GLA		
Supportable Apparel Space in GLA, Annual Increase		68,142	69,247	70,369	71,510	72,670
Cumulative Increase (Adjusted for higher sales requirement per square foot)		68,142	135,404	201,830	267,461	332,341
Supportable Funiture/Furnishings Space in GLA, Annual Increase		55,317	56,214	57,125	58,051	58,993
Cumulative Increase (Adjusted for higher sales requirement per square foot)		55,317	109,920	163,844	217,123	269,791
SupportableSpecialtyRetail Space in GLA, Annual Increase		180,940	183,873	186,854	189,883	192,962
Cumulative Increase (Adjusted for higher sales requirement per square foot)		180,940	359,542	535,924	710,198	882,474

Source: HRA, Inc.; W & W, Inc.

The proposed Expansion Project space allocation for each retail category is compared to the projected increase in supportable retail space within the RMA over the period 2007-2012 in Table 11. This analysis indicates that the proposed Expansion allocation for Apparel and Accessories space represents 43 percent of the projected net increase in supportable space in that category for the RMA between 2007 and 2012. Similarly, the proportion of Expansion Project space that will be developed for Household retail goods represents nine percent of the potential market increase in Household Goods supportable space over the same period, and the proposed allocation for Specialty retail goods represents about eight percent of the total market increase in supportable space for that space category.

Table 11 COMPARISON OF PROJECTED INCREASE IN MARKET DEMAND WITH PROJECTED EXPANSION SUPPLY SELECTED SHOPPER GOODS SPACE WESTFIELD FASHION SQUARE REGIONAL MARKET AREA (RMA) 2007-2012

	In Square Feet GLA											
	2007	2008	2009	2010	<u>2011</u>	2012						
Total Supportable Apparel Space in RMA		68,142	135,404	201,830	267,461	332,341						
Westfield Fashion Square Apparel Space				144,000	144,000	144,000						
Westfield Market Share of Increase in Demand				71%	54%	43%						
Total Supportable Furniture/Furnishings Space in RMA		55,317	109,920	163,844	217,123	269,791						
Westfield Fashion Square Furniture/Furnishings Space				24,000	24,000	24,000						
Westfield Market Share of Increase in Demand				15%	11%	9%						
Total Supportable Specialty Retail Space in RMA		180,940	359,542	535,924	710,198	882,474						
Westfield Fashion Square Specialty Retail Space				72,000	72,000	72,000						
Westfield Market Share of Increase in Demand				13%	10%	8%						

Source: HRA, Inc.; W & W, Inc.

Given the size of the existing RMA and its likely continued growth in population and per capita personal income over the five year period 2007 through 2012, the proposed Expansion Project will not have a significant impact on the existing base of Shopper Goods retail space in the RMA. Moreover, given Westfield management's intent to offer stores that will market to higher income households, the Expansion Project should not have a significant impact on the older centers on the periphery of the RMA in the San Fernando Valley that serve residents with more modest incomes.

В. **Eating and Drinking Facilities Impact Analysis**

While the demand for the Expansion Project's Eating and Drinking Facilities would logically be generated from the entire RMA, because shoppers at the center would be the most likely customers to patronize the available restaurants, it can be argued that the major source of market support for the Expansion Project's major dinner restaurants would come from the local residents of Sherman Oaks and other nearby communities that are found near the site. As a consequence, the Eating and Drinking Facilities analysis utilizes a 3.0-mile market radius as the basis for determining the magnitude of market support that exists for proposed Eating and Drinking Facilities at the Westfield Fashion Square site. The 3.0-mile market radius is identified in Figure 4 below:



Figure 4: 3.0-Mile Market Radius

Tables 12 (summary) and 13 (detailed presentation) provide a projection of the increase in Eating and Drinking Facilities demand for the period 2007through 2012 by utilizing an analytic approach similar to the one presented above that assessed the need for additional Shoppers' Goods retail space. The analysis considers two types of restaurant space for the Expansion: Dinner Restaurants, which would constitute about 30,000 square feet of the Expansion Project's GLA; and Fast Food Units organized around a central food court, representing about 10,000 square feet of GLA. In Table 12, the fast food units are considered to be comparable to restaurants that the California State Board of Equalization characterizes as "Restaurants, No Alcohol", while dinner restaurants would be considered as comparable to the State's category of "Restaurants with Alcohol." The anticipated growth in demand within the 3.0-mile market area for eating and drinking facilities over the period 2007-2012 should approach \$155.7 million.

Table 12 SUMMARY OF PROJECTED INCREASE IN EATING & DRINKING SALES DEMAND IN A 3-MILE MARKET AREA AROUND FASHION SQUARE, 2007-2012

		(in Millions				
Restaurant Category		200	7-2012			
Restaurants, No Alcohol		\$	79.6			
Restaurants with Alcohol		\$	76.1			
	Total	\$	155.7			

Source: HR&A, Inc.; W & W, Inc.

PROJECTED GROWTH IN DEMAND FOR EATING AND DRINKING FACILITIES WESTFIELD FASHION SQUARE 3.0-MILE MARKET AREA 2007-2012

			et Change 007-2012		2007		2008		2009		<u>2010</u>	<u>2011</u>	<u>2012</u>
Eating & Drinking Facility Market Area			15,504		228,558		231,578		234,638		237,738	240,879	244,062
Per Capita Personal Income (per Census Definition)		\$	9,661	\$	51,476	\$	53,278	\$	55,142	\$	57,072	\$ 59,070	\$ 61,137
Aggregate Regional Market Area Income ('000s)		\$	3,156,050	\$	11,765,252	\$	12,337,930	\$	12,938,485	\$	13,568,271	\$ 14,228,713	\$ 14,921,302
Potential Demand for Retail Sales ('000s)		\$	1,436,003	\$	5,353,189	\$	5,613,758	\$	5,887,011	\$	6,173,563	\$ 6,474,064	\$ 6,789,192
Calculation of Demand for Eating and Drinking Facilities by Major Category:													
	of Total emand		et Change ('000s) 007-2012										
Restaurants, No Alcohol	5.54%	\$	79,555	\$	296,567	\$	311,002	\$	326,140	\$	342,015	\$ 358,663	\$ 376,121
Incremental Growth in Demand by Year ('000s)						\$	14,436	\$	15,138	\$	15,875	\$ 16,648	\$ 17,458
Cumulative Growth in Demand ('000s)						\$	14,436	\$	29,574	\$	45,449	\$ 62,096	\$ 79,555
Restaurants with Alcohol	5.30%	\$	76,108	\$	283,719	\$	297,529	\$	312,012	\$	327,199	\$ 343,125	\$ 359,827
Incremental Growth in Demand by Year ('000s)						\$	13,810	\$	14,482	\$	15,187	\$ 15,927	\$ 16,702
Cumulative Growth in Demand ('000s)						\$	13,810	\$	28,293	\$	43,480	\$ 59,406	\$ 76,108

Source: California State Board of Equalization; Claritas, Inc.; HRA, Inc.; W & W, Inc.

Allowing for both classes of restaurants to achieve sales volumes approaching \$550 per square feet in 2007 as a threshold support requirement, by 2012 the anticipated increase in local area demand should be able to sustain additional restaurant space in an amount approaching 100,000 square feet for fast food units and over 95,000 square feet for restaurants serving alcohol. These projections are shown in Table 14 below:

PROJECTED INCREASE IN SUPPORTABLE SPACE FOR EATING AND DRINKING FACILITIES WESTFIELD FASHION SQUARE 3.0-MILE MARKET AREA 2007-2012

	2007	2008	2009		<u>2010</u>		<u>2011</u>	2012
Projected Increase in Supportable Retail Space:								
Sales per Square Foot of GLA Requirement, Average: Base \$ 550 Annual Increase in Required Support 3.0%	\$ 550	\$ 567	\$ 583	\$	601	\$	619	\$ 638
			In	Squ	are Feet Gl	LA		
Supportable Fast Food Restaurant Space in GLA, Annual Increase		25,482	25,944		26,414		26,893	27,381
Supportable Fast Food Restaurant Space in GLA, Cumulative Increase		25,482	50,684		75,622		100,313	124,772
Comparishing Dispuse Destaurant Concer in CLA Appropriate								20.405
Supportable Dinner Restaurant Space in GLA, Annual Increase		24,378	24,820		25,270		25,728	26,195

Source: HRA, Inc.; W & W, Inc.

Table 15 provides a comparison of the projected increase in supportable eating and drinking facilities space over the period 2007 through 2012 from local market sources with the proposed supply to be developed in the Expansion Project. The analysis indicates that for fast food units the Expansion Project represents about eight percent of the anticipated increase in supportable space; for dinner restaurants (restaurants serving alcohol), the Expansion Project represents about 25% of the total supportable space.

Table 15 COMPARISON OF PROJECTED MARKET DEMAND WITH PROJECTED EXPANSION SUPPLY **EATING AND DRINKING FACILITIES SPACE WESTFIELD FASHION SQUARE 3.0-MILE MARKET AREA** 2007-2012

	In Square Feet GLA											
Total Supportable Fast Food Restaurant Space in Market Area	2007	2008 25,482	<u>2009</u> 50,684	2010 75,622	2011 100,313	2012 124,772						
Westfield Fashion Square Fast Food Restaurant Space				10,000	10,000	10,000						
Westfield Market Share of Increase in Demand				13%	10%	8%						
Total Supportable Dinner Restaurant Space in RMA		24,378	48,488	72,346	95,967	119,366						
Westfield Fashion Square Dinner Restaurant Space				30,000	30,000	30,000						
Westfield Market Share of Increase in Demand				41%	31%	25%						

Source: HRA, Inc.; W & W, Inc.

Given the relatively small proportion of future supportable space that is represented by the Expansion Project's Eating and Drinking Facilities, it can be concluded that the development of this additional space at Westfield Fashion Square is not likely to have a major impact on the existing base of restaurants in the local market area, and most certainly is not likely to contribute to conditions that would lead to urban decay as defined by CEQA.

In addition to the quantitative demand/supply analysis presented above, field surveys were conducted of the 3.0-mile radius Eating and Drinking Facilities Market Area in order to determine whether there were any signs of vacancy or other physical conditions that might be exacerbated or otherwise negatively impacted by the proposed eating and drinking facility development program at Westfield Fashion Square. Based on findings from two visits to the market area, the primary commercial corridor in the region, Ventura Boulevard, exhibits significant economic vitality along its entire length within the six-mile diameter of the Eating and Drinking Facility market area. Only three vacant parcels of land were observed over the six miles, and these parcels were each under two acres in size. A thumbnail sketch of the road segments of Ventura Boulevard that are within the 3.0-Mile Market Area and that were evaluated is provided below, starting from the eastern perimeter of the study zone:

Ventura Boulevard, Woodley Avenue to Interstate-405. This is an area of mixed uses that is going through substantial redevelopment with higher density projects, particularly mid-rise office space. Retail recycling includes the development of two-story retail projects such as Encino Place that offer subterranean parking. There are also larger chain stores such as Marshalls, a hospitality use at the Interstate-405 freeway interchange, and a number of

residential and institutional uses. There is also a mix of fast food and chain sit-down restaurants, though no major concentrations per se of such restaurant uses. There are very few retail vacancies and virtually no vacant sites.

- Ventura Boulevard, Interstate-405 to Van Nuys Boulevard. This segment is dominated by the intersection of Ventura Boulevard with Sepulveda Boulevard, where there are high-rise office buildings and the transformed Sherman Oaks Galleria, which has become a life-style retail center and office space. Similar to the development activity on Ventura Boulevard west of the freeway, this segment is also undergoing intensification with recycling of older, single-story retail buildings to two-story retail complexes and mixed use projects. There was only one vacant lot fronting the Boulevard noted in this segment, a small, fenced parcel on the south side of the street with no obvious signs of development activity or presence of real estate signs. Vacancy rates for retail space are very low in this segment, probably three percent or less.
- Ventura Boulevard, Van Nuys Boulevard to Woodman Avenue. This segment is characterized by a great variety of smaller community-oriented retail and service businesses that occupy an array of different types of buildings, including newly-built commercial spaces and converted residential bungalows. The one significant redevelopment opportunity on Ventura Boulevard within the 3.0 mile radius is located in this area – a property located on the north side of Ventura Boulevard across from its intersection with Stansbury Avenue that is an assemblage comprised of an old restaurant named Barone's, a used car lot and possibly other smaller vacant properties that front on Moorpark Street. Barone's Restaurant has relocated to a new site on Woodman Avenue, thus remaining in the immediate area. According to a realtor at Piken Company who is representing the developer, the site is being redeveloped with a mixed use project that will include 16,500 square feet of commercial space, including 10,200 square feet of eating and drinking facilities, and affordable residential condominiums. The asking rates for commercial space is \$4.95 per square foot, triple-net (NNN).

Discussions with local realtors also revealed the following:

- There is significant demand for retail space on Ventura Boulevard, including demand from local-oriented restaurateurs who would never seek a mall location like a national chain restaurant that would locate in a regional mall.
- Demand for space is particularly high on the street segments situated between Woodman Avenue and Laurel Canyon Boulevard. Typically, empty retail spaces will be re-tenanted in about three weeks, and achievable rents are approaching \$5.00 to \$5.25 per square foot NNN, rents that are purported by realtors to be higher typically than rents obtainable for properties located on Ventura Boulevard in Encino.
- Part of the low availability of space relates to the fact that leases are typically longer-term, running from five to 10 years. Since there is low turnover, there is low availability.

HR&A ADVISORS, INC. Page 21 August 2007

- A second restaurant site, formerly the "Ventura," was also located in this segment. The property is on the south side of Ventura Boulevard at the southeast corner of Stern Avenue. While real estate signs indicating the availability of the property were noted in an initial field survey conducted in May 2007, they were no longer present during the June field survey and demolition of the existing structure had begun — an indication that redevelopment was imminent.
- Ventura Boulevard, Woodman Avenue to Coldwater Canyon Avenue. Along this segment, commercial uses and building composition continues to follow a pattern that is similar to the eastern portion of the Van Nuys-to-Woodman Ventura Boulevard segment until Fulton Avenue, where the south side of Ventura Boulevard changes to residential use until Van Noord Avenue. However, retail and service uses continue on the north side of the street for the entire length of the segment. Some vacancies in smaller, older facilities were noted on the south side of the Boulevard at Dixie Canyon, otherwise existing space is virtually fully occupied or undergoing renovation. Finally, there are several well-known local restaurants located along this segment that have been extremely successful over the years, and serve an older, local market that is likely to be different in demographic character from the likely composition of future patrons of eating and drinking establishments at Westfield Fashion Square.
- Ventura Boulevard, Coldwater Canyon Avenue to Laurel Canyon Boulevard. This segment resumes the pattern of commercial strip development on both sides of the Boulevard, featuring the well-known Sportsmen's Lodge Hotel and restaurant facility and a Ralph's neighborhood center at the Coldwater Canyon intersection. It then continues with an eclectic mix of retail uses that include large delicatessens, automotive-related retail, architects' offices and high-end boutiques. As noted above, rents in this area are at \$5.00 per square foot NNN, and there is very little space available.

The June 4, 2007 edition of the Los Angeles Business Journal carried an article indicating that the Sportmen's Lodge had been sold to local investor who has plans to renovate the existing hotel and add retail space. Reportedly, the project would retain the Sportmen's Lodge name, and would be renovated to evoke the "mid-century cool" of the San Fernando Valley of the 1950s. The article further indicated that as much as 300,000 square feet of retail space could be constructed on the site, but for the immediate future the property will continue to operate as a hotel, restaurant and banquet facility.

Ventura Boulevard, Laurel Canyon Boulevard to Tujunga Avenue. East of Laurel Canyon the Boulevard changes to a diverse mix of larger and smaller retail, service and officeoriented uses with generally less retail intensity. Major uses include the CBS Studio City Center; a shopping center anchored by Marshalls; a set of automobile-oriented retail stores and services; and an abundance of Chinese and Japanese restaurants. Once again, there are a few vacancies in the older residential buildings that have been converted to commercial use, but no vacant land or even parcels with obviously underperforming commercial uses that would be ripe for redevelopment.

Field investigations were also conducted along Van Nuys Boulevard, Coldwater Canyon Avenue, Woodman Avenue, Laurel Canyon Boulevard, Victory Boulevard, Burbank Boulevard, Magnolia Boulevard, Riverside Drive and Moorpark Street, the other major streets that serve as locations for retail activities within the 3.0 mile Eating and Drinking Facilities Market Area. Typically, these streets provide neighborhood-oriented and community-oriented convenience retail facilities, with the exception of Van Nuys Boulevard and Laurel Canyon Boulevard which are discussed below.

- Van Nuys Boulevard. The commercial character of this street provides for a number of different functions from its southern terminus near its intersection with Ventura Boulevard until it leaves the market area at Vanowen Street to the north. From Ventura Boulevard to the Ventura Freeway (U.S. Highway 101) the development pattern is very similar to that found along Ventura Boulevard a rich diversity of shops and services. From the Ventura Freeway to Magnolia Boulevard, the Boulevard is dominated by medical facilities and related services, including the Sherman Oaks Hospital and Health Center. From Magnolia Boulevard to Calvert Street the Boulevard performs as a region-serving automobile row with a series of auto dealers and auto-related service and parts businesses. From Calvert Street to Vanowen Street, the Boulevard becomes a convenience-oriented district with both public services and local retail uses, many with a South American character. While there are restaurants along the Boulevard's entire length, they are not likely to compete with the facilities planned for Westfield Fashion Square.
- Laurel Canyon Boulevard. Retail developments on Laurel Canyon Boulevard within the 3.0-Mile Market Area are dominated by an older complex centered at the Boulevard's intersection with Victory Boulevard. Two former regional centers have effectively merged together at this site: Valley Plaza north of the Victory Boulevard intersection; and Laurel Plaza to the south. Plans were announced in 2006 indicating that a joint venture between JH Snyder and Federated Department Stores would result in a mega-mall north of Victory Boulevard featuring Macy's as an anchor. To date, a new high school is under construction on portions of the project fronting Laurel Canyon Boulevard north of Hamlin Street. While no restaurant program has been announced for the project, it is unlikely that new development at this site would directly compete with new restaurant development at Fashion Square, because they would be serving different market needs and because the developer of the new center would have ample market knowledge of the Westfield Fashion Square program and plan the new facility's use mix accordingly.

In summary, several older restaurants located on sites along Ventura Boulevard have been closed in recent years and the underlying properties put up for redevelopment. Based on field surveys and interviews with real estate brokers and other professionals knowledgeable about the area, these closures do not appear to be indications of impending urban decay resulting from a condition of oversupply of dining opportunities in the market place, but are more likely a reflection that these facilities were "victims" of the strong real estate market that will support higher and better uses. Many of these older restaurants have experienced increasing difficulty providing adequate parking for their patrons, and landowners have found that the relatively large sites can be recycled to higher and more efficient uses.

III. CONCLUSIONS

Based on the foregoing analysis, it can be concluded that although the Expansion Project may be a new source of competitive supply in both the RMA for specified retail uses and the 3.0mile local market area for eating and drinking facilities, there is little possibility that the operation of the Expansion Project uses will result in significant adverse economic competition leading to a threat of "urban decay."

More specifically, the analysis of potential impacts has revealed the following:

Shopper Goods (Apparel, Furniture/Home Furnishings and Specialty Goods). Based on an analysis of the RMA for Westfield Fashion Square, the 240,000 square feet GLA of Shopper Goods space in the Expansion Project is projected to capture less than significant market shares of the anticipated growth in demand of Apparel and Accessories space; Furniture, Furnishings and Appliances space; and Specialty or "Other" retail space over the period 2007 through 2012, as shown in Table 16.

Table 16 EXPANSION PROJECT'S SHARE OF SUPPORTABLE SHOPPER GOODS SPACE IN THE FASHION SQUARE REGIONAL MARKET AREA

Expansion Area	Percent of RMA
Square Feet GLA	Supportable Space
144,000	43%
24,000	9%
72,000	8%
240,000	
	Square Feet GLA 144,000 24,000 72,000

Source: HR&A, Inc.: W & W, Inc.

This leaves substantial market share to be captured by other retailers in the RMA as well as allowing existing stores to expand their sales at rates above anticipated the inflationary growth rate. Thus, it may be concluded that the development of Shopper Goods uses in the Expansion Project will not be a cause of urban decay at any of the existing shopping centers and business districts found in the market area served by Westfield Fashion Square.

Eating and Drinking Facilities. Analysis of the potential impact of the proposed Eating and Drinking Facility component of the Expansion Project indicates that there is ample market support generated by the local resident population within a 3.0-mile market radius around Fashion Square to support the proposed addition of 40,000 square feet GLA of space planned for the Expansion Project. As summarized in Table 17, the market shares required to sustain the Expansion Project allow for significant future demand to be captured by existing and future competition.

Table 17 EXPANSION PROJECT'S SHARE OF SUPPORTABLE EATING & DRINKING FACILITIES SPACE IN A 3-MILE MARKET RADIUS AROUND FASHION SQUARE

		Ex	pansion Area	Percent of Local Market
Restaurant Category		<u>Squ</u>	are Feet GLA	Supportable Space
Fast Food Restaurants			10,000	8%
Restaurants with Alcohol			30,000	25%
	Total	_	40,000	

Source: HR&A, Inc.: W & W, Inc.

Because the addition of the proposed eating and drinking uses in the Expansion Project will not have a significant negative impact on the existing supply of competitive uses in the local market area, this component of the Expansion Project will not lead to urban decay at any of the existing shopping centers and business districts found in the competitive market area.

Field surveys of the primary commercial streets adjacent to the Expansion Project confirm that the area is experiencing significant reinvestment as older facilities and larger sites transition into new commercial development, including new retail and eating and drinking facilities.

Given the finding that the scale of supportable retail and dining space that can be supported by future customer demand exceeds the retail and restaurant floor area planned for each respective category of retail and eating and drinking facility space planned for the Expansion Project, and that the commercial area surrounding the site is experiencing significant reinvestment, it can be concluded that no adverse economic impacts are likely to result in the market areas applicable to the Expansion Project. Therefore, there is no requirement to further evaluate potential changes in the physical environment that are associated with the economic interactions between the Expansion Project and its market context.

In addition, field surveys indicate that although several older restaurants located on sites along Ventura Boulevard have been closed in recent years and the underlying properties put up for redevelopment, these closures do not appear to be indications of impending urban decay resulting from a condition of oversupply of dining opportunities in the market place. Rather, this is a consequence of a strong real estate market that will support higher and better uses.





HR&A ADVISORS, INC.

Economic Development, Real Estate Advisory & Public Policy Consultants

QUALIFICATIONS TO PREPARE CEQA/NEPA DOCUMENTATION ON SOCIOECONOMIC ISSUES

HR&A Advisors, Inc. (HR&A) is a full service policy, financial and management consulting firm. Founded in 1976, the firm has a distinguished track record of providing realistic answers to complex economic, economic development, public finance, real estate, housing and strategic planning problems. HR&A clients include Fortune 500 corporations, all levels of government, the nation's leading foundations, and not-for-profit agencies. The firm has extensive experience working for the legal community in such roles as court-appointed special master, consent decree monitor, technical advisor and expert witness.

HR&A's practice lines include local and regional economic analysis, economic development program formulation and analysis, fiscal impact analysis, real estate analysis and advisory services, housing policy research and analysis, population forecasting and demographic analysis, and transportation and other capital facilities analysis and financing.

Among the qualities for which HR&A is widely known and respected are the impeccable quality of its analysis, ability to invent new analytic methods and approaches to suit the needs of a particular client, independent professional judgment honed through extensive exposure to the rigors of the public review process and the scrutiny of the judicial system, the ability to translate complex technical analysis for a variety of non-technical audiences, and the extensive involvement of its Partners in every project it accepts.

The firm's domestic and international consulting is provided by a staff of 30 people located in offices in Los Angeles and New York. Staff members include public finance professionals, planners, economists, architects, lawyers, and experienced project managers. Virtually every member of the firm has substantial public or private sector experience in economic, financial and policy analysis, real estate development and planning.

HR&A has frequently been called on by its public and private sector clients to provide analysis of population, housing, employment, economic, public school facilities and induced growth impacts for projects subject to the California Environmental Policy Act and the National Environmental Policy Act. The following are examples of projects that illustrate this experience.

Los Angeles New York

For Public Sector Clients

- For the City of Lancaster, HR&A is preparing economic, fiscal and "urban decay" analysis for EIRs on the Lane Ranch Towne Center and The Commons at Quartz Hill, two regional shopping centers planned for opposite corners at 60th and Avenue L.
- For Los Angeles World Airports, HR&A prepared all of the economic impact analyses needed to evaluate alternative Master Plan concepts for future development of Los Angeles International Airport. The project included extensive econometric modeling of future baseline (pre-project) economic conditions and forecasts of conditions under alternative development scenarios in the City of Los Angeles, the County of Los Angeles, incorporated and unincorporated areas adjacent to the airport, and the surrounding five-county region.
- For the City of Chicago Department of Aviation, HR&A prepared regional and local economic and fiscal impact analyses of the O'Hare Modernization Program (OMP), which was be used by the Federal Aviation Administration to prepare an Environmental Impact Statement on the project. The analysis includes econometric modeling of the six-county Chicago regional area to forecast the employment, total economic output, population and households, among other factors, that would be associated with the \$16-billion OMP project, as compared with a No Project scenario.
- For the City of Los Angeles Environmental Affairs Department, HR&A prepared draft Initial Study screening criteria, thresholds of significance and recommendations for analysis approach on the topics of housing, population and employment impacts.
- For Central City West Association and the City of Los Angeles, HR&A prepared a demographic portrait and forecast, and baseline "jobs/housing balance" analysis as part of the Central City West Specific Plan, a transitional neighborhood located directly north of Pico-Union, and across the Harbor Freeway, from the Los Angeles central business district. HR&A's analysis was used as the technical basis for the population, housing and employment sections of the EIR on the Plan. The firm also assisted counsel for interested parties regarding these issues during subsequent litigation over the adequacy of the Final EIR, which was ultimately decided in favor of the City.
- For the Santa Monica-Malibu Unified School District, HR&A managed a detailed review of the options available to the District to consolidate use of its four properties in the Ocean Park neighborhood of Santa Monica, an area which had been experiencing significant enrollment declines. The project included managing the preparation and certification of an EIR on the multi-site strategy adopted by the Board of Education, which included construction of the first new elementary school since the 1950s.
- For the University of California, Los Angeles, the firm prepared an analysis of the degree to which employment and housing associated with UCLA's 1991 Long Range Development Plan was consistent with the emerging regional planning concept of "jobs-housing balance." The firm's analysis was included as a technical appendix to the Final EIR on the Plan, which received approval by the Regents of the University.
- Also for the University of California, Los Angeles, HR&A prepared the population and housing section, and contributed to the induced growth section of the EIR on the 2000-2010 Long-Range Development Plan Update for the campus. The Final EIR was certified by the Regents.
- For the University of California, Santa Barbara, HR&A analyzed the public school impacts of the 1992 Long-Range Development Plan for the Santa Barbara campus, and prepared a Supplemental Environmental Impact Report on this issue, pursuant to a judgment against the University in an action brought by the Goleta Union School District. The Supplemental EIR was certified by the Regents of the University. Upon return to the writ, the court found that the analysis adequately supported the Regent's action. This determination was upheld by the Second District Court of Appeal in Goleta Union School District v. Regents of the University of California, 36 Cal. App. 4th 1121 (1995) (opinion on rehearing), holding that the University was not required to pay school mitigation fees.

• For the Southern California Association of Governments (SCAG), HR&A prepared the economic and fiscal impact sections of the EIR on SCAG's 1996 Regional Comprehensive Plan and Guide.

For Private Sector Clients

- For Westfield Corporation, HR&A prepared "urban decay" and public services impact analyses for a 100,000 square foot addition to the existing Westfield Santa Anita super-regional shopping center in Arcadia.
- For Bisno Development Company, HR&A is preparing technical reports on the population, housing employment and school facilities impacts of a 2,300-unit condominium project proposed for a former US Navy housing site in the San Pedro-Wilmington area of Los Angeles.
- For General Growth Properties, HR&A prepared detailed comments on various socio-economic issues in the Draft and Final EIR for the Americana at Brand, a "lifestyle" mall proposed for a site immediately adjacent to the Glendale Galleria in Glendale.
- For Universal Studios, Inc., HR&A analyzed the employment, housing, population and economic and fiscal impacts in Los Angeles County of a proposed \$3 billion Specific Plan that will nearly double the intensity of development at Universal City, the home of Universal Studios, Inc.'s film studio, studio tour, various entertainment retail uses, commercial office buildings and hotels. HR&A's analyses were included in the project's Draft EIR. HR&A is now preparing similar analyses for the EIR on the new Universal City Vision Plan being proposed by NBC Universal.
- For the Ratkovitch-Villaneuva Partnership, HR&A prepared the employment, housing, population and public schools impact analyses for the EIR on a proposal to construct 10 million square feet of new commercial and residential development around the City of Los Angeles' Union Station. The Draft EIR was certified by the Los Angeles City Council.
- For St. John's Hospital and Health Center, HR&A prepared analyses of the economic and fiscal impact of current health center impact on the economy of the City of Santa Monica, and the impact that will result from each of two phases of a major reconstruction of the health center following the 1994 Northridge earthquake. The analysis was relied on by the City's consultants in preparing the project's EIR, which was certified by the Santa Monica City Council. HR&A also prepared analysis for the Health Center on the degree to which draft police services mitigation measures being considered by the City met the requirements of CEQA.
- For The Walt Disney Company, HR&A prepared a comprehensive analysis of the employment, population, housing, "jobs-housing balance" and vehicle miles traveled impacts of Downtown Disney and Disney's California Adventure, in Anaheim. The firm's analysis is contained in a series of technical appendices to the EIR, which was certified by the Anaheim City Council.
- Also for The Walt Disney Company, HR&A analyzed the "jobs-housing balance" implications of a proposal to consolidate all of Disney's studio and studio-related administrative facilities on a single site in the City of Burbank. HR&A's analysis was included as a technical appendix to the project's EIR, which was certified by the Burbank City Council.
- For Wilshire-Barrington Associates, HR&A analyzed the population, housing, employment and jobs-housing balance impacts of a preliminary concept for converting the Barrington Apartments in West Los Angeles into a mixed-use project consisting of 700 apartments, a 262-room hotel, 210,000 s.f. of office space plus miscellaneous retail.
- For the Santa Monica Beach Hotel Development Partnership, HR&A coordinated an extensive review and prepared the Draft EIR comment letter for the developer of a proposed 160-room luxury hotel and community center proposed for a parcel of State-owned land along Santa Monica Beach.

- For Reliance Development Group, HR&A coordinated an extensive review and prepared the Draft EIR comment letter for the developer of a 1.8 million square foot office park and studio complex proposed for surplus land at Santa Monica Airport.
- For Maguire Thomas Partners, HR&A coordinated an extensive review and prepared the Draft EIR comment letter for the developer of a proposed office building and hotel project to be developed on Ocean Avenue in the City of Santa Monica.

REPRESENTATIVE LIST OF CLIENTS

Financial Institutions & Investment Companies

American Council on Life Insurance Citibank Private Banking Group

Citicorp Real Estate, Inc.

Community Preservation Corporation

First Union National Bank Fleet Financial Group

Goldman Sachs

Hartland Asset Management

Lehman Bros.

Shorebank Corporation

Real Estate Development Organizations and Private Companies

ARC Development

ARCORP Properties

Bermant Development Company

Boeing Realty Corporation

Casden Properties, Inc.

Castle & Cook Development Company

Centex Homes

Continental Development Corporation

Daniel Island Development Company

Disney Development Corporation

Edward J. Minskoff Equities

Gaylord Entertainment

General Growth Properties

Gibson Speno LLC

Home Depot Company

JMB Urban Realty Corporation

K. Hovnanian Companies of California

Landmark Land Company

Madison Square Garden

Maefield Development Corporation

Maserich Company

Maguire Thomas Partners

Millennium Partners

Newhall Land & Farming Company

New York Times Company

Olympia & York (USA)

The Related Companies

Reliance Development Group

Santa Monica Beach Development

Corporation

Starrett Housing Corporation

Sunset Development Corporation

Tishman Speyer Properties

Trammell Crow Company

Trammell Crow Residential

TransAction Companies, Ltd.

Twentieth Century Fox

Universal Studios, Inc.

The Walt Disney Company

Westfield Corporation, Inc.

William Lyon Homes

World Financial Properties

Public Development Agencies

Alliance for Downtown New York

Battery Park City Authority

Brooklyn Bridge Park Development

Brooklyn Navy Yard Development

Corporation

Catskill Watershed Corporation

Catholic Charities of Brooklyn

Cincinnati Business Committee

Columbus Downtown Redevelopment

Corporation

Downtown Brooklyn Local Development

Corporation

Economic Development Growth

Enterprises, Oneida Co., NY

Empire State Development Corporation

Inland Valley Development Agency

Memphis Riverfront Development Corp.

National Capital Revitalization Corp.

New York City Economic Development

Corporation

New York State Urban Development

Corporation

Penmar Development Corporation

Port Authority of New York and

New Jersey

Queens West Development Corporation

Cultural, Recreational & Special Events Clients

American Museum of Natural History

Brooklyn Academy of Music

Corporation

Brooklyn Museum of Art

City of New Haven Arts &

Entertainment Facilities Committee

Lincoln Center for the Performing Arts

Madison Square Garden

New Jersey Performing Arts Center

NYC2008

Public Space for Public Life

Randall's Island Sports Foundation

The Trust for Public Land

Other Quasi-Public and Non-Profit Organizations and Foundations

Apartment Association of Greater

Los Angeles

The Bowery Mission

Common Ground Community

Cornell University

Corporation for Supportive Housing

Community Services Society of

New York

Other Quasi-Public and Non-Profit Organizations and Foundations (con't.)

The Enterprise Foundation

Ford Foundation

Gay Men's Health Crisis

Griffiss Local Development Corporation

Harry Frank Guggenheim Foundation

Kaiser Permanente

Local Initiatives Support Corporation

Los Angeles Collaborative for Community

Development

Metropolitan Boston Housing Partnership

Metropolitan Jewish Geriatric Center

National Equity Fund

Neighborhood Progress, Inc.

New York Blood Center

Newark Alliance

Saint John's Hospital and Health Center

Saint Vincent's Hospital

San Gabriel Valley Council of Governments

Spanish-American Merchant's Assoc.

University of California, Los Angeles

University of California, Santa Barbara

Upper Manhattan Empowerment Zone

Development Corp.

Williamsburg Affordable Housing

Westside Urban Forum

Governmental Agencies

Boulder Urban Renewal Authority

City of Berkeley Rent Stabilization Board

City of Beverly Hills

City of Chester (PA)

City of Columbus

City of Culver City (CA)

City of Detroit

City of Houston

City of Huntington Beach (CA)

City of Indianapolis

City of Lancaster

City of Los Angeles

City of New York

City of Olathe (KS)

City of Phoenix

City of San Luis Obispo (CA)

City of Santa Monica

City of West Hollywood (CA)

City of Yonkers

Community Redevelopment Agency of the

City of Los Angeles

Compton Unified School District (CA)

County of Santa Barbara

District of Columbia

New Jersey Department of Commerce and **Economic Development**

Redevelopment Authority of the

City of Philadelphia

San Diego Association of Governments

Santa Ana Unified School District (CA)

Santa Monica-Malibu Unified

School District

Southern California Association of

Governments

Yonkers Office of Downtown &

Waterfront Development

Transportation Agencies

City of Chicago Department of Airports Connecticut Dept. of Transportation Delaware Dept. of Transportation

Los Angeles County Metropolitan

Transportation Authority

Los Angeles World Airports

Massachusetts Bay Transportation

Authority

New Jersey Transportation Corp.

New York Metropolitan Transportation

Authority

San Diego County Regional Airport

Authority

U.S. Dept. of Transportation

Housing Agencies

Chicago Housing Authority

Community Redevelopment Agency of the

City of Los Angeles

Cuyahoga Metropolitan Housing Authority

(IN)

Detroit Housing Commission

Housing Authority of Baltimore City

Housing Authority of the City of Houston

Housing Authority of the County of Los

Angeles

Housing Authority of the City of Santa

Monica

Housing Bureau, City of Long Beach

Indianapolis Housing Authority

Los Angeles Housing Department

New York City Housing Authority

New York City Housing Development

Corporation

New York State Housing Finance Agency

Omaha Housing Authority (NE)

Philadelphia Housing Authority

Redevelopment Authority of the City of

Philadelphia

St. Louis Housing Authority (MO)

United States Department of Housing and Urban Development

WHITNEY & WHITNEY, INC.

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Whitney & Whitney, Inc. (W&W) is a real estate development advisory services firm located in Los Angeles, California. The company was founded by William H. Whitney, Ph.D. in 1984. After six years of serving the southern California and Hawaii markets, W&W reduced the scope of its activities when Mr. Whitney was recruited by Arthur Andersen to assist their Real Estate and Hospitality/Leisure consulting practices in establishing both a national and international presence.

Mr. Whitney served with Arthur Andersen for over nine years, participating on major real estate and hospitality consulting engagements in over 40 different countries throughout the world. Activities during this period also included starting Arthur Andersen's Asia/Pacific Region real estate consulting practice in Manila, and spending three years in Andersen's London offices serving as a resource for the European and Middle East real estate consulting practices.

Following his return to the United States in March 2000 Mr. Whitney has re-activated Whitney & Whitney, Inc. The firm's major focus is on the provision of real estate consulting services to both public and private clients in the following areas:

- Due diligence services for companies involved with the acquisition and operation of real estate assets;
- Participation on multi-disciplinary teams with architects, planners and other design professionals in the planning of resorts, new communities and urban mixed-use projects
- Advisory services related to the maximization of returns from corporate real estate assets;
- Advisory services related to the maximization of public benefits from proper utilization of public lands;
- Market feasibility studies for large scale land development programs, including waterfront projects, shopping centers, resorts, and new communities;
- Master planning for large-scale urban parks and open space programs;
- Financial feasibility studies for proposed real estate investments;
- Negotiation assistance related to the formation and implementation of public/private partnerships;
- Fiscal impact, economic impact, cost-revenue and cost-benefit evaluations of proposed real estate development activities for public agencies and private developers;
- Valuation/expert witness services related to complex real estate transactions and/or arbitration and litigation proceedings; and
- Implementation services related to attaining necessary development entitlements and funding for real estate programs.

W & W's recent projects include the following: since the early 1990s has served as a real estate economic and financial advisor to the State of Hawaii Aloha Tower Development Corporation related to the redevelopment of the downtown Honolulu waterfront; performed a market and financial analysis of a proposed "high technology" park/mixed-use commercial development program in Dubai, United Arab Emirates known as Dubai Internet City; conducted an analysis of the economic feasibility of converting the 4,700-acre El Toro Marine Corps Air Station to an urban park; conducted an analysis of the redevelopment potentials for tourist-serving projects in the Old City of Shanghai; provided a market analysis of the retail redevelopment potential for the International Market Place in Waikiki for the Queen Emma Foundation; performed an evaluation of redevelopment potentials and the resultant fiscal impacts from conversion of certain industrial lands to retail and other uses for the City of San Jose; provided an evaluation of the market feasibility for residential and commercial retail uses on surplus lands owned by Ohlone Community College, Fremont, California; evaluated the market and financial opportunity for development of a major shopping center near Mililani Town on the Island of Oahu, Hawaii for Forest City: and reviewed the market for office and retail commercial uses near the East Eisenhower Transit Station for the City of Alexandria, Virginia; and a market study for a C. J. Segerstrom & Sons development project located near South Coast Plaza in Orange County. Currently, the firm is serving as an advisor to Castle

& Cooke on the preparation of a master plan and development strategy for 28,000+/- acres of land located on the North Shore of the Island of Oahu; providing a review of the master plan for the Sa'adiyat Island resort located in Abu Dhabi, United Arab Emirates; and preparing market/financial analyses and a business plan for a proposed destination spa to be located in the Santa Monica Mountains.

Mr. Whitney's background in the analysis of major shopping center developments and the planning of their adjacent lands supersedes the formation of W & W. He has been conducting investigations of retail development opportunities for nearly 40 years, starting with the re-use of the Chevron properties located in El Segundo and Manhattan Beach that ultimately led to the development of Manhattan Beach Village. One such project, the planning of the Puente Hills Mall and its immediate surrounding lands for the Western Harness Racing Association in 1970, was the inspiration for his doctoral dissertation, "An Investigation of Selected Impacts on Surrounding Lands Which are Generated by Development of Regional Shopping Centers" (UCLA, 1975).

A partial listing of Mr. Whitney's shopping center experience includes the following:

ERNEST W. HAHN, INC. (NOW TRIZECHAHN): Regional Shopping Center Market Analysis and Economic/Fiscal Impact Studies, California and Washington

Conducted numerous market feasibility and economic/fiscal impact studies of proposed regional shopping centers for the Ernest W. Hahn Company, forerunner to TrizecHahn, including analyses for the following existing regional shopping centers: Puente Hills Mall, City of Industry; Mariner's Island, San Mateo; North County Fair, Escondido; Kelso Mall, Kelso, Washington; and Sierra Vista, Clovis, California.

PSB REALTY CORPORATION: Costa Mesa Courtyards, Costa Mesa, California

Performed market and financial feasibility studies for the Costa Mesa Courtyards, a 173,000 square foot shopping center once honored as the "Best Retail Development" in the Western States at the Pacific Coast Builders Conference. The 11-acre project has been an important stimulus to the revitalization of the City of Costa Mesa's old central business district.

JAMES YOUNGBLOOD, DEVELOPER: The Lumberyard, Encinitas, California

Conducted market and financial feasibility studies for the project, a specialty retail center with 80,000 square feet of retail space located in the City of Encinitas. The center has been successfully developed, and has performed at or above initial market expectations.

THE IRVINE COMPANY: Fashion Island and Spectrum Center Impact Studies, Newport Beach and Irvine. California

Conducted economic and fiscal impact evaluations of these two major centers as part of their submissions for general plan amendments to the Cities of Newport Beach and Irvine, respectively. The Fashion Island expansion program focused on the interactive benefits that could be generated between the existing and proposed retail uses and the surrounding hotel and office developments; in contrast, the central concern regarding the proposed Spectrum project was its potential sales and property tax generation for the municipality.

LIVERPOOL DEPARTMENT STORE AND THE FRANSEN COMPANY: Regional Shopping Center Market Evaluations, Various Metropolitan Areas, Mexico

Conducted detailed investigations of the market opportunities for Liverpool Department Store to serve as an anchor tenant and developer of regional shopping centers throughout Mexico. A number of sites in major metropolitan locations were evaluated, and projections were made of potential store sales and supportable retail space. As of 2001, the study had resulted in one new shopping center currently operating in the Mexico City metro area and a second project under construction.

MITSUI TRUST & BANKING CO., LTD.: Aloha Tower Marketplace, Honolulu, Oahu, Hawaii Provided a market validation study for a festival marketplace that was under construction in downtown Honolulu. The development program, which ultimately became the Aloha Tower marketplace, called for approximately 200,000 square feet of retail and restaurant space at Honolulu Harbors Piers 7, 8 and 9 adjacent to the historic Aloha Tower. The analysis included a thorough examination of each segment of the potential customer base and an assessment of the potential expenditure patterns at the center from

those identified market segments. The results of the market studies were then utilized to generate sales projections for the center.

THE ROBERTS GROUP: Wood Ranch Development Program, Simi Valley, California

Performed an analysis of retail commercial potentials for a major community shopping center located in the Wood Ranch planned community. The study involved a detailed assessment of competitive retail projects found within the immediate market area surrounding Wood Ranch and a determination of market support generated by Wood Ranch residents. The center is open and operating successfully.

A&B HAWAII, INC./VANGUARD PROPERTIES: Triangle Square Factory Stores, Kahului, Maui, Hawaii

Provided a market analysis of a proposed factory outlet center in Kahului, Maui near the Kahului Airport. The development program called for 110,000 square feet of retail space to be built at one of Maui's most important highway junctions. The analysis included an examination of the potential customer base, consideration of the potential expenditure patterns by the major market segments, and a projection of potential sales at the project. The project has been developed and is operating successfully.

CITY OF VISALIA: Regional Shopping Center Location Studies; Visalia, California

Served the City of Visalia as market and planning consultants in the evaluation of potential locations for new regional shopping center facilities in the City of Visalia. The analysis included an assessment of the market, fiscal, transportation and other economic and social impacts related to the alternative sites under consideration for the new center.

AMFAC/JMB HAWAII, INC.: Kaanapali North Beach Entertainment / Retail Center Feasibility Studies, Kaanapali, West Maui, Hawaii

Provided a detailed assessment of a proposed themed entertainment/retail attraction at North Beach. A number of different retail and entertainment concepts were evaluated for the property, including specialty retail alternatives similar to Whaler's Village and more elaborate commercial recreation complexes featuring entertainment venues similar to Church Street Station in Orlando, Florida. The major finding of the study was that the most profitable use in terms of land utilization and environmental constraints was a major health spa, as this use generated the highest visitor expenditures per unit of land area and required relatively low market penetration of the existing visitor base.

CASTLE & COOKE PROPERTIES, INC.: Iwilei District Market Feasibility Study, Honolulu, Hawaii Conducted market feasibility studies to provide development guidelines for the redevelopment of the 50-acre Iwilei property. The site is located near downtown Honolulu in an area transitioning from industrial to commercial uses, and was previously occupied by the Dole Cannery. The market analysis concentrated primarily on the market potential for outlet-type retail shopping activities and "bull-pen"-type office space. Major issues raised by the study pertained to the site's relative accessibility for both local residents and visitors.

CASTLE & COOKE PROPERTIES, INC.: Mililani Town Center Market Assessment, Mililani Town, Oahu, Hawaii

Conducted a market analysis of the existing Mililani Town Center, a 166,500 square foot community shopping center located in central Oahu. The primary purposes of the investigation were to first, assess the current market performance of the center given its location, configuration and competitors; second, determine a strategy for expansion of the center to 400,000 square feet of space after giving full consideration to future market positioning, product mix and anchor tenants. Attention also focused on expanding the range of activities at the center to include a variety of service functions in addition to the retail tenants.

CITY OF LAWNDALE: South Bay Galleria Buyout, Redondo Beach, California

Provided a financial evaluation of the ownership interest held by the City of Lawndale in the South Bay Galleria, a regional shopping center that was undergoing renovation by Forest City Development Company. The work performed by the consultant formed the basis for the city's successful sale of its interest in the project to the developer.

CITY OF PASADENA: Lake/Washington Neighborhood Shopping Center, Pasadena, California Analyzed the development potential for a major new neighborhood shopping center intended to revitalize an older shopping district in Pasadena. The study involved an extensive review of existing businesses in order to assess both the positive and negative impacts of the new facility. The center has been constructed with a supermarket and drug store as the anchor tenants, and has successfully fostered revitalization of the entire district with new commercial development.

MAGUIRE THOMAS PARTNERS: Peter's Landing Specialty Center, Huntington Harbour, California Provided market and financial consulting services to Peter's Landing, a specialty retail center and marina complex located in the affluent waterfront residential community of Huntington Harbour. Initially, the focus was on evaluating the market potentials for boat slips and retail and office uses. Later, attention was focused on evaluating the financial trade-offs between retention of the marina as a rental program and sale of the berths under a "dockominium" concept.

THE IRVINE COMPANY: Mervyn's Retail Location Study, Various Locations, Orange County
Assisted The Irvine Company (TIC) in evaluating potential alternative locations for Mervyn's department
stores on various properties owned by TIC. The study considered both the provision of "blanket"
coverage by the chain store throughout Orange County with multiple locations as well as an evaluation of
specific sites on TIC lands. Presented results of the study to Mervyn's leadership in Minneapolis.

SAN DIEGO UNIFIED PORT DISTRICT: Embarcadero Master Planning Program Feasibility Studies San Diego, California

Performed market studies leading to the establishment of Seaport Village, a leading specialty retail center of about 200,000 square feet located on the San Diego waterfront. Other market and related investigations have led to development of hotel, marina, convention center and cruise ship terminal facilities along the Embarcadero.

CITY OF IRVINE: Retail Commercial Needs Assessment Study, Irvine, California

Prepared a retail commercial needs assessment for the City of Irvine that considered the long term demand for and supply of retail commercial space in the community. One of the sites investigated ultimately became the Spectrum specialty/entertainment center. The results of the study were somewhat controversial, as the analysis was critical of a number of the existing and proposed retail locations in the residential villages of Irvine with respect to their long term economic viability.

DAVID HOCKER & ASSOCIATES: Shelter Cove Shopping Centers, Palmetto Dunes, Hilton Head, South Carolina

Performed market investigations of the potential for (1) a 200,000 square foot specialty retail shopping center anchored by "downsized" department stores, and a (2) 120,000 square foot convenience retail center. While the convenience center was accepted and completed as originally conceived, there was significant resistance from department stores to the concept of the specialty center in a resort setting because of the low visitation at Hilton Head during the prime Christmas season.

ARROWHEAD REGIONAL DEVELOPMENT COMMISSION: Downtown Duluth Regional Center Evaluation, Duluth, Minnesota

Performed a comprehensive economic and fiscal analysis of alternative locations for a regional shopping center in the Duluth region. While the study clearly showed the advantages to the community of utilizing the downtown as a location for the facility, these potential benefits did not convince potential chain retailers that there was sufficient market support for the facility or that the center city location could be successfully "retrofitted" with large quantities of retail space.

NANSAY CORPORATION: Market Assessment of Retail Potentials, Westwood Mixed Use Project Westwood, California

Analyzed the market potential for development of a major new retail center in Westwood. The study documented the need for quality retail stores and restaurants in the Westwood area, though the stigma associated with Westwood following several crimes of violence plus the recession of the early 1990s effectively doomed the project. Notwithstanding, in recent years Westwood has been rejuvenated on a piecemeal basis with many of the retail activities proposed in the study.

PRUDENTIAL REALTY/MELVIN SIMON COMPANY: Marina Place Economic/Fiscal Impact Study, Culver City, California

Provided market assessments and economic and fiscal impact analyses of the proposed Marina Place regional shopping center as part of the consultant team that was successful in obtaining approvals for the proposed development on a 30+/- acre site near Marina del Rey. Unfortunately, regional economic conditions coupled with the decline in performance of traditional department stores led to the project's demise; the site was developed instead with a Costco department store.

HAWAII OMORI CORPORATION: Lahaina Cannery Shopping Center Evaluation, Lahaina, Maui Performed a series of market evaluations for three properties owned by Hawaii Omori Corporation that were located in the Town of Lahaina, Maui. One of the properties serves as the site for the Lahaina Cannery Shopping Center, an existing 180,000 square foot facility. The study examined the possibility of developing a multi-centered retail complex with both specialty and convenience retail nodes designed to serve the full range of resident and tourist retail needs.

MAUNA LANI RESORT, INC.: Specialty Retail Center Market Studies, Mauna Lani, South Kohala, Big Island of Hawaii

Analyzed the market potentials for the development of a specialty retail center at Mauna Lani Resort. The analysis focused on upper-income visitors and their propensities to support specialty retail shops in hotels and at "boutique" centers similar to The Shops at Kapalua. The study identified candidate tenants for the development, provided recommendations regarding store mix, and offered suggestions with respect to the optimum location for the facility within the resort.

ALOHA TOWER DEVELOPMENT CORPORATION: Aloha Tower Development Program, Phases I and II, Honolulu, Hawaii

Prepared developer selection criteria and evaluated business terms of proposals for redevelopment of the Aloha Tower complex, a \$1 billion redevelopment program for the downtown Honolulu waterfront featuring a "festival market" specialty retail center, the precursor to current "entertainment/retail" projects. The first phase of the project, Aloha Tower Marketplace, was completed in 1994. Following the selection of the preferred developer, Enterprise Development Company, provided leasing advisory services and negotiated the business terms of the lease agreement between parties.

STATE OF HAWAII EMPLOYEES RETIREMENT SYSTEM (ERS): Kaahumanu Regional Center Expansion, Kahului, Maui, Hawaii

Provided a market and financial evaluation of the proposed expansion of Kaahumanu Center from 316,600 square feet of gross leasable area (GLA) to 542,600 square feet. The only regional center located on Maui, the property was owned by Maui Land & Pineapple Company, developers of Kapalua Resort. The analysis measured investment returns to the State of Hawaii ERS under a range of future outcomes. Of particular significance were the assessments of potential competitive impacts on the center from Mainland retailers entering the Maui market. The expansion program was successfully completed.

STATE OF HAWAII EMPLOYEES RETIREMENT SYSTEM (ERS): Waikele Shopping Center, Central Oahu, Hawaii

Completed a due diligence review of a proposed power center and an outlet mall which were developed on 40+ / - acres of freeway frontage in the Waikele master-planned community. The services provided to the ERS included a review of major sources of demand for retail goods and services, a survey of existing and proposed competitive facilities on Oahu, and a detailed examination of the developer's proposed tenant mix and pro forma financial projections. Also compared actual leases with the pro-forma rent schedules to ensure that the project would achieve its target levels of return.

QUEEN LILIUOKALANI TRUST/FIRST HAWAIIAN BANK: Mauka Lands Evaluation, Kailua-Kona, Big Island of Hawaii

Served the Queen Liliuokalani Trust as market and financial advisors for 1,200 acres of land located in Kailua-Kona on the Big Island of Hawaii. Following its re-classification to urban use by the State Land Use Commission, provided assistance to the Trust by performing market studies for the site and reviewing proposals for the first phase of development from shopping center developer candidates. The

project has gone forward successfully, and several increments of retail commercial development have been completed.

T & S DEVELOPMENT, INC.: Regional Shopping Center Assessment, Riverside, California Provided a critique of the market study that supported the expansion of the existing Tyler Mall regional shopping center. Also presented a comparative analysis of the economic benefits resulting from the proposed expansion of Tyler Mall with an alternative program to develop a new regional center at Canyon Springs Road.

DONAHUE/SHRIBER AND THE IRVINE COMPANY: Comparative Analysis of Alternative Sites, City of Irvine. California

Assisted the shopping center developer and the Irvine Company in evaluating alternative locations for the development of Target department stores. Primary focus was on two sited in the City of Irvine – Interstate-5/Myford and Culver/Barranca. The principal basis for comparison was the demographic characteristics of the primary market areas served by the two locations.

HOMART DEVELOPMENT CORP. (SEARS): Proposed Regional Shopping Center, Eugene, Oregon Evaluated the market potential for a regional shopping center to be located in the Eugene, Oregon metropolitan area. The results of the study suggested that the market was likely too small to absorb the retail space proposed in the Homart project.

THE IRVINE COMPANY: Proposed Regional Shopping Center, Orange County, California
Provided a market analysis of the future potentials for a regional shopping center located on Santiago
Canyon Road easterly of the City of Orange. The primary purpose of the study was to guide the master
planning for the area and make necessary allocations for lands sufficient to accommodate future
commercial space requirements.

AHMANSON COMMERCIAL DEVELOPMENT CORPORTATION: Palm Desert Community Shopping Center, Palm Desert, California

Performed market and financial feasibility studies for this recently completed community shopping center located on Highway 111 adjacent to the Palm Desert Town Center regional mall. One purpose of the study was to consider a tenant mix that would be able to effectively compete with the regional mall.

LOS ANGELES COUNTY CHIEF ADMINISTRATIVE OFFICE: Civic Center Mall Retail Analysis Civic Center Mall, Los Angeles

Evaluated the market potential for specialty retail and service commercial uses at a potential retail location on the Civic Center Mall near the Music Center. The purpose of the facility was to provide for the needs of governmental workers and visitors to County Hall of Administration. Consulting services also included lease negotiations with candidate tenants for the project.



This Appendix provides additional explanatory detail for the population, income and retail sales projections that are presented in the preceding urban decay analysis, and how potential conflicts among some of the data sources were reconciled.

Population

The baseline population forecasts underlying this analysis were prepared by Claritas, Inc., a nationally-recognized provider of demographic information for market analyses and other purposes. As presented in Table B-1, Claritas provided population and baseline income data for the Westfield Fashion Square Regional Market Area (RMA); a 3.0-Mile Market Radius around Westfield Fashion Square that serves as the Eating and Drinking Facility market area; and Los Angeles County. Data were prepared for several time periods: the baseline year 2000, per information collected from the U.S. Census; a current estimate for the year 2007; and a five-year projection for the year 2012. These estimates and projections were then evaluated for internal consistency and for comparability with other data sources, including the California State Department of Finance and the Los Angeles County Economic Development Commission.

Table B-1

BASELINE DEMOGRAPHIC ESTIMATES AND PROJECTIONS.

LOS ANGELES COUNTY AND WESTFIELD FASHION SQUARE MARKET AREAS

Data Category	Regional <u>Market Area</u>	Eating & Drinking Facility Market Area 3.0-Mile Radius	Los Angeles <u>County</u>
Population			
2000	680,152	120,800	9,519,338
2007	728,332	131,195	10,164,031
2012	770,434	140,232	10,734,503
Number of House	holds		
2000	283,462	44,140	3,133,774
2007	300,049	47,183	3,314,263
2012	315,697	49,907	3,486,188
Average Per Capi	ta Income		
2000 1	\$32,882	\$29,268	\$20,683
2007	\$35,328	\$32,354	\$23,618
Average Househo	old Income		
2000 1	\$75,225	\$69,278	\$61,811
2007	\$85,221	\$78,285	\$71,592

¹ Data are actually for calendar year 1999.

Source: Claritas, Inc.

Income

Table B-1 also provides Claritas' current household and per capita income estimates for the RMA, the 3.0-Mile Market Radius and Los Angeles County. While these statistics may be indicative to the degree that they reflect that there are basic differences between the Regional Market Area (RMA), the Eating & Drinking Facility 3.0-Mile Market Radius and Los Angeles County with respect to income levels, the current estimates made by Claritas appear to be conservative. For example, Claritas' household and per capita income growth estimates for Los Angeles County between 1999 and 2007 is measured at about 1.7 percent, while other forecasts for the area suggest that incomes were growing at a rate above 3.6 percent. Given what the analysts believe are unrealistically low estimates by Claritas, further analysis was conducted to arrive at more realistic projections of current and future income levels for the RMA and the 3.0-Mile Market Radius. These projections are noted in Table B-2.

There are two basic measures of per capita personal income that are commonly used in retail market analysis: Per Capita Personal Income as measured by the U.S. Bureau of Economic Analysis (BEA); and Per Capita Personal Income as reported in the United States Census. The BEA definition is a broad definition of per capita personal income that includes both money receipts and changes in assets; it usually is a substantially higher figure for a given population than the per capita amount reported by the U.S. Census, which reports a more limited concept of "money" income that is estimated by census respondents. As noted in Table B-2, the U.S. Census figure for Los Angeles County per capita income was equivalent to only 73.1 percent of the BEA County per capita income measure estimate in 1999, and comparative data for other time periods suggest that the ratio between these two per capita income measures has stayed fairly consistent over time.

In the preparation of per capita personal income estimates and projections for the RMA and the 3.0-Mile Market Radius, the baseline estimates made by Claritas were adjusted upward to reflect both: (1) the recent 2005 and 2006 BEA estimates of per capita income for State of California residents; (2) the recent 2005 and 2006 BEA estimates of per capita income for County of Los Angeles residents; and (3) the relatively higher per capita incomes historically found in the RMA and the 3.0-Mile Market Radius *vis a vis* Los Angeles County as measured by Claritas. The results of these adjustments are presented in Table B-2 in the form of per capita income estimates in 2007 for the RMA and for the 3.0-Mile Market Radius residents.

Per capita personal incomes for the RMA are projected to 2012 using an annual compound growth rate of 3.5 percent that is applied to the baseline 2007 estimates. This magnitude of growth is consistent with the State's annual per capita income growth that has been experienced over the 7-year period 1999-2006, a period that reflects both recession and expansion phases in the general economy. The projection also reflects the fact that the RMA and 3.0-Mile Market Radius both include some of the wealthiest residential communities found within the United States.

Table B-2
COMPARISON OF PER CAPITA INCOMES FOR STATE OF CALIFORNIA, LOS ANGELES COUNTY AND WESTFIELD FASHION SQUARE MARKET AREAS

	<u>1998</u>	<u>1999</u>	2000	<u>2001</u>	2002	2003	2004	<u>2005</u>	Preliminary 2006	Projected 2007	Projected 2012
State of California Per Capita Personal Income, BEA Definition	\$29,489	\$30,152	\$32,588	\$32,964	\$32,751	\$33,202	\$35,172	\$36,936	\$ 38,956	\$ 40,319	\$ 47,887
County of Los Angeles Per Capita Personal Income, BEA Definition	\$27,479	\$28,294	\$29,314	\$30,478	\$30,535	\$31,193	\$32,619	\$34,335	\$ 36,307	\$ 37,577	\$ 44,631
County as Percent of State	93.2%	93.8%	90.0%	92.5%	93.2%	93.9%	92.7%	93.0%	93.2%	93.2%	93.2%
Money Income as Percent of Personal Income		73.10%						73.10%	73.10%	73.10%	73.10%
County of Los Angeles Per Capita Personal Income, Census Definition (Claritas):	:	\$ 20,683					:	\$ 22,727		\$ 23,618	\$ 25,813
Adjusted County of Los Angeles Per Capita Personal Income, Census Definition	:	\$ 20,683					;	\$ 25,099	\$ 26,450	\$ 27,469	\$ 32,625
Westfield Fashion Square Market Areas Per Capita Personal Income:											
Regional Market Area Per Capita Personal Income, Census Definition (Claritas)	:	\$ 32,882								\$ 35,328	\$ 37,974
Adjusted Per Capita Personal Income, Census Definition	;	\$ 32,882								\$ 41,088	\$ 47,995
Per Capita Personal Income: BEA Definition	:	\$ 44,982					;	\$ 52,175	\$ 54,472	\$ 56,208	\$ 66,757
Eating & Drinking Facilities Market Area Per Capita Personal Income, Census Definition (Claritas)	:	\$ 29,268								\$ 32,354	\$ 34,783
Adjusted Per Capita Personal Income, Census Definition	:	\$ 29,268								\$ 37,629	\$ 43,961
Per Capita Personal Income: BEA Definition	;	\$ 40,038					:	\$ 47,422	\$ 49,942	\$ 51,476	\$ 61,137

Source: US Bureau of Economic Analysis; U S Census of Retail Trade; State of California: Department of Finance, Employment Development Department, State Board of Equalization; Bureau of Labor Statistics; Los Angeles County Economic Development Commission; Claritas, Inc.; W & W, Inc.; HRA, Inc.

Retail Sales Demand

Future retail demand has been calculated by determining the percent of personal income that has historically been expended for retail sales in the State of California and applying it to existing and future population and income levels in the market areas for the Westfield Fashion Square site. This percentage has been calculated by comparing total retail sales as measured by the U.S. Census of Retail Trade in census years 1997 and 2002 with the BEA measure of California Personal Income for those two corresponding periods. This comparison is noted below in Table B-3:

Table B-3
RETAIL SALES AS PERCENT OF INCOME, US BEA AND US CENSUS

	Total Personal	Total Retail	Retail Sales	Retail Sales
	Income	Sales <u>1</u> /	as % of Personal	as % of Personal
<u>Year</u>	<u>('000s)</u>	<u>('000s)</u>	Income (BEA)	Income (Census)
1997	\$ 860,544,880	\$ 285,356,629	33.2%	45.4%
2002	\$ 1,147,868,177	\$ 383,296,602	33.4%	45.7%
		A	verage 33.3%	45.5%

^{1/} Excludes e-sales and vending machines; adds Eating and Drinking facility sales. Source: U S Bureau of Economic Analysis (BEA); U S Census; W & W, Inc.; HRA, Inc.

Allocations of retail sales to individual retail categories and store types have been developed following the retail store classification system utilized by the State of California State Board of Equalization. As shown in Table B-4, annual retail sales measured by the State and by the U.S. Census correspond reasonably well after adjustments are made in the State's taxable sales statistics that convert them to total retail sales. The adjustments that convert the State's taxable retail sales to total retail sales are based on a review of years 1997 and 2002, years where comparative data are available from both the U.S. Census and the Board of Equalization.

After the adjustments to retail sales by store category are made at the state level for 2002 and 2005 (see Tables B-5 and B-6), they are then refined to reflect local tastes and preferences by utilizing the retail sales distributions to various store categories per the percentage distributions that are found in Los Angeles County. The final retail sales distributions to individual retail store categories utilized in this analysis for the RMA and 3.0-Mile Market areas are derived from data presented in Tables B-7 and B-8. The data in these tables show the distribution of taxable and total retail sales in Los Angeles County for 2002 and 2005. In this regard, it should be noted that 2005 is the most recent calendar year for which annual data are available as of the date of preparation of this report.

Table B-4 COMPARATIVE ANALYSIS, STATE BOARD OF EQUALIZATION AND U S CENSUS OF RETAIL TRADE RETAIL SALES BY MAJOR RETAIL CATEGORY, STATE OF CALIFORNIA 2002

(in Thousands of Current Dollars)

Retail Store Category	2002 <u>State</u>	Adjust. <u>Factor</u>	State <u>Adjusted</u>		2002 <u>Census</u>
Apparel Stores	14,029,200		14,029,200		
Clothing, Accessories, Jewelry, luggage					22,661,146
General Merchandise Stores	42 741 257		40 741 057		4/ /0/ 215
Department Stores & Other General Merchandise Drug Stores	42,741,257 5,745,634	3.07	42,741,257 17,635,808		46,696,215 17,635,808
Total, General Merchandise	48,486,891		60,377,065	_	64,332,023
Food Store Group					
Food Stores	18,951,412	3.06	57,964,493		57,964,493
Liquor Stores	2,137,065		2,137,065	_	2,278,760
Total, Food & Beverage	21,088,477		60,101,558		60,243,253
Eating & Drinking Group					
Restaurants, no Alcohol	17,202,160		17,202,160		
Resaurants with Alcohol	20,877,670		20,877,670		
Total, Eating & Drinking	38,079,830		38,079,830		
Household Furnishings Group	13,983,287		13,983,287		
Furniture & Home Furnishings					11,605,138
Electronics & Appliances					13,186,464
Building Materials and Farm Supplies					
Building Materials and Supplies	25,816,009		25,816,009		24,515,132
Lawn/Garden Supplies, including Farm Eqpt	4,671,072		4,671,072	_	2,265,209
Total, Building Materials and Garden Supplies	30,487,081		30,487,081		26,780,341
Automotive Group					
Auto Dealers/Parts	63,821,146		90,664,859	1.421	90,664,859
Service Stations	23,928,351		23,928,351	_	23,421,136
Total, Automotive Group	87,749,497		114,593,210		114,085,995
All Other: State Board of Equalization					
Specialty Group(Calif definition)	43,539,120		43,539,120		
Used Merchandise	520,999		520,999		E 400 44E
Mobile Home, RV, Motorcycle, Boat, Plane Dealers All Other: US Census	3,647,924		3,647,924		5,692,445
Health & Personal Care(less Drug Stores/Pharmacies)					3,108,465
Sporting Goods, Hobby, Books, Music, et al					9,789,031
Misc. Retail: Florists, Office Supplies, Used Merch., Pets, Art, et al					10,786,260
Total, Other	47,708,043	_	47,708,043		29,376,201
Grand Total, Store Groups Noted Above	301,612,306		379,359,274		342,270,561
Less: Eating & Drinking		-	(38,079,830)		
Total Retail Store Sales, Selected Categories			341,279,444		342,270,561
State as Percent of Census			99.71%		

Source: State of California, State Board of Equalization; U S Census of Retail Trade; W & W, Inc.

Table B-5 DISTRIBUTION OF RETAIL SALES BY MAJOR RETAIL CATEGORY STATE OF CALIFORNIA 2002

(in Thousands of Current Dollars)

Retail Store Category Apparel Stores	State <u>Baseline</u> 14,029,200	Percent of Total 4.65%	Adjust <u>Factor</u>	State <u>Adjusted</u> 14,029,200	Percent of Total 3.70%
General Merchandise Stores Department Stores & Other General Merchandise Drug Stores	42,741,257 5,745,634	14.17% 1.90%	3.07	42,741,257 17,639,096	11.28% 4.65%
Total, General Merchandise Group	48,486,891	16.08%		60,380,353	15.93%
Food Store Group					
Food Stores Liquor Stores	18,951,412 2,137,065	6.28% <u>0.71%</u>	3.06	57,991,321 2,137,065	15.30% <u>0.56%</u>
Total, Food & Beverage Group	21,088,477	6.99%		60,128,386	15.87%
Eating & Drinking Facilities	17 202 1/0	F 700/		17 202 1/0	4 F 40/
Restaurants, no Alcohol Restaurants with Alcohol	17,202,160 20,877,670	5.70% <u>6.92%</u>		17,202,160 20,877,670	4.54% <u>5.51%</u>
Total, Eating & Drinking Group	38,079,830			38,079,830	10.05%
Household Furnishings Group	13,983,287	4.64%		13,983,287	3.69%
Building Materials and Farm Supplies Building Materials and Supplies	25,816,009	8.56%		25,816,009	6.81%
Lawn/Garden Supplies, incl Farm Eqpt Total, Building Materials and Garden Supplies	4,671,072 30,487,081	<u>1.55%</u> 10.11%		4,671,072 30,487,081	<u>1.23%</u> 8.04%
Total, building Materials and Garden Supplies	30,407,001	10.1170		30,467,061	0.0470
Automotive Group	(0.004.44)	04.4707	4.44	00.040.100	00.010/
Auto Dealers/Parts Service Stations	63,821,146 23,928,351	21.16% 7.93%	1.41	90,243,100 23,928,351	23.81% <u>6.31%</u>
Total, Automotive Group	87,749,497	29.09%		114,171,451	30.13%
Specialty Group, incl Used Merchandise	44,060,119	14.61%		44,060,119	11.63%
Mobile Home, RV, Motorcycle, Boat, Plane Dealers	3,647,924	1.21%		3,647,924	0.96%
Total, Retail Store Sales	301,612,306	100.00%		378,967,632	100.00%

Source: State of California, State Board of Equalization; U S Census of Retail Trade; W & W, Inc.

Table B-6 DISTRIBUTION OF RETAIL SALES BY MAJOR RETAIL CATEGORY STATE OF CALIFORNIA 2005

(in Thousands of Current Dollars)

Retail Store Category Apparel Stores	State <u>Baseline</u> 18,712,125	Percent of Total 4.98%	Adjust <u>Factor</u>	State <u>Adjusted</u> 18,712,125	Percent of Total 4.04%
General Merchandise Stores					
Department Stores & Other General Merchandise	50,588,297	13.46%	0.07	50,588,297	10.93%
Drug Stores	6,198,856	<u>1.65%</u>	3.07	19,030,488	4.11%
Total, General Merchandise Group	56,787,153	15.11%		69,618,785	15.05%
Food Store Group					
Food Stores	21,128,469	5.62%	3.06	64,653,115	13.97%
Liquor Stores	2,511,183	0.67%		2,511,183	0.54%
Total, Food & Beverage Group	23,639,652	6.29%		67,164,298	14.52%
Eating & Drinking Facilities					
Restaurants, no Alcohol	21,341,643	5.68%		21,341,643	4.61%
Restaurants with Alcohol	25,071,204	6.67%		25,071,204	5.42%
Total, Eating & Drinking Group	46,412,847	12.35%		46,412,847	10.03%
Household Furnishings Group	17,388,704	4.63%		17,388,704	3.76%
Building Materials and Farm Supplies					
Building Materials and Supplies	36,152,218	9.62%		36,152,218	7.81%
Lawn/Garden Supplies, incl Farm Eqpt	6,541,010	<u>1.74%</u>		6,541,010	<u>1.41%</u>
Total, Building Materials and Garden Supplies	42,693,228	11.36%		42,693,228	9.23%
Automotive Group					
Auto Dealers/Parts	73,601,374	19.58%	1.41	104,072,343	22.50%
Service Stations	38,566,548	<u>10.26%</u>		38,566,548	<u>8.34%</u>
Total, Automotive Group	112,167,922	29.85%		142,638,891	30.83%
Specialty Group, incl Used Merchandise	52,928,654	14.08%		52,928,654	11.44%
Mobile Home, RV, Motorcycle, Boat, Plane Dealers	5,077,840	<u>1.35%</u>		5,077,840	<u>1.10%</u>
Total, Retail Store Sales	375,808,125	100.00%		462,635,372	100.00%

Source: State of California, State Board of Equalization; U S Census of Retail Trade; W & W, Inc.

Table B-7 DISTRIBUTION OF RETAIL SALES BY MAJOR RETAIL CATEGORY LOS ANGELES COUNTY 2002

(in Thousands of Current Dollars)

Retail Store Category Apparel Stores	County Baseline 4,036,630	Percent of Total 5.41%	Adjust <u>Factor</u>	County Adjusted 4,036,630	Percent of Total 4.09%
General Merchandise Stores Department Stores & Other General Merchandise Drug Stores	9,704,153 1,492,554	13.02% <u>2.00%</u>	3.26	9,704,153 4,861,770	9.83% <u>4.92%</u>
Total, General Merchandise Group	11,196,707	15.02%		14,565,923	14.75%
Food Store Group Food Stores Liquor Stores	4,235,299 544,140	5.68% <u>0.73%</u>	3.50	14,821,554 544,140	15.01% <u>0.55%</u>
Total, Food & Beverage Group	4,779,439	6.41%		15,365,694	15.56%
Eating & Drinking Facilities Restaurants, no Alcohol	5,364,930	7.20%		5,364,930	5.43%
Restaurants with Alcohol	5,176,950	6.94%		5,304,730	5.24%
Total, Eating & Drinking Group	10,541,880	14.14%		10,541,880	10.68%
Household Furnishings Group	3,378,316	4.53%		3,378,316	3.42%
Building Materials and Farm Supplies Building Materials and Supplies Lawn/Garden Supplies, incl Farm Eqpt Total, Building Materials and Garden Supplies	5,528,888 512,038 6,040,926	7.42% <u>0.69%</u> 8.10%		5,528,888 512,038 6,040,926	5.60% <u>0.52%</u> 6.12%
Automotive Group Auto Dealers/Parts Service Stations	15,869,231 6,404,120	21.29% <u>8.59%</u>	1.64	26,095,296 6,404,120	26.43% 6.49%
Total, Automotive Group	22,273,351	29.88%		32,499,416	32.92%
Specialty Group, incl Used Merchandise	11,739,640	15.75%		11,739,640	11.89%
Mobile Home, RV, Motorcycle, Boat, Plane Dealers	561,088	<u>0.75%</u>		561,088	<u>0.57%</u>
Total, Retail Store Sales	74,547,977	100.00%		98,729,513	100.00%

Source: State of California, State Board of Equalization; U S Census of Retail Trade; W & W, Inc.

Table B-8 DISTRIBUTION OF RETAIL SALES BY MAJOR RETAIL CATEGORY LOS ANGELES COUNTY 2005

(in Thousands of Current Dollars)

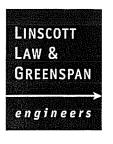
Retail Store Category Apparel Stores	County Baseline 5,248,349	Percent of Total 5.69%	Adjust <u>Factor</u>	County Adjusted 5,248,349	Percent of Total 4.41%
General Merchandise Stores Department Stores & Other General Merchandise Drug Stores	11,504,506 1,672,209	12.47% <u>1.81%</u>	3.26	11,504,506 5,451,401	9.66% <u>4.58%</u>
Total, General Merchandise Group	13,176,715	14.28%		16,955,907	14.24%
Food Store Group Food Stores Liquor Stores	4,532,723 602,264	4.91% <u>0.65%</u>	3.50	15,864,531 602,264	13.32% <u>0.51%</u>
Total, Food & Beverage Group	5,134,987	5.57%		16,466,795	13.83%
Eating & Drinking Facilities Restaurants, no Alcohol	6,590,968	7.14%		6,590,968	5.54%
Restaurants with Alcohol	6,313,342	6.84%		6,313,342	<u>5.30%</u>
Total, Eating & Drinking Group	12,904,310	13.99%		12,904,310	10.84%
Household Furnishings Group	4,263,142	4.62%		4,263,142	3.58%
Building Materials and Farm Supplies Building Materials and Supplies Lawn/Garden Supplies, incl Farm Eqpt Total, Building Materials and Garden Supplies	7,701,383 676,879 8,378,262	8.35% <u>0.73%</u> 9.08%		7,701,383 676,879 8,378,262	6.47% <u>0.57%</u> 7.04%
Automotive Group Auto Dealers/Parts Service Stations	18,263,829 10,261,639	19.79% <u>11.12%</u>	1.64	29,952,680 10,261,639	25.16% <u>8.62%</u>
Total, Automotive Group	28,525,468	30.91%		40,214,319	33.77%
Specialty Group, incl Used Merchandise	13,944,113	15.11%		13,944,113	11.71%
Mobile Home, RV, Motorcycle, Boat, Plane Dealers	695,809	0.75%		695,809	<u>0.58%</u>
Total, Retail Store Sales	92,271,155	100.00%		119,071,005	100.00%

Source: State of California, State Board of Equalization; U S Census of Retail Trade; W & W, Inc.

HR&A ADVISORS, INC. B-10 August 2007

APPENDIX I

TRAFFIC STUDY



TRAFFIC IMPACT, PARKING AND SITE ACCESS STUDY

WESTFIELD FASHION SQUARE EXPANSION PROJECT

City of Los Angeles, California Revised August 5, 2008

Prepared for:

Westfield Corporation, Inc. 11601 Wilshire Boulevard, 11th Floor Los Angeles, California 90025

LLG Ref. 1-053606-1



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TABLE OF CONTENTS

SECT	ION		PAGE
Exe	cutive	Summary	v
1.0	Intro	oduction	1
2.0	Proj	ect Description	4
	2.1	Site Location	
	2.2	Existing Project Site	
	2.3	Proposed Project Description	4
3.0	Site.	Access and Circulation	
	3.1	Existing Site Access	
	3.2	Proposed Project Site Access and Circulation	7
4.0		ect Parking	
	4.1	Existing Parking Requirement	
	4.2	Existing Parking Utilization	
	4.3	Proposed Project	
	4.4	Code Parking Calculation for Proposed Future Fashion Square Development	
	4.5	Shared Parking Demand Analysis	
		4.5.1 Shared Parking Demand Analysis for Existing Conditions	
		4.5.2 Shared Parking Demand Analysis for Future Conditions	17
5.0		ting Street System	
	5.1	Regional Highway System	
	5.2	Local Street System	
	5.3	Roadway Classifications	
	5.4	Roadway Descriptions	
	5.5	Existing Public Bus Transit Service	28
6.0	Traf	fic Counts	31
7.0	Traf	fic Forecasting Methodology	36
	7.1	Project Trip Generation.	36
	7.2	Project Trip Distribution	37
8.0	Cum	nulative Development Projects	43
	8.1	Related Projects	43
	8.2	Ambient Growth Factor	49
9.0	Traf	fic Impact Analysis Methodology	50
	9.1	Impact Criteria and Thresholds	50
	9.2	LADOT ATSAC/ATCS	
	9.3	Traffic Impact Analysis Scenarios	52

TABLE OF CONTENTS (continued)

SECT	ON I	PAGE
10.0	Traffic Analysis	
	10.1 Existing Conditions	
	10.2 Existing With Ambient Growth Conditions	55
	10.3 Future Pre-Project Conditions	
	10.4 Future With Project Conditions	58
11.0	Transportation Mitigation Measures	64
	11.1 Summary of Project Mitigation	64
	11.2 Traffic Signal Warrants Analysis	66
12.0	Neighborhood Street Segment Analysis	72
13.0	Weekend Project Impact Analysis	
	13.1 Existing Traffic Counts	
	13.2 Traffic Forecasting Methodology	
	13.2.1 Weekend Project Trip Generation	
	13.2.2 Project Trip Distribution	
	13.3 Cumulative Development Projects	
	13.3.1 Related Projects	
	13.3.2 Ambient Growth Factor	
	13.4 Traffic Analysis	
	13.4.1 Existing Conditions	
	13.4.2 Existing With Ambient Growth Conditions	
	13.4.3 Future Pre-Project Conditions	
	13.4.4 Future With Project Conditions	
	13.4.5 Summary of Project Mitigation	
	13.5 Neighborhood Street Segment Analysis	88
14.0	Congestion Management Program Traffic Impact Assessment	
	14.1 Intersections	
	14.2 Freeways	
	14.3 Transit	90
15.0	Voluntary Transportation Improvements	92
160	Conclusions	93

TABLE OF CONTENTS (continued)

LIST OF TABLES

SECTIO	N—TABLE#	PAGE				
4–1	Summary of Project Floor Area	12				
4–2	4–2 Weekday Shared Parking Demand Analysis – Existing Conditions					
4–3	Weekend Shared Parking Demand Analysis – Existing Conditions	16				
4-4	Weekday Shared Parking Demand Analysis – Future Conditions (July)	18				
4–5	Weekend Shared Parking Demand Analysis – Future Conditions (July)	19				
4–4	Weekday Shared Parking Demand Analysis – Future Conditions (December)	21				
4–5	Weekend Shared Parking Demand Analysis – Future Conditions (December)	22				
5-1	Existing Transit Routes	29				
6–1	Existing Traffic Volumes	32				
7–1	Weekday Project Trip Generation	38				
8-1	List of Related Projects	44				
8-2	Related Project Trip Generation	46				
9-1	City of Los Angeles Intersection Impact Threshold Criteria	50				
9-2	Weekday Summary of Volume to Capacity Ratios and Levels of Service Summary	53				
12-1	Street Segment Impact Threshold Criteria.	72				
12-2	Neighborhood Street Segment Analysis Summary	73				
13-1	Weekend Existing Traffic Volumes.	75				
13–2	Weekend Project Trip Generation.	78				
13-3	Related Project Trip Generation	80				
13-4	Weekend Summary of Volume to Capacity Ratios and Levels of Service Summary	82				
	APPENDICES					
APPEN	DIX					
A.	Proposed Site Access and Intersection Improvements					
B.	Manual Traffic Counts					
C.	2. Fashion Square Driveway Traffic Counts					
D.	CMA and Levels of Service Explanation,					
	CMA Data Worksheets – AM and PM Peak Hours					
E.	Traffic Signal Warrant Worksheets					
F.	24-Hour Machine Counts					

TABLE OF CONTENTS (continued)

LIST OF FIGURES

SECTIO	N—Figure#	Page
1-1	Vicinity Map	2
2-1	Site Plan	5
5-1	Existing Lane Configurations	25
5-2	Existing Public Transit Routes	30
6–1	Existing Traffic Volumes – Weekday AM Peak Hour	34
6-2	Existing Traffic Volumes – Weekday PM Peak Hour	35
7-1	Project Trip Distribution	40
7-2	Project Traffic Volumes – Weekday AM Peak Hour	41
7-3	Project Traffic Volumes – Weekday PM Peak Hour	42
8-1	Location of Related Projects	45
8-2	Related Projects Traffic Volumes – Weekday AM Peak Hour	47
8-3	Related Projects Traffic Volumes – Weekday PM Peak Hour	48
10-1	Existing With Ambient Growth Traffic Volumes – Weekday AM Peak Hour	56
10-2	Existing With Ambient Growth Traffic Volumes – Weekday PM Peak Hour	57
10-3	Future Pre-Project Traffic Volumes – Weekday AM Peak Hour	59
10-4	Future Pre-Project Traffic Volumes – Weekday PM Peak Hour	60
10-5	Future With Project Traffic Volumes – Weekday AM Peak Hour	62
10-6	Future With Project Traffic Volumes – Weekday PM Peak Hour	63
13-1	Existing Traffic Volumes – Weekend Mid-Day Peak Hour	76
13-2	Project Traffic Volumes – Weekend Mid-Day Peak Hour	79
13-3	Related Projects Traffic Volumes – Weekend Mid-Day Peak Hour	81
13-4	Existing With Ambient Growth Traffic Volumes – Weekend Mid-Day Peak Hour	84
13-5	Future Pre-Project Traffic Volumes – Weekend Mid-Day Peak Hour	85
13-6	Future With Project Traffic Volumes – Weekend Mid-Day Peak Hour	87

EXECUTIVE SUMMARY

This traffic analysis has been conducted to identify and evaluate the potential traffic impacts of the proposed Westfield Fashion Square Expansion project. The project would develop the remaining approximately 108,000 square feet of the center's existing entitlement and an additional approximately 172,000 square feet. In full compliance with all applicable City, County and other regulatory requirements, the traffic report concludes that the project would cause no potentially significant impacts at 11 of the 18 studied intersections, the local neighborhood streets, or relevant segments of the 101 Freeway. The impacts to the remaining 7 intersections will be fully mitigated.

These findings by the traffic report were based on a number of conservative assumptions (as detailed throughout the report). In addition, the project includes a number of improvements that will enhance access to and from the center. Finally, the center will continue to provide adequate parking based on studies of actual parking demand during the peak holiday season in December.

The traffic analysis follows City of Los Angeles traffic study guidelines and is consistent with traffic impact assessment guidelines set forth in the 2004 Congestion Management Program for Los Angeles County. This traffic analysis evaluates potential project-related impacts at 18 key intersections and two local residential street segments in the vicinity of the project site. The study intersections and local residential street segments were determined in consultation with City of Los Angeles Department of Transportation staff. This traffic analysis also evaluates the potential impact of the proposed project within the context of cumulative impact of all ongoing developments (17 related projects) in the area. In addition, a review was conducted of Los Angeles County Metropolitan Transportation Authority intersection and freeway monitoring stations to determine if a Congestion Management Program transportation impact assessment analysis is required for the proposed project.

The existing center and the proposed expansion project is located on the south side of Riverside Drive between Hazeltine Avenue and Woodman Avenue in the Sherman Oaks area of the City of Los Angeles. The existing shopping center currently contains approximately 867,000 square feet of gross leasable floor area (i.e., GLSF). The shopping center was previously approved for development of up to 975,000 GLSF, and 867,000 square feet of the permitted development has been built. The proposed project would entail the construction of the remaining approximately 108,000 square feet, and the development of an additional approximately 172,000 square feet of gross leasable floor area. Thus, the proposed Westfield Fashion Square Expansion project consists of the net addition of approximately 280,000 GLSF as compared to existing conditions, providing a total of 1,147,000 GLSF for the shopping center. Occupancy of the proposed project is anticipated in the year 2012.

A portion of the existing parking structure south of the mall building will be removed to accommodate the expanded commercial floor space. A new grade plus five parking structure, including one subterranean parking level, will be provided south of the Macy's building, as well as a new grade plus three parking structure on the southern portion of the site along Woodman Avenue, as part of the proposed project to replace the existing parking spaces to be removed, as well as accommodate additional parking for the expanded commercial floor area. Parking utilization observations conducted at the site during the December 2005 and 2006 holiday seasons revealed that the demand for parking peaked at a ratio equivalent to 4.03 parking spaces per 1,000 GLSF (observed at 4:00 PM on December 26). Therefore, parking provided at the site in conjunction with the proposed expansion at a rate of 4.25 parking spaces per 1,000 GLSF, with the potential to provide parking at a maximum rate of 4.50 parking spaces per 1,000 GLSF, is expected to be adequate to accommodate peak parking demands during the December holiday season, as well as throughout the year.

Vehicular access to the existing project site will be provided via five project driveways: two existing driveways on Hazeltine Avenue, two new driveways on Riverside Drive, and one existing driveway on Woodman Avenue. As part of the expansion project, it is proposed that the existing Fashion Square driveways on Riverside Drive be closed and two new driveways be provided on Riverside Drive: New Easterly Fashion Square Driveway-Matilija Avenue and the New Fashion Square Westerly Driveway. While not specifically required for traffic mitigation purposes, it is recommended that, as part of the proposed project, traffic signals be installed at the Matilija Avenue-New Easterly Fashion Square Driveway/Riverside Drive intersection and the New Fashion Square Westerly Driveway/Riverside Drive intersection. The proposed traffic signals would facilitate vehicular movements to and from the Fashion Square site, particularly in consideration of the proposed parking structure to be constructed south of the Macy's building.

The proposed project is expected to generate a net increase of 95 vehicle trips (58 inbound trips and 37 outbound trips) during the weekday AM commuter peak hour. During the weekday PM commuter peak hour, the proposed project is expected to generate a net increase of 476 vehicle trips (229 inbound trips and 247 outbound trips). Over a 24-hour period, the proposed project is forecast to generate a net increase of 4,964 daily trip ends during a typical weekday (2,482 inbound trips and 2,482 outbound trips). On a typical Saturday, the proposed project is expected to generate a net increase of 632 vehicle trips (329 inbound trips and 303 outbound trips) during the weekend mid-day peak hour, and 6,252 trips ends over a 24-hour period (3,126 inbound trips and 3,126 outbound trips). The trip generation forecast was made using rates published in the Institute of Transportation Engineers' (ITE) Trip Generation manual, 7th Edition, 2003. It is noted that the trip generation forecast provided herein likely overstates the actual amount of vehicular traffic that would be generated by the proposed expansion. By example, traffic counts were conducted at the existing Fashion Square driveways during the weekday morning and afternoon commuter peak periods as well as the Saturday mid-day peak period. The actual peak hour traffic count data is approximately 25% less than the estimates based on the ITE trip rates for shopping centers of similar size. This trip generation characteristic is likely due to the Fashion Square providing "high end" tenants which require relatively fewer patrons to achieve revenue goals. However, to provide a conservative, "worst-case" traffic analysis, the trip

generation forecast based on the ITE trip rates have been utilized in the review of potential impacts associated with the project.

Application of the City's threshold criteria to the "With Proposed Project" scenario indicates that six of the 18 study intersections are anticipated to be significantly impacted by the proposed project during the weekday conditions. Incremental but not significant impacts are noted at the remaining 12 study intersections. It is recommended that the significant transportation impacts be mitigated through a contribution to the City of Los Angeles' Adaptive Traffic Control System installation. This recommended mitigation measure is anticipated to reduce the forecast project-related significant impacts to less than significant levels.

In order to address the issue of non-residential traffic using local streets in neighborhoods adjacent to the proposed project site, two street segments located near the project site have been analyzed for potential significant impacts. Application of LADOT's threshold criteria for residential street segment analysis indicates that the proposed project is not anticipated to significantly impact the analyzed street segments.

A focused analysis was also prepared to evaluate the potential traffic impacts of the proposed Westfield Fashion Square Expansion project during the Saturday mid-day peak hour at seven study intersections located immediately adjacent to the project site. Application of the City's threshold criteria to the "With Proposed Project" scenario indicates that four of the seven study intersections are anticipated to be significantly impacted by the proposed project during the weekend conditions. Incremental but not significant impacts are noted at the remaining three study intersections. The aforementioned contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation at the four impacted study intersections is recommended to mitigate potential impacts. In addition, at the Woodman Avenue/Riverside Drive intersection, it is recommended that the southbound Woodman Avenue approach to the Riverside Drive intersection be reconfigured to provide one left-turn lane, two through lanes and one optional through/right-turn lane to reduce the forecast project-related significant impacts during the Saturday mid-day peak hour to less than significant levels. If required by LADOT, the existing four-foot wide median island on the south leg of the intersection could be replaced by striping and/or lane delineators (e.g., two feet wide or less) so that additional width could be provided to the existing three southbound Woodman Avenue through lanes on the departure side of the intersection.

In summary, the following intersections are forecast to be impacted by the proposed project:

- Int. No. 1: Van Nuys Boulevard/Riverside Drive (Weekday Impact Only)
- Int. No. 4: Tyrone Avenue/Moorpark Street (Weekday Impact Only)
- Int. No. 7: Hazeltine Avenue/Riverside Drive (Weekday and Weekend Impact)
- Int. No. 8: Hazeltine Avenue/Fashion Square Lane (Weekend Impact Only)
- Int. No. 12: Woodman Avenue/Riverside Drive (Weekday and Weekend Impact)

Int. No. 13: Woodman Ave/U.S. 101 WB Ramps (Weekday and Weekend Impact)

Int. No. 15: Woodman Avenue/Moorpark Street (Weekday Impact Only)

The Fashion Square, in consultation with LADOT, will also volunteer to implement the following two measures to further improve traffic operations in the vicinity of the project site:

- Fund the development and implementation of a Neighborhood Traffic Management Plan (NTMP) to address potential existing and future regional "cut-through" traffic on residential streets north of Fashion Square.
- Design and install protected/permissive left-turn traffic signal phasing for Hazeltine Avenue and Riverside Drive at the Hazeltine Avenue/Riverside Drive intersection (i.e., all approaches to the intersection). Based on recent discussions with LADOT staff, the southbound left-turn phasing on Hazeltine Avenue is currently under construction by LADOT. Therefore, the Fashion Square will volunteer to implement the installation of the protected/permissive left-turn phasing at the remaining approaches to the intersection (i.e., northbound approach on Hazeltine Avenue and eastbound and westbound approaches on Riverside Drive).
- Design and install upgraded traffic delineators along Hazeltine Avenue between Riverside Drive and Fashion Square Lane using "quik-kurb" or similar installation approved by LADOT.

The Congestion Management Program (CMP) is a state-mandated program that was enacted by the State Legislature with the passage of Proposition 111 in 1990. The program is intended to address the impact of local growth on the regional transportation system. The analysis has been prepared in accordance with procedures outlined in the 2004 Congestion Management Program for Los Angeles County, County of Los Angeles Metropolitan Transportation Authority, July 2004. It is concluded that the project will not result in significant traffic impacts at designated monitoring locations on the CMP highway system, including on the nearby U.S. 101 (Ventura) Freeway. Also, in compliance with the CMP, the project is not anticipated to cause a significant impact to local public transit services.

TRAFFIC IMPACT, PARKING AND SITE ACCESS STUDY

WESTFIELD FASHION SQUARE EXPANSION PROJECT

City of Los Angeles, California Revised August 5, 2008

1.0 Introduction

This traffic analysis has been conducted to identify and evaluate the potential traffic impacts of the proposed Westfield Fashion Square Expansion project. The proposed project is located on the south side of Riverside Drive between Hazeltine Avenue and Woodman Avenue in the Sherman Oaks area of the City of Los Angeles. The proposed project site location and general vicinity are shown in *Figure 1-1*.

The traffic analysis follows City of Los Angeles traffic study guidelines¹ and is consistent with traffic impact assessment guidelines set forth in the 2004 Congestion Management Program for Los Angeles County.² This traffic analysis evaluates potential project-related impacts at 18 key intersections and two local residential street segments in the vicinity of the project site. The study intersections and local residential street segments were determined in consultation with City of Los Angeles Department of Transportation staff. The Critical Movement Analysis method was used to determine Volume-to-Capacity ratios and corresponding Levels of Service at the study intersections. In addition, a review was conducted of Los Angeles County Metropolitan Transportation Authority intersection and freeway monitoring stations to determine if a Congestion Management Program transportation impact assessment analysis is required for the proposed project.

This study (i) presents existing traffic volumes, (ii) forecasts future traffic volumes with the related projects, (iii) forecasts future traffic volumes with the proposed project, (iv) determines project-related impacts, and (v) recommends mitigation measures, where necessary.

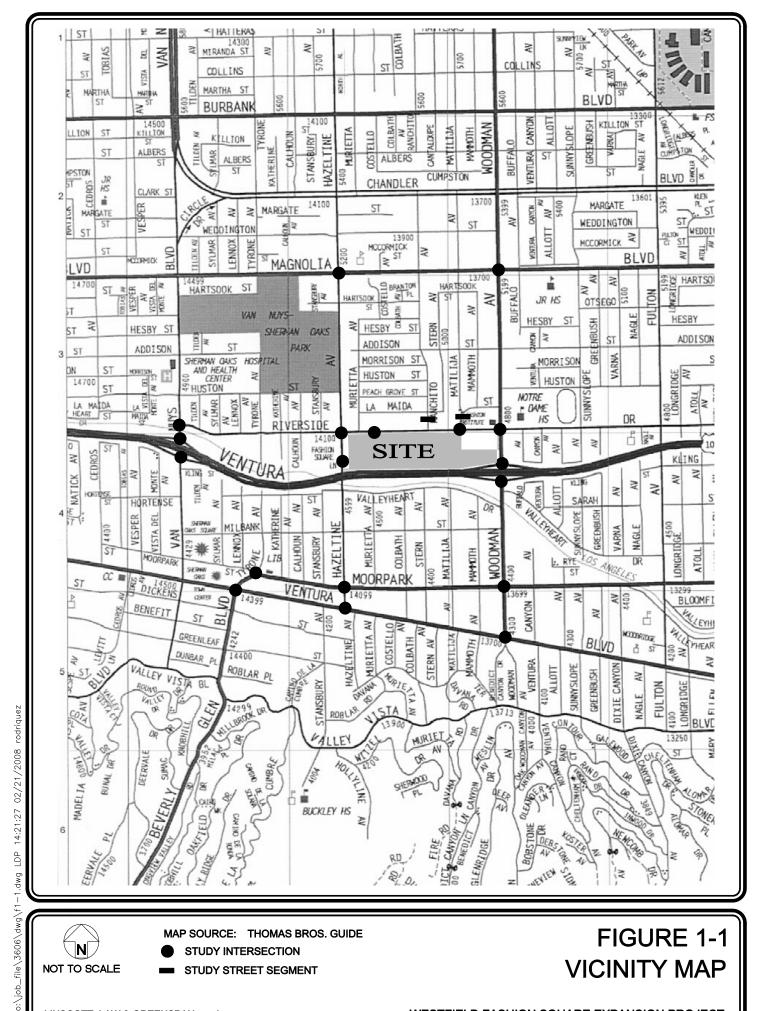
1.1 Study Area

Upon coordination with the City of Los Angeles Department of Transportation staff, 18 study intersections and two local residential street segments have been identified for evaluation. The 18 intersections and two local residential street segments provide local access to the study area and define the extent of the boundaries for this traffic impact analysis. Further discussion of the existing street system and study area is provided in Section 5.0.

The general location of the project in relation to the study locations and surrounding street system is presented in *Figure 1-1*. The traffic analysis study area is generally comprised of those locations which have the greatest potential to experience significant traffic impacts due to the

¹ Traffic Study Policies and Procedures, City of Los Angeles Department of Transportation, March 2002.

² Congestion Management Program for Los Angeles County, Los Angeles County Metropolitan Transportation Authority, July 2004.





MAP SOURCE: THOMAS BROS. GUIDE

STUDY INTERSECTION

STUDY STREET SEGMENT

FIGURE 1-1 VICINITY MAP

LINSCOTT, LAW & GREENSPAN, engineers

WESTFIELD FASHION SQUARE EXPANSION PROJECT

proposed project as defined by the Lead Agency. In the traffic engineering practice, the study area generally includes those intersections that are:

- a. Immediately adjacent or in close proximity to the project site;
- b. In the vicinity of the project site that are documented to have current or projected future adverse operational issues; and
- c. In the vicinity of the project site that are forecast to experience a relatively greater percentage of project-related vehicular turning movements (e.g., at freeway ramp intersections).

The locations selected for analysis were based on the above criteria, proposed Fashion Square peak hour vehicle trip generation, the anticipated distribution of project vehicular trips and existing intersection/corridor operations.

2.0 PROJECT DESCRIPTION

2.1 Site Location

The proposed project is located on the south side of Riverside Drive between Hazeltine Avenue and Woodman Avenue in the Sherman Oaks area of the City of Los Angeles. The project site is bounded by Riverside Drive to the north, the Ventura Freeway to the south, Woodman Avenue to the east, and Hazeltine Avenue to the west.

2.2 Existing Project Site

The existing shopping center is located at 14006 Riverside Drive. The existing shopping center currently contains approximately 867,000 square feet of gross leasable floor area (i.e., GLSF). Vehicular access to the existing project site is currently provided via five project driveways: two driveways on Hazeltine Avenue, two driveways on Riverside Drive, and one driveway on Woodman Avenue. A service/loading driveway is also located on Riverside Drive, east of the Hazeltine Avenue.

2.3 Proposed Project Description

The existing shopping center currently contains approximately 867,000 square feet of gross leasable floor area (i.e., GLSF). The shopping center was previously approved for development of up to 975,000 GLSF, and 867,000 square feet of the permitted development has been built. The proposed project would entail the construction of the remaining 108,000 square feet, and the development of an additional 172,000 square feet of gross leasable floor area. Thus, the proposed Westfield Fashion Square Expansion project consists of the net addition of approximately 280,000 GLSF as compared to existing conditions, providing a total of 1,147,000 GLSF for the shopping center. Occupancy of the proposed project is anticipated in the year 2012.

A portion of the existing parking structure south of the mall building will be removed to accommodate the expanded commercial floor space. A new grade plus five parking structure, including one subterranean parking level, will be provided south of the Macy's building, as well as a new grade plus three parking structure on the southern portion of the site along Woodman Avenue, as part of the proposed project to replace the existing parking spaces to be removed, as well as accommodate additional parking for the expanded commercial floor area. The site plan for the proposed project is illustrated in *Figure 2-1*.

Vehicular access to the project site will be provided via five project driveways: two existing driveways on Hazeltine Avenue, two new driveways on Riverside Drive, and one existing driveway on Woodman Avenue. As part of the expansion project, it is proposed that the existing Fashion Square driveways on Riverside Drive be closed and two new driveways be provided on Riverside Drive: New Easterly Fashion Square Driveway-Matilija Avenue and the New Fashion Square Westerly Driveway. Further discussion of the project's access and circulation scheme is provided in Section 3.0.

SITE PLAN

FIGURE 2-

NOT TO SCALE

SOURCE: WESTFIELD DESIGN + CONSTRUCTION



3.0 SITE ACCESS AND CIRCULATION

The site access scheme for the proposed project is displayed in *Figure 2-1*. Descriptions of the existing site access and proposed project site access and circulation schemes are provided in the following subsections.

3.1 Existing Site Access

Vehicular access to the existing Fashion Square is currently provided via five driveways: two driveways on Hazeltine Avenue, two driveways on Riverside Drive, and one driveway on Woodman Avenue. The Hazeltine Avenue north project driveway and the Woodman Avenue project driveway currently accommodate right-turn ingress and egress movements only. The Hazeltine Avenue south driveway currently accommodates left-turn and right-turn ingress and egress movements. The westerly driveway on Riverside Drive (i.e., located immediately east of the Macy's building) accommodates left-turn and right-turn ingress movements, and right-turn only egress movements. The easterly driveway on Riverside Drive (located at the easterly end of the Fashion Square Riverside Drive frontage) also accommodates left-turn and right-turn ingress movements, and right-turn only egress movements. A service/loading driveway is also located on Riverside Drive, east of the Hazeltine Avenue.

Based on field observations at the site, current traffic circulation issues with the Fashion Square driveways on Riverside Drive and Hazeltine Avenue are as follows:

- Both Riverside Drive driveways are stop sign controlled (i.e., requiring motorists to find gaps in traffic prior to completing left-turns and right-turns). It has been noted that during busy shopping periods, City traffic control officers have been stationed at the Riverside Drive driveways to assist in facilitating the flow of traffic to and from the Fashion Square.
- The left-turn volume from Riverside Drive entering the Fashion Square is relatively high, with the queue of left-turn vehicles occasionally exceeding the storage capacity of the existing westbound Riverside Drive left-turn pockets.
- The right-turn only egress movements from the two Riverside Drive driveways (i.e., left-turns from these driveways are not permitted) unnecessarily causes additional vehicles exiting these driveways to turn towards the busy Woodman Drive/Riverside Drive intersection.
- The existing easterly Riverside Drive driveway is located immediately adjacent to a driveway that serves the adjacent Riverside Shopping Center located east of the Fashion Square. In fact, the left-turn pocket on westbound Riverside Drive which accommodates traffic turning left into the Fashion Square easterly driveway also serves traffic turning left into the Riverside Shopping Center driveway. The shared use of this left-turn pocket has been observed to cause some motorists confusion and related congestion.
- The existing Hazeltine Avenue driveway at Fashion Square Lane currently has one lane entering the parking structure. The existing parking spaces located on the south side of Fashion Square Lane conflict with travel lanes creating congestion and block traffic flow.

3.2 Proposed Project Site Access

The Westfield Fashion Square Expansion project site access scheme is displayed in *Figure 2-1*. Brief descriptions of the project site access driveways on Hazeltine Avenue and Woodman Avenue are provided in the following paragraphs.

Hazeltine Avenue North Project Driveway:

The Hazeltine Avenue north project driveway is located on the east side of Hazeltine Avenue, south of Riverside Drive. The Hazeltine Avenue north project driveway will continue to provide access to the existing parking structure located south of the mall building. The Hazeltine Avenue north project driveway will continue to accommodate right-turn ingress and egress movements only.

Hazeltine Avenue South Project Driveway

The Hazeltine Avenue south project driveway is located on the east side of Hazeltine Avenue at Fashion Square Lane. The intersection of Hazeltine Avenue and Fashion Square Lane is currently controlled by traffic signals. The Hazeltine Avenue south project driveway will continue to provide access to the existing parking structure located south of the mall building, as well as provide access to the proposed parking structure to be located south of the Macy's building. The Hazeltine Avenue south project driveway will continue to accommodate left-turn and right-turn ingress and egress movements.

As part of the proposed expansion, the Hazeltine Avenue south project driveway will be modified to accommodate two inbound lanes and two outbound lanes on Fashion Square Lane. Parking along the south side of Fashion Square Lane adjacent to the entrance/exit will be removed to allow better movement of vehicles entering the parking structure. In addition, an unimpeded road through Fashion Square will be provided from Hazeltine Avenue to Riverside Drive. The schematic plan of the proposed site access improvements on Hazeltine Avenue is contained in *Appendix A*.

Woodman Avenue Project Driveway:

The Woodman Avenue project driveway is located on the west side of Woodman Avenue, just south of Riverside Drive. The Woodman Avenue project driveway will provide access to the remaining surface parking areas located on the project site as well as the new grade plus three parking structure to be constructed as part of the proposed project. The existing Woodman Avenue project driveway will be modified to accommodate right-turn ingress movements only.

As part of the expansion project, it is proposed that the existing Fashion Square driveways on Riverside Drive be closed and two new driveways be provided on Riverside Drive. The new westerly driveway will be provided approximately 540 feet east of Hazeltine Avenue. The new easterly project driveway will be provided approximately 100 feet west of the existing westerly driveway to align with Matilija Avenue to the north. The new easterly driveway will form the south leg of the existing Matilija Avenue/Riverside Drive intersection. The schematic plan of the proposed site access improvements on Riverside Drive is contained in *Appendix A*.

Old HE HILL 2600 Report 4.2000 aproduc

Briefly, the following improvements are recommended in conjunction with the proposed new Fashion Square driveways on Riverside Drive:

New Westerly Fashion Square Driveway

The new westerly driveway access is proposed to be approximately 40 feet in width and accommodate one inbound lane and two outbound lanes. At the Riverside Drive intersection, the driveway exit (i.e., northbound approach) would provide one left-turn lane and one right-turn lane. The new westerly driveway access currently serves as an existing service driveway and historically served as a customer driveway. The new westerly driveway would provide access to a new subterranean parking level to be constructed at the south side of the center.

New Easterly Fashion Square Driveway

The new easterly driveway access is proposed to be approximately 60 feet in width and accommodate two inbound lanes and three outbound lanes. The new easterly driveway would be constructed opposite Matilija Avenue so as to provide a traditional four-leg intersection on Riverside Drive. At the Riverside Drive intersection, the driveway exit (i.e., northbound approach) would provide one left-turn lane and two right-turn lanes (i.e., no through movements would be permitted onto Matilija Avenue north of Riverside Drive). The new easterly driveway would provide access to the existing two-level Macy's parking garage, as well as to the new six-level parking structure proposed south of Macy's.

Riverside Drive Improvements

The Riverside Drive approaches to the relocated easterly driveway would be improved to accommodate the new intersection as follows:

- o Widen the south side of Riverside Drive beginning at a point approximately 290 feet west of the Matilija Avenue centerline by 10 feet. The widening would also require a concurrent dedication of up to 10 feet (thus resulting in a 50-foot wide half roadway and a 60-foot wide half right-of-way).
- o Widen the south side of Riverside Drive beginning at a point approximately 600 feet east of the Matilija Avenue centerline by 3 feet. The widening would also require a concurrent dedication of 2 feet along the Fashion Square frontage (no dedication required by the adjacent Riverside Shopping Center). Thus, the resulting cross-section would be a 40-foot wide half roadway and a 52-foot wide half right-of-way (remaining a 50-foot half right-of-way adjacent to the Riverside Shopping Center).
- o Restripe the eastbound Riverside Drive approach to the intersection with the new easterly Fashion Square driveway to provide two through lanes and one right-turn lane, plus retention of the existing eastbound bike lane. No left-turns to Matilija Avenue north of Riverside Drive would be permitted.
- o Restripe the westbound Riverside Drive approach to the intersection with the new easterly Fashion Square driveway to provide two left-turn lanes, one through lane, and one optional through/right-turn lane, plus retention of the existing westbound bike lane.

The Riverside Drive approaches to the new westerly driveway would be improved to accommodate the new intersection as follows:

- o Restripe the eastbound Riverside Drive approach to the intersection with the new westerly Fashion Square driveway to provide two through lanes and one right-turn lane.
- o Restripe the westbound Riverside Drive approach to the intersection with the new westerly Fashion Square driveway to provide one left-turn lane, and two through lanes, plus retention of the existing westbound bike lane.

Matilija Avenue Improvements

A channelization median island may be installed on the Matilija Avenue approach to the Riverside Drive intersection so as to facilitate right-turn movements only to and from Matilija Avenue. This will limit the potential for Fashion Square or other regional traffic from using Matilija Avenue north of Riverside Drive.

■ <u>Traffic Signal Control</u>

Install traffic signals at the new project driveways on Riverside Drive. The traffic signals at the new project driveways would feature separate westbound left-turn phasing for vehicles turning left into the Fashion Square site. In addition, at the New Fashion Square Driveway-Matilija Avenue/Riverside Drive intersection, a northbound right-turn overlap phasing for vehicles exiting the Fashion Square at the new easterly project driveway may be installed.

Riverside Shopping Center Access:

The Riverside Shopping Center is an existing commercial development located immediately adjacent to the southwest corner of the Woodman Avenue/Riverside Drive intersection. The Riverside Shopping Center is not a part of the Fashion Square. In conjunction with the Fashion Square site access improvements, it is proposed that the existing Riverside Shopping Center driveway, which is currently located immediately adjacent to the Fashion Square site, can remain open. However, left-turn ingress directly to the Riverside Shopping Center driveway would no longer be available under the proposed access modifications as it would conflict with the new double left-turn lanes on the westbound Riverside Drive approach to the new Fashion Square driveway intersection. However, to improve access to the Riverside Shopping Center site, the Fashion Square proposes to construct an internal roadway connection between the two sites so as to provide Riverside Shopping Center patrons with convenient access to the newly created signalized intersection. Therefore, the Riverside Shopping Center site will be provided with improved left-turn ingress access from Riverside Drive via a signalized intersection (i.e., as compared to the existing non-signalized left-turn access). addition, with the provision for the new internal roadway connection to the newly created intersection, Riverside Shopping Center patrons will be able to safely complete left-turn exits onto westbound Riverside Drive (a movement which is currently not available from any of the Riverside Shopping Center driveways along Riverside Drive).

It is noted that the owners of the Riverside Shopping Center may not permit an internal roadway connection between the Fashion Square and its center. In this scenario, the Riverside Shopping Center will continue to have vehicular access from its existing driveways along Riverside Drive and Woodman Avenue, albeit with restricted left-turn ingress from Riverside Drive (which is common at many commercial centers in Los Angeles located immediately adjacent to intersections of major roadways due to traffic operational safety issues). Therefore, the potential restrictions of left-turn ingress directly to the Riverside Shopping Center from Riverside Drive would not create a significant adverse traffic impact.

■ Landscaped Raised Median Island:

In conjunction with the Matilija Avenue improvements, construct a landscaped raised median island along Riverside Drive from Matilija Avenue to Ranchito Avenue to prohibit left-turn movements on Riverside Drive to Matilija Avenue. This will limit the potential for Fashion Square or other regional traffic from using Matilija Avenue north of Riverside Drive.

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4.0 PROJECT PARKING

This shared parking analysis has been prepared for the proposed expansion of the Westfield Fashion Square. This shared parking analysis demonstrates that combining compatible land uses in a single development results in less parking demand than would be required for separate free-standing land uses of similar types as required under Section 12.21.A.4 of the Los Angeles Municipal Code for each of the new uses proposed for the expansion. Specifically, the project will request the Zoning Administrator to issue a finding that Shared Parking is applicable to the project under the provisions of Section 12.24.X.20 of the Los Angeles Municipal Code. The shared parking analysis has been prepared based on data published in the second edition of the Shared Parking manual published by the Urban Land Institute (ULI)³, and supplemented by observations of existing parking demand at the site which together demonstrate the adequacy of the proposed on-site parking supply for the project.

4.1 Existing Parking Requirement

Prior development approvals at the Westfield Fashion Square (e.g., ZA-95-0899 (CUZ) and CPC 94-0287 (ZC)) have established the parking requirement for the site at 4.5 parking spaces per 1,000 square feet of gross leasable floor area (applicable to retail, restaurant, office, etc.). The existing Westfield Fashion Square provides approximately 867,000 square feet of gross leasable floor area, thereby yielding a current parking requirement for approximately 3,902 parking spaces on-site. Parking is currently provided in on-site parking structures and surface parking lots. Vehicular access to the on-site parking facilities is provided by Hazeltine Avenue, Riverside Drive and Woodman Avenue. During periods of high parking demand (e.g., during the holiday shopping period), the shopping center management has by practice instructed employees to park in the surface parking lot located at the far easterly portion of the site (i.e., adjacent to Woodman Avenue).

4.2 Existing Parking Utilization

To determine the adequacy of the existing parking requirement (i.e., 4.5 parking spaces per 1,000 square feet of gross leasable floor area), observations of parking demand were conducted at the Fashion Square during the 2005 and 2006 holiday shopping periods on three days of typically high patronage: the day after Thanksgiving, the Saturday before Christmas, and the day after Christmas.

The average peak parking demand observed at the Fashion Square during each of the three days during the 2005 and 2006 holiday shopping periods is as follows:

- 2005/2006 Friday after Thanksgiving: 3,367 average peak demand (observed at 2:00 p.m.)
- 2005/2006 Saturday before Christmas: 3,362 average peak demand (observed at 4:00 p.m.)
- 2005/2006 Day after Christmas: 3,498 average peak demand (observed at 4:00 p.m.)

³ Shared Parking, Second Edition, Urban Land Institute, 2005, Washington D.C.

Based on the current 867,000 square feet of gross leasable floor area, the peak demand for parking at the Fashion Square during the 2005 and 2006 holiday seasons (3,498 parking spaces on the day after Christmas) was equivalent to approximately 4.03 spaces per 1,000 square feet of gross leasable floor area. Thus, the observed peak parking rate is significantly less than the current requirement to provide 4.5 parking spaces per 1,000 square feet of gross leasable floor area. It is noted that during the 2005 and 2006 holiday seasons that Fashion Square employees were parked on-site (primarily in the existing surface parking area near the Woodman Avenue driveway).

It is noted as a community benefit, that the Fashion Square currently permits students from the nearby Notre Dame High School and Buckley School to park vehicles in the surface parking lot near Woodman Avenue. This parking occurs during weekday school hours, which have typically been a relatively low parking demand period, generated by the Fashion Square. Should Fashion Square continue to accommodate student parking at this area of the site, it would not impact parking availability for the center (which typically is at its peak on weekends and holiday periods when schools are not in session).

4.3 Proposed Project

The Fashion Square proposes to provide additional floor area at the site for retail and restaurant uses. Floor area is expressed in terms of the Los Angeles Municipal Code (LAMC) definition, as well as in gross leasable floor area (which is the common floor area calculation used for enclosed shopping malls such as Fashion Square). The summary of existing and proposed floor area is provided in the *Table 4-1*.

Table 4-1 SUMMARY OF PROJECT FLOOR AREA										
	Existing Floor Area		Net New Floor Area		Total Floor Area					
Use	LAMC	Leasable	LAMC	Leasable	LAMC	Leasable				
Retail	956,422	842,045	355,227	233,178	1,311,649	1,075,223				
Restaurant	31,694	24,955	71,329	46,822	103,023	71,777				
Total	988,116	867,000	426,556	280,000	1,414,672	1,147,000				

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4.4 Code Parking Calculation for Proposed Future Fashion Square Development

A calculation has been prepared of the parking needed for the Fashion Square based on parking rates provided in the Los Angeles Municipal Code (LAMC) in conjunction with a proposed expansion project at the center. Specifically, Section 12.21.A.4 of the LAMC provides the following parking rates applicable to the project floor area⁴:

- Retail: 1 space per 250 square feet of retail floor area.
- Restaurant Space: 1 space per 100 square feet of restaurant floor area.

As currently proposed, the Fashion Square expansion will provide 355,227 square feet of new retail space and 71,329 square feet of new restaurant space per LAMC⁵. This new development will be in addition to the existing 988,116 square feet of commercial floor area. Based on the parking rates provided in the LAMC, the calculated parking for the project is as follows:

- New Retail (1 space/250 SF for 355,227 SF): 1,421 parking spaces
- New Restaurants (1 space/100 SF for 71,329 SF): 713 parking spaces
- Existing shopping center (by permit): 3,902 parking spaces
- Total calculated project parking by Code: 6,036 parking spaces

The project proposes to provide parking that is less than the number of parking spaces that would otherwise be required under Section 12.21.A.4 of the LAMC. Specifically, the Westfield Fashion Square proposes to provide parking at a rate of 4.25 parking spaces per 1,000 square feet of gross leasable floor area (i.e., 4,875 spaces based on a total center of 1,147,000 gross leasable square feet), with the potential to provide parking at a maximum rate of 4.50 parking spaces per 1,000 square feet of gross leasable floor area (i.e., 5,162 spaces based on a total center of 1,147,000 gross leasable square feet). Thus, the project will request the Zoning Administrator to issue a finding that Shared Parking is applicable to the project under the provisions of Section 12.24.X.20 of the LAMC. The basis for reduced parking under the Shared Parking provisions in the LAMC is demonstrated hereafter by the shared parking analysis which has been prepared based on data published in the second edition of the Shared Parking manual published by the Urban Land Institute (ULI), and supplemented by the observations of existing parking demand at the site which together demonstrate the adequacy of the proposed on-site parking supply for the project.

4.5 **Shared Parking Demand Analysis**

This shared parking analysis incorporates the analysis procedures recommended in the Shared Parking manual published by the ULI, and is consistent with methodology used by the City of Los Angeles in the review and approval of shared parking applications for other major retail centers.

⁴ Floor area as defined by the Los Angeles Municipal Code, which differs from gross leasable floor area.

⁵ All floor areas in this section as defined by the Los Angeles Municipal Code.

The *Shared Parking* manual provides recommendations with respect to the following characteristics of parking demand at shopping centers:

- Hourly Parking Indices. The Shared Parking manual provides hourly parking indices for various land uses. For the Fashion Square, the hourly parking indices for retail, and restaurants (sit-down and fast-food type restaurants) were utilized. The indices show, for example, that the hourly parking demand for retail (which generates its peak parking demand during the early afternoon period) is different than the parking demand seen at sit-down type restaurants (which generates its peak parking demand in the evening hours).
- <u>Day of Week Parking Variations</u>. The *Shared Parking* manual provides recommendations for day of week parking factors. For example, retail and restaurant uses generate their peak parking during weekends.
- <u>Monthly Parking Variations</u>. The *Shared Parking* manual considers that some uses have substantial parking variations based on the month of the year. Retail uses, for example typically generates its highest parking demand December while restaurant uses have a generally consistent parking demand throughout the year.
- <u>Internal Capture</u>. Parking demand at mixed-use centers can be reduced through internal capture characteristics. For example, a person working in a retail establishment within a mall may walk to the restaurants in the center to eat during lunch. The *Shared Parking* manual indicates that parking demand may be reduced by at least 10% at a mixed-use center based on these internal capture characteristics.

4.5.1 Shared Parking Demand Analysis for Existing Conditions

A shared parking demand analysis has been prepared for existing conditions at Westfield Fashion Square to demonstrate the validity of the parking indices provided in the ULI *Shared Parking* manual. The following existing land uses and corresponding floor areas⁶ were entered into the shared parking analysis:

- Existing Retail: 842,045 square feet of gross leasable area
- Existing "Fast Food" Restaurants: 20,275 square feet of gross leasable area
- Existing "Family" Restaurants: 4,680 square feet of gross leasable area

As previously noted, observations of existing parking utilization were conducted at the Fashion Square during the 2005 and 2006 holiday seasons (i.e., the Friday after Thanksgiving, the Saturday before Christmas, and the day after Christmas). Accordingly, the shared parking analysis was prepared for a December weekday (shown in *Table 4-2*) and weekend (shown in *Table 4-3*) condition for the existing floor area at Fashion Square. As shown in *Table 4-2*, a

⁶ The ULI *Shared Parking* manual is based on gross leasable floor areas for retail and restaurant uses, which differs from the floor area definitions provided in the LAMC. To be consistent with the *Shared Parking* manual methodologies, gross leasable floor areas are utilized in the shared parking analysis provided herein.

Table 4-2
WEEKDAY SHARED PARKING DEMAND ANALYSIS [1]

Westfield Fashion Square - Existing Conditions

Month: DEC

					Month. DEC
Land Use	Retail	Family Restaurant	Fast-Food Restaurant		
Size	842.0 KSF	4.7 KSF	20.3 KSF		
Peak Pkg Rate[2]	4.0 /KSF	15.0 /KSF	15.0 /KSF		
Weekday Pkg Rate[3]	3.6 /KSF	10.5 /KSF	15.0 /KSF		
Gross Spaces	3,031 Spc.	49 Spc.	304 Spc.		Comparison w/
Adjusted Gross	2,879 Spc.	44 Spc.	274 Spc.		Parking Supply
Spaces[4]				Shared	3902 Spaces
	Number of	Number of	Number of	Parking	Surplus
Time of Day	Spaces	Spaces	Spaces	Demand	(Deficiency)
6:00 AM	79	13	18	110	3,792
7:00 AM	200	24	31	255	3,647
8:00 AM	572	28	59	659	3,243
9:00 AM	1,232	34	86	1,352	2,550
10:00 AM	1,983	38	159	2,180	1,722
11:00 AM	2,503	40	239	2,782	1,120
12:00 PM	2,763	44	274	3,081	821
1:00 PM	2,879	40	274	3,193	709
2:00 PM	2,763	25	249	3,037	865
3:00 PM	2,647	22	169	2,838	1,064
4:00 PM	2,647	22	153	2,822	1,080
5:00 PM	2,735	35	169	2,939	963
6:00 PM	2,735	36	235	3,006	896
7:00 PM	2,735	36	223	2,994	908
8:00 PM	2,359	36	142	2,537	1,365
9:00 PM	1,580	28	86	1,694 _	2,208
10:00 PM	920	25	59	1,004	2,898
11:00 PM	316	23	31	370	3,532
12:00 AM	0	12	20	32	3,870

Notes:

- [1] Source: ULl Urban Land Institute "Shared Parking," Second Edition, 2005.
- [2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.
- [3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2 of the "Shared Parking" manual.
- [4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

Table 4-3
WEEKEND SHARED PARKING DEMAND ANALYSIS [1]
Westfield Fashion Square - Existing Conditions

Month:

DEC

					Mondi. DEC
Land Use	Retail	Family Restaurant	Fast-Food Restaurant		
Size	842.0 KSF	4.7 KSF	20.3 KSF		
Peak Pkg Rate[2]	4.0 /KSF	15.0 /KSF	15.0 /KSF		
Weekend Pkg Rate[3]	4.0 /KSF	15.0 /KSF	14.0 /KSF		
Gross Spaces	3,368 Spc.	70 Spc.	284 Spc.		Comparison w/
Adjusted Gross	3,200 Spc.	63 Spc.	256 Spc.		Parking Supply
Spaces[4]				Shared	3902 Spaces
	Number of	Number of	Number of	Parking	Surplus
Time of Day	Spaces	Spaces	Spaces	Demand	(Deficiency)
6:00 AM	90	10	17	117	3,785
7:00 AM	224	21	29	274	3,628
8:00 AM	512	32	55	599	3,303
9:00 AM	1,248	46	81	1,375	2,527
10:00 AM	1,824	58	148	2,030	1,872
11:00 AM	2,272	58	223	2,553	1,349
12:00 PM	2,688	63	256	3,007	895
1:00 PM	2,944	55	256	3,255	647
2:00 PM	3,200	44	232	3,476	426
3:00 PM	3,200	29	157	3,386	516
4:00 PM	3,072	31	142	3,245	657
5:00 PM	2,912	41	157	3,110	792
6:00 PM	2,592	47	219	2,858	1,044
7:00 PM	2,432	47	208	2,687	1,215
8:00 PM	2,144	44	132	2,320	1,582
9:00 PM	1,696	23	81	1,800	2,102
10:00 PM	1,184	20	55	1,259	2,643
11:00 PM	480	14	29	523	3,379
12:00 AM	0	8	18	26	3,876

Notes:

- [1] Source: ULl Urban Land Institute "Shared Parking," Second Edition, 2005.
- [2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.
- [3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2
- of the "Shared Parking" manual.
- [4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

peak parking demand for 3,193 parking spaces (at 1:00 p.m.) is forecast for a weekday while *Table 4-3* shows a peak parking demand for 3,476 parking spaces (at 2:00 p.m.). By comparison, during the most recent 2005 and 2006 holiday seasons, an average peak parking demand of 3,367 spaces as observed on the Friday after Thanksgiving, 3,362 spaces on the Saturday before Christmas, and 3,498 on the day after Christmas. Thus, the parking demand model developed for the Fashion Square using the ULI methodology is highly correlated to the observed parking demand during the 2005 and 2006 holiday seasons. Therefore, it is concluded that the shared parking methodology provides a reasonable model for purposes of forecasting future parking demand at Fashion Square following build-out of the proposed expansion project.

4.5.2 Shared Parking Demand Analysis for Future Conditions

A shared parking demand analysis has been prepared for future conditions at Westfield Fashion Square following build-out of the proposed expansion program (i.e., the addition of 280,000 square feet of gross leasable floor area, providing a total of 1,147,000 square feet of gross leasable floor area). The forecast parking demand based on the shared parking analysis has been compared to a proposed parking supply of 4,875 parking spaces, which is equivalent to a ratio of 4.25 parking spaces per 1,000 square feet of gross leasable floor area. Based on building programming information provided by Westfield, the following future build-out land uses and corresponding floor areas were entered into the shared parking analysis:

- Future Retail: 1,075,223 square feet of gross leasable area
- Future "Fast-Food" Restaurants: 39,097 square feet of gross leasable area
- Future "Family" Restaurants: 4,680 square feet of gross leasable area
- Future "Fine/Casual Dining" Restaurants: 28,000 square feet of gross leasable area

The shared parking analysis has been prepared for weekday and weekend conditions. Further, evaluations have been prepared for both non-holiday month conditions (e.g., July), as well as holiday conditions. Hourly parking forecasts have been prepared from 6:00 a.m. to 12:00 a.m. for each of the analysis days to evaluate parking demand during operating hours of the center during typical (non-holiday) and non-typical (holiday) conditions. Therefore, the analysis is deemed to be in compliance with the requirements of Section 12.24.X.20 of the LAMC whereby a review of parking demand for "24 hours per day, for seven consecutive days" is required. It is noted that the parking demand forecasts account for parking generated by both shopping center employees and patrons.

Tables 4-4 and 4-5 provide the weekday and weekend shared parking analysis for Fashion Square for a non-holiday month (i.e., July). As shown in *Table 4-4*, the forecast peak demand for parking at the site for a July weekday condition is expected to occur at 1:00 p.m. when approximately 3,371 parking spaces are expected to be utilized. Similarly, the parking demand forecast for a July weekend condition is summarized in *Table 4-5* with an expected demand of approximately 3,474 parking spaces at 2:00 p.m. In conclusion, a proposed parking supply of 4,875 parking spaces (i.e., 4.25 parking spaces per 1,000 square feet of gross leasable floor area) would result in a substantial surplus in parking at the site during non-holiday periods (i.e., a

Table 4-4
WEEKDAY SHARED PARKING DEMAND ANALYSIS [1]
Westfield Fashion Square - Future Conditions (July)

Month: JUL

Land Use	Retail	Fine/Casual Dining	Family Restaurant	Fast-Food Restaurant		
Size	1,075.2 KSF	28.0 KSF	4.7 KSF	39.1 KSF		
Peak Pkg Rate[2]	4.0 /KSF	20.0 /KSF	15.0 /KSF	15.0 /KSF		
Weekday Pkg Rate[3]	3.6 /KSF	18.0 /KSF	10.5 /KSF	15.0 /KSF		
Gross Spaces	3,871 Spc.	504 Spc.	49 Spc.	586 Spc.		Comparison w/
Adjusted Gross	3,677 Spc.	454 Spc.	44 Spc.	527 Spc.		Parking Supply
Spaces[4]					Shared	4875 Spaces
	Number of	Number of	Number of	Number of	Parking	Surplus
Time of Day	Spaces	Spaces	Spaces	Spaces	Demand	(Deficiency)
6:00 AM	76	0	12	34	122	4,753
7:00 AM	181	14	24	60	279	4,596
8:00 AM	513	35	27	112	687	4,188
9:00 AM	1,092	52	33	164	1,341	3,534
10:00 AM	1,718	119	38	300	2,175	2,700
11:00 AM	2,154	213	40	452	2,859	2,016
12:00 PM	2,373	345	43	518	3,279	1,596
1:00 PM	2,468	345	40	518	3,371	1,504
2:00 PM	2,373	307	25	470	3,175	1,700
3:00 PM	2,278	203	22	318	2,821	2,054
4:00 PM	2,278	241	22	288	2,829	2,046
5:00 PM	2,344	352	34	318	3,048	1,827
6:00 PM	2,344	427	36	444	3,251	1,624
7:00 PM	2,344	446	36	422	3,248	1,627
8:00 PM	2,032	446	36	267	2,781	2,094
9:00 PM	1,377	446	27	164	2,014	2,861
10:00 PM	798	427	24	112	1,361	3,514
11:00 PM	276	342	23	60	701	4,174
12:00 AM	0	118	11	38	167	4,708

Notes:

^[1] Source: ULI - Urban Land Institute "Shared Parking," Second Edition, 2005.

^[2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.

^[3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2 of the "Shared Parking" manual.

^[4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

Table 4-5
WEEKEND SHARED PARKING DEMAND ANALYSIS [1]
Westfield Fashion Square - Future Conditions (July)

Month: JUL

						Mondi. JCL
Land Use	Retail	Fine/Casual Dining	Family Restaurant	Fast-Food Restaurant		
Size	1,075.2 KSF	28.0 KSF	4.7 KSF	39.1 KSF		
Peak Pkg Rate[2]	4.0 /KSF	20.0 /KSF	15.0 /KSF	15.0 /KSF		
Weekend Pkg Rate[3]	4.0 /KSF	20.0 /KSF	15.0 /KSF	14.0 /KSF		
Gross Spaces	4,301 Spc.	560 Spc.	70 Spc.	547 Spc.		Comparison w/
Adjusted Gross	4,086 Spc.	504 Spc.	63 Spc.	492 Spc.		Parking Supply
Spaces[4]					Shared	4875 Spaces
	Number of	Number of	Number of	Number of	Parking	Surplus
Time of Day	Spaces	Spaces	Spaces	Spaces	Demand	(Deficiency)
6:00 AM	86	0	10	32	128	4,747
7:00 AM	203	15	20	55	293	4,582
8:00 AM	470	23	32	104	629	4,246
9:00 AM	1,118	46	45	152	1,361	3,514
10:00 AM	1,602	57	57	280	1,996	2,879
11:00 AM	1,981	120	57	422	2,580	2,295
12:00 PM	2,328	267	62	484	3,141	1,734
1:00 PM	2,537	288	54	484	3,363	1,512
2:00 PM	2,746	246	43	439	3,474	1,401
3:00 PM	2,746	246	28	297	3,317	1,558
4:00 PM	2,642	246	31	269	3,188	1,687
5:00 PM	2,504	328	41	297	3,170	1,705
6:00 PM	2,230	453	46	415	3,144	1,731
7:00 PM	2,092	474	46	394	3,006	1,869
8:00 PM	1,850	495	43	249	2,637	2,238
9:00 PM	1,471	453	23	152	2,099	2,776
10:00 PM	1,026	453	19	104	1,602	3,273
11:00 PM	412	442	14	55	923	3,952
12:00 AM	0	248	8	35	291	4,584

Notes:

- [1] Source: ULI Urban Land Institute "Shared Parking," Second Edition, 2005.
- [2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.
- [3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2
- of the "Shared Parking" manual.
- [4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

minimum surplus of over 1,500 parking spaces during weekdays and over 1,400 parking spaces during weekends).

Tables 4-6 and 4-7 provide the December shared parking analysis for weekday and weekend conditions, respectively. For a weekday condition in December, *Table 4-6* indicates a peak demand for approximately 4,595 parking spaces at 1:00 p.m., which can be accommodated by the proposed supply of 4,875 parking spaces. *Table 4-7* shows a peak demand for 4,827 parking spaces at 2:00 p.m. for a weekend condition during the holiday season. Thus, sufficient parking would be provided at the site to accommodate the parking demand generated throughout the day during the holiday season based on a supply of 4.25 parking spaces per 1,000 square feet of gross leasable floor area. It is noted that the parking demand forecasts account for parking generated by both shopping center employees and patrons.

Table 4-6
WEEKDAY SHARED PARKING DEMAND ANALYSIS [1]
Westfield Fashion Square - Future Conditions (December)

Month: DEC Fine/Casual Fast-Food Family Land Use Retail Dining Restaurant Restaurant 1,075.2 KSF 28.0 KSF 4.7 KSF 39.1 KSF Size Peak Pkg Rate[2] 4.0 /KSF 20.0 /KSF 15.0 /KSF 15.0 /KSF Weekday Pkg Rate[3] 3.6 /KSF 18.0 /KSF 10.5 /KSF 15.0 /KSF 3,871 Spc. 504 Spc. 586 Spc. **Gross Spaces** 49 Spc. Comparison w/ 3,677 Spc. 454 Spc. **Adjusted Gross** 44 Spc. 527 Spc. Parking Supply Spaces[4] Shared 4875 Spaces Number of Number of Number of Number of Parking Surplus Time of Day Spaces Spaces Spaces Spaces Demand (Deficiency) 6:00 AM 102 0 13 34 149 4,726 7:00 AM 255 14 24 61 354 4,521 8:00 AM 730 35 28 114 907 3,968 9:00 AM 52 1,573 34 166 1,825 3,050 10:00 AM 2,533 120 38 305 2,996 1,879 11:00 AM 3,197 216 40 460 3,913 962 12:00 PM 3,529 351 44 527 4,451 424 1:00 PM 3,677 351 40 527 4,595 280 2:00 PM 3,529 312 25 478 4,344 531 3:00 PM 324 3,381 206 22 3,933 942 4:00 PM 3,381 245 22 293 3,941 934 5:00 PM 3,493 358 35 324 4,210 665 6:00 PM 3,493 435 36 452 4,416 459 454 36 429 7:00 PM 3,493 4,412 463 8:00 PM 3,014 454 36 271 1,100 3,775 9:00 PM 2,017 454 28 166 2,665 2,210 10:00 PM 1,175 435 25 114 1,749 3,126 11:00 PM 403 348 23 61 835 4,040

12

38

170

Notes:

12:00 AM

0

120

4,705

^[1] Source: UL1 - Urban Land Institute "Shared Parking," Second Edition, 2005.

^[2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.

^[3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2 of the "Shared Parking" manual.

^[4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

Table 4-7
WEEKEND SHARED PARKING DEMAND ANALYSIS [1]
Westfield Fashion Square - Future Conditions (December)

Month:

DEC

Land Use	Retail	Fine/Casual	Family Restaurant	Fast-Food		
		Dining		Restaurant		
Size	1,075.2 KSF	28.0 KSF	4.7 KSF	39.1 KSF		
Peak Pkg Rate[2]	4.0 /KSF	20.0 /KSF	15.0 /KSF	15.0 /KSF		
Weekend Pkg Rate[3]	4.0 /KSF	20.0 /KSF	15.0 /KSF	14.0 /KSF		
Gross Spaces	4,301 Spc.	560 Spc.	70 Spc.	547 Spc.		Comparison w/
Adjusted Gross	4,086 Spc.	504 Spc.	63 Spc.	492 Spc.		Parking Supply
Spaces[4]					Shared	4875 Spaces
	Number of	Number of	Number of	Number of	Parking	Surplus
Time of Day	Spaces	Spaces	Spaces	Spaces	Demand	(Deficiency)
6:00 AM	115	0	10	32	157	4,718
7:00 AM	286	15	21	56	378	4,497
8:00 AM	654	23	32	105	814	4,061
9:00 AM	1,594	46	46	155	1,841	3,034
10:00 AM	2,329	57	58	285	2,729	2,146
11:00 AM	2,901	121	58	429	3,509	1,366
12:00 PM	3,432	271	63	492	4,258	617
1:00 PM	3,759	292	55	492	4,598	277
2:00 PM	4,086	250	44	447	4,827	48
3:00 PM	4,086	250	29	302	4,667	208
4:00 PM	3,923	250	31	274	4,478	397
5:00 PM	3,718	333	41	302	4,394	481
6:00 PM	3,309	461	47	422	4,239	636
7:00 PM	3,106	483	47	401	4,037	838
8:00 PM	2,738	504	44	253	3,539	1,336
9:00 PM	2,166	461	23	155	2,805	2,070
10:00 PM	1,512	461	20	105	2,098	2,777
11:00 PM	613	450]4	56	1,133	3,742
12:00 AM	0	252	8	35	295	4,580

Notes:

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^[1] Source: ULI - Urban Land Institute "Shared Parking," Second Edition, 2005.

^[2] Peak parking rates for all land uses based on the recommended base parking ratios as contained in Table 2-2 of the "Shared Parking" manual.

^[3] Weekday parking rates based on the weekday parking demand ratios, as summarized in Table 2-2

of the "Shared Parking" manual.
[4] Gross spaces adjusted to reflect parking demand reduction due to captive market, internal capture, transit, and/or walk-in reduction.

5.0 5.0 EXISTING STREET SYSTEM

5.1 Regional Highway System

Regional access to the project site is provided by U.S. 101 (Ventura) Freeway, as shown in *Figure 1-1*. Northbound and southbound ramps are provided on U.S. 101 Freeway at Woodman Avenue and Van Nuys Boulevard in the project vicinity. A brief description of U.S. 101 Freeway is provided in the following paragraph.

U.S. 101 (Ventura) Freeway is a major north-south freeway that extends across northern and southern California. In the project vicinity, five mainline travel lanes are provided in each direction on U.S. 101 Freeway. Both northbound and southbound ramps are provided on U.S. 101 Freeway at Woodman Avenue, which borders the project site to the east. Northbound and southbound ramps are also provided on U.S. 101 Freeway at Van Nuys Boulevard, which is located approximately one-half mile west of the project site.

5.2 Local Street System

Access to the project site is provided via Hazeltine Avenue, Riverside Drive and Woodman Avenue. The following 18 study intersections were selected for analysis by LADOT staff in order to determine potential impacts related to the proposed project:

- 1. Van Nuys Boulevard/Riverside Drive.
- 2. Van Nuys Boulevard/U.S. 101 Freeway Westbound (WB) Ramps.
- 3. Van Nuys Boulevard/U.S. 101 Freeway Eastbound (EB) Ramps.
- 4. Tyrone Avenue/Moorpark Street.
- 5. Tyrone Avenue-Beverly Glen Boulevard/Ventura Boulevard.
- 6. Hazeltine Avenue/Magnolia Boulevard.
- 7. Hazeltine Avenue/Riverside Drive.
- 8. Hazeltine Avenue/Fashion Square Lane.
- 9. Hazeltine Avenue/Moorpark Street.
- 10. Hazeltine Avenue/Ventura Boulevard.
- 11. Woodman Avenue/Magnolia Boulevard.
- 12. Woodman Avenue/Riverside Drive.
- 13. Woodman Avenue/U.S. 101 Freeway Westbound (WB) Ramps.
- 14. Woodman Avenue/U.S. 101 Freeway Eastbound (EB) Ramps.

- 15. Woodman Avenue/Moorpark Street.
- 16. Woodman Avenue/Ventura Boulevard.
- 17. Matilija Avenue/Riverside Drive
- 18. New Westerly Project Driveway/Riverside Drive

Sixteen of the 18 study intersections selected for analysis are presently controlled by traffic signals. The Matilija Avenue/Riverside Drive intersection is currently two-way stop sign controlled with the stop signs facing the minor street. The existing lane configurations at the 18 study intersections are displayed in *Figure 5-1*.

5.3 Roadway Classifications

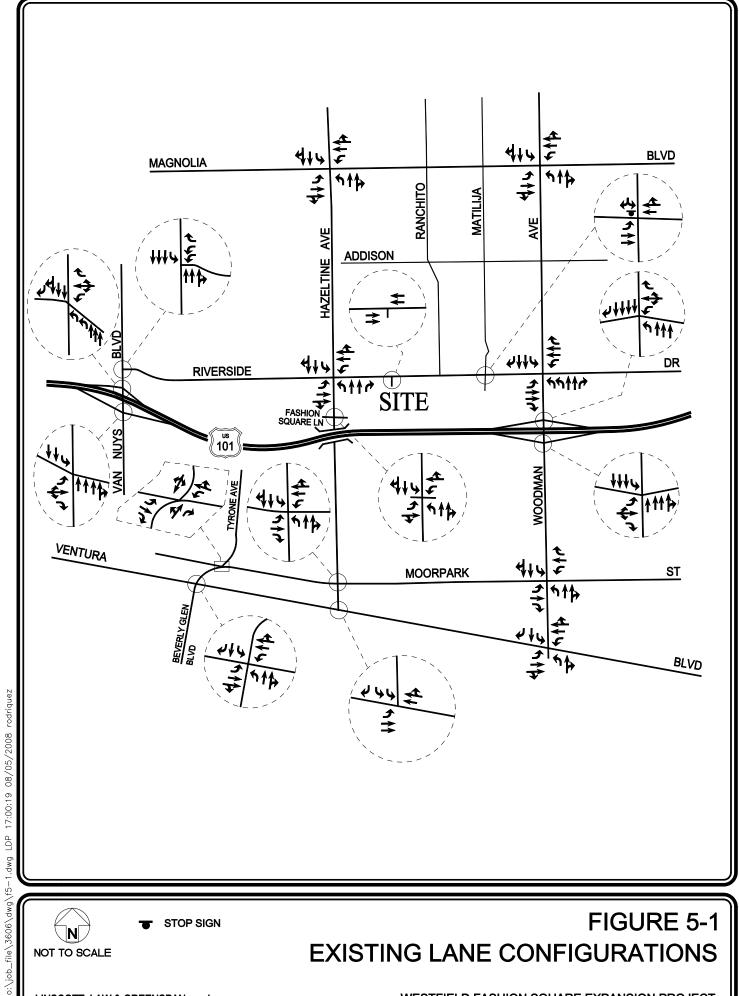
The City of Los Angeles utilizes the roadway categories recognized by regional, state and federal transportation agencies. There are four categories in the roadway hierarchy, ranging from freeways with the highest capacity to two-lane undivided roadways with the lowest capacity. The roadway categories are summarized as follows:

Freeways are limited-access and high speed travel ways included in the state and federal highway systems. Their purpose is to carry regional through-traffic. Access is provided by interchanges with typical spacing of one mile or greater. No local access is provided to adjacent land uses.

Arterial roadways are major streets that primarily serve through-traffic and provide access to abutting properties as a secondary function. Arterials are generally designed with two to six travel lanes and their major intersections are signalized. This roadway type is divided into two categories: principal and minor arterials. For the City of Los Angeles, these are referred to as Major and Secondary Highways. Principal arterials are typically four-or-more lane roadways and serve both local and regional through-traffic. Minor arterials are typically two-to-four lane streets that service local and commute traffic.

Collector roadways are streets that provide access and traffic circulation within residential and non-residential (e.g., commercial and industrial) areas. They connect local streets to arterials and are typically designed with two through travel lanes (i.e., one through travel lane in each direction) that may accommodate on-street parking. They may also provide access to abutting properties.

Local roadways distribute traffic within a neighborhood or similar adjacent neighborhoods and are not intended for use as a through-street or a link between higher capacity facilities such as collector or arterial roadways. Local streets are fronted by residential uses and do not typically serve commercial uses.





STOP SIGN

FIGURE 5-1 **EXISTING LANE CONFIGURATIONS**

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WESTFIELD FASHION SQUARE EXPANSION PROJECT

5.4 Roadway Descriptions

A brief description of the important roadways in the project site vicinity is provided in the following paragraphs.

Van Nuys Boulevard is a north-south oriented roadway that is located west of the project site. Van Nuys Boulevard is designated as a Major Highway Class II in the City of Los Angeles Transportation Element of the General Plan. Three travel lanes are provided in each direction on Van Nuys Boulevard within the study area. Exclusive left-turn lanes are provided in the southbound direction on Van Nuys Boulevard at the U.S. 101 Freeway Eastbound Ramps and Riverside Drive intersections. Dual left-turn lanes are provided in the northbound direction on Van Nuys Boulevard at the U.S. 101 Freeway Westbound Ramps intersection. Parking is prohibited along both sides of Van Nuys Boulevard between Riverside Drive and just south of the U.S. 101 Freeway Eastbound Ramps. Two-hour metered parking between the hours of 9:00 AM and 3:00 PM is provided along both sides of Van Nuys Boulevard north of Riverside Drive. Van Nuys Boulevard is posted for a speed limit of 35 miles per hour near the project site.

Tyrone Avenue is a north-south oriented roadway that is located west of the project site. Tyrone Avenue is designated as a Secondary Highway south of Moorpark Street in the City of Los Angeles General Plan Transportation Element. North of Moorpark Street, Tyrone Avenue is designated as a Local Street. One through travel lane is provided in each direction on Tyrone Avenue within the study area. An exclusive left-turn lane is provided in the southbound direction on Tyrone Avenue at the Ventura Boulevard intersection. Exclusive right-turn lanes are provided on Tyrone Avenue in the northbound direction at the Moorpark Street intersection and in the southbound direction at the Ventura Boulevard intersection. Parking is allowed along both sides of Tyrone Avenue in the project vicinity, except between Moorpark Street and Ventura Boulevard where parking is prohibited along both sides of Tyrone Avenue. There is no posted speed limit on Tyrone Avenue within the project study area, thus it is assumed to be a prima facie speed limit of 25 miles per hour.

Beverly Glen Boulevard is a north-south oriented roadway that is located west of the project site. Beverly Glen Boulevard is designated as a Secondary Highway in the City of Los Angeles General Plan Transportation Element. One through travel lane is provided in each direction on Beverly Glen Boulevard within the study area. An exclusive left-turn lane is provided in the northbound direction on Beverly Glen Boulevard at the Ventura Boulevard intersection. Parking is prohibited along both sides of Beverly Glen Boulevard in the project vicinity. Beverly Glen Boulevard is posted for a speed limit of 25 miles per hour near the project site.

Hazeltine Avenue is a north-south oriented roadway that borders the project site to the west. Hazeltine Avenue is designated as a Secondary Highway in the City of Los Angeles General Plan Transportation Element. Two through travel lanes are provided in each direction on Hazeltine Avenue within the study area. Exclusive left-turn lanes in each direction are provided on Hazeltine Avenue at the Magnolia Boulevard, Riverside Drive, Fashion Square Lane, Moorpark Street and Ventura Boulevard intersections. An exclusive right-turn lane is provided in the northbound direction on Hazeltine Avenue at the Riverside Drive intersection. Parking is allowed along both sides of Hazeltine Avenue in the project vicinity, except between Riverside

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LLG Ref. 1-053606-1

Westfield Fashion Square Expansion Project

Drive and Fashion Square Lane where parking is prohibited. Hazeltine Avenue is posted for a speed limit of 35 miles per hour near the project site.

Matilija Avenue is a north-south oriented roadway that is located north of the project site. Matilija Avenue is designated as a Local Street in the City of Los Angeles General Plan Transportation Element. One through travel lane is provided in each direction on Matilija Avenue within the study area. Parking is allowed along both sides of Matilija Avenue in the project vicinity. There is no posted speed limit on Matilija Avenue within the project study area, thus it is assumed to be a prima facie speed limit of 25 miles per hour.

Woodman Avenue is a north-south oriented roadway that borders the project site to the east. Woodman Avenue is designated as a Major Highway Class II in the City of Los Angeles General Plan Transportation Element. Two through travel lanes are provided in each direction on Woodman Avenue within the study area. Exclusive left-turn lanes are provided in each direction on Woodman Avenue at the Magnolia Boulevard, Moorpark Street and Ventura Boulevard intersections. Exclusive left-turn lanes are provided on Woodman Avenue in the northbound direction at the U.S. 101 Freeway Westbound Ramp intersection and in the southbound direction at the U.S. 101 Freeway Westbound Ramps and Riverside Drive intersections. Dual left-turn lanes are provided in the northbound direction on Woodman Avenue at the Riverside Drive intersection. Exclusive right-turn lanes are provided on Woodman Avenue in each direction at the Riverside Drive intersection and in the southbound direction at the Ventura Boulevard intersection. Curbside parking is allowed along both sides of Woodman Avenue in the project vicinity, except north of Riverside Drive where one-hour parking between the hours of 8:00 AM and 4:00 PM is provided along the west side of Woodman Avenue and south of Moorpark Street, where two-hour parking between the hours of 8:00 AM and 6:00 PM is provided along both sides of Woodman Avenue. Woodman Avenue is posted for a speed limit of 35 miles per hour near the project site.

Magnolia Boulevard is an east-west oriented roadway that is located north of the project site. Magnolia Boulevard is designated as a Secondary Highway in the City of Los Angeles General Plan Transportation Element. Two through travel lanes in the each direction are provided on Magnolia Boulevard in the project vicinity. Exclusive left-turn lanes are provided in each direction on Magnolia Boulevard at the Hazeltine Avenue and Woodman Avenue intersections. Two-hour parking between the hours of 8:00 AM and 6:00 PM is provided along both sides of Magnolia Boulevard in the project vicinity. Magnolia Boulevard is posted for a speed limit of 35 miles per hour near the project site.

Riverside Drive is an east-west oriented roadway that borders the project site to the north. Riverside Drive is designated as a Major Highway Class II in the City of Los Angeles General Plan Transportation Element. Two through travel lanes in the each direction are provided on Riverside Drive in the project vicinity. Exclusive left-turn lanes are provided in each direction on Riverside Drive at the Hazeltine Avenue and Woodman Avenue intersections. Dual left-turn lanes are provided in the westbound direction on Riverside Drive at the Van Nuys Boulevard intersection. Exclusive right-turn lanes are provided on Riverside Drive in each direction at the Woodman Avenue intersection and in the westbound direction at the Van Nuys Boulevard

intersection. One-hour parking between the hours of 8:00 AM and 6:00 PM is provided along the north side of Riverside Drive in the project vicinity. Two-hour parking between the hours of 8:00 AM and 6:00 PM is provided along the south side of Riverside Drive in the project vicinity. Class II bike lanes are provided in each direction on Riverside Drive between Riverside Drive and Moorpark Street. Riverside Drive is posted for a speed limit of 35 miles per hour near the project site.

Moorpark Street is an east-west oriented roadway that is located south of the project site. Moorpark Street is designated as a Secondary Highway in the City of Los Angeles General Plan Transportation Element. One through travel lane in each direction is provided on Moorpark Street in the project vicinity. Exclusive left-turn lanes are provided in each direction on Moorpark Street at the Tyrone Avenue, Hazeltine Avenue and Woodman Avenue intersections. Exclusive right-turn lanes are provided in the eastbound direction on Moorpark Street at the Tyrone Avenue and Hazeltine Avenue intersections and in both directions at the Woodman Avenue intersection. Curbside parking is allowed along both sides of Moorpark Street in the project vicinity, except east of Woodman Avenue where two-hour parking between the hours of 8:00 AM and 6:00 PM is provided along both sides of Moorpark Street. Moorpark Street is posted for a speed limit of 35 miles per hour near the project site.

Ventura Boulevard is an east-west oriented roadway that is located south of the project site. Ventura Boulevard is designated as a Major Highway Class II in the City of Los Angeles General Plan Transportation Element. Two through travel lanes are provided in each direction on Ventura Boulevard in the project vicinity. Exclusive left-turn lanes are provided in each direction on Ventura Boulevard at the Tyrone Avenue/Beverly Glen Boulevard, Hazeltine Avenue and Woodman Avenue intersections. Two-hour metered parking is provided from 8:00 AM to 6:00 PM along both sides of Ventura Boulevard in the project vicinity. Ventura Boulevard is posted for a speed limit of 35 miles per hour near the project site.

5.5 Existing Public Bus Transit Service

Public bus transit service in the project study area is currently provided by the Los Angeles County Metropolitan Transportation Authority (MTA), and the City of Los Angeles Department of Transportation (LADOT). A summary of the existing transit routes, including the transit route, destinations and peak hour headways is presented in *Table 5-1*. The existing public transit routes in the proposed project site vicinity are illustrated in *Figure 5-2*.

It is noted on *Table 5-1* and *Figure 5-2* that the project area is served by the MTA's Orange Line, a rapid bus transit service operating in a separate dedicated right-of-way that provides east-west service across the San Fernando Valley. The LADOT DASH service provides a convenient connection between the Orange Line's stop in Van Nuys to the Fashion Square.

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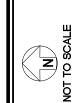
Table 5-1
EXISTING TRANSIT ROUTES [1]

		ROADWAY	1	NO, OF BU	
ROUTE	DESTINATIONS	NEAR SITE	DIR	AM	PM
MTA Route 96	Downtown LA to Sherman Oaks (via Griffith Park, Burbank, Universal City)	Riverside Dr, Van Nuys Blvd, Ventura Blvd, Tyrone Ave, Moorpark St	EB WB	2 2	1 2
MTA Route 150/240	Canoga Park to Universal City (via Woodland Hills, Tarzana, Sherman Oaks)	Ventura Blvd	EB WB	6 8	6 7
MTA Route 158	Chatsworth to Sherman Oaks (via Northridge, Arleta, Van Nuys)	Woodman Ave, Ventura Blvd, Moorpark St	NB SB	3 3	2
MTA Route 183	Giendale to Sherman Oaks (via Burbank, North Hollywood)	Magnolia Blvd, Ventura Blvd	EB WB	2 2	2 2
MTA Route 233	Lakeview Terrace to Westwood (via Pacoima, Van Nuys, Sherman Oaks, UCLA)	Van Nuys Blvd, Moorpark St, Ventura Blvd, Tyrone Ave	NB SB	12 12	13 12
MTA Route 237	Encino to Sherman Oaks (via Van Nuys, Northridge, Granada Hills)	Van Nuys Blvd, Moorpark St, Ventura Blvd, Tyrone Ave	NB SB	1	1 2
MTA Route 750	Universal City to Woodland Hills (via Sherman Oaks, Tarzana)	Ventura Blvd	EB WB	6 10	11 7
MTA Route 761	Pacoima to Westwood (via Panorama City, Sherman Oaks)	Van Nuys Bivd, Ventura Bivd	NB SB	5 9	11 5
LADOT Dash Van Nuys/Studio City	Van Nuys to Studio City (via Sherman Oaks)	Moorpark St, Hazeltine Ave	NB SB	3	3
Metro Orange Line Route 901	North Hollywood to Warner Center (via Valley Village, Van Nuys, Tarzana, Winnetka)	Woodman Avenue	WB EB	12 12	12 12

[1] Sources: Los Angeles County Metropolitan Transportation Authority (LACMTA) and City of Los Angeles Department of Transportation (LADOT).

FIGURE 5-2 EXISTING PUBLIC TRANSIT ROUTES

MAP SOURCE: METROPOLITAN TRANSPORTATION AUTHORITY



6.0 Traffic Counts

Manual counts of vehicular turning movements were conducted at each of the 18 study intersections during the weekday morning (AM) and afternoon (PM) commuter periods to determine the peak hour traffic volumes. The manual counts were conducted in November 2005 at the 18 study intersections from 7:00 to 10:00 AM to determine the AM peak commuter hour, and from 3:00 to 6:00 PM to determine the PM peak commuter hour. The traffic count data were increased at a rate of 2.0 percent (2.0%) per year to reflect year 2007 conditions. Traffic volumes at the study intersections show the typical peak periods between 7:00 to 10:00 AM and 3:00 to 6:00 PM generally associated with metropolitan Los Angeles peak commuter hours.

The AM and PM peak period manual counts of vehicle movements at the 18 study intersections are summarized in *Table 6-1*. The existing traffic volumes at the study intersections during the AM and PM peak hours are shown in *Figures 6-1 and 6-2*, respectively. Summary data worksheets of the manual traffic counts at the study intersections are contained in *Appendix B*.

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Table 6-1
WEEKDAY EXISTING (2007) TRAFFIC VOLUMES

		<u> </u>	ſ	AM PEA	K HOUR	PM PEAK HOUR		
NO.	INTERSECTION	DATE	DIR	BEGAN	VOLUME	BEGAN	VOLUME	
1	Van Nuys Boulevard/ Riverside Drive	11/17/05	NB SB EB WB	8:15	1,956 1,385 0 745	3:00	2,156 1,647 0 709	
2	Van Nuys Boulevard/ U.S. 101 Freeway Westbound Ramps	11/17/05	NB SB EB WB	8:15	1,776 1,723 0 834	3:00	2,422 1,871 0 755	
3	Van Nuys Boulevard/ U.S. 101 Freeway Eastbound Ramps	11/17/05	NB SB EB WB	8:45	1,273 1,586 1,315 0	3:00	2,029 1,415 1,306 0	
4	Tyrone Avenue/ Moorpark Street	11/17/05	NB SB EB WB	7:30	279 51 324 1,064	5:00	798 75 549 1,010	
5	Tyrone Avenue- Beverly Glen Boulevard/ Ventura Boulevard	11/17/05	NB SB EB WB	7:30	321 361 1,392 1,230	3:45	885 390 1,267 1,298	
6	Hazeltine Avenue/ Magnolia Boulevard	11/17/05	NB SB EB WB	7:30	581 1,032 1,015 1,228	5:00	1,145 825 1,356 673	
7	Hazeltine Avenue/ Riverside Drive	11/16/05	NB SB EB WB	7:30	545 1,132 830 1,327	5:00	1,229 1,056 817 994	
8	Hazeltine Avenue/ Fashion Square Lane	11/16/05	NB SB EB WB	7:30	558 1,231 9 3	5:00	1,094 1,089 17 130	
9	Hazeltine Avenue/ Moorpark Street	11/15/05	NB SB EB WB	7:45	284 1,421 537 900	4:45	629 908 1,064 743	
10	Hazeltine Avenue/ Ventura Boulevard	11/15/05	NB SB EB WB	7:45	0 835 1,197 1,479	5:00	0 397 1,676 1,506	

^[1] Counts conducted by Accutek Traffic Data and increased by 2 percent per year to reflect year 2007 conditions.

Table 6-1 (Continued) WEEKDAY EXISTING (2007) TRAFFIC VOLUMES

				AM PEA	K HOUR	PM PEA	K HOUR
NO.	INTERSECTION	DATE	DIR	BEGAN	VOLUME	BEGAN	VOLUME
11	Woodman Avenue/ Magnolia Boulevard	11/17/05	NB SB EB WB	7:30	938 1,549 1,019 1,184	5:00	1,239 1,000 1,139 733
12	Woodman Avenue/ Riverside Drive	11/16/05	NB SB EB WB	7:30	1,137 1,505 1,091 1,486	3:15	1,456 1,161 1,386 1,400
13	Woodman Avenue/ U.S. 101 Freeway Westbound Ramps	11/15/05	NB SB EB WB	7:30	1,239 1,607 0 583	5:00	1,500 1,403 0 765
14	Woodman Avenue/ U.S. 101 Freeway Eastbound Ramps	11/15/05	NB SB EB WB	8:00	1,145 1,433 734 0	4:45	1,458 1,297 825 0
15	Woodman Avenue/ Moorpark Street	11/15/05	NB SB EB WB	7:45	526 1,354 702 1,087	5:00	893 1,248 972 880
16	Woodman Avenue/ Ventura Boulevard	11/15/05	NB SB EB WB	7:45	270 803 1,250 1,250	4:45	279 523 1,340 1,054
17	Matilija Avenue- New Project Driveway/ Riverside Drive	11/15/05	NB SB EB WB	7:30	0 59 1,023 1,112	5:00	0 46 1,067 1,198
18	New Project Driveway/ Riverside Drive	11/15/05	NB SB EB WB	7:30	0 0 1,023 1,127	5:00	0 0 1,067 1,187

^[1] Counts conducted by Accutek Traffic Data and increased by 2 percent per year to reflect year 2007 conditions.

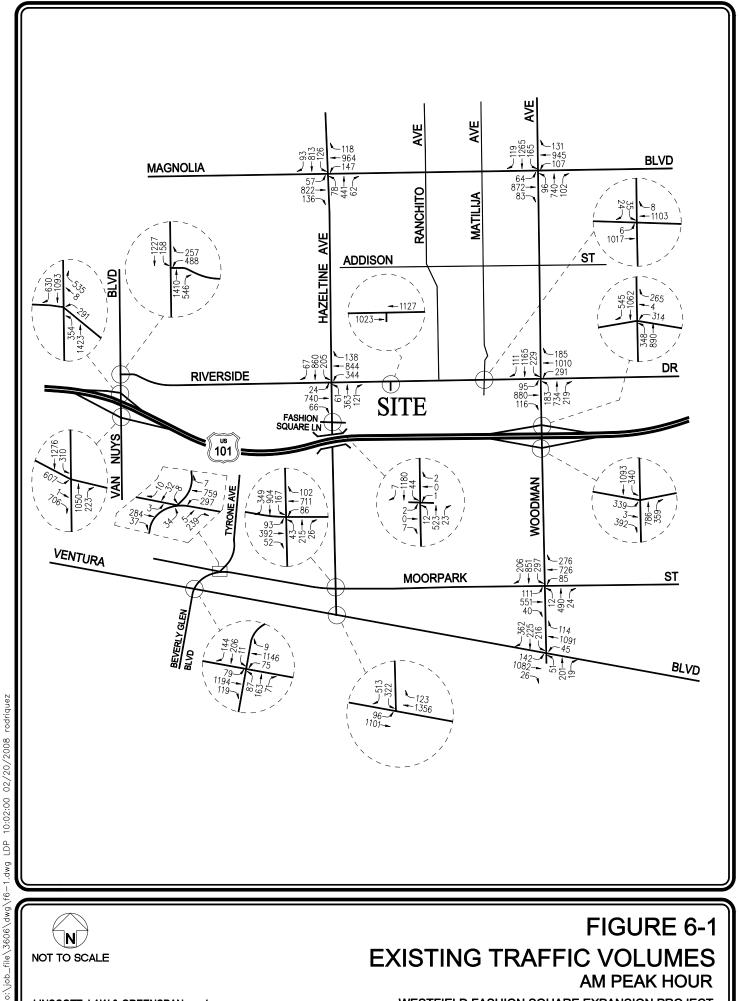




FIGURE 6-1 **EXISTING TRAFFIC VOLUMES AM PEAK HOUR**

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

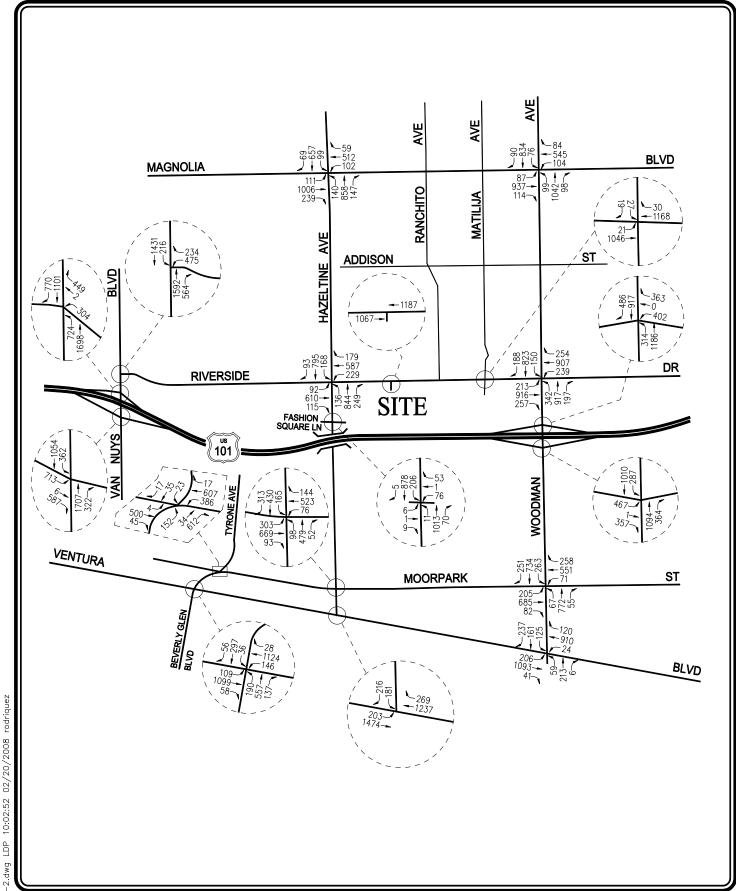




FIGURE 6-2 EXISTING TRAFFIC VOLUMES PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

7.0 Traffic Forecasting Methodology

In order to estimate the traffic impact characteristics of the proposed project, a multi-step process has been utilized. The first step is trip generation, which estimates the total arriving and departing traffic volumes on a peak hour and daily basis. The traffic generation potential is forecast by applying the appropriate vehicle trip generation equations or rates to the project development tabulation.

The second step of the forecasting process is trip distribution, which identifies the origins and destinations of inbound and outbound project traffic volumes. These origins and destinations are typically based on demographics and existing/anticipated travel patterns in the study area.

The third step is traffic assignment, which involves the allocation of project traffic to study area streets and intersections. Traffic assignment is typically based on minimization of travel time, which may or may not involve the shortest route, depending on prevailing operating conditions and travel speeds. Traffic distribution patterns are indicated by general percentage orientation, while traffic assignment allocates specific volume forecasts to individual roadway links and intersection turning movements throughout the study area.

With the forecasting process complete and project traffic assignments developed, the impact of the proposed project is isolated by comparing operational (i.e., Levels of Service) conditions at the selected key intersections using expected future traffic volumes with and without forecast project traffic. The need for site-specific traffic improvements can then be evaluated and the significance of the project's impacts identified.

7.1 Project Trip Generation

Traffic generation is expressed in vehicle trip ends, defined as one-way vehicular movements, either entering or exiting the generating land use. Traffic volumes expected to be generated by the proposed project during the AM and PM peak hours, as well as on a daily basis, were estimated using rates published in the Institute of Transportation Engineers' (ITE) *Trip Generation* manual. Traffic volumes expected to be generated by the proposed shopping center project were based upon per thousand square feet of gross leasable area. The proposed Westfield Fashion Square Expansion project includes both the existing uses at the shopping center plus the added square footage. ITE Land Use Code 820 (Shopping Center) trip generation equation rates were used to forecast the traffic volumes expected to be generated by the proposed project.

Traffic volumes expected to be generated by the existing shopping center located on the project site were also estimated using rates published in the ITE *Trip Generation* manual. ITE Land Use Code 820 (Shopping Center) trip generation equation rates were used to forecast the traffic volumes expected to be generated by the existing shopping center located on the project site.

LLG Ref. 1-053606-1 Westfield Fashion Square Expansion Project

⁷ Trip Generation Manual, Institute of Transportation Engineers, 7th Edition, 2003.

In addition to the trip generation forecast for the proposed project (which is essentially an estimate of vehicles that could be expected to enter and exit the site access points), a forecast was made of the likely pass-by trips that could be anticipated at the site. Pass-by trips are made as intermediate stops on the way from an origin to a primary trip destination without a route diversion. Pass-by trips are attracted from traffic passing the site on an adjacent street or roadway that offers direct access to the generator. The pass-by traffic forecast has been estimated based on existing traffic volumes at the study intersections, recommended practice in Chapter 5 of the ITE *Trip Generation Handbook*⁸, and LADOT's policy on pass-by trips as stated in the City's *Traffic Study Policies and Procedures*. A 10 percent (10%) pass-by adjustment has been applied to the project AM and PM peak hour traffic volume forecasts, as well as to the daily traffic volume forecast for the existing shopping center and the proposed shopping center project.

The trip generation forecast for the proposed project is summarized in *Table 7-1*. The trip generation forecast for the proposed project was submitted for review and approval by LADOT staff. As presented in *Table 7-1*, the proposed project is expected to generate a net increase of 95 vehicle trips (58 inbound trips and 37 outbound trips) during the AM peak hour. During the PM peak hour, the proposed project is expected to generate a net increase of 476 vehicle trips (229 inbound trips and 247 outbound trips). Over a 24-hour period, the proposed project is forecast to generate a net increase of 4,964 daily trip ends during a typical weekday (2,482 inbound trips and 2,482 outbound trips).

It is noted that the trip generation forecast provided herein likely overstates the actual amount of vehicular traffic that would be generated by the proposed expansion. By example, traffic counts were conducted at the existing Fashion Square driveways during the weekday morning and afternoon commuter peak periods. As shown in *Appendix C*, the Fashion Square currently generates 418 AM peak hour trips (327 inbound trips and 91 outbound trips) and 1,850 PM peak hour trips (836 inbound trips and 1,014 outbound trips). The actual peak hour traffic count data is approximately 25% less than the estimates based on the ITE trip rates for shopping centers of similar size. This trip generation characteristic is likely due to the Fashion Square providing "high end" tenants which require relatively fewer patrons to achieve revenue goals. However, to provide a conservative, "worst-case" traffic analysis, the trip generation forecast based on the ITE trip rates have been utilized in the review of potential impacts associated with the project.

7.2 Project Trip Distribution

Project generated traffic was assigned to the local roadway system based on a trip distribution pattern developed in consultation with City staff. The traffic distribution pattern was based on the proposed project land uses, the existing and planned project site access schemes, existing traffic patterns, characteristics of the surrounding roadway system, and nearby population and employment centers. The trip distribution pattern for the proposed project was developed in consultation with LADOT staff.

LLG Ref. 1-053606-1 Westfield Fashion Square Expansion Project

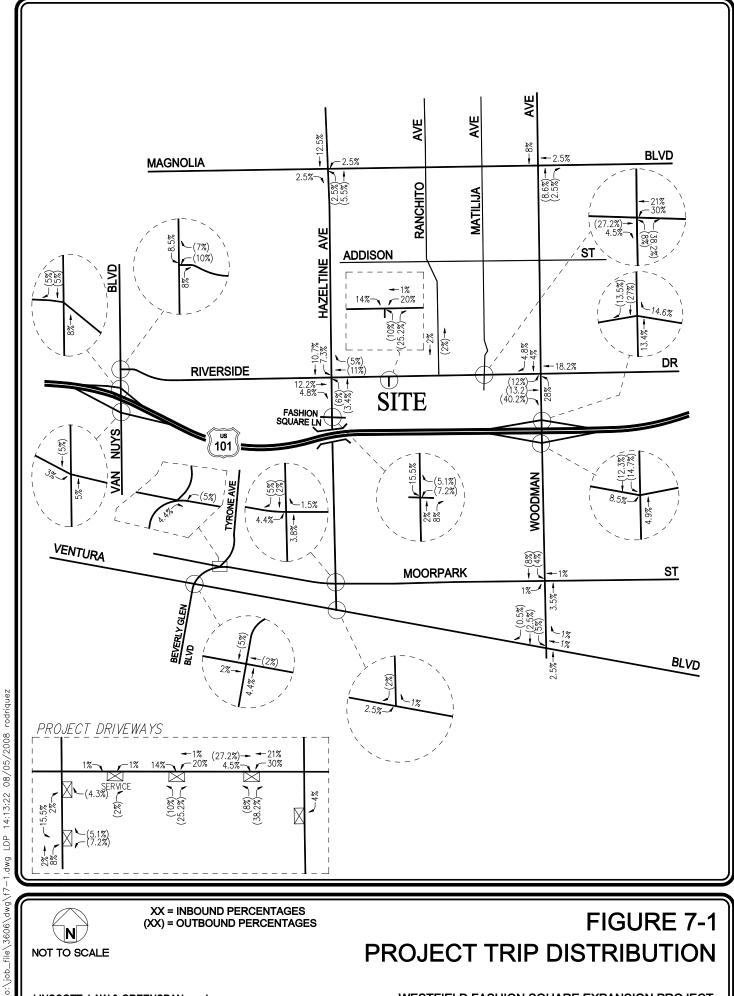
⁸ Trip Generation Handbook, Institute of Transportation Engineers, June, 2004.

Table 7-1
WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY	AM PEAK HOUR			PM PEAK HOUR		
		TRIP ENDS [2]	V	OLUME	S [2]	V	OLUME	S [2]
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed								
Shopping Center	1,147,000 GLSF	33,162	413	264	677	1,504	1,629	3,133
Less 10% Pass-by [4]		(3,316)	(41)	(26)	(67)	(150)	(163)	(313)
Subtotal		29,846	372	238	610	1,354	1,466	2,820
Existing								
Shopping Center	867,000 GLSF	27,647	349	223	572	1,250	1,354	2,604
Less 10% Pass-by [4]		(2,765)	(35)	(22)	(57)	(125)	(135)	(260)
Subtotal		24,882	314	201	515	1,125	1,219	2,344
NET CHANGE	280,000 GLSF	4,964	58	37	95	229	247	476

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

The project traffic volume distribution percentages during AM and PM peak hours at the 18 study intersections are illustrated in Figure 7-1. The forecast project traffic volumes at the study intersections for the AM and PM peak hours are displayed in Figures 7-2 and 7-3, respectively.





XX = INBOUND PERCENTAGES (XX) = OUTBOUND PERCENTAGES

FIGURE 7-1 PROJECT TRIP DISTRIBUTION

LINSCOTT, LAW & GREENSPAN, engineers

WESTFIELD FASHION SQUARE EXPANSION PROJECT

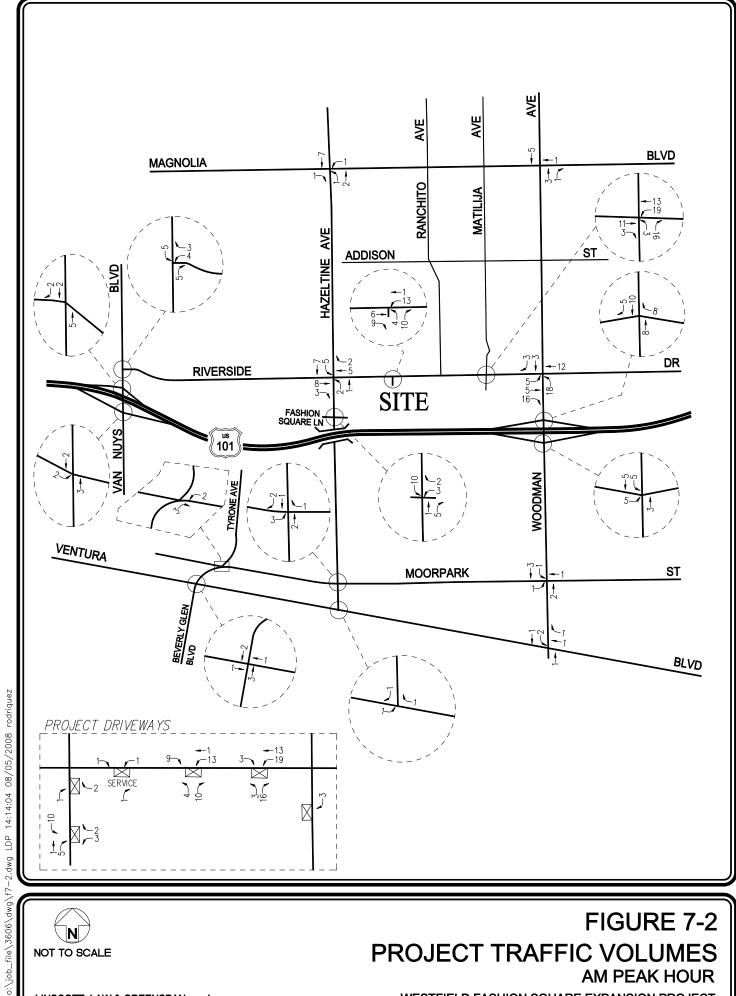
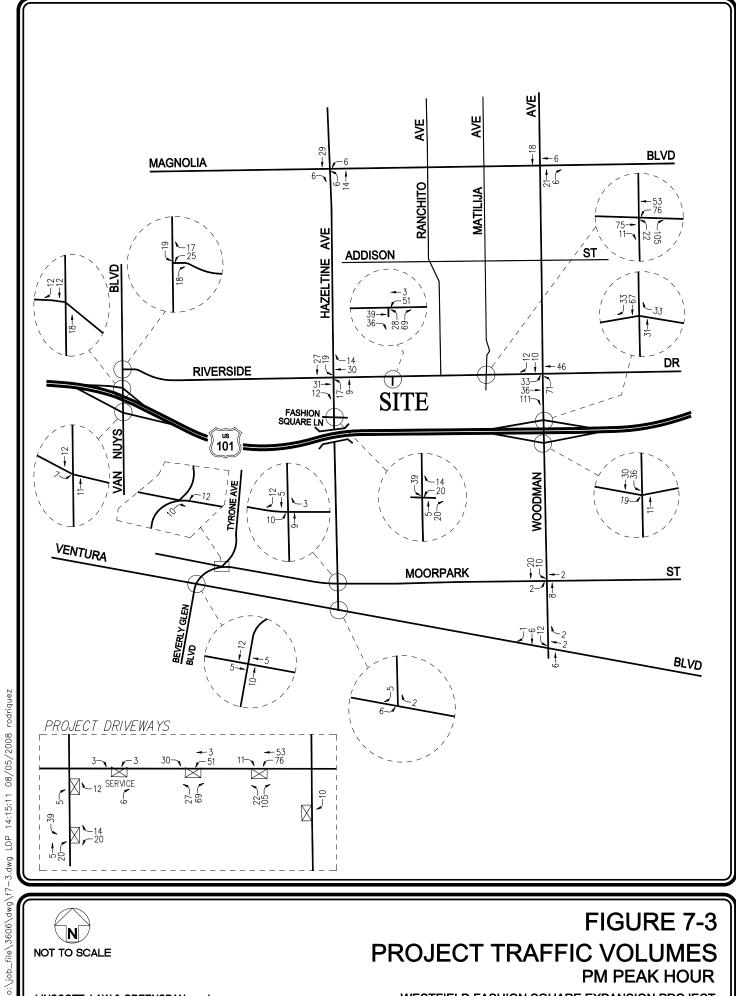




FIGURE 7-2 PROJECT TRAFFIC VOLUMES **AM PEAK HOUR**

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE





LINSCOTT, LAW & GREENSPAN, engineers

FIGURE 7-3 PROJECT TRAFFIC VOLUMES PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

8.0 CUMULATIVE DEVELOPMENT PROJECTS

The forecast of future pre-project conditions was prepared in accordance to procedures outlined in Section 15130 of the CEQA Guidelines. Specifically, the CEQA Guidelines provides two options for developing the future traffic volume forecast:

- "(A) A list of past, present, and probable future projects producing related or cumulative impacts, including, if necessary, those projects outside the control of the [lead] agency, or
- (B) A summary of projections contained in an adopted general plan or related planning document, or in a prior environmental document which has been adopted or certified, which described or evaluated regional or areawide conditions contributing to the cumulative impact. Any such planning document shall be referenced and made available to the public at a location specified by the lead agency."

Accordingly, the traffic analysis provides a highly conservative estimate of future pre-project traffic volumes as it incorporates both the "A" and "B" options outlined in CEQA Guidelines for purposes of developing the forecast. Option A is considered through the review and analysis of the potential traffic generated by related projects. Option B is incorporated through the use of an annual traffic growth rate as adopted by the Congestion Management Agency for Los Angeles County.

8.1 Related Projects

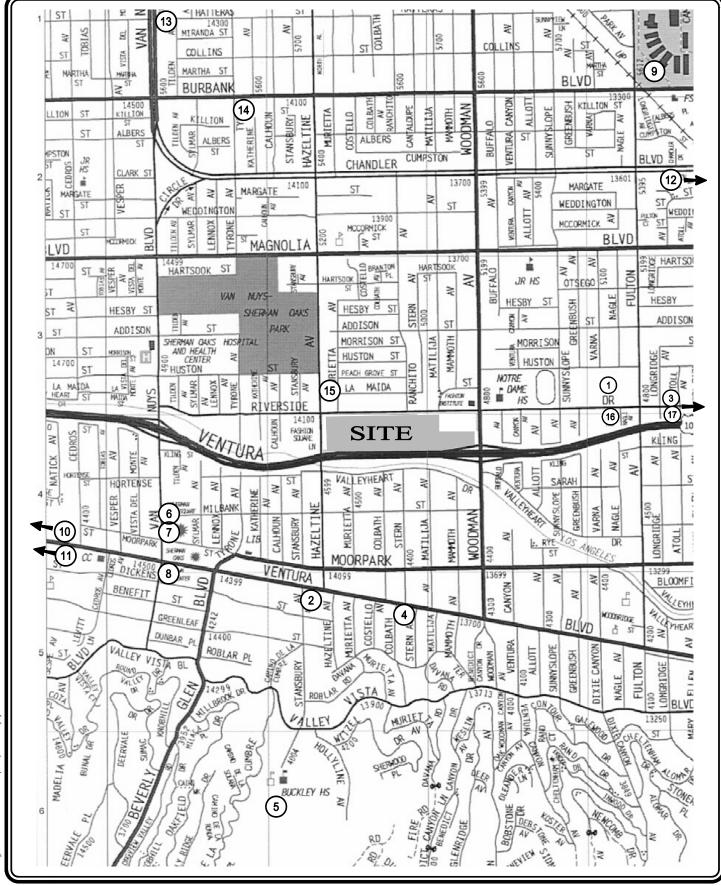
A forecast of on-street traffic conditions prior to occupancy of the proposed project was prepared by incorporating the potential trips associated with other known development projects (related projects) in the area. With this information, the potential impact of the proposed project can be evaluated within the context of the cumulative impact of all ongoing development. The related projects research was based on information on file on July 31, 2007 at the City of Los Angeles Departments of Planning and Transportation. The list of related projects in the project site area is presented in *Table 8-1*. The location of the related projects is shown in *Figure 8-1*. The list of related projects was submitted to LADOT staff for review and approval.

Traffic volumes expected to be generated by the related projects were calculated using rates provided in the ITE *Trip Generation* manual. The related projects' respective traffic generation for the AM and PM peak hours, as well as on a daily basis for a typical weekday, is summarized in *Table 8-2*. The anticipated distribution of the related projects traffic volumes to the study intersections during the AM and PM peak hours is displayed in *Figures 8-2 and 8-3*, respectively.

Table 8-1 LIST OF RELATED PROJECTS [1]

MAP NO.	FILE/PROJECT NUMBER	PROJECT NAME LOCATION	LAND USE	SIZE	STATUS
1	VEN 2004 -273	Chase Knolls Apartments 13401 Riverside Drive	Apartments Senior Apartments	102 DU 40 DU	Proposed
2	VEN 2004-5/ EAF 2002-6453	· · · · · · · · · · · · · · · · · · ·		88 DU 6,000 SF 7,000 SF 3,500 SF	Proposed
3	VEN 2003-2/ EAF 2003-1757	Riverside Drive Office Buildings 12828 Riverside Drive	Office	29,475 SF	Proposed
4	VEN 2003-15	Walgreens 13920 Ventura Boulevard	Drugstore	11,244 SF	Proposed
5	VEN 2003-194	Buckley School - 3900 Stansbury Avenue	School	80 Students	Proposed
6	VEN 2003-79	Best Buy 4500 Van Nuys Boulevard	Retail	60,000 SF	Proposed
7	VEN 2004-33	Sherman Oaks Square 4454 Van Nuys Boulevard	Apartments	98 DU	Proposed
8	VEN 2003-13	Gas Station Expansion 14478 Ventura Boulevard	Gas Station	392 SF	Proposed
9	VEN 2003 -19	Los Angeles Valley College College 5800 Fulton Avenue		2,300 Students	Proposed
10	VEN 2004-86	15222 Ventura Boulevard	Condominiums Specialty Retail	52 DU 7,460 SF	Proposed
11	VEN 2004 -26	N 2004 -26 Il Villaggio Toscano 4805 Sepulveda Boulevard Retail Existing Apartments Existing Residence Existing Office		500 DU 45,000 SF 10,000 SF (24 DU) (11 DU) (52,452 SF)	Proposed
12	EAF 2001-3806	5300 Coldwater Canyon Avenue	Self Storage Demolish Health Club	60,250 SF (14,624 SF)	Proposed
13	EAF 2004-0661	5829 Van Nuys Boulevard	New Car Sales	85,038 SF	Proposed
14	ENV 2005-5273-MND	14242 West Burbank Boulevard	Condominium	26 DU	Proposed
15	ENV 2005-6373-MND	4838 North Hazeltine Avenue	Condominium	23 DU	Propose
16	2006-44	Merdinian Evangelical School 13330 Riverside Drive	Private School	300 Students	Propose
17	2006-130	Sherman Village 12629 Riverside Drive	Condominium	247 DU	Propose

[1] Source: City of Los Angeles Departments of Planning and Transportation.





MAP SOURCE: THOMAS BROS. GUIDE

FIGURE 8-1
LOCATION OF RELATED PROJECTS

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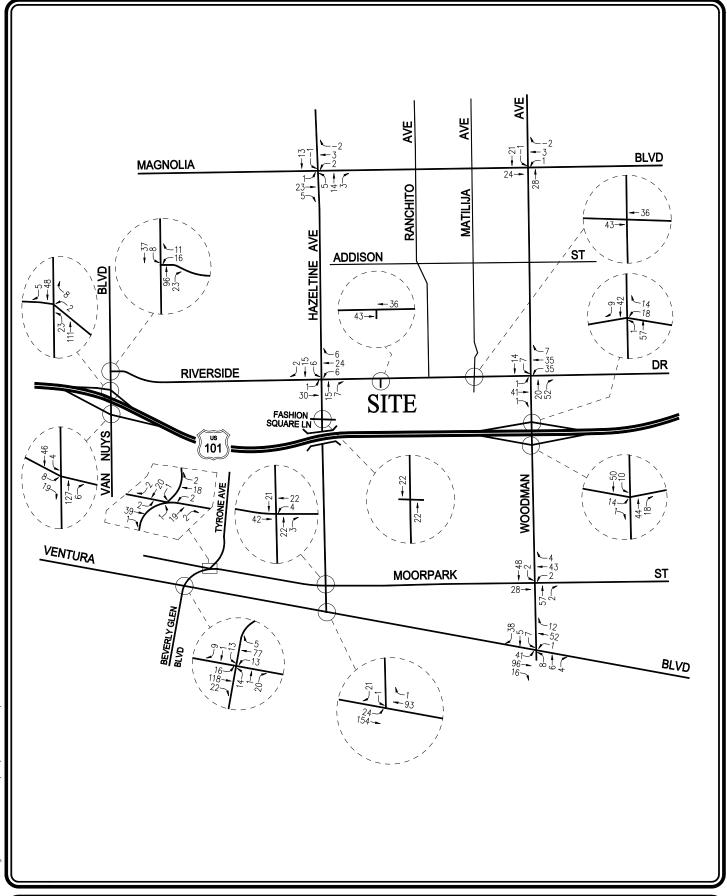
WESTFIELD FASHION SQUARE EXPANSION PROJECT

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Table 8-2
WEEKDAY RELATED PROJECTS TRIP GENERATION [1]

		DAILY		AM PEAK HOUR		PM PEAK HOUR		
LAND USE	SIZE	TRIP ENDS [2] VOLUMES	IN	OLUME	S [2] TOTAL	VOLUME IN OUT		CS [2] TOTAL
LAID USE	SIZIE	VOLUMES	114	001	TOTAL	щ	001	TOTAL
I Apartment [3]	142 DU	724	13	29	42	32	23	55
2 Camino Real Mixed-Use Development [4]	146,463 SF	1,310	58	73	131	64	49	113
3 Office [5]	29,475 GSF	325	40	6	46	7	37	44
4 Drugstore [6]	11,244 GSF	1,013	21	15	36	48	47	95
5 Private School [7]	80 Students	198	50	41	91	29	32	61
6 Retail [8]	60,000 GSF	2,576	5	2	7	30	42	72
7 Apartment [9]	98 DU	659	10	40	50	40	21	61
8 Gas Station [10]	392 SF	520	21	21	42	26	26	52
9 Community College [11]	2,300 Students	5,380	441	97	538	212	120	332
10 Mixed-Use [12]	10,551 SF	470	9	23	32	27	20	47
11 Il Villaggio Toscano [13]		5,500	96	225	321	323	227	550
12 Self- Storage [14]	60,250 GSF	(729)	(31)	(43)	(74)	(37)	(35)	(72)
13 New Car Sales [15]	85,038 GSF	1,787	78	27	105	52	81	133
14 Condominium [16]	26 DU	152	2	9	11	9	5	1,4
15 Condominium [16]	23 DU	135	2	8	10	8	4	12
16 Private School [7]	300 Students	744	165	78	243	(38)	(34)	(72)
17 Condominium [16]	247 DU	1,447	18	88	106	84	42	126
TOTAL		22,211	998	739	1,737	916	707	1,623

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] Source: "Traffic Impact Analysis, Chase Knolls Project," prepared by Linscott, Law & Greenspan, Engineers, 2005.
- [4] LADOT trip generation forecast. The AM peak hour traffic volumes represent ten percent of the daily trip generation forecast.
- [5] ITE Land Use Code 710 (General Office Building) trip generation average rates.
- [6] ITE Land Use Code 881 (Pharmacy/Drugstore without Drive-Through Window) trip generation average rates.
- [7] LADOT trip generation forecast. Daily trip generation rate for Private School (K-12) obtained from ITE "Trip Generation," 7th Edition, 2003.
- [8] LADOT trip generation forecast. Daily trip generation rate for shopping center obtained from ITE "Trip Generation," 7th Edition, 2003.
- [9] ITE Land Use Code 220 (Apartment) trip generation average rates.
- [10] LADOT trip generation forecast. The PM peak hour traffic volumes represent ten percent of the daily trip generation forecast.
- [11] LADOT trip generation forecast. Daily trip generation rate and directional distribution for Junior/Community College obtained from ITE "Trip Generation," 7th Edition, 2003.
- [12] LADOT trip generation forecast. The PM peak hour traffic volumes represent ten percent of the daily trip generation forecast.
- [13] LADOT trip generation forecast. The PM peak hour traffic volumes represent ten percent of the daily trip generation forecast.
- [14] LADOT trip generation forecast. Directional distribution for Health Club obtained from ITE "Trip Generation," 7th Edition, 2003. Existing Health Club at 14,624 square feet will be demolished as part of the project.
- [15] LADOT trip generation forecast. Directional distribution for New Car Sales obtained from ITE "Trip Generation," 7th Edition, 2003.
- [16] ITE Land Use Code 230 (Residential Condominium/Townhome) trip generation average rates.



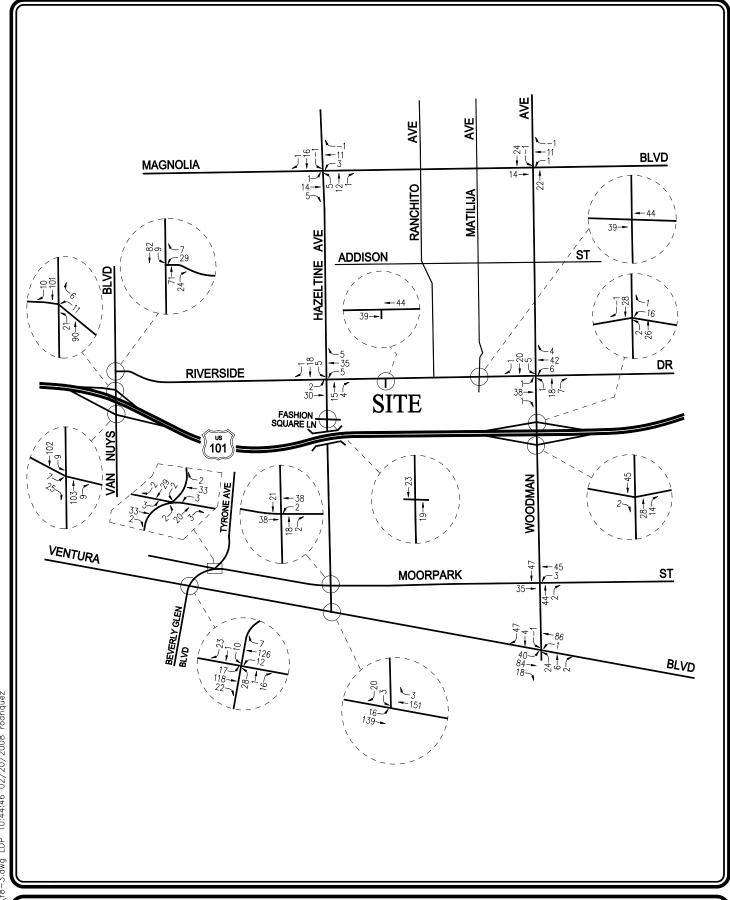
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FIGURE 8-2 RELATED PROJECTS TRAFFIC VOLUMES AM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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FIGURE 8-3 RELATED PROJECTS TRAFFIC VOLUMES

PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

8.2 Ambient Growth Factor

In order to account for unknown related projects not included in this analysis, the existing traffic volumes were increased at an annual rate of two percent (2.0%) from year 2007 to the year 2012 (i.e., the anticipated year of project build-out). Application of this ambient growth factor allows for a conservative forecast of future traffic volumes in the project study area. The ambient growth factor was based on general traffic growth factors provided in the 2004 Congestion Management Program for Los Angeles County (the "CMP manual") and determined in consultation with LADOT staff. The CMP's traffic growth rate is intended to anticipate future traffic generated by development projects in the project vicinity. It is also noted that based on review of empirical data and the general traffic growth factors provided in the CMP manual for the San Fernando Valley area, it is anticipated that the existing traffic volumes are actually expected to increase at an annual rate of less than 1.0% per year between the years 2005 and 2012. Thus, the inclusion in this traffic analysis of both a forecast of traffic generated by known related projects plus the use of an ambient growth traffic factor based on CMP traffic model data likely overstates future pre-project conditions and future traffic volumes at the study intersections.

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9.0 TRAFFIC IMPACT ANALYSIS METHODOLOGY

The 18 study intersections were evaluated using the Critical Movement Analysis (CMA) method of analysis which determines Volume-to-Capacity (v/c) ratios on a critical lane basis. The overall intersection v/c ratio is subsequently assigned a Level of Service (LOS) value to describe intersection operations. Level of Service varies from LOS A (free flow) to LOS F (jammed condition). A description of the CMA method and corresponding Level of Service is provided in *Appendix D*.

9.1 Impact Criteria and Thresholds

The relative impact of the added project traffic volumes to be generated by the proposed project during the AM and PM peak hours was evaluated based on analysis of future operating conditions at the 18 study intersections, without and with the proposed project. The previously discussed capacity analysis procedures were utilized to evaluate the future v/c relationships and service level characteristics at each study intersection.

The significance of the potential impacts of project generated traffic at each study intersection was identified using criteria set forth in the LADOT's *Traffic Study Policies and Procedures*. According to the City's Sliding Scale Method for calculating the level of impact due to traffic generated by the proposed project, a significant transportation impact is determined based on the sliding scale criteria presented in *Table 9-1*.

INTERSE	Table 9-1 CITY OF LOS ANGELES INTERSECTION IMPACT THRESHOLD CRITERIA							
Final v/c	Final v/c Level of Service Project Related Increase in v/c							
> 0.700 - 0.800	С	equal to or greater than 0.040						
> 0.800 - 0.900	D	equal to or greater than 0.020						
>0.900	>0.900 E or F equal to or greater than 0.010							

The City's Sliding Scale Method requires mitigation of project traffic impacts whenever traffic generated by the proposed development causes an increase of the analyzed intersection v/c ratio by an amount equal to or greater than the values shown above.

As previously mentioned, an annual two percent (2.0%) ambient growth rate was assumed so as to account for unknown related projects in the vicinity of the proposed project. Additionally, it was assumed that the proposed project will be completed and occupied in the year 2012.

9.2 LADOT ATSAC/ATCS

The City of Los Angeles has announced it will receive \$150 million in State of California transportation bond funds for upgrading traffic signals in the City. In November 2006, California voters approved Proposition 1B, which committed \$20 billion to statewide and regional transportation projects. Designed to enhance mobility, expand public transit, reduce air pollution, improve port security and repair local roads, this bond measure included \$250 million for traffic signal improvements across the state. The City has stated it will use its share of the funds to synchronize every traffic signal in Los Angeles.

Subsequent to the City's announcement, LADOT has stated effective November 20, 2007, Automated Traffic Surveillance and Control (ATSAC)/Adaptive Traffic Control System (ATCS) is no longer available as a mitigation option due to the full funding of the ATSAC/ATCS program for the entire City. Additionally, all future traffic studies should assume the ATSAC/ATCS credit in the future baseline analysis conditions (e.g., future pre-project, future with project, etc.).

ATSAC provides computer control of traffic signals allowing automatic adjustment of signal timing plans to reflect changing traffic conditions, identification of unusual traffic conditions caused by accidents, the ability to centrally implement special purpose short-term traffic timing changes in response to incidents, and the ability to quickly identify signal equipment malfunctions. ATCS provides real time control of traffic signals and includes additional loop detectors, closed-circuit television, an upgrade in the communications links, and a new generation of traffic control software. LADOT estimates that the ATSAC system reduces critical ν/c ratios by seven percent (0.07). The ATCS upgrade further reduces the critical ν/c ratios by three percent (0.03). Therefore, a reduction of 0.10 was assumed in the calculation of the ν/c ratios for the signalized study intersections in the existing and future baseline analysis conditions.

It should be noted that prior to the City's announcement of full funding of the ATSAC/ATCS program for the entire City and implementation of the City's policy, the project applicant had provided funding for the installation of LADOT's Adaptive Traffic Control System (ATCS) at the following seven signalized study intersections:

Int. No. 1: Van Nuys Boulevard/Riverside Drive

Int. No. 4: Tyrone Avenue/Moorpark Street

Int. No. 7: Hazeltine Avenue/Riverside Drive

Int. No. 8: Hazeltine Avenue/Fashion Square Lane

Int. No. 12: Woodman Avenue/Riverside Drive

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

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Int. No. 15: Woodman Avenue/Moorpark Street

As such, a reduction of 0.07 was assumed in the calculation of the v/c ratios for these seven signalized study intersections in the existing and future baseline analysis conditions based on the current ATSAC operation (not ATCS). Further, the project may utilize ATCS for purposes of mitigating potential traffic impacts associated with the project.

9.3 Traffic Impact Analysis Scenarios

Traffic impacts at the study intersections were analyzed for the following conditions:

- (a) Existing conditions.
- (b) Condition (a) plus two percent (2.0%) ambient traffic growth through year 2012.
- (c) Condition (b) with completion and occupancy of the related projects.
- (d) Condition (c) with completion and occupancy of the proposed project.
- (e) Condition (d) with implementation of project mitigation measures where necessary.

The traffic volumes for each new condition were added to the volumes in the prior condition to determine the change in capacity utilization at the 18 study intersections.

Summaries of the v/c ratios and LOS values for the study intersections during the AM and PM peak hours are shown in **Table 9-2**. The CMA data worksheets for the analyzed intersections are contained in *Appendix D*.

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SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE AM AND PM PEAK HOURS

[3] [4] [5]	VEAR 2012 YEAR 2012 (TED W/ PROPOSED CHANGE SIGNIF. W/ PROJEC CTS PROJECT V/C IMPACT MITIGATIC LOS v/c LOS [(4)–(3)] [c] v/c Lo	0.802 D 0.808 D 0.006 NO 0.778 C -0.024 0.893 D 0.920 E 0.027 YES 0.890 D -0.003 YES	0.721 C 0.722 C 0.001 NO 0.722 C 0.001 0.881 D 0.885 D 0.004 NO 0.885 D 0.004	0.877 D 0.878 D 0.001 NO 0.878 D 0.001	0.622 B 0.622 B 0.000 NO 0.592 A -0.030 0.983 E 0.994 E 0.011 YES 0.964 E -0.019 YES	0.717 C 0.718 C 0.001 NO 0.718 C 0.001 0.863 D 0.873 D 0.010 NO 0.873 D 0.010	0.766 C 0.770 C 0.004 NO 0.770 C 0.004 0.884 D 0.900 D 0.016 NO 0.900 D 0.016	0.882 D 0.890 D 0.008 NO 0.860 D -0.022 — 0.819 D 0.000 YES	0.412 A 0.414 A 0.002 NO 0.384 A -0.028 0.580 A 0.630 B 0.050 NO 0.600 A 0.020	0.779 C 0.780 C 0.001 NO 0.780 C 0.001 0.824 D 0.829 D 0.005 NO 0.829 D 0.005	0.907 E 0.908 E 0.001 NO 0.908 E 0.001
[2]	YEAR 2012 W/AMBIENT GROWTH v/c LOS	0.762 C 0.854 D	0.698 B 0.843 D	0.850 D	0.600 A 0.955 E	0.651 B (0.748 C (0.863 D (0.404 A (0.757 C 0.790 C	0.853 D C
	YEA EXI	0.687 B	0.655 B 0.787 C	0.793 C 0.955 E	0.539 A 0.862 D	0.613 B 0.738 C	0.701 C 0.814 D	0.778 C 0.718 C	0.361 A 0.515 A	0.709 C 0.739 C	0.797 C
	PEAK HOUR	AM PM	AM PM	AM PM	AM PM	AM PM	AM PM	AM PM	AM PM	AM PM	AM PM
	INTERSECTION	Van Nuys Boulevard/ Riverside Drive	Van Nuys Boulevard/ US-101 Freeway Westbound Ramps	Van Nuys Boulevard/ US-101 Freeway Eastbound Ramps	Tyrone Avenue/ Moorpark Street	Tyrone Avenue-Beverly Glen Boulevard/ Ventura Boulevard	Hazeltine Avenuc/ Magnolia Boulevard	Hazeltine Avenue/ Riverside Drive	Hazeltine Avenuc/ Fashion Square Lane	Hazeltine Avenue/ Moorpark Street	Hazeltine Avenue/ Ventura Roulevard
	NO.		2	m	4	٧	9	7	∞	6	01

Table 9-2 (Continued) SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE AM AND PM PEAK HOURS

			Ξ		12.1		[3]				[4]			[5]		
			VEAR 2007	20,7	YEAR 2012 W/AMBIENT	212 FN	YEAR 2012 W/RELATED	1	YEAR 2012 W/ PROPOSED		CHANGE SIGNIF	STCNTF	YEAR 2012 W/PROJECT		CHANGE	MITT.
Ş	NOLLOGOGALNI	PEAK	EXISTING	NG I OS	GROWTH	E	PROJECTS		PROJECT		V/C	IMPACT	MITIGATION		V/C	GATED
=	Woodman Avenue/	AM	į.	Q	1	ш	1	ш		ш	0.002	ON	0.929	ш	0.002	
	Magnolia Boulevard	PM	0.780	U	0.835		0.847	Ω	0.849	Δ	0.002	O _N	0.849	۵	0.002	3
12	Woodman Avenue/	AM	0.959	lī	1.061	12.	1.107	Į.	1.117	12.	0.010	YES	1.016	Įz.	-0.091	YES
	Riverside Drive	PM	0.880	Ω	0.975	ш	1.003	ŀτ'	1.038	11.	0.035	YES	0.986	'n	-0.017	YES
13	Woodman Avenue/	ΑM	0.743	ပ	0.824	Д	0.841	D	0.847	Д	0.006	OX	0.817	Ω	-0.024	I
	US-101 Freeway Westbound Ramps	PM	0.733	ပ	0.813	Ω	0.819	Ω	0.853	Ω	0.034	YES	0.823	О	0.004	YES
14	Woodman Avenue/	AM	0.654	ш	969.0	Д	0.720	U	0.725	U	0.005	ON ON	0.725	Ú	0.005	ı
	US-101 Freeway Eastbound Ramps	PM	0.648	В	0.690	В	0.700	മ	0.731	ပ	0.031	NO	0.731	U	0.031	1
15	Woodman Avenue/	ΑM	0.850	۵	0.942	ш	0.991	ш	0.993	ш	0.002	O Z	0.963	ш	-0.028	1
	Moorpark Street	PM	0.867	О	096'0	ш	1.005	13_	1.017	ıι	0.012	YES	0.987	m	-0.018	YES
91	Woodman Avenue/	AM	0.717	υ	0,766	υ	0.826	D	0.829	Ω	0.003	ON	0.829	Q	0.003	i
	Ventura Boulevard	PM	0.640	ш	0.681	en en	0.741	U	0.754	U	0.013	ON ON	0.754	U	0.013	
11	Project Driveway-Matilija Avenue	AM	0.518	∢ •	0.570	∢ (0.585	< 4	0.412	< ⋅	-0.173	0 2	0.412	∢ ·	-0.173	1
	Kiverside Drive [a]	Σ.	0.555	<	0.610	n a	0.028	n	0.565	<	-0.063	O Z	0.565	∢	-0.063	i
8	New Project Driveway (Tunnel Access)	AM 8	0.000	< <	0.000	< <	0.000	< <	0.378	۲ ک	0.378	0 0	0.378	۱ ≻	0.378	ı
	Kiveiside Diive [b]	Σ	0.000		0.000		0.000		0.049	۵	0.049	2	0.049	n	0,049	

Intersection proposed to be signalized as part of the proposed project. v/c ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

ATSAC/ATCS as part of the Victory System No. 6.

According to LADOT's "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection 回回

shall be deemed significant in accordance with the following table:

<u></u>

equal to or greater than 0.010	> 0.900	E/F
equal to or greater than 0,020	> 0.800 - 0.900	Q
equal to or greater than 0.040	> 0,700 - 0.800	U
Project-Related Increase in V/C	Final V/C	Level of Service
	1	

10.0 TRAFFIC ANALYSIS

10.1 Existing Conditions

As indicated in column [1] of *Table 9-2*, 16 of the 18 study intersections are presently operating at LOS D or better during the AM and PM peak hours under existing conditions. The following two study intersections are currently operating at LOS E during the peak hours shown below:

Int. No. 3: Van Nuys Boulevard/U.S. 101 EB Ramps PM Peak Hour: v/c=0.955, LOS E

Int. No. 12: Woodman Avenue/Riverside Drive AM Peak Hour: v/c=0.959, LOS E

As previously mentioned, the existing traffic volumes at the study intersections during the AM and PM peak hours are displayed in *Figures 6-1 and 6-2*, respectively.

10.2 Existing With Ambient Growth Conditions

Growth in traffic due to the combined effects of continuing development, intensification of existing developments and other factors was assumed to be two percent (2.0%) per year through year 2012. This ambient growth incrementally increases the v/c ratios at all of the study intersections. As shown in column [2] of Table 9-2, 13 of the 18 study intersections are expected to continue to operate at LOS D or better during the AM and PM peak hours with the addition of ambient growth traffic through the year 2012. The following five study intersections are expected to operate at LOS E or F during the peak hours shown below with the addition of ambient growth traffic:

Int. No. 3: Van Nuys Boulevard/U.S. 101 EB Ramps	PM Peak Hour: $v/c=1.027$, LOS F
Int. No. 4: Tyrone Avenue/Moorpark Street	PM Peak Hour: <i>v/c</i> =0.955, LOS E
Int. No. 11: Woodman Avenue/Magnolia Boulevard	AM Peak Hour: v/c =0.919, LOS E
Int. No. 12: Woodman Avenue/Riverside Drive	AM Peak Hour: v/c =1.061, LOS F PM Peak Hour: v/c =0.975, LOS E
Int. No. 15: Woodman Avenue/Moorpark Street	AM Peak Hour: v/c =0.942, LOS E PM Peak Hour: v/c =0.960, LOS E

The existing with ambient growth traffic volumes at the study intersections during the AM and PM peak hours are shown in *Figures 10-1 and 10-2*, respectively.

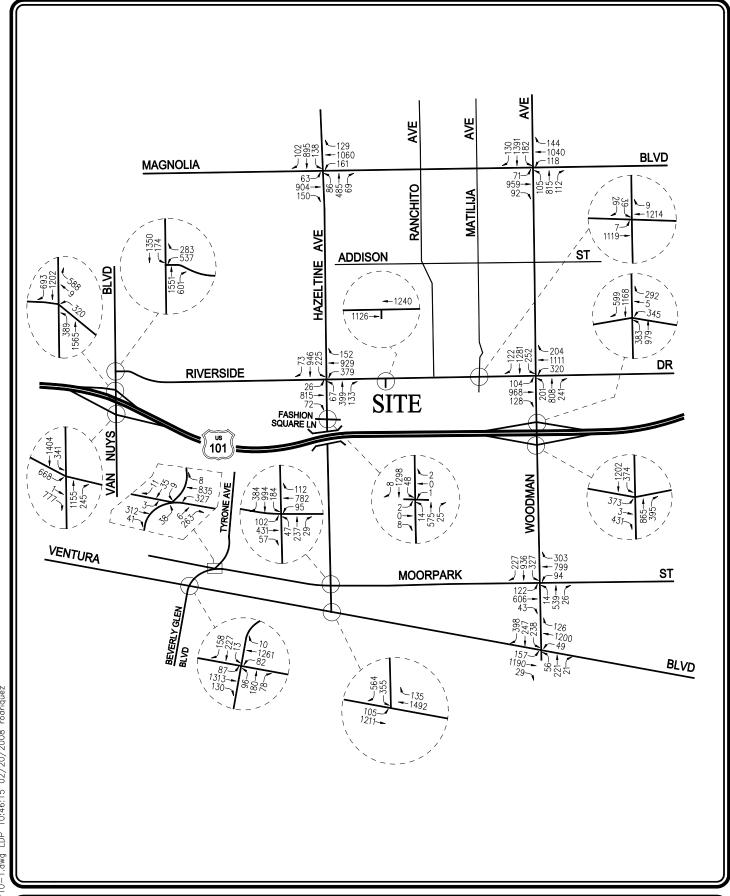




FIGURE 10-1 EXISTING WITH AMBIENT GROWTH TRAFFIC VOLUMES AM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

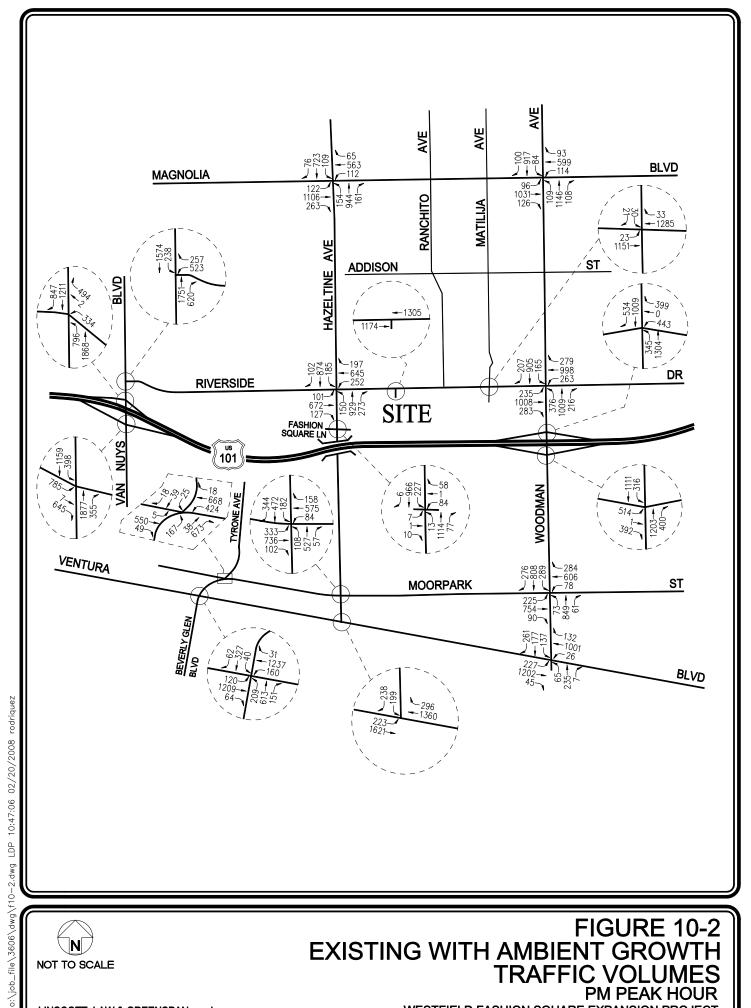




FIGURE 10-2 EXISTING WITH AMBIENT GROWTH PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

10.3 Future Pre-Project Conditions

The v/c ratios at all 18 study intersections are incrementally increased with the addition of traffic generated by the related projects listed in *Table 8-1*. As presented in column [3] of *Table 9-2*, 12 of the 18 study intersections are expected to continue operating at LOS D or better during the AM and PM peak hours with the addition of growth in ambient traffic and the traffic due to the related projects. The following six study intersections are expected to operate at LOS E or F during the peak hours shown below with the addition of ambient traffic and the traffic due to the related projects:

Int. No. 3: Van Nuys Boulevard/U.S. 101 EB Ramps	PM Peak Hour: v/c =1.063, LOS F
Int. No. 4: Tyrone Avenue/Moorpark Street	PM Peak Hour: v/c =0.983, LOS E
Int. No. 10: Hazeltine Avenue/Ventura Boulevard	AM Peak Hour: v/c=0.907, LOS E
Int. No. 11: Woodman Avenue/Magnolia Boulevard	AM Peak Hour: v/c =0.927, LOS E
Int. No. 12: Woodman Avenue/Riverside Drive	AM Peak Hour: v/c =1.107, LOS F PM Peak Hour: v/c =1.003, LOS F
Int. No. 15: Woodman Avenue/Moorpark Street	AM Peak Hour: v/c =0.991, LOS E PM Peak Hour: v/c =1.005, LOS F

The future pre-project (existing, ambient growth and related projects) traffic volumes at the study intersections during the AM and PM peak hours are presented in *Figures 10-3 and 10-4*, respectively.

10.4 Future With Project Conditions

As shown in column [4] of *Table 9-2*, application of the City's threshold criteria to the "With Proposed Project" scenario indicates that the proposed project is expected to create significant impacts at six of the 18 study intersections. The proposed project is expected to create significant impacts at the following locations according to the City's impact criteria during the peak hour shown below with the addition of ambient growth, related projects traffic, and project-related traffic:

Int. No. 1: Van Nuys Boulevard/Riverside Drive

PM peak hour v/c ratio increase of 0.027 [to 0.920 (LOS E) from 0.893 (LOS D)]

Int. No. 4: Tyrone Avenue/Moorpark Street

PM peak hour v/c ratio increase of 0.011 [to 0.994 (LOS E) from 0.983 (LOS E)]

Int. No. 7: Hazeltine Avenue/Riverside Drive

PM peak hour v/c ratio increase of 0.030 [to 0.849 (LOS D) from 0.819 (LOS D)]

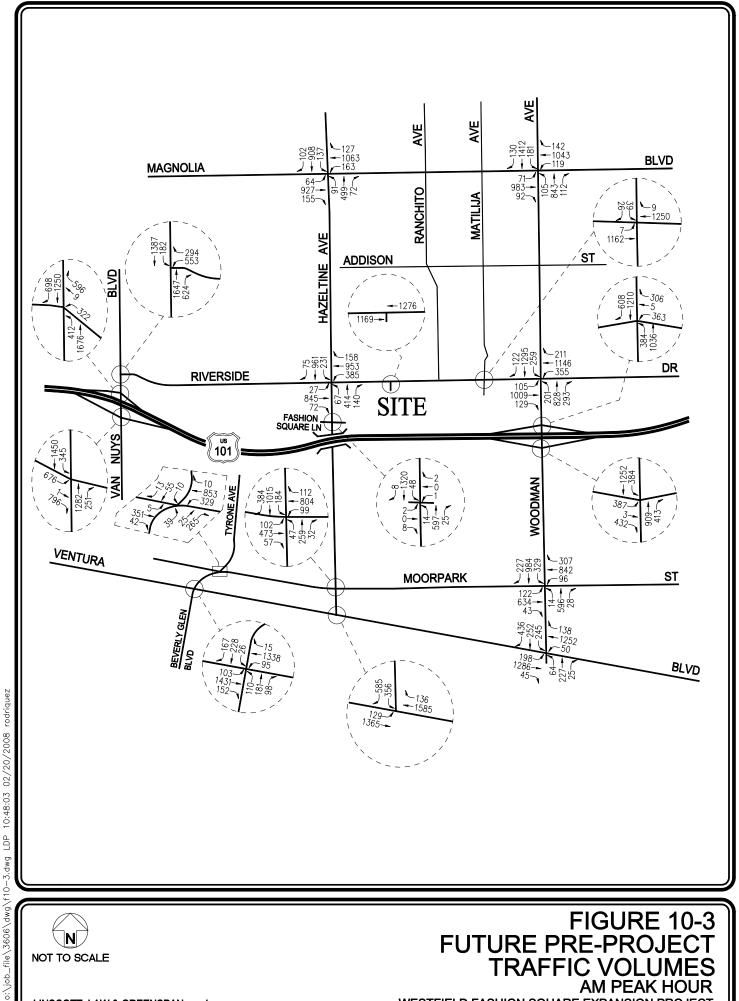




FIGURE 10-3 FUTURE PRE-PROJECT TRAFFIC VOLUMES AM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

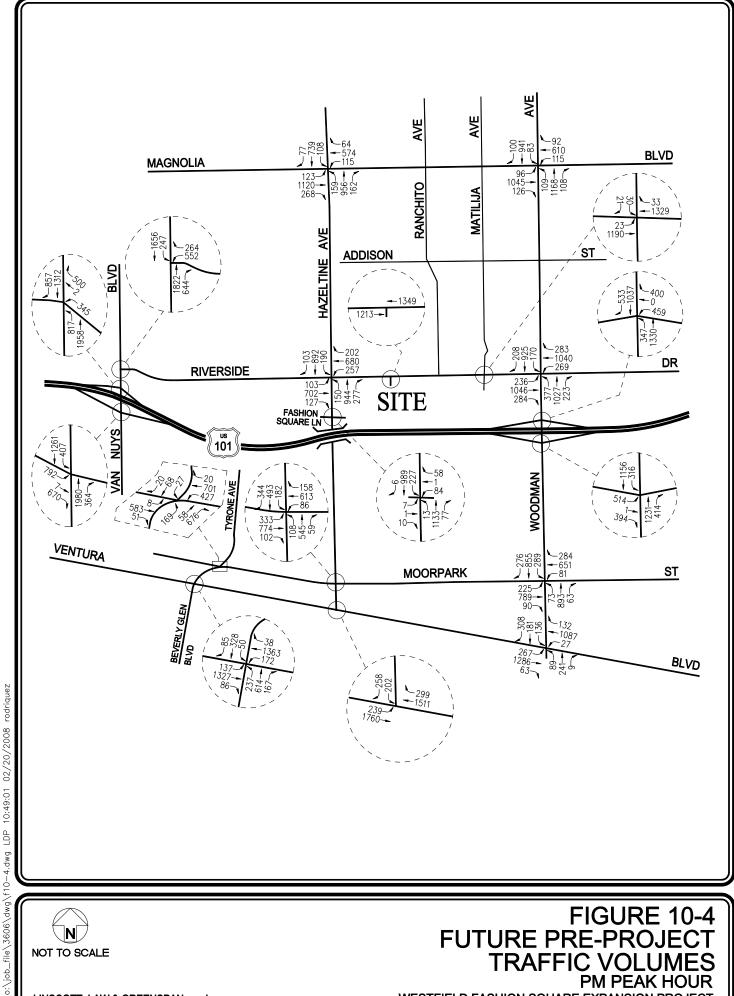




FIGURE 10-4 FUTURE PRE-PROJECT TRAFFIC VOLUMES PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

Int. No. 12: Woodman Avenue/Riverside Drive

AM peak hour v/c ratio increase of 0.010 [to 1.117 (LOS F) from 1.107 (LOS F)] PM peak hour v/c ratio increase of 0.035 [to 1.038 (LOS F) from 1.003 (LOS F)]

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

PM peak hour v/c ratio increase of 0.034 [to 0.853 (LOS D) from 0.819 (LOS D)]

Int. No. 15: Woodman Avenue/Moorpark Street

PM peak hour v/c ratio increase of 0.012 [to 1.017 (LOS F) from 1.005 (LOS F)]

Incremental but not significant impacts are noted at the remaining 12 study intersections as presented in *Table 9-2*. The future with project (existing, ambient growth, related projects and project) traffic volumes at the study intersections during the AM and PM peak hours are illustrated in *Figures 10-5 and 10-6*, respectively.

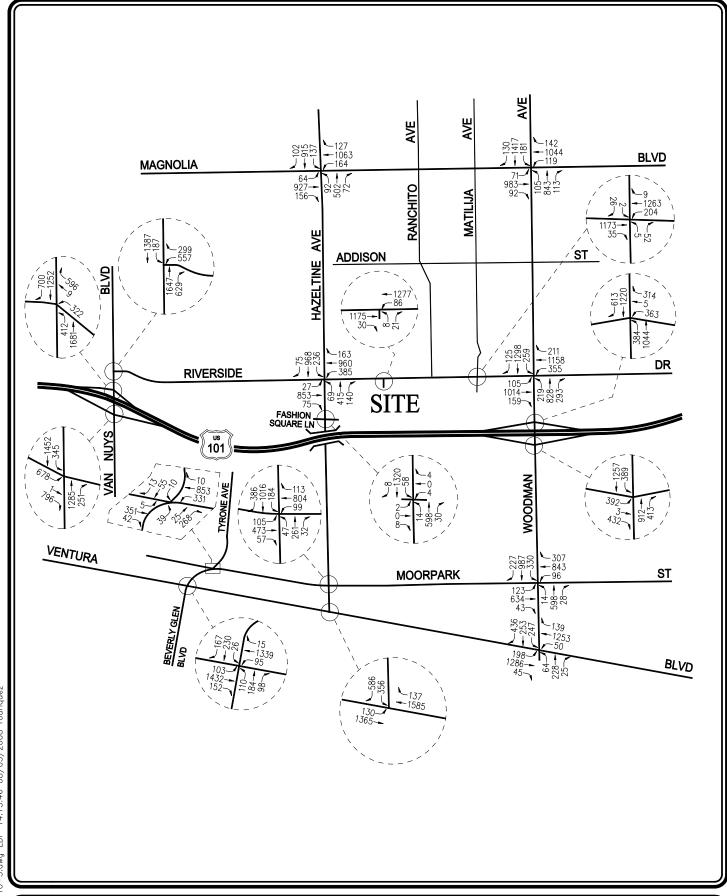




FIGURE 10-5
FUTURE WITH PROJECT
TRAFFIC VOLUMES
AM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

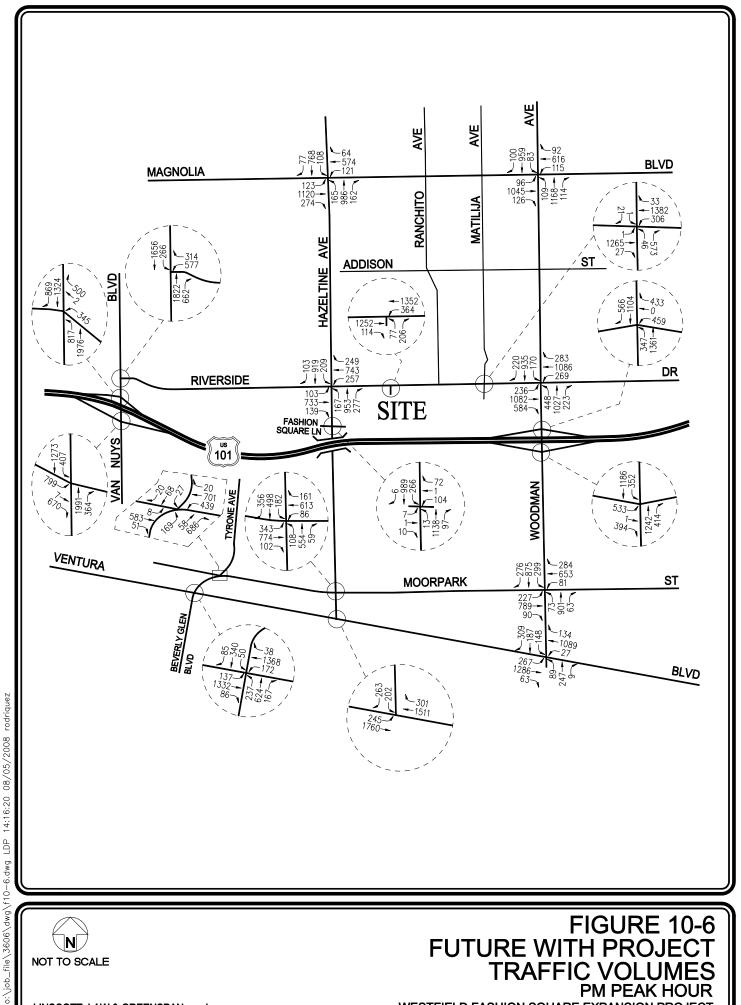




FIGURE 10-6 FUTURE WITH PROJECT TRAFFIC VOLUMES PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

11.0 TRANSPORTATION MITIGATION MEASURES

The following sections provide an overview of transportation improvement measures that are anticipated to address project impacts to the local roadway network associated with the proposed Fashion Square project to less than significant levels. In addition, it is recommended that traffic signals be installed at the Matilija Drive-New Project Driveway/Riverside Drive intersection to improve safety and operations at the intersection.

11.1 Summary of Project Mitigation

As summarized in the Future With Project Conditions section (see Subsection 10.4) of this study, application of the City's threshold criteria to the "With Proposed Project" scenario indicates that the proposed project is anticipated to create significant impacts at the following eight study intersections:

Int. No. 1: Van Nuys Boulevard/Riverside Drive

Int. No. 4: Tyrone Avenue/Moorpark Street

Int. No. 7: Hazeltine Avenue/Riverside Drive

Int. No. 12: Woodman Avenue/Riverside Drive

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

Int. No. 15: Woodman Avenue/Moorpark Street

The following paragraphs summarize the recommended transportation mitigation measures for the study intersections.

Int. No. 1: Van Nuys Boulevard/Riverside Drive

The recommended mitigation consists of funding provided by the project for the installation of LADOT's Adaptive Traffic Control System (ATCS) at this intersection. This intersection is already included as part of the Victory ATSAC system. ATSAC provides computer control of traffic signals allowing automatic adjustment of signal timing plans to reflect changing traffic conditions, identification of unusual traffic conditions caused by accidents, the ability to centrally implement special purpose short-term traffic timing changes in response to incidents, and the ability to quickly identify signal equipment malfunctions. ATCS provides real time control of traffic signals and the funding provided by the project includes additional loop detectors, closed-circuit television, an upgrade in the communications links, and a new generation of traffic control software. LADOT estimates that the ATSAC system reduces critical ν/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical ν/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the ν/c ratios for this intersection.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 0.890 (LOS D) from 0.920 (LOS E) during the PM peak hour. Thus, the significant impact at this intersection during the PM peak hour would be reduced to less than significant levels.

Int. No. 4: Tyrone Avenue/Moorpark Street

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 0.964 (LOS E) from 0.994 (LOS E) during the PM peak hour. Thus, the significant impact at this intersection during the PM peak hours would be reduced to less than significant levels.

Int. No. 7: Hazeltine Avenue/Riverside Drive

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 0.819 (LOS D) from 0.849 (LOS D) during the PM peak hour. Thus, the significant impact at this intersection during the PM peak hour would be reduced to less than significant levels.

Int. No. 12: Woodman Avenue/Riverside Drive

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection. In addition, redesignate the curb lane on the southbound approach on Woodman Avenue to an optional through/right-turn lane. The resultant lane configurations at the southbound approach will be one left-turn lane, two through lanes and one optional through/right-turn lane. If required by LADOT, the existing four-foot wide median island on the south leg of the intersection could be replaced by striping and/or lane delineators (e.g., two feet wide or less) so that additional width could be provided to the existing three southbound Woodman Avenue through lanes on the departure side of the intersection. The schematic plan of the proposed improvement at the Woodman Avenue/Riverside Drive intersection is contained in *Appendix A*.

In addition, LADOT recommends that left-turns from northbound Woodman Avenue to La Maida Street be restricted during the weekday PM peak period. After the mitigation is implemented, LADOT will review the Woodman Avenue/La Maida Street intersection and determine if additional turn restriction measures are required.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 1.016 (LOS F) from 1.117 (LOS F) during the AM peak hour and to 0.986 (LOS E) from 1.038 (LOS F) during the PM peak hour. The improvement in the calculated v/c ratio is due to both the installation of ATCS, as well as the redesignation of the southbound Woodman Avenue right-turn lane to an optional through/right-turn lane. Thus, the significant impact at this intersection during the AM and PM peak hours would be reduced to less than significant levels.

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 0.823 (LOS D) from 0.853 (LOS D) during the PM peak hour. Thus, the significant impact at this intersection during the PM peak hour would be reduced to less than significant levels.

Int. No. 15: Woodman Avenue/Moorpark Street

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection.

As shown in *Table 9-2*, the proposed mitigation is expected to improve the v/c ratio to 0.987 (LOS E) from 1.017 (LOS F) during the PM peak hour. Thus, the significant impact at this intersection during the PM peak hour would be reduced to less than significant levels.

11.2 Traffic Signal Warrant Analysis

While not specifically required for traffic mitigation purposes, it is recommended that, as part of the proposed project, traffic signals be installed at the new Fashion Square driveways on Riverside Drive. Based on discussions with LADOT staff, traffic signal warrant analyses have been prepared for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection and the New Fashion Square Westerly Driveway/Riverside Drive intersection (Tunnel Access), associated with the proposed project. The traffic signal warrants were prepared for the Existing With Project Conditions.

The determination of whether the installation of a traffic signal is warranted was based on criteria set forth in Section 353, Traffic Signal Warrants, of the Manual of Policies and Procedures (MPP)⁹. This is also consistent with criteria set forth in Chapter 4C of the Manual on Uniform Traffic Control Devices (MUTCD)¹⁰. Traffic signal warrants were prepared for the Matilija Avenue-New Fashion Square Driveway/Riverside Drive intersection. Specifically, Warrant No. 1 (Eight Hour Vehicular Volume), Warrant No. 2 (Four Hour Vehicular Volume), Warrant No. 3 (Peak Hour Volume) and Warrant No. 6 (Coordinated Signal System) traffic signal warrants were prepared. The traffic signal warrant analysis was prepared based on existing with project traffic volume data. The traffic signal warrant worksheets are provided in *Appendix E*.

In reviewing the traffic signal warrant analysis, it is important to note the following:

- In the signal warrant analysis for both intersections, Riverside Drive was assumed to be the major street which provides two or more approach lanes to the intersection.
- The eastbound approach on Riverside Drive at both project driveways are assumed to be two through lanes and one right-turn only lane.
- The westbound approach on Riverside Drive at the proposed westerly project driveway is assumed to be one left-turn lane and two through lanes. The westbound approach on Riverside Drive at the proposed easterly project driveway is assumed to be two left-turn lanes, one through lane and one combination through/right-turn lane.
- The New Fashion Square Easterly Driveway was assumed to be the minor street which provides three approach lanes to the intersection (i.e., one left-turn lane and two right-turn lanes).
- The New Fashion Square Westerly Driveway was assumed to be the minor street which provides two approach lanes to the intersection (i.e., one left-turn lane and one right-turn lane).
- Based on the California MUTCD document, for intersections with a high left-turn volume from the major street, the stated numerical values used to determine whether traffic signal warrants are met are based on the major street volume, which is the sum of both approaches on the major street (e.g., eastbound and westbound Riverside Drive approaches) minus the higher of the major street left turn volume (e.g., westbound Riverside Drive left-turn volume), and the minor street volume, which is the higher-volume of the major street left-turn volume (e.g., westbound Riverside Drive left-turn volume) plus the higher volume minor street approach to the intersection (e.g., the New Fashion Square Driveways).
- In addition, based on the LADOT MPP, for signal warrant analysis purposes, right-turn traffic volume that is delayed less than 45 seconds under Stop control should be subtracted from the minor street approach volumes in Warrant Nos. 1, 2, and 3. The Highway

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⁹ Manual of Policies and Procedures, Section 353, Traffic Signal Warrants, LADOT, October 2005.

¹⁰ Manual on Uniform Traffic Control Devices (MUTCD), 2003 California Supplement, May 20, 2004.

Capacity Manual (HCM) methodology for stop-controlled intersections was utilized to determine the delay of the minor street approaches to the intersection (e.g., the New Fashion Square Driveways) for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersections. It has been determined that four out of the eight hours analyzed for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection, the northbound right-turn movement had a delay of 45 seconds or more. As such, the traffic signal warrants were prepared both with and without the consideration of the northbound right-turn volumes on the New Fashion Square Easterly Driveway (i.e., minor street approach) for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection.

The following paragraphs provide detailed discussions of the traffic signal warrants prepared for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections.

Warrant 1: Eight-Hour Vehicular Volume

The Eight Hour Vehicular Volume warrant consists of three conditions: Condition A - The Minimum Vehicular Volume, Condition B - The Interruption of Continuous Traffic, and the Combination of Conditions A and B.

The Minimum Vehicular Volume warrant (Condition A) is intended for application where a large volume of intersecting traffic is the principal reason for consideration of a signal installation. The warrant is satisfied when for each of any 8 hours of an average day the traffic volumes provided in the LADOT table for Warrant 1 under Condition A exist on the major street and on the higher-volume minor street approach to the intersection.

The Interruption of Continuous Traffic warrant applies to operating conditions where Condition A is not satisfied and where the traffic volume on a major street is so heavy that traffic on a minor intersecting street suffers excessive delay or hazard in entering or crossing the major street. The warrant is satisfied when, for each of any 8 hours of an average day, the traffic volumes given in the table exist on the major street and on the higher-volume minor street approach to the intersection, and the signal installation will not seriously disrupt progressive traffic flow.

The Combination of Conditions A and B warrant applies at locations where Conditions A and B are not satisfied but where Conditions A and B are satisfied to the extent of 80 percent or more of the stated numerical values.

As shown in the worksheets provided in *Appendix E*, with or without consideration of the right-turn volume on the minor street, Conditions A and B associated with Warrant No. 1-Eight Hour Vehicular are met for the existing with project conditions for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection. Therefore, Warrant No. 1 is satisfied for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection.

As shown in the worksheets provided in *Appendix E*, without consideration of the right-turn volume on the minor street, Conditions A and B associated with Warrant No. 1-Eight Hour Vehicular are met for the existing with project conditions for the New Fashion Square Westerly Driveway/Riverside Drive intersection. Therefore, Warrant No. 1 is satisfied for the New Fashion Square Westerly Driveway/Riverside Drive intersection.

Warrant 2: Four-Hour Vehicular Volume Warrant

The Four Hour Vehicular Volume Warrant is satisfied, when for each of any four hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher volume minor street approach (one direction only) all fall above the curve in Figure A for the combination of approach lanes.

As indicated in Figure A provided in *Appendix E*, with or without consideration of the right-turn volume on the minor street, all of the plotted points for the four highest hours of the day during existing with project conditions fall well above the applicable curve for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection. Thus, Warrant No. 2 is satisfied for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection.

As indicated in Figure A provided in *Appendix E*, without consideration of the right-turn volume on the minor street, all of the plotted points for the four highest hours of the day during existing with project conditions fall well above the applicable curve for the New Fashion Square Westerly Driveway/Riverside Drive intersection. Thus, Warrant No. 2 is satisfied for the New Fashion Square Westerly Driveway/Riverside Drive intersection.

Warrant 3: Peak Hour Volume Warrant

The Peak Hour Volume Warrant is intended for application where traffic conditions are such that for one hour of the day minor street traffic suffers undue delay in entering or crossing the major street. Warrant No. 3 is satisfied when the plotted point, representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per on the higher volume minor street approach (one direction only) for one hour of an average day, falls above the curve in Figure C for the applicable number of approach lanes. The lower threshold for a minor street approach with one lane is 100 vehicles per hour and with two or more lanes is 150 vehicles per hour. As shown in the worksheet, the signal warrant is met when the plotted point falls above the appropriate curve.

As shown on the attached Figure C provided in *Appendix E*, with or without consideration of the right-turn volume on the minor street, the plotted points for the peak hours fall above the applicable curve for the existing with project conditions for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection. Therefore, Warrant No. 3 is satisfied for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection.

As shown on the attached Figure C provided in *Appendix E*, without consideration of the right-turn volume on the minor street, the plotted points for the peak hours fall above the applicable

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curve for the existing with project conditions for the New Fashion Square Westerly Driveway/Riverside Drive intersection. Therefore, Warrant No. 3 is satisfied for the New Fashion Square Westerly Driveway/Riverside Drive intersection.

Warrant 6: Coordinated Signal System

The Coordinated Signal System warrant applies when the following criteria is satisfied:

- The distance to the nearest traffic signal is greater than 1,000 feet, and
- On an isolated one-way street or street with one way traffic significance adjacent signals are so far apart that necessary that necessary platooning and speed control would be lost; or
- On a two-way street, where the adjacent signals do not provide the necessary degree of platooning and speed control, proposed signals could constitute a progressive signal system.

Warrant 6 is satisfied if the distance to the nearest traffic signal is greater than 1,000 feet; if the adjacent signals did not provide the necessary degree of platooning and the proposed and adjacent signals could constitute a progressive signal system.

For the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection, the distance to the nearest traffic signal to the west (i.e., at Hazeltine Avenue) on Riverside Drive is 2,000 feet. Although the traffic signals on Riverside Drive are coordinated, the traffic signals do not provide the necessary degree of platooning and speed control due to the significant distance between Woodman Avenue and Hazeltine Avenue. Thus, Warrant 6 is satisfied for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection.

For the New Fashion Square Westerly Driveway/Riverside Drive intersection, the distance to the nearest traffic signal to the east (i.e., at Woodman Avenue) on Riverside is 2,140 feet. Although the traffic signals on Riverside Drive are coordinated, the traffic signals do not provide the necessary degree of platooning and speed control due to the significant distance between Woodman Avenue and Hazeltine Avenue. Thus, Warrant 6 is satisfied for the New Fashion Square Westerly Driveway/Riverside Drive intersection.

In summary, based on the traffic signal warrants analysis prepared for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections, the following conclusions are reached:

- The existing with project traffic volumes at the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections satisfy the minimum thresholds shown for Conditions A and B associated with Warrant No. 1 (Eight Hour Vehicular Volume).
- The existing with project traffic volumes at the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections satisfy the minimum thresholds shown for Warrant No. 2 (Four-Hour Vehicular Volume).

- The existing with project traffic volumes at the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections satisfy the minimum thresholds shown for Warrant No. 3 (Peak Hour) during the AM and PM peak hours.
- The Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections satisfy the minimum criteria for Warrant No. 6 (Coordinated Signal System).

Based on the existing and future land uses in the project study area, future traffic volumes, and observed travel speeds on Riverside Drive, it is recommended that traffic signals be considered for installation at the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive and New Fashion Square Westerly Driveway/Riverside Drive intersections to improve overall safety and/or operation of the intersections. In addition, the traffic signal installation at these intersections would accommodate turning movements (particularly left-turn egress turning movements) in a safe and efficient manner. The accommodation of a left-turn ingress and egress movements at the two proposed Fashion Square Driveways on Riverside Drive is anticipated to alleviate the existing vehicular queuing at the existing Fashion Square driveways, to discourage vehicles from traveling through the adjacent residential areas, and to eliminate the potential safety concerns associated with the existing illegal left-turn maneuvers onto westbound Riverside Drive.

As stated in the LADOT MPP document, the decision to install a traffic signal is not based solely upon these warrants. Delay, congestion, approach conditions, driver confusion, future land use or other evidence of the need for right-of-way assignment beyond that which could be provided by stop sign control may be demonstrated.

It is noted that the owners of the Riverside Shopping Center may not permit an internal roadway connection between the Fashion Square and its center. In this scenario, the Riverside Shopping Center will continue to have vehicular access from its existing driveways along Riverside Drive and Woodman Avenue, with restricted left-turn ingress from Riverside Drive. Traffic signal warrants prepared for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection without traffic from the Riverside Shopping Center have been previously submitted to LADOT. The traffic signal warrants conclude that the traffic signal for the Matilija Avenue-New Fashion Square Easterly Driveway/Riverside Drive intersection is warranted without traffic from the Riverside Shopping Center.

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12.0 NEIGHBORHOOD STREET SEGMENT ANALYSIS

In order to address the issue of non-residential traffic using local streets in neighborhoods adjacent to the proposed project site, two local residential street segments located near the project site have been analyzed for potential significant impacts. These street segments were selected for analysis as they would have the highest potential to incur project-related trips. The street segments listed below were selected for analysis by LADOT:

- 1. Ranchito Avenue north of Riverside Drive
- 2. Matilija Avenue north of Riverside Drive

The significance of the potential impacts of project generated traffic at the study street segment was identified using criteria set forth in the LADOT's *Traffic Study Policies and Procedures*. According to the City's published traffic study guidelines, a transportation impact on a local residential street shall be deemed significant based on an increase in the project Average Daily Traffic (ADT) volumes as shown in *Table 12-1*.

	le 12-1 MENT IMPACT THRESHOLD CRITERIA
Projected Average Daily Traffic With Project (Final ADT)	Project-Related Increase in ADT
0 to 999	16 percent or more of final ADT
1,000 or more	12 percent or more of final ADT
2,000 or more	10 percent or more of final ADT
3,000 or more	8 percent or more of final ADT

The 24-hour machine traffic counts were conducted during a typical mid-week day (Tuesday, Wednesday, or Thursday) for the analyzed street segments. The traffic count data were increased at a rate of 2.0 percent (2.0%) per year to reflect year 2007 conditions. Copies of the 24-hour machine counts are contained in Appendix F.

The forecast traffic conditions at the analyzed street segment for existing, future pre-project and future with project scenarios are summarized in *Table 12-2*. The actual 24-hour count data was utilized to evaluate the existing conditions. As shown in Column [2] of *Table 12-2*, for purposes of estimating future pre-project traffic volume, a two percent (2.0%) annual growth rate through the year 2012 was conservatively added to the existing ADT volume to account for traffic generated by the related projects, as well as increases in general ambient traffic.

As presented in Column [5] of *Table 12-2*, the proposed project daily trips will incrementally affect traffic volumes on the analyzed street segments. As shown in *Table 12-2*, application of LADOT's threshold criteria for local residential street segment analysis indicates that the proposed project is not anticipated to significantly impact the analyzed street segment.

LINSCOTT, LAW & GREENSPAN, engineers

LLG Ref. 1-053606-1

Westfield Fashion Square Expansion Project

Table 12-2
NEIGHBORHOOD STREET SEGMENT ANALYSIS SUMMARY

		[1] YEAR 2007 EXISTING 24-HOUR	[2] YEAR 2012 FUTURE PRE-PROJECT	PROF	3] POSED DECT T %	[4] DAILY PROJECT BUILD-OUT	[5] YEAR 2012 FUTURE WITH PROJECT	[6] PERCENT ADT INCREASE WITH	[7] SEGMENT
NO.	STREET SEGMENT	VOLUME	VOLUME	IN	OUT	TRIP ENDS	[(2)+(4)]	PROJECT	IMPACT
1	Ranchito Avenue north of Riverside Drive	1,568	1,725	2.0%	2.0%	99	1,824	5.4%	МО
2	Matilija Avenue north of Riverside Drive	802	882	0.0%	0.0%	0	882	0.0%	МО

- [1] The existing average daily traffic (ADT) volume was determined based on a count conducted by City Traffic Counters.
 - An ambient growth rate of two percent (2.0%) per year was assumed to derive the year 2007 existing conditions. A copy of the ADT summary data worksheet is provided in Appendix C.

[2] An ambient growth rate of two percent (2.0%) per year was assumed to derive the year 2012 future pre-project volume. [3] Distribution of inbound and outbound daily project traffic at the analyzed street segment.

[4] Project build-out daily trip ends include inbound and outbound trips based on a net increase of 4,964 daily trips.

[5] Total of columns [2] and [4].

- [6] The ADT percentage increase due to project traffic was calculated by dividing [4] by [5].
- [7] According to LADOT's "Traffic Study Policies & Procedures," March, 2002, page 10: "A local residential street shall be deemed significantly impacted based on an increase in the projected average daily traffic (ADT) volumes."

Projected Average
Daily Traffic with
Project (Final ADT)
0 to 999
1,000 or more
2,000 or more
3,000 or more

Project-Related
Increase in ADT
16% or more of final ADT
12% or more of final ADT
10% or more of final ADT
8% or more of final ADT

13.0 WEEKEND PROJECT IMPACT ANALYSIS

While not specifically required by LADOT staff, additional analysis was prepared to evaluate the potential traffic impacts of the proposed Westfield Fashion Square Expansion project to the local street system during the Saturday mid-day peak hour. Specifically, the focus of this analysis is to determine the potential traffic impacts at the following seven study intersections located immediately adjacent to the project site:

Int. No. 7: Hazeltine Avenue/Riverside Drive

Int. No. 8: Hazeltine Avenue/Fashion Square Lane

Int. No. 12: Woodman Avenue/Riverside Drive

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

Int. No. 14: Woodman Avenue/U.S. 101 Eastbound Ramps

Int. No. 17: Matilija Avenue-New Project Driveway/Riverside Drive

Int. No. 18: New Westerly Project Driveway/Riverside Drive

These intersections were selected for analysis during the Saturday mid-day peak hour conditions as they are immediately adjacent or in close proximity to the site, and therefore have the highest potential to incur potential impacts due to the project during this time period.

13.1 Existing Traffic Counts

Manual counts of vehicular turning movements were conducted at the seven adjacent study intersections during the Saturday mid-day period to determine the peak hour traffic volumes. The manual counts were conducted in March 2007 at the seven study intersections from 1:00 PM to 3:00 PM on Saturday to determine the Saturday mid-day peak hour. The Saturday mid-day peak period manual counts of vehicle movements at the seven adjacent study intersections are summarized in *Table 13-1*. The existing traffic volumes at the study intersections during the Saturday mid-day peak hour are shown in *Figure 13-1*. Summary data worksheets of the Saturday manual traffic counts at the seven adjacent study intersections are contained in *Appendix B*.

13.2 Traffic Forecasting Methodology

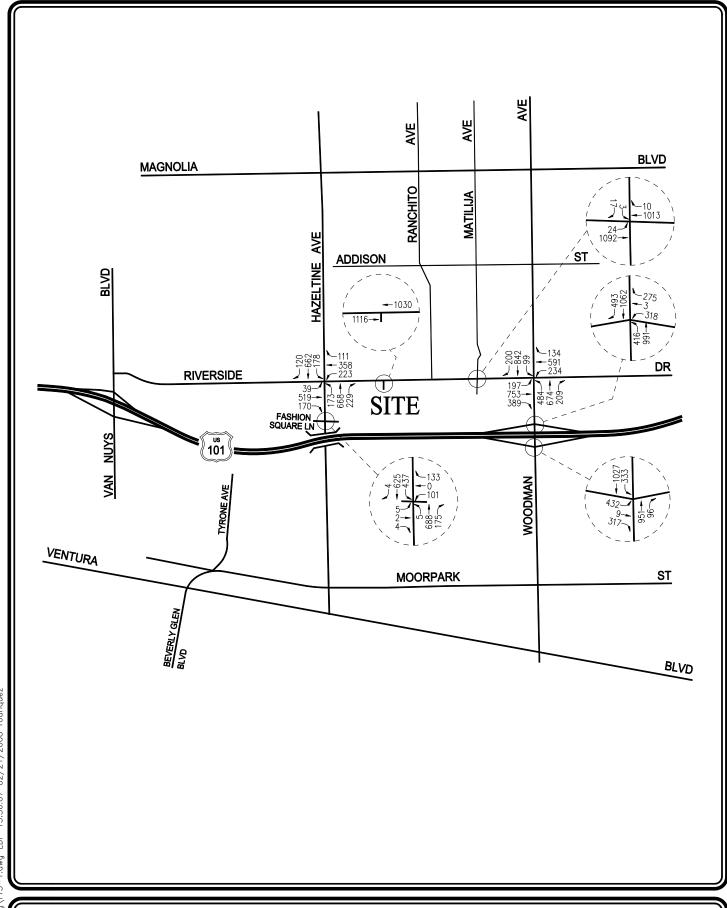
13.2.1 Weekend Project Trip Generation

Traffic volumes expected to be generated by the proposed project during the Saturday mid-day peak hour, as well as on a daily basis, were estimated using rates published in the Institute of Transportation Engineers' (ITE) *Trip Generation* manual. Traffic volumes expected to be generated by the proposed shopping center project were based upon per thousand square feet of gross leasable area. ITE Land Use Code 820 (Shopping Center) trip generation equation rates

Table 13-1
WEEKEND EXISTING TRAFFIC VOLUMES [1]

				SATURDA	Y MIDDAY
NO.	INTERSECTION	DATE	DIR	BEGAN	VOLUME
7	Hazeltine Avenue/ Riverside Drive	03/17/07	NB SB EB WB	1:45	1,070 960 728 692
8	Hazeltine Avenue/ Fashion Square Lane	03/17/07	NB SB EB WB	1:45	868 1,066 11 234
12	Woodman Avenue/ Riverside Drive	03/17/07	NB SB EB WB	1:45	1,367 1,141 1,339 959
13	Woodman Avenue/ U.S. 101 Freeway Westbound Ramps	03/17/07	NB SB EB WB	1:00	1,407 1,555 0 596
14	Woodman Avenue/ U.S. 101 Freeway Eastbound Ramps	03/17/07	NB SB EB WB	1:00	1,047 1,360 758 0
17	Matilija Avenue- New Project Driveway/ Riverside Drive	03/17/07	NB SB EB WB	2:00	0 20 1,116 1,023
18	New Project Driveway/ Riverside Drive	03/17/07	NB SB EB WB	2:00	0 0 1,116 1,030

^[1] Counts conducted by City Traffic Counters.





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FIGURE 13-1 EXISTING TRAFFIC VOLUMES WEEKEND MID-DAY PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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for the Saturday peak hour were used to forecast the traffic volumes expected to be generated by the proposed project as well as the existing shopping center.

The Saturday trip generation forecast for the proposed project is summarized in *Table 13-2*. As presented in *Table 13-2*, the proposed project is expected to generate a net increase of 632 vehicle trips (329 inbound trips and 303 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, the proposed project is forecast to generate a net increase of 6,252 daily trip ends during a typical Saturday (3,126 inbound trips and 3,126 outbound trips).

It is noted that the trip generation forecast provided herein likely overstates the actual amount of vehicular traffic that would be generated by the proposed expansion. By example, traffic counts were conducted at the existing Fashion Square driveways during the Saturday mid-day peak period. As shown in *Appendix C*, the Fashion Square currently generates 2,854 Saturday mid-day peak hour trips (1,627 inbound trips and 1,227 outbound trips). The actual peak hour traffic count data is approximately 25% less than the estimates based on the ITE trip rates for shopping centers of similar size. This trip generation characteristic is likely due to the Fashion Square providing "high end" tenants which require relatively fewer patrons to achieve revenue goals. However, to provide a conservative, "worst-case" traffic analysis, the trip generation forecast based on the ITE trip rates have been utilized in the review of potential impacts associated with the project.

13.2.2 Project Trip Distribution

Similar to the weekday analysis, the project generated traffic was assigned to the local roadway system based on a trip distribution pattern developed in consultation with City staff. The forecast project traffic volumes at the study intersections for the Saturday mid-day peak hour are displayed in *Figure 13-2*.

13.3 Cumulative Development Projects

13.3.1 Related Projects

Traffic volumes expected to be generated by the related projects presented in *Table 8-1* were calculated using rates provided in the ITE Trip Generation manual. The related projects' respective traffic generation for the Saturday mid-day peak hour, as well as on a daily basis for a typical Saturday, is summarized in *Table 13-3*. The anticipated distribution of the related projects traffic volumes to the study intersections during the Saturday mid-day peak hour is displayed in *Figure 13-3*.

13.3.2 Ambient Growth Factor

Similar to the weekday analysis, the existing Saturday traffic volumes were increased at an annual rate of two percent (2.0%) to the year 2012 (i.e., the anticipated year of project build-out).

13.4 Traffic Analysis

Summaries of the v/c ratios and LOS values for the seven adjacent study intersections during the Saturday mid-day peak hour are shown in *Table 13-4*. The CMA data worksheets for the analyzed intersections are contained in *Appendix D*.

Table 13-2 WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]	ľ	AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	. IN	OUT	TOTAL
Proposed Shopping Center	1,147,000 GLSF	42,972	2,198	2,029	4,227
Less 10% Pass-by [4] Subtotal		(4,297)	(220) 1,978	(203)	(423)
Subtotal		38,675	1,976	1,826	3,804
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal		32,423	1,649	1,523	3,172
NET CHANGE	280,000 GLSF	6,252	329	303	632

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

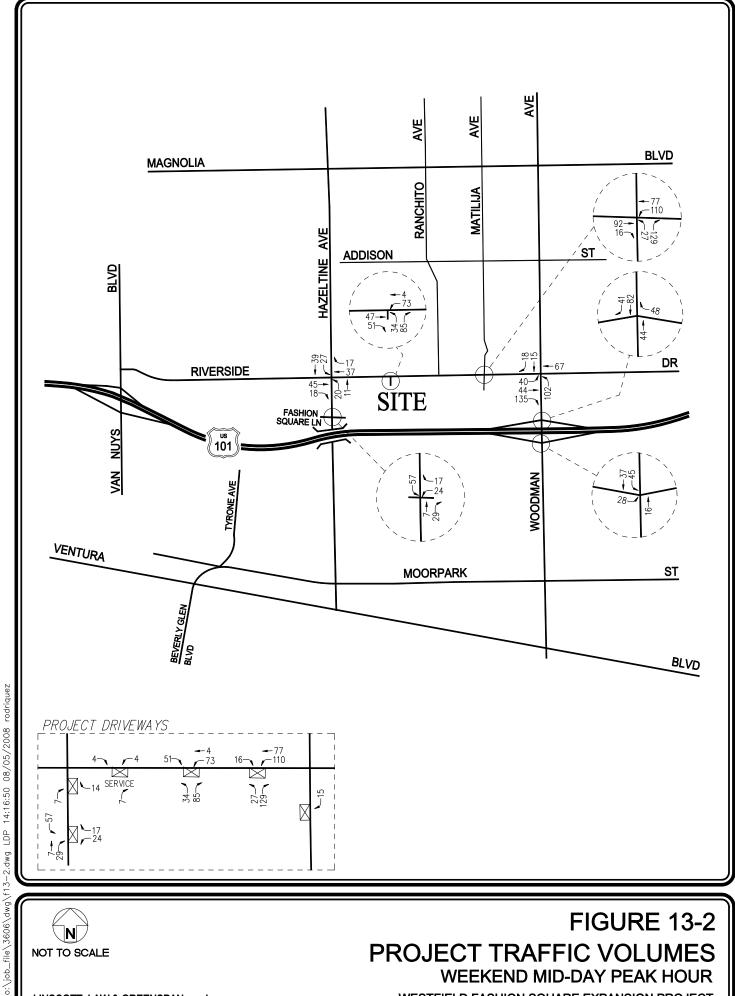




FIGURE 13-2 PROJECT TRAFFIC VOLUMES WEEKEND MID-DAY PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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Table 13-3 WEEKEND RELATED PROJECTS TRIP GENERATION [1]

Г				DAILY TRIP ENDS [2]		PEAK H	
	LAND USE	ST	ZE	VOLUMES	IN	OLUMES	TOTAL
<u> </u>	2210 002			· ODG/ALS	 -	001	TOTAL
1	Apartment [3]	142	טם	859	15	62	77
2	Camino Real Mixed-Use Development						
	Condominium [4]		DU	516	31	15	46
l	Retail [5]	6,000		300	16	14	30
1	Quality Restaurant [6]	7,000		661	45	31	76
	Fast-Food Restaurant with Drive-Through [7]	3,500	GSF	2,527	106	101	207
3	Office [8]	29,475	GSF	82	8	6	14
4	Drugstore [9]	11,244	GSF	880	44	44	88
5	Private School [10]	80	Students	Nom.	Nom.	Nom.	Nom.
6	Retail [5]	60,000	GSF	2,998	155	143	298
7	Apartment [3]	98	שע	513	12	47	59
8	Gas Station [11]	392	SF	380	19	19	38
9	Community College [12]	2,300	Students	966	66	49	115
10	Condominium [4]	52	טט	305	18	9	27
	Retail [5]	7,460	GSF	373	19	18	37
11	Villaggio Toscano Project						
1	Apartment [3]	500	DU	3,669	45	179	224
ŀ	Retail [5]	10,000	GSF	500	26	24	50
İ	Supermarket [13]	45,000	GSF	7,992	247	237	484
ı	Existing Apartments [14]	24	DU	(153)	(6)	(6)	(12)
į .	Existing Single Family Detached Housing [15]		טט	(111)	(5)	(5)	(10)
	Existing Office [8]	52,452	GSF	(131)	(12)	(10)	(22)
12	Self- Storage [16]	60,250	GSF	74	4	3	7
	Existing Health/Fitness Club [17]	14,624	GSF	305	19	19	38
13	New Car Sales [18]	85,038	GSF	1,788	129	124	253
14	Condominium [4]	26	DU	152	9	5	14
15	Condominium [4]	23	DU	135	8	4	12
16	Private School [9]	300	Students	Nom.	Nom.	Nom.	Nom.
17	Condominium [4]	247	טם	1,447	86	42	128
	TOTAL			27,027	1,104	1,174	2,278

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [4] ITE Land Use Code 230 (Apartment) trip generation equation rates for Saturday daily and peak hour of generator.

 [4] ITE Land Use Code 230 (Townhome/Condominium) trip generation average rates for Saturday daily and
- [4] ITE Land Use Code 230 (Townhome/Condominium) trip generation average rates for Saturday daily and peak hour of generator.
- [5] ITE Land Use Code 820 (Shopping Center) trip generation average rates for Saturday daily and peak hour of generator.
- [6] ITE Land Use Code 931 (Quality Restaurant) trip generation average rates for Saturday daily and peak hour of generator.
- [7] ITE Land Use Code 933 (Fast-Food Restaurant without Drive-Through) trip generation average rates for Saturday daily and peak hour of generator.
- [8] ITE Land Use Code 710 (General Office Building) trip generation equation rates for Saturday daily and peak hour of generator.
- [9] ITE Land Use Code 881 (Pharmacy/Drugstore without Drive-Through Window) trip generation average rates for Saturday peak hour of generator. The peak hour traffic volumes represent ten percent of the daily trip generation forecast.
- [10] Please note that the weekend daily and peak hour traffic volumes for Private School are assumed to be nominal.
- [11] As the ITE Trip Generation Manual does not provide weekend trip generation rates for this land use, ITE Land Use Code 945 (Gasoline Station with Convenience Market) weekday trip generation average rates were utilized.
- [12] ITE Land Use Code 540 (Junior/Community College) trip generation average rates for Saturday daily and peak hour of generator.
- [13] ITE Land Use Code 850 (Supermarket) trip generation average rates for Saturday daily and peak hour of generator.
- [14] ITE Land Use Code 220 (Apartment) trip generation average rates for Saturday daily and peak hour of generator.
- [15] ITE Land Use Code 210 (Single Family Housing) trip generation average rates for Saturday daily and peak hour of
- [16] ITE Land Use Code 841 (New Car Sales) trip generation average rates for Saturday daily and peak hour of generator.
- [17] ITE Land Use Code 492 (Health/Fitness Club) trip generation average rates for Saturday daily and peak hour of generator.
- [18] ITE Land Use Code 841 (New Car Sales) trip generation average rates for Saturday daily and peak hour of generator.

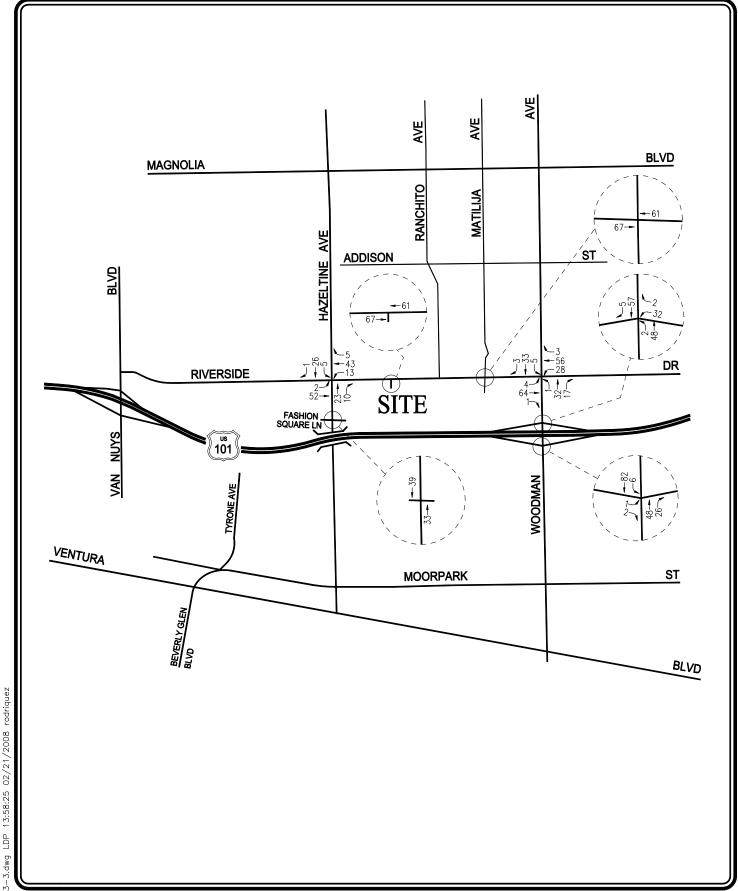




FIGURE 13-3
RELATED PROJECTS TRAFFIC VOLUMES
WEEKEND MID-DAY PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

			[]		[2]	<u> </u>	[3]	\vdash			[4			5		
			2		YEAR 2012	012	YEAR 2012	112	YEAR 2012	Г			YEAR 2012	2101		
		PEAK	YEAR 2007 EXISTING	700 VG	W/ AMBIENT GROWTH	LNE E	W/RELATED PROJECTS		W/ PROPOSED CHANGE SIGNIF. PROJECT V/C IMPACT	SED	CHANGE V/C	SIGNIE. IMPACT	W/PROJECT MITIGATION		CHANGE V/C	MITI- GATED
NO.	INTERSECTION	HOUR	A/C	ros	V/C	ros	V/C I	ros	V/C	ros	[(4)-(3)]	ق	A/C	ros	[(5)-(3)]	
7	Hazeltine Avenue/ Riverside Drive	Saturday Mid-day	0.684	Ф	0.760	υ	0.795	υ	0.842	Ω	0.047	YES	0.812	D	0.017	YES
∞	Hazeltine Avenue/ Fashion Square Lane	Saturday Mid-day	0.636	В	0.707	ပ	0.719	υ	0.764	C	0.045	YES	0.734	υ	0.015	YES
12	Woodman Avenue/ Riverside Drive	Saturday Mid-day	0.874	Q.	0.968	ы	1.024	ļz.,	1.086	р -	0.062	YES	766'0	ъī	-0.027	YES
13	Woodman Avenue/ US-101 Freeway Westbound Ramps	Saturday Mid-day	0.757	Ú	0.840	D	0.856	D ,	0.900	D	0.044	YES	0.870	Q	0.014	YES
14	Woodman Avenue/ US-101 Freeway Eastbound Ramps	Saturday Mid-day	0.590	Y	0.626	В	0.644	В	0.688	В	0.044	ON O	0.688	Д	0.044	I
17	Project Driveway-Matilija Avenue Riverside Drive [a]	Saturday Mid-day	0.472	4	0.519	¥	0.547	A	0.606	В	0.059	NO	0.606	ф	0.059	1
18	New Project Driveway (Tunnel Access) Riverside Drive [b]	Saturday Mid-day	0.000	¥	0.000	∢	0.000	A	0.755	C	0.755	ON .	0,755	Ú	0.755	ı

Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. Intersection currently does not exist. Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. <u>p</u> <u>g</u>

<u>ত</u>

According to LADOT's "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection shall be deemed significant in accordance with the following table: Project-Related Increase in V/C equal to or greater than 0.020 equal to or greater than 0.010 equal to or greater than 0.040 > 0.700 - 0.800 > 0,800 - 0,900 Final V/C > 0.900 Level of Service D F υ

13.4.1 Existing Conditions

As indicated in column [1] of *Table 13-4*, all of the seven adjacent study intersections are presently operating at LOS D or better during the Saturday mid-day peak hour under existing conditions. As previously mentioned, the existing traffic volumes at the study intersections during Saturday mid-day peak hour are displayed in *Figure 13-1*.

13.4.2 Existing With Ambient Growth Conditions

As shown in column [2] of *Table 13-4*, six of the seven adjacent study intersections are presently operating at LOS D or better during the Saturday mid-day peak hour with the addition of ambient growth traffic through the year 2012. The following study intersection is expected to operate at LOS E during the during Saturday mid-day peak hour with the addition of ambient growth traffic:

Int. No. 12: Woodman Avenue/Riverside Drive Mid-day Peak Hour: v/c=0.968, LOS E

The existing with ambient growth traffic volumes at the study intersections during all of the seven adjacent study intersections are presently operating at LOS D or better during the Saturday mid-day peak hour are shown in *Figure 13-4*.

13.4.3 Future Pre-Project Conditions

As presented in column [3] of *Table 13-4*, five of the seven adjacent study intersections are expected to continue operating at LOS D or better during the Saturday mid-day peak hour with the addition of growth in ambient traffic and the traffic due to the related projects. The following study intersection is expected to operate at LOS E during the Saturday mid-day peak hour with the addition of ambient traffic and the traffic due to the related projects:

Int. No. 12: Woodman Avenue/Riverside Drive Mid-day Peak Hour: v/c=1.024, LOS F

The future pre-project (existing, ambient growth and related projects) traffic volumes at the study intersections during the Saturday mid-day peak hour are presented in *Figure 13-5*.

13.4.4 Future With Project Conditions

As shown in column [4] of *Table 13-4*, application of the City's threshold criteria to the "With Proposed Project" scenario indicates that the proposed project is expected to create significant impacts at four of the seven adjacent study intersections. The proposed project is expected to create significant impacts at the following locations according to the City's impact criteria during the Saturday mid-day peak hour with the addition of ambient growth, related projects traffic, and project-related traffic:

Int. No. 7: Hazeltine Avenue/Riverside Drive

Mid-Day peak hour v/c ratio increase of 0.047 [to 0.842 (LOS D) from 0.795 (LOS C)]

Int. No. 8: Hazeltine Avenue/Fashion Square Lane

Mid-Day peak hour v/c ratio increase of 0.045 [to 0.764 (LOS C) from 0.719 (LOS C)]

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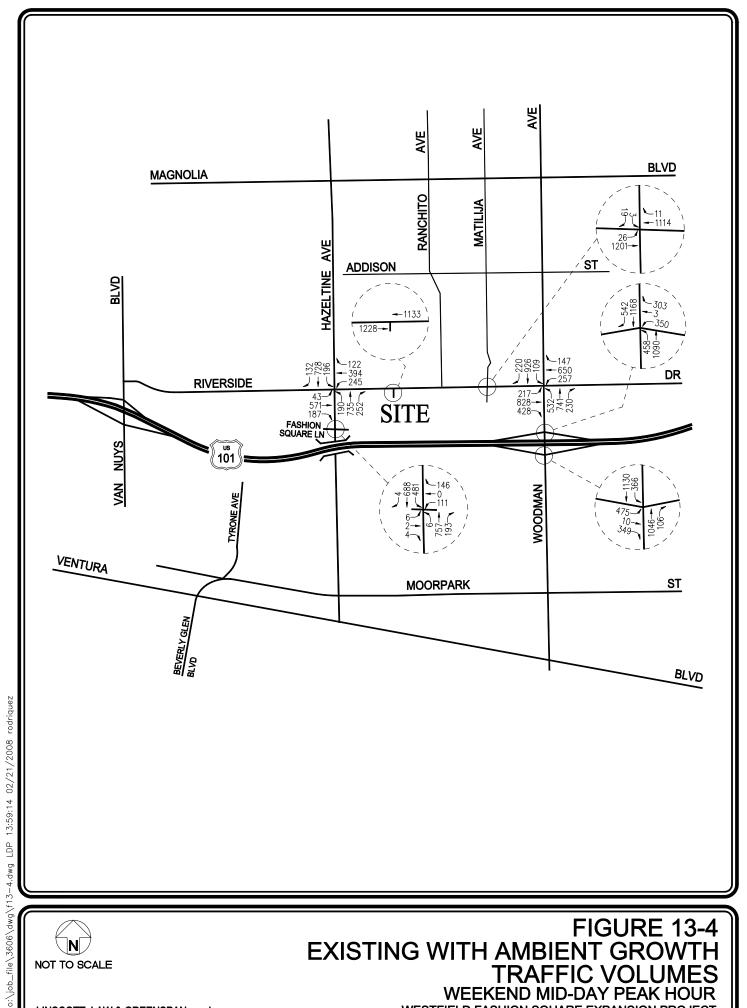




FIGURE 13-4 EXISTING WITH AMBIENT GROWTH

WEEKEND MID-DAY PEAK HOUR WESTFIELD FASHION SQUARE EXPANSION PROJECT

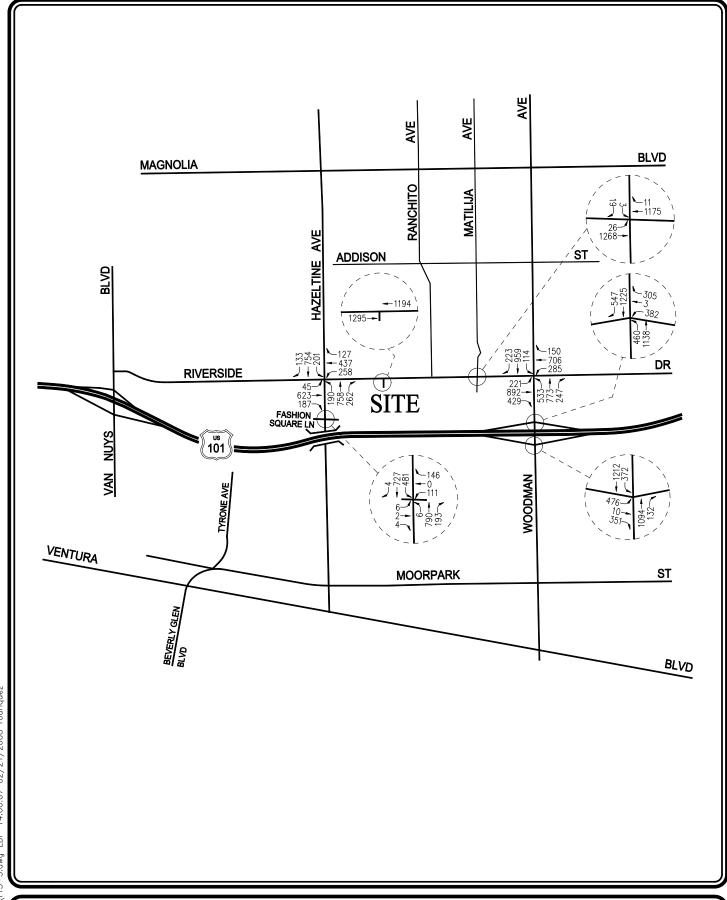
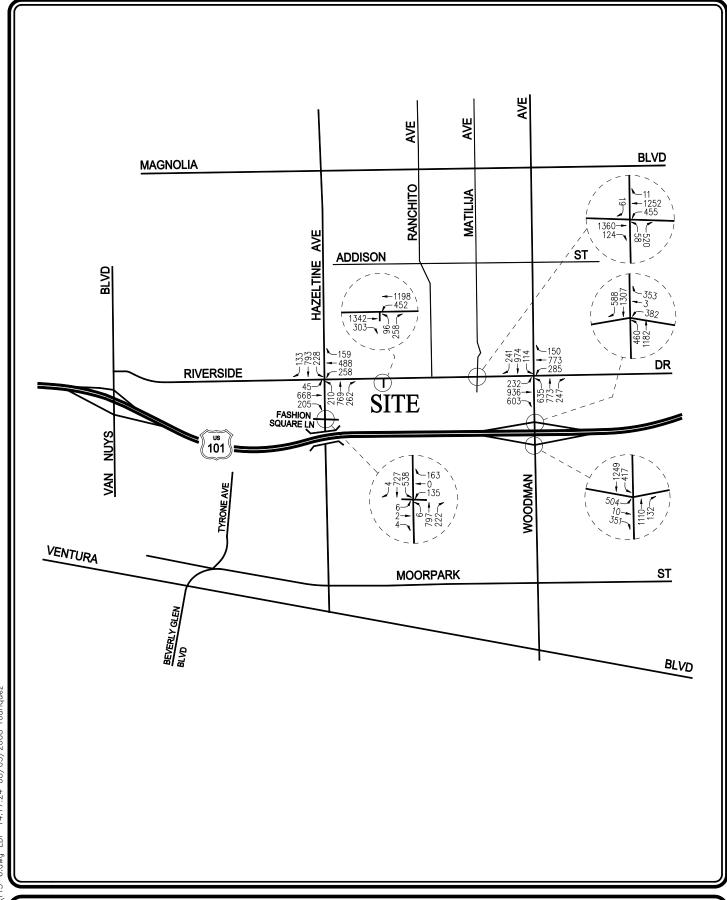




FIGURE 13-5
FUTURE PRE-PROJECT
TRAFFIC VOLUMES
WEEKEND MID-DAY PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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FIGURE 13-6
FUTURE WITH PROJECT
TRAFFIC VOLUMES

WEEKEND MID-DAY PEAK HOUR WESTFIELD FASHION SQUARE EXPANSION PROJECT

The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection. In addition, redesignate the curb lane on the southbound approach on Woodman Avenue to an optional through/right-turn lane. The resultant lane configurations at the southbound approach will be one left-turn lane, two through lanes and one optional through/right-turn lane. If required by LADOT, the existing four-foot wide median island on the south leg of the intersection could be replaced by striping and/or lane delineators (e.g., two feet wide or less) so that additional width could be provided to the existing three southbound Woodman Avenue through lanes on the departure side of the intersection. The schematic plan of the proposed improvement at the Woodman Avenue/Riverside Drive intersection is contained in Appendix A.

In addition, LADOT recommends that left-turns from northbound Woodman Avenue to La Maida Street be restricted during the weekday PM peak period. After the mitigation is implemented, LADOT will review the Woodman Avenue/La Maida Street intersection and determine if additional turn restriction measures are required.

As shown in *Table 13-4*, the proposed mitigation is expected to improve the v/c ratio to 0.997 (LOS E) from 1.086 (LOS F) during the Saturday mid-day peak hour. The improvement in the calculated v/c ratio is due to both the installation of ATCS, as well as the redesignation of the southbound Woodman Avenue right-turn lane to an optional through/right-turn lane. Thus, the significant impact at this intersection during the Saturday mid-day peak hour would be reduced to less than significant levels.

Int. No. 13: Woodman Avenue/U.S. 101 Westbound Ramps

The recommended mitigation consists of funding provided by the project for the installation of LADOT's ATCS at this intersection. This intersection is already included as part of the Victory ATSAC system. LADOT estimates that the ATSAC system reduces critical v/c ratios by seven percent (0.07) which has already been accounted for in the analysis of intersection operations. The ATCS upgrade further reduces the critical v/c ratios by three percent (0.03). Therefore, an additional reduction of 0.03 was assumed in the calculation of the v/c ratios for this intersection.

As shown in *Table 13-4*, the proposed mitigation is expected to improve the v/c ratio to 0.870 (LOS D) from 0.900 (LOS D) during the Saturday mid-day peak hour. Thus, the significant impact at this intersection during the Saturday mid-day peak hour would be reduced to less than significant levels.

13.5 Neighborhood Street Segment Analysis

As described in the Neighborhood Street Segment Analysis section (see Subsection 12.0) of this study, the potential impacts to the local residential streets north of the project site are deemed to be less than significant. As shown in *Table 12-2*, the incremental increase due to the proposed project is 5 percent (5%), which is half of the threshold of significance (10%) based on the LADOT approved methodology. Therefore, traffic impacts to the local residential streets north of the project site it is not anticipated to occur during the weekend conditions, even with a higher level of project-generated traffic on weekends.

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14.0 Congestion Management Program Traffic Impact Assessment

The Congestion Management Program (CMP) is a state-mandated program that was enacted by the State Legislature with the passage of Proposition 111 in 1990. The program is intended to address the impact of local growth on the regional transportation system.

As required by the 2004 Congestion Management Program for Los Angeles County, a Traffic Impact Assessment (TIA) has been prepared to determine the potential impacts on designated monitoring locations on the CMP highway system. The analysis has been prepared in accordance with procedures outlined in the 2004 Congestion Management Program for Los Angeles County, County of Los Angeles Metropolitan Transportation Authority, July, 2004.

According to Section B.9.1 (Appendix B, page B-6) of the 2004 CMP manual, the criteria for determining a significant impact is as follows:

"A significant transportation impact occurs when the proposed project increases traffic demand by 2% of capacity (V/C \geq 0.02), causing or worsening LOS F (V/C \geq 1.00)."

The CMP impact criteria apply for analysis of both intersection and freeway monitoring locations.

14.1 Intersections

The following CMP intersection monitoring locations in the project vicinity have been identified:

Ħ	CMP Station	<u>Intersection</u>
	Int. No. 74	Ventura Boulevard/Laurel Canyon Boulevard
	Int. No. 76	Ventura Boulevard/Sepulveda Boulevard
	Int. No. 78	Ventura Boulevard/Woodman Avenue (Study Int. No. 16)

The CMP TIA guidelines require that intersection monitoring locations must be examined if the proposed project will add 50 or more trips during either the AM or PM weekday peak hours. The proposed project will not add 50 or more trips during the AM or PM peak hours at any of the CMP monitoring intersections which is the threshold for preparing a traffic impact assessment, as stated in the CMP manual. Therefore, no further review of potential impacts to intersection monitoring locations that are part of the CMP highway system is required.

It should be noted that the Woodman Avenue/Ventura Boulevard intersection was analyzed as part of the traffic study. The Woodman Avenue /Ventura Boulevard intersection was evaluated using the CMA method of analysis which determines Volume-to-Capacity (ν /c) ratios on a critical lane basis. The overall intersection ν /c ratio is subsequently assigned a Level of Service (LOS) value to describe intersection operations. A description of the CMA method and corresponding Level of Service is provided in *Appendix D*. A summary of the ν /c ratios and LOS values for this intersection during the AM and PM peak hours is shown in *Table 9-2*. The CMA data worksheets for this intersection are contained in *Appendix D*.

As shown in *Table 9-2*, the proposed project is not expected to create a significant impact at the Woodman Avenue /Ventura Boulevard intersection based on the CMP significant impact criteria. Therefore, no further review of potential impacts to intersection monitoring locations that are part of the CMP highway system is required.

14.2 Freeways

The following CMP freeway monitoring location in the project vicinity has been identified:

CMP Station Segment
 Seg. No. 1038
 U.S. 101 Freeway at Coldwater Canyon Avenue

The CMP TIA guidelines require that freeway monitoring locations must be examined if the proposed project will add 150 or more trips (in either direction) during either the AM or PM weekday peak periods. The proposed project will not add 150 or more trips (in either direction) during either the AM or PM weekday peak hours to the CMP freeway monitoring location which is the threshold for preparing a traffic impact assessment, as stated in the CMP manual. Specifically, based on the data provided in *Figures 7-2 and 7-3*, the project is forecast to add four (4) eastbound trips and seven (7) westbound trips to the freeway monitoring location during the AM peak hour. Similarly, during the PM peak hour, an additional 30 eastbound trips and 27 westbound trips are forecast at the monitoring location due to the project. These forecast additional trips are substantially less than the CMP threshold for additional analysis. Therefore, no further review of potential impacts to freeway monitoring locations that are part of the CMP highway system is required.

14.3 Transit

As required by the 2004 Congestion Management Program for Los Angeles County, a review has been made of the CMP transit service. As previously discussed, existing transit service is provided in the vicinity of the proposed project.

The project trip generation, as shown in *Table 7-1*, was adjusted by values set forth in the CMP (i.e., person trips equal 1.4 times vehicle trips, and transit trips equal 3.5 percent of the total person trips) to estimate transit trip generation. Pursuant to the CMP guidelines, the proposed project is forecast to generate demand for 5 net new transit trips (3 inbound trips and 2 outbound trips) during the weekday AM peak hour. During the PM peak hour, the proposed project is forecast to generate demand for 23 net new transit trips (11 inbound trips and 12 outbound trips). Over a 24-hour period, the proposed project is forecast to generate a demand for 243 daily transit trips. The calculations are as follows:

- AM Peak Hour Trips = $95 \times 1.4 \times 0.035 = 5$ Transit Trips
- PM Peak Hour Trips = $476 \times 1.4 \times 0.035 = 23$ Transit Trips
- Daily Trips = $4,964 \times 1.4 \times 0.035 = 243$ Transit Trips

It is anticipated that the existing transit service in the project area will adequately accommodate the project generated transit trips. Thus, given the relatively low number of generated transit trips, no project impacts on existing or future transit services in the project area are expected to occur as a result of the proposed project.

15.0 VOLUNTARY TRANSPORTATION IMPROVEMENTS

As described in the prior section, based on an analysis of potential traffic impacts during weekday and weekend peak hours, the proposed project is forecast to cause impacts that may be potentially significant at ten study intersections. Transportation improvement measures have been identified herein to mitigate the impacts to levels of significance. Further, based on the LADOT approved methodology, the potential impacts to the local residential streets north of the project site are deemed to be less than significant. Therefore, with the exception of the previously identified traffic mitigation measures, no additional measures are required.

The project applicant, in consultation with LADOT, has volunteered to implement two additional transportation improvements so as to improve local traffic operations:

- Fund the development and implementation of a Neighborhood Traffic Management Plan (NTMP) to address potential existing and future regional "cut-through" traffic on residential streets north of Fashion Square.
- Design and install protected/permissive left-turn traffic signal phasing for Hazeltine Avenue and Riverside Drive at the Hazeltine Avenue/Riverside Drive intersection (i.e., all approaches to the intersection). Based on recent discussions with LADOT staff, the southbound left-turn phasing on Hazeltine Avenue is currently under construction by LADOT. Therefore, the Fashion Square will volunteer to implement the installation of the protected/permissive left-turn phasing at the remaining approaches to the intersection (i.e., northbound approach on Hazeltine Avenue and eastbound and westbound approaches on Riverside Drive).
- Design and install upgraded traffic delineators along Hazeltine Avenue between Riverside Drive and Fashion Square Lane using "quik-kurb" or similar installation approved by LADOT.

16.0 CONCLUSIONS

This traffic analysis has been conducted to identify and evaluate the potential impacts of traffic generated by the proposed Westfield Fashion Square Expansion project. The proposed project is located on the south side of Riverside Drive between Hazeltine Avenue and Woodman Avenue. The proposed project consists of the expansion of the existing shopping center by 280,000 square feet of gross leasable floor area to provide a total of approximately 1,147,000 square feet of gross leasable floor area.

This traffic analysis evaluates potential project-related impacts at 18 intersections and two street segments. Application of the City's threshold criteria to the "With Proposed Project" scenario indicates that six of the 18 study intersections are anticipated to be significantly impacted by the proposed project during the weekday conditions. Incremental but not significant impacts are noted at the remaining 12 study intersections, as well as at the two local residential street segments evaluated in the analysis. It is recommended that the significant transportation impacts be mitigated through a contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation. This recommended mitigation measure is anticipated to reduce the forecast project-related significant impacts to less than significant levels.

While not specifically required for traffic mitigation purposes, it is recommended that, as part of the proposed project, traffic signals be installed at the Matilija Avenue-New Easterly Fashion Square Driveway/Riverside Drive intersection and the New Fashion Square Westerly Driveway/Riverside Drive intersection (Tunnel Access)

A focused analysis was also prepared to evaluate the potential traffic impacts of the proposed Westfield Fashion Square Expansion project during the Saturday mid-day peak hour at seven study intersections located immediately adjacent to the project site. Application of the City's threshold criteria to the "With Proposed Project" scenario indicates that four of the seven study intersections are anticipated to be significantly impacted by the proposed project during the weekend conditions. Incremental but not significant impacts are noted at the remaining three study intersections. The aforementioned contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation at the four impacted study intersections is recommended to mitigate potential impacts. In addition, at the Woodman Avenue/Riverside Drive intersection, it is recommended that the southbound Woodman Avenue approach to the Riverside Drive intersection be reconfigured to provide one left-turn lane, two through lanes and one optional through/right-turn lane to reduce the forecast project-related significant impacts to less than significant levels.

Parking utilization observations conducted at the site during the 2005 and 2006 holiday shopping periods revealed that the demand for parking peaked at a ratio equivalent to 4.03 parking spaces per 1,000 GLSF (observed at 4:00 PM on December 26). Therefore, parking provided at the site in conjunction with the proposed expansion at a rate of 4.25 parking spaces per 1,000 GLSF is expected to be adequate to accommodate peak parking demands during the December holiday season, as well as throughout the year.

APPENDIX A

PROPOSED SITE ACCESS AND INTERSECTION IMPROVEMENTS

APPENDIX FIGURE A-1 HAZELTINE AVENUE IMPROVEMENTS

- UP TO PARKING

Parking Garage

MAP SOURCE: WESTFIELD DESIGN & CONSTRUCTION

NOT TO SCALE

APPENDIX FIGURE A-3 NEW EASTERLY PROJECT DRIVEWAY ON RIVERSIDE DRIVE

WESTFIELD FASHION SQUARE EXPANSION PROJECT

SOURCE: WESTFIELD DESIGN & CONSTRUCTION

N

NOT TO SCALE

LINSCOTT, LAW & GREENSPAN, engineers

APPENDIX FIGURE A-2 PROPOSED CIRCULATION ACCESS ROAD

NOT PART OF FASHION SQUAR

macyš

GARAGE G+3

NEW PARKING GARAGE G+5

MAP SOURCE: WESTFIELD DESIGN & CONSTRUCTION



NOT TO SCALE

APPENDIX FIGURE A-4 NEW WESTERLY PROJECT DRIVEWAY ON RIVERSIDE DRIVE

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

LINSCOTT, LAW & GREENSPAN, engineers

APPENDIX B MANUAL TRAFFIC COUNTS

APPENDIX B-1 WEEKDAY CONDITIONS

File Name : 355501 Site Code : 00355501 Start Date : 11/17/2005 Page No : 1

		VAN NU'	ve pi i/r	<u> </u>		Gr RIVERSI		inted- Tur		vement VAN NU	Ve bi Vi	n					
	İ		ro bevi bound	J.	-		bound	√ □	,		ibound	D.		East	bound		
			1	App.				App.				Арр.				App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Tota
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	345	17	362	28	0	84	112	33	190	0	223	0	0	0	0	697
07:15 AM	0	368	22	390	36	0	138	174	62	186	0	248	0	0	0	0	812
07:30 AM	0	306	11	317	47	0	158	205	134	191	0	325	0	0	0	0	847
07:45 AM	0	248	15	263	82	0	144	226	129	259	0	388	0	0	0	0	877
Total	0	1267	65	1332	193	0	524	717	358	826	0	1184	0	0	0	0	3233
08:00 AM	0	247	7	254	113	0	167	280	105	315	0	420	0	0	0	0	954
08:15 AM	0	324	22	346	88	0	177	265	107	357	0	464	0	0	0	0	1075
08:30 AM	0	268	41	309	· 65	0	108	173	129	316	0	445	0	0	0	0	927
08:45 AM	0	270	49	319	48	0	111	159	138	332	0	470	0	0	0	0	948
Total	0	1109	119	1228	314	0	563	877	479	1320	0	1799	0	0	0	0	3904
09:00 AM	0	318	40	358	46	0	73	119	151	351	0	502	0	0	0	0	979
09:15 AM	0	335	52	387	62	0	75	137	98	309	0	407	0	0	0	0	931
09:30 AM	0	311	30	341	41	0	58	99	99	288	0	387	0	0	0	0	827
09:45 AM	0	336	34	370	37	0	56_	93	75	318	0	393	0	0	0	0	856
Total	0	1300	156	1456	186	0	262	448	423	1266	0	1689	0	0	0	0	3593
** BREAK **	*																
03:00 PM	0	327	42	369	63	0	96	159	120	411	0	531	0	0	0	0	1059
03:00 PM 03:15 PM	0 0	327 353	42 54	369 407	63 48	0	96 117	159 165	120 118	411 399	0	531 517	0	0	0 0	0	
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03:15 PM 03:30 PM	0	353 369	54 54	407 423	48 49	0	117 126	165 175	118 163	399 351	0	517 514	0	0	0	0 0	1089 1112 1079
03:15 PM 03:30 PM 03:45 PM Total 04:00 PM	0 0 0 0	353 369 327 1376	54 54 58 208	407 423 385 1584 328	48 49 65 225	0 0 0 0	117 126 118 457	165 175 183 682	118 163 141 542	399 351 370 1531 394	0 0 0	517 514 511 2073	0 0 0 0	0 0 0 0	0 0 0	0 0 0	1089 1112 1079 4339
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Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:0	O AM to	09:45	AM - Pea	k 1 of 1	L											
Intersection	08:15	AM															
Volume	0	1180	152	1332	247	0	469	716	525	1356	0	1881	0	0	0	0	3929
Percent	0.0	88.6	11.4		34.5	0.0	65.5		27.9	72.1	0.0		0.0	0.0	0.0		
08:15 Volume	0	324	22	346	88	0	177	265	107	357	0	464	0	0	0	0	1075
Peak Factor																	0.914
High Int.	09:00	AM			08:15	AM			09:00	AM			6:45:0	0 AM			
Volume Peak Factor	0	318	40	358 0.930	88	0	177	265 0.675	151	351	0	502 0.937					

File Name : 355501 Site Code : 00355501 Start Date : 11/17/2005 Page No : 2

	Ĭ		YS BLV	D.	R		DE DRIV	/E	\		YS BLVE).		Eastl	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00) PM to	05:45		k 1 of 1												
Intersection	03:00	PM															
Volume	0	1376	208	1584	225	0	457	682	542	1531	0	2073	0	0	0	0	4339
Percent	0.0	86.9	13.1		33.0	0.0	67.0		26.1	73.9	0.0		0.0	0.0	0.0		
03:30	0	369	54	423	49	0	126	175	163	351	0	514	0	0	0	0	1112
Volume	Ŭ	505	٠.			_											
Peak Factor																	0.975
High Int.	03:30	PM			03:45	PM			03:00	PM							
Volume	0	369	54	423	65	0	118	183	120	411	0	531					
Peak Factor				0.936				0.932				0.976					

File Name : 355502 Site Code : 00355502 Start Date : 11/17/2005 Page No : 1

						Gr	oups Pri	nted- Tur	ning Mo	vement					. ugo		
	\	VAN NU	S BLV).	US		OFF RA				YS BLV	D.	US		ON RA	MP	
		South	bound	**************************************	<u> </u>	West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	128	292	0	420	73	0	59	132	0	135	97	232	0	0	0	0	784
07:15 AM	211	301	0	512	59	3	33	95	0	185	119	304	0	0	0	0	911
07:30 AM	210	255	0	465	59	1	41	101	0	267	122	389	0	0	0	0	955
07:45 AM	172	213	0	385	80	1	40	121	0	328	96	424	0	0	0	0	930
Total	721	1061	0	1782	271	5	173	449	0	915	434	1349	0	0	0	0	3580
08:00 AM	187	220	0	407	112	3	64	179	0	319	91	410	0	0	0	0	996
08:15 AM	190	306	0	496	136	0	54	190	0	330	89	419	0	0	0	0	1105
08:30 AM	159	231	0	390	113	5	62	180	0	332	82	414	0	0	0	0	984
08:45 AM	140	242	0	382	124	2_	79	205	0_	341	82	423	0	0	0_	0	1010
Total	676	999	0	1675	485	10	259	754	0	1322	344	1666	0	0	0	0	4095
09:00 AM	117	272	0	389	141	1	85	227	0	365	87	452	0	0	0	0	1068
09:15 AM	108	288	Ö	396	127	5	109	241	0	283	86	369	0	0	0	0	1006
09:30 AM	123	285	Ö	408	119	0	92	211	0	286	84	370	0	0	0	0	989
09:45 AM	89	290	0	379	116	3	89	208	0	267	100	367	0	0	0	0	954
Total	437	1135	0	1572	503	9	375	887	0	1201	357	1558	0	0	0	0	4017
*** BREAK **		257		454	1			470		41.0	100	607		0	0	o l	1202
03:00 PM	164	257	0 0	421	115	1 0	63 74	179 180	0	416 399	186 185	602 584	0	0	0	0	1202
03:15 PM 03:30 PM	192 221	266 270	0	458 491	106 107	1	7 4 75	183	0	407	150	557	0	0	0	0	1231
03:45 PM	163	266	0	429	107	0	80	184	0	411	175	586	0	0	ő	ő	1199
Total	740	1059	0	1799	432	2	292	726	0	1633	696	2329	0	0	0	0	4854
04:00 PM	131	238	0	369	108	1	60	169	0	426	170	596	0	0	0	0	1134
04:15 PM	180	256	Ö	436	80	1	56	137	0	448	158	606	0	0	0	0	1179
04:30 PM	187	243	Ō	430	99	1	68	168	0	394	157	551	0	0	0	0	1149
04:45 PM	183	261	Ō	444	92	2	68	162	0	409	168	577	0	0	0	0	1183
Total	681	998	0	1679	379	5	252	636	0	1677	653	2330	0	0	0	0	4645
05:00 PM	168	231	0	399	89	1	97	187	0	469	192	661	0	0	0	0	1247
05:15 PM	145	200	0	345	57	0	72	129	0	435	159	594	0	0	0	0	1068
05:30 PM	139	167	0	306	69	0	50	119	0	417	171	588	0	0	0	0	1013
05:45 PM	161	212	0	373	75	0	67	142	0	418	151	569	0	0	0	0	1084
Total	613	810	0	1423	290	1	286	577	0	1739	673	2412	0	0	0	0	4412
Grand Total	3868	6062	0	9930	2360	32	1637	4029	0	8487	3157	11644	0	0	0	0	25603
Apprch %	39.0	61.0	0.0		58.6	0.8	40.6		0.0	72.9	27.1		0.0	0.0	0.0		
Total %	15.1	23.7	0.0	38.8	9.2	0.1	6.4	15.7	0.0	33.1	12.3	45.5	0.0	0.0	0.0	0.0	

	`		YS BLV	D.	US		OFF RA	AMP	\		YS BLVI	Э.	US		ONRA	MP	
1		South	nbound			Wes	lbound			Nortr	nbound		<u> </u>	East	bound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
Start Time	ixigin.	111114	LCT	Total	, wg.ic	11110	LC.C	Total	14.9.10			Total				<u>Total</u>	Total
Peak Hour From	m 07:0	0 AM to	09:45	AM - Pea	k 1 of 1												1
Intersection	08:15	AM															
Volume	606	1051	0	1657	514	8	280	802	0	1368	340	1708	0	0	0	0	4167
Percent	36.6	63.4	0.0		64.1	1.0	34.9		0.0	80.1	19.9		0.0	0.0	0.0		
08:15	190	306	0	496	136	0	54	190	0	330	89	419	0	0	0	0	1105
Volume	100	500	U	150	130	U	٥,	130		550				-	_		
Peak Factor																	0.943
High Int.	08:15	AM			09:00	AM			09:00	AM			6:45:0	MA 0			
Volume	190	306	0	496	141	1	85	227	0	365	87	452					
Peak Factor				0.835				0.883				0.945	[

File Name : 355502 Site Code : 00355502 Start Date : 11/17/2005 Page No : 2

	\	/AN NU	YS BLV	Ο.	US	101 NE	OFF RA	AMP	,		YS BLV	D.	US		3 ON RA	MP	
		South	nbound			West	lbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From	m 03:00) PM to	05:45	PM - Pea	k 1 of 1												
Intersection	03:00	PM															
Volume	740	1059	0	1799	432	2	292	726	0	1633	696	2329	0	0	0	0	4854
Percent	41.1	58.9	0.0		59.5	0.3	40.2		0.0	70.1	29.9		0.0	0.0	0.0		
03:30	221	270	0	491	107	1	75	183	0	407	150	557	0	0	0	o	1231
Volume	221	2,0	·	131	10,	_	, 5	105	Ū	,	150	557	J	•	_	-	
Peak Factor																	0.986
High Int.	03:30	PM			03:45	PM			03:00	PM							
Volume	221	270	0	491	104	0	80	184	0	416	186	602					
Peak Factor				0.916				0.986				0.967					

File Name : 355503 Site Code : 00355503 Start Date : 11/17/2005 Page No : 1

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	Τ	VAN NI	IYS BLV	<u> </u>	110	Gr 3 101 SE		inted- Tur			YS BLVI	`	110	101 00	3 OFF R	A M/D	
			hbound	υ.	0,		bound	AIVII"	ļ		nbound	J.	03		tbound	HIVIF	
Chart Times	Diele		1	App.	D: 11			App.	5			App.	5: 1.	ĺ	T	App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	242	101	343	0	0	0	0	72	160	0	232	96	0	69	165	740
07:15 AM	0	209	116	325	0	0	0	0	65	233	0	298	95	0	72	167	790
07:30 AM	0	210	93	303	0	0	0	0	81	231	0	312	124	2	160	286	901
07:45 AM	0	179	76	255	0	0	0	0	69	260	0	329	109	0	162	271	855
Total	0	840	386	1226	0	0	0	0	287	884	0	1171	424	2	463	889	3286
08:00 AM	0	242	51	293	0	0	0	0	67	245	0	312	162	0	170	332	937
08:15 AM	0	254	80	334	0	0	0	0	62	256	0	318	138	2	168	308	960
08:30 AM	0	213	67	280	0	0	0	0	62	249	0	311	153	0	172	325	916
08:45 AM	0	308	64	372	0	0	0	0	54	251	0	305	147	1	162	310	987
Total	0	1017	262	1279	0	0	0	0	245	1001	0	1246	600	3	672	1275	3800
09:00 AM	0	289	75	364	0	0	0	0	52	279	0	331	174	0	154	328	1023
09:15 AM	0	321	83	404	0	Ō	Ō	Ō	59	251	ō	310	163	ō	132	295	1009
09:30 AM	0	309	76	385	0	Ō	0	Ō	49	229	Ō	278	195	Ō	136	331	994
09:45 AM	0	291	88	379	0	0	0	0	44	245	0	289	194	0	119	313	981
Total	0	1210	322	1532	0	0	0	0	204	1004	0	1208	726	0	541	1267	4007
*** BREAK *** 03:00 PM	* 0	248	84	332	0	0	0	0	92	428	0	520	138	1	163	302	1154
03:15 PM	Ō	256	82	338	0	Ö	Ö	0	74	417	0	491	151	1	173	325	1154
03:30 PM	0	258	101	359	ő	Ö	Ö	ő	79	401	Ö	480	136	2	159	297	1136
03:45 PM	0	251	81	332	0	Ō	ō	ō	65	395	Ö	460	139	2	191	332	1124
Total	0	1013	348	1361	0	0	0	0	310	1641	0	1951	564	6	686	1256	4568
04:00 PM	0	212	76	288	0	0	0	0	78	428	0	506	147	4	174	325	1119
04:15 PM	0	244	57	301	0	0	0	0	83	426	0	509	134	3	171	308	1118
04:30 PM	0	220	74	294	0	0	0	0	62	397	0	459	125	2	165	292	1045
04:45 PM	0	256	83	339	0	0	0	0	77	413	0	490	149	2	167	318	1147
Total	0	932	290	1222	0	0	0	0	300	1664	0	1964	555	11	677	1243	4429
05:00 PM	0	256	83	339	0	0	0	0	61	487	0	548	141	2	163	306	1193
05:15 PM	0	215	67	282	0	0	0	0	59	415	0	474	151	1	191	343	1099
05:30 PM	0	168	46	214	0	0	0	0	68	409	0	477	155	1	169	325	1016
05:45 PM	0	229	57	286	0	0	0	0	49	402	0	451	170	4	163	337	1074
Total	0	868	253	1121	0	0	0	0	237	1713	0	1950	617	8	686	1311	4382
Grand Total	0	5880	1861	7741	0	0	0	0	1583	7907	0	9490	3486	30	3725	7241	24472
Apprch %	0.0	76.0	24.0		0.0	0.0	0.0		16.7	83.3	0.0		48.1	0.4	51.4		·
Total %	0.0	24.0	7.6	31.6	0.0	0.0	0.0	0.0	6.5	32.3	0.0	38.8	14.2	0.1	15.2	29.6	

	1		YS BLV	D.	US	101 SE	3 ON RAI	ИP	,	VAN NU	YS BLVD		US	101 SB	OFF R	AMP	
		South	nbound			West	lbound			North	nbound			Eastl	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From	m 07:00	0 AM to	09:45	AM - Pea	k 1 of 1									***************************************			
Intersection	08:45	AM						i									
Volume	0	1227	298	1525	0	0	0	0	214	1010	0	1224	679	1	584	1264	4013
Percent	0.0	80.5	19.5		0.0	0.0	0.0		17.5	82.5	0.0		53.7	0.1	46.2		
09:00 Volume	0	289	75	364	0	0	0	0	52	279	0	331	174	0	154	328	1023
Peak Factor												·					0.981
High Int.	09:15	AM			6:45:0	MA 0			09:00	AM			09:30	AM			
Volume	0	321	83	404	0	0	0	0	52	279	0	331	195	0	136	331	
Peak Factor				0.944								0.924				0.955	

File Name : 355503 Site Code : 00355503 Start Date : 11/17/2005

		-	
Page	No	;	2

	'		YS BLVI	D.	US		3 ON RA	MP	\		YS BLVI	D.	US		OFF RA	AMP	
		South	ibound			vves	luounu			NOIL	ibound				DOUING 1		T 1
				App.				App.	D'ala	TI	1 -64	App.	Diabt	Then	Left	App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Leit	Total	Total
Peak Hour Fro	m 03:0	PM to	05:45	PM - Pea	k 1 of 1											!	
Intersection	03:00	PM															
Volume	0	1013	348	1361	0	0	0	0	310	1641	0	1951	564	6	686	1256	4568
Percent	0.0	74.4	25.6		0.0	0.0	0.0		15.9	84.1	0.0		44.9	0.5	54.6		
03:15	0	256	82	338	0	n	0	0	74	417	0	491	151	1	173	325	1154
Volume	U	250	02	550		U	•	•			_						
Peak Factor																	0.990
High Int.	03:30	PM							03:00	PM			03:45	PM			
Volume	0	258	101	359	0	0	0	0	92	428	0	520	139	2	191	332	
Peak Factor	Ü	2.50	101	0.948		Ū		_			_	0.938				0.946	
i can i actor				0.5 10	I				1								

File Name : 355505 Site Code : 00355505 Start Date : 11/17/2005 Page No : 1

						_									Page	NO :	1
	BF	VERI Y	GLEN B	IVD	l		roups Pri PARK ST		rning Mo		t NE AVE.		1	MOORI	PARK ST.		1
***			hbound				tbound				hbound				bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int Tota
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	2	1	0	3	0	82	98	180	14	1	5	20	10	17	0	27	230
07:15 AM	2	3	2	7	0	137	84	221	32	1	7	40	19	37	0	56	324
07:30 AM	2	8	1	11	2	170	76	248	67	1	7	75	10	55	1	66	400
07:45 AM	<u>3</u>	8	3_	14	2	172	97	271	52	3	9	64	10	67	2	79	428
Total	9	20	6	35	4	561	355	920	165	6	28	199	49	176	3	228	1382
MA 00:80	3	11	2	16	2	185	66	253	57	0	7	64	9	76	0	85	41
08:15 AM	2	4	2	8	1	203	47	251	54	1	10	65	7	75	0	82	400
08:30 AM	4	4	0	8	3	167	49	219	51	4	11	66	16	72	1	89	38:
08:45 AM	2	5	4	11	0	150	68	218	67	2	12	81	3	91	2	96	406
Total	11	24	8	43	6	705	230	941	229	7	40	276	35	314	3	352	1612
09:00 AM	2	3	1	6	2	139	85	226	60	1	11	72	13	115	0	128	432
09:15 AM	0	8	1	9	3	116	105	224	57	2	10	69	17	83	1	101	403
09:30 AM	0	2	2	4	2	110	96	208	66	3	7	76	12	69	1	82	370
09:45 AM	1	5_	1	7_	2	86	100	188	60	9	13	82	9	64	0	73	35
Total	3	18	5	26	9	451	386	846	243	15	41	299	51	331	2	384	155
** BREAK **	*																
03:00 PM	8	4	10	22	5	92	81	178	145	4	36	185	16	96	0	112	497
03:15 PM	4	9	4	17	5	106	72	183	157	8	27	192	15	115	0	130	522
03:30 PM	0	3	2	5	2	111	85	198	144	7	36	187	18	132	1	151	54:
03:45 PM	6	7	5	18	1	116	51	168	156	4	33	193	20	112	1	133	512
Total	18	23	21	62	13	425	289	727	602	23	132	757	69	455	2	526	2072
04:00 PM	1	11	7	19	3	103	80	186	147	5	31	183	14	118	1	133	521
04:15 PM	2	12	5	19	3	113	75	191	159	11	28	198	9	123	Ō	132	540
04:30 PM	2	8	1	11	2	121	64	187	172	5	35	212	11	115	Ö	126	536
04:45 PM	5	6	4	15	3	128	77	208	167	6	34	207	10	138	1	149	579
Total	10	37	17	64	11	465	296	772	645	27	128	800	44	494	2	540	2176
05:00 PM	3	7	4	14	2	126	78	206	144	9	40	193	19	122	1	142	555
05:15 PM	4	9	4	17	4	151	81	236	153	11	30	194	6	136	1	143	590
05:30 PM	5	6	7	18	2	158	106	266	146	9	32	187	8	100	1	109	580
05:45 PM	4	12	7	23	8	149	106	263	145	4	44	193	10	123	1	134	613
Total	16	34	22	72	16	584	371	971	588	33	146	767	43	481	4	528	2338
Grand Total	67	156	79	302	59	3191	1927	5177	2472	111	515	3098	291	2251	16	2558	11135
Apprch %	22.2	51.7	26.2		1.1	61.6	37.2		79.8	3.6	16.6		11.4	88.0	0.6	ŀ	
Total %	0.6	1.4	0.7	2.7	0.5	28.7	17.3	46.5	22.2	1.0	4.6	27.8	2.6	20.2	0.1	23.0	
·	500	/EDLY :	OI EN E	11.75	· ·												
	DE/		GLEN BI	LVD.			PARK ST				VE AVE.	1			ARK ST.		
						* * * * * * * * * * * * * * * * * * * *	- Vuilu			INCHES	11/11/11/11			7.510	45 21 21 15 1		

		BE'		GLEN B	LVD.			PARK ST tbound	Γ.			NE AVE. ibound				PARK ST bound		
	Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Pe	ak Hour Fro	m 07:0	O AM to	09:45	AM - Pea	k 1 of 1		L		·								
I	ntersection	07:30	AM															
	Volume	10	31	8	49	7	730	286	1023	230	5	33	268	36	273	3	312	1652
	Percent	20.4	63.3	16.3		0.7	71.4	28.0		85.8	1.9	12.3		11.5	87.5	1.0		
	07:45 Volume	3	8	3	14	2	172	97	271	52	3	9	64	10	67	2	79	428
F	Peak Factor																	0.965
	High Int.	08:00	AM			07:45	AM			07:30	AM			08:00	AM			
	Volume	3	11	2	16	2	172	97	271	67	1	7	75	9	76	0	85	
F	Peak Factor				0.766				0.944				0.893				0.918	

File Name : 355505 Site Code : 00355505 Start Date : 11/17/2005 Page No : 2

	BE		GLEN B nbound	LVD.			PARK ST	Γ.			NE AVE	•			ARK ST	Γ.	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00	9 PM to	05:45	PM - Pea	k 1 of 1				·				·			- I Otal	rotur
Intersection	05:00	PM															
Volume	16	34	22	72	16	584	371	971	588	33	146	767	43	481	4	528	2338
Percent	22.2	47.2	30.6		1.6	60.1	38.2		76.7	4.3	19.0		8.1	91.1	0.8		
05:45 Volume	4	12	7	23	8	149	106	263	145	4	44	193	10	123	1	134	613
Peak Factor																	0.054
-	05:45				05:30	PM			05:15	PM			05:15	PM			0.954
Volume	4	12	7	23	2	158	106	266	153	11	30	194	6	136	1	143	
Peak Factor				0.783				0.913				0.988				0.923	

Groups Printed- Turning Movement

File Name : 355504 Site Code : 00355504 Start Date : 11/17/2005 Page No : 1

	BEV	VERLY C	GLEN BL	VD.	,		RA BLVD				GLEN BL	.VD.			RA BLVE	Э.	
		South	bound			West	bound			North	bound			East	bound		TL
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	39	65	2	106	1	198	21	220	3	17	15	35	50	173	8	231	592
07:15 AM	41	59	8	108	1	273	22	296	8	29	29	66	31	209	14	254	724
07:30 AM	35	54	5	94	2	292	12	306	12	44	18	74	38	327	18	383	857
07:45 AM	42	68	2	112	1	276	19	296	19	34	21	74	29	259	20	308	790
Total	157	246	17	420	5	1039	74	1118	42	124	83	249	148	968	60	1176	2963
MA 00:80	36	42	3	81	1	293	14	308	24	43	19	86	26	282	19	327	802
08:15 AM	25	34	1	60	5	241	27	273	13	36	26	75	21	280	19	320	728
08:30 AM	27	36	2	65	0	276	11	287	13	49	17	79	11	287	23	321	752
08:45 AM	20	55	1	76	1	220	25	246	19	49	23	91	30	253	18	301	714
Total	108	167	7	282	7	1030	77	1114	69	177	85	331	88	1102	79	1269	2996
09:00 AM	33	64	2	99	6	210	31	247	19	47	12	78	35	273	14	322	746
09:15 AM	31	89	4	124	3	234	40	277	30	36	18	84	31	261	12	304	789
09:30 AM	18	86	6	110	6	209	41	256	29	43	19	91	35	240	20	295	752
09:45 AM	25	71	11	107	0	159	36	195	21	48	27	96	31	248	20	299	697
Total	107	310	23	440	15	812	148	975	99	174	76	349	132	1022	66	1220	2984
*** BREAK **	*																
03:00 PM	22	66	10	98	5	239	31	275	37	94	52	183	16	237	23	276	832
03:15 PM	15	65	11	91	13	242	29	284	24	127	52	203	20	258	26	304	882
03:30 PM	9	89	7	105	8	267	30	305	30	102	53	185	9	290	23	322	917
03:45 PM	12	61	9	82	8	282	28	318	31	137	56	224	16	271	18	305	929
Total	58	281	37	376	34	1030	118	1182	122	460	213	795	61	1056	90	1207	3560
04:00 PM	18	80	9	107	7	284	39	330	38	120	51	209	14	266	28	308	954
04:15 PM	13	77	11	101	6	251	32	289	33	134	37	204	15	248	25	288	882
04:30 PM	11	68	6	85	6	264	41	311	30	145	39	214	11	272	34	317	927
04:45 PM	15	72	10	97	2	242	29	273	35	124	46_	205	15	256	28	299	874
Total	57	297	36	390	21	1041	141	1203	136	523	173	832	55	1042	115	1212	3637
05:00 PM	21	74	8	103	1	250	38	296	35	102	47	184	12	245	35	292	875
05:15 PM	23	72	4	99	9	254	46	309	46	106	35	187	18	267	43	328	923
05:30 PM	36	83	3	122	3	223	44	270	41	118	46	205	16	257	20	293	890
05:45 PM	42	82	4	128	8	237	29	274	32	112	42	186	8	269	28_	305	893
Total	122	311	19	452	28	964	157	1149	154	438	170	762	54	1038	126	1218	3581
Grand Total	609	1612	139	2360	110	5916	715	6741	622	1896	800	3318	538	6228	536	7302	19721
Apprch %	25.8	68.3	5.9		1.6	87.8	10.6		18.7	57.1	24.1		7.4	85.3	7.3		
Total %	3.1	8.2	0.7	12.0	0.6	30.0	3.6	34.2	3.2	9.6	4.1	16.8	2.7	31.6	2.7	37.0	

	BE		GLEN BI	LVD.	,		RA BLV).	BEV		GLEN BL	.VD.	-		RA BLVI	D.	
1		Souti	hbound		ļ.	Wes	tbound			Nortr	bound			East	bound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
July 1 mile	ragine	111114		Total	ingine	11114		Total				Total				Total	Total
Peak Hour Fro	m 07:0	0 AM to	09:45	AM - Pea	k 1 of 1								1				ı
Intersection	07:30	AM															
Volume	138	198	11	347	9	1102	72	1183	68	157	84	309	114	1148	76	1338	3177
Percent	39.8	57.1	3.2		0.8	93.2	6.1		22.0	50.8	27.2		8.5	85.8	5.7		
07:30	35	54	5	94	,	292	12	306	12	44	18	74	38	327	18	383	857
Volume	JJ	JΤ	ر	דכ	4	232	14	200	12		10	, ,	30	J.,			
Peak Factor																	0.927
High Int.	07:45	AM			08:00	AM			08:00	AM			07:30	AM			
Volume	42	68	2	112	1	293	14	308	24	43	19	86	38	327	18	383	
Peak Factor				0.775				0.960				0.898				0.873	

File Name : 355504 Site Code : 00355504 Start Date : 11/17/2005 Page No : 2

	BEV		GLEN Bi	_VD.			RA BLVE).	BEV		GLEN B	LVD.	,		RA BLVE bound).	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00) PM to	05:45	PM - Pea	k 1 of 1											1	
Intersection Volume Percent	03:45 54 14.4	PM 286 76.3	35 9.3	375	27 2.2	1081 86.6	140 11.2	1248	132 15.5	536 63.0	183 21.5	851	56 4.6	1057 86.8	105 8.6	1218	3692
04:00 Volume	18	80	9	107	7	284	39	330	38	120	51	209	14	266	28	308	954
Peak Factor High Int. Volume Peak Factor	04:00 18	PM 80	9	107 0.876	04:00 7	PM 284	39	330 0.945	03:45 31	PM 137	56	224 0.950	04:30 11	PM 272	34	317 0.961	0.968

File Name : 355506 Site Code : 00355506 Start Date : 11/17/2005 Page No : 1

						_	_								Page	No :	1
		HAZELT	INE AVE	=	1		roups Pr LIA BLV	inted- Tu				_		MACNO	1 1 A DI \ '	<u> </u>	1
			nbound	Ξ.	'		LIA BLV tbound	D.			TINE AVI	Ξ.	'		LIA BLV bound	D.	
G:				App.		i .	T T	App.			T T	Арр.		Lasi	bouria	App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0	1000	1.0	1.0	1.0	Total	1.0	1.0	1.0	10101	10101
07:00 AM	18	169	14	201	9	125	28	162	8	64	11	83	12	83	11	106	552
07:15 AM	16	178	27	221	17	178	24	219	10	71	16	97	15	119	6	140	677
07:30 AM	23	160	34	217	24	238	35	297	14	105	24	143	27	176	12	215	872
07:45 AM	20	213	32	265	36	238	33	307	18	120	24	162	31	207	20	258	992
Total	77	720	107	904	86	779	120	985	50	360	75	485	85	585	49	719	3093
					, 50			300	, 55	500	, ,	.03	, 05	303	,,,	, 13	, 3033
08:00 AM	22	212	31	265	34	231	46	311	13	107	12	132	43	194	15	252	960
08:15 AM	24	197	24	245	19	220	27	266	15	92	15	122	30	213	8	251	884
08:30 AM	21	208	17	246	9	158	23	190	19	89	12	120	25	181	7	213	769
08:45 AM	33	202	25	260	12	170	21	203	12	81	20	113	34	151	14	199	775
Total	100	819	97	1016	74	779	117	970	59	369	59	487	132	739	44	915	3388
					, , ,			3,0	, 55	303	33	107	152	,55		713	3300
09:00 AM	19	225	20	264	15	128	25	168	14	80	22	116	29	170	12	211	759
09:15 AM	22	201	20	243	10	112	26	148	14	95	17	126	25	142	10	177	694
09:30 AM	20	178	21	219	17	85	24	126	13	92	20	125	32	121	10	163	633
09:45 AM	18	179	14	211	10	99	18	127	17	78	17	112	26	91	7	124	574
Total	79	783	75	937	52	424	93	569	58	345	76	479	112	524	39	675	2660
*** BREAK **	*																
03:00 PM	15	154	26	195	16	111	25	152	36	178	31	245	40	186	16	242	834
03:15 PM	11	153	21	185	28	158	37	223	34	142	35	211	29	190	26	245	864
03:30 PM	13	162	19	194	15	157	31	203	29	195	18	242	43	172	26	241	880
03:45 PM	21	176	14	211	17	129	22	168	32	183	45	260	41	164	15	220	859
Total	60	645	80	785	76	555	115	746	131	698	129	958	153	712	83	948	3437
04:00 PM	25	142	18	185	21	126	70	176	27	105	42	264	40	100	4.0	acel	000
04:15 PM	18	168	25	211	26	140	29 22	188	27 29	195	42	264	48	189	18	255	880
04:30 PM	17	185	16	218	17	144	26	187	29	197 208	31 39	257 276	44 43	197 218	22 23	263 284	919 965
04:45 PM	18	185	21	224	19	132	30	181	30	196	39 37	263	43 47	207	23 24	278	965 946
Total	78	680	80	838	83	542	<u></u>	732	115	796	149	1060	182	811	87	1080	3710
10141	, 0	000	00	050	00	J72	107	/32	113	790	149	1000	102	011	07	1000	3/10
05:00 PM	19	158	26	203	12	118	24	154	32	214	31	277	50	209	23	282	916
05:15 PM	13	153	26	192	14	125	24	163	34	184	34	252	59	275	25	359	966
05:30 PM	19	163	19	201	15	126	27	168	35	219	39	293	52	242	31	325	987
05:45 PM	15	158	24	197	16	123	23	162	40	208	31	279	69	241	28	338	976
Total	66	632	95	793	57	492	98	647	141	825	135	1101	230	967	107	1304	3845
Grand Total	460	4279	534	E272	420	2574	CEO	4640	r- 4	2202	633	4570	00.4	4220	400	FC 44 1	20122
Apprch %	8.7	81.1	534 10.1	5273	428	3571	650	4649	554	3393	623	4570	894	4338	409	5641	20133
Total %	2.3	21.3	2.7	26.2	9.2 2.1	76.8 17.7	14.0	77.4	12.1	74.2	13.6	77.7	15.8	76.9	7.3	20.0	
10(a) 70	2.3	21.3	2./	20.2	2.1	1/./	3.2	23.1	2.8	16.9	3.1	22.7	4.4	21.5	2.0	28.0	

			INE AV	Ε.	N	MAGNO	LIA BLV	D.		HAZELT	INE AVE	=.	١	/AGNO	LIA BLV	D.	
		South	bound			Wes	tbound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:00	O AM to	09:45	AM - Pea	k 1 of 1							,	<u></u>				
Intersection	07:30	AM															
Volume	89	782	121	992	113	927	141	1181	60	424	75	559	131	790	55	976	3708
Percent	9.0	78.8	12.2		9.6	78.5	11.9		10.7	75.8	13.4		13.4	80.9	5.6		
07:45 Volume	20	213	32	265	36	238	33	307	18	120	24	162	31	207	20	258	992
Peak Factor																	0.934
High Int.	07:45	AM			08:00	AM			07:45	AM			07:45	AM			
Volume	20	213	32	265	34	231	46	311	18	120	24	162	31	207	20	258	
Peak Factor				0.936				0.949				0.863				0.946	

File Name : 355506 Site Code : 00355506 Start Date : 11/17/2005 Page No : 2

	1		INE AVE	-	1		LIA BLV	D.	ŀ		INE AVE	Ξ.	N		LIA BLV	D.	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Froi	m 03:00) PM to	05:45 F		k 1 of 1			Total				10(0)	L			TOtal	Total
Intersection	05:00																
Volume	66	632	95	793	57	492	98	647	141	825	135	1101	230	967	107	1304	3845
Percent	8.3	79.7	12.0		8.8	76.0	15.1		12.8	74.9	12.3		17.6	74.2	8.2	100.	00.0
05:30	19	163	10	201		126	27	4.50	25	240	20	202					
Volume	19	102	19	201	15	126	27	168	35	219	39	293	52	242	31	325	987
Peak Factor																	0.974
High Int.	05:00	PM			05:30	PM			05:30	PM			05:15	РМ			0.57
Volume	19	158	26	203	15	126	27	168	35	219	39	293	59	275	25	359	
Peak Factor				0.977				0.963				0.939	-			0.908	

File Name : 355507 Site Code : 00355507 Start Date : 11/16/2005 Page No : 1

						C.	D-	inted To	ania - NA		L				Page	No :	1
		HAZELT	INE AV	E.	F	RIVERSI		inted- Tu VE	rning ivid		I TINE AV	E.	T	RIVERSI	DE DRI	/F	٦
		South	bound			Wes	tbound				hbound				bound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
Factor	1.0	1.0	1.0	Total	1.0			Total	_			Total				Total	Total
07:00 AM	9	149	1.0	176	1.0	1.0 125	1.0 51	182	1.0	1.0 37	1.0	66	1.0	1.0 36	1.0	40	472
07:15 AM	22	162	38	222	19	161	75	255	20	57	9	86	7	122	3	48 132	695
07:30 AM	18	180	34	232	40	204	79	323	40	79	17	136	10	166	6	182	873
07:45 AM	16	214	60	290	27	236	85	348	27	79	15	121	18	195	7	220	979
Total	65	705	150	920		726	290	1108	104	252	53	409	43	519	20	582	
08:00 AM	17	241	58	316	32	216	77	325	28	93	14	135	24	194	2	220	996
08:15 AM	13	192	45	250	34	156	90	280	21	98	13	132	11	157	8	176	838
08:30 AM	21	180	51	252	20	135	74	229	20	85	9	114	17	189	5	211	806
08:45 AM	20	174	45	239	23	144	57	224	33	82	12	127	18	190	3	211	801
Total	71	787	199	1057	109	651	298	1058	102	358	48	508	70	730	18	818	
09:00 AM	21	205	54	280	25	142	58	225	19	83	10	112	10	181	2	193	810
09:15 AM	16	178	48	242	21	144	51	216	26	87	17	130	18	145	11	174	762
09:30 AM	21	167	57	245	28	107	55	190	24	77	18	119	25	141	4	170	724
09:45 AM	15	173	52	240	21	93	40	154	30	70_	17	117	23	120	12	155	666
Total	73	723	211	1007	95	486	204	785	99	317	62	478	76	587	29	692	2962
*** BREAK **	*																
03:00 PM	38	173	27	238	36	125	44	205	85	158	29	272	26	132	17	175	890
03:15 PM	27	172	21	220	29	164	70	263	70	172	39	281	28	125	14	167	931
03:30 PM	39	147	24	210	34	138	62	234	67	150	42	259	27	141	15	183	886
03:45 PM	23	140	37	200	43	108	60	211	64	165	43	272	21	150	12	183	866
Total	127	632	109	868	142	535	236	913	286	645	153	1084	102	548	58	708	3573
04:00 PM	28	155	44	227	38	117	43	198	57	183	35	275	26	130	20	176	876
04:15 PM	22	176	41	239	49	128	42	219	43	173	40	256	20	133	29	182	896
04:30 PM	23	163	43	229	43	119	45	207	65	186	38	289	26	146	19	191	916
04:45 PM	27	194	40	261	36	125	56	217	68	198	41	307	22	128	27	177	962
Total	100	688	168	956	166	489	186	841	233	740	154	1127	94	537	95	726	3650
05:00 PM	19	179	36	234	51	137	44	232	65	193	34	292	34	156	21	211	969
05:15 PM	29	193	35	257	45	136	55	236	53	212	32	297	23	147	25	195	985
05:30 PM	18	180	43	241	39	145	52	236	62	204	33	299	28	146	20	194	970
05:45 PM Total	23 89	212 764	48 162	283 1015	37 172	146 564	69 220	252 956	59 239	203 812	32 131	294	26 111	<u>138</u> 	22 88	186	1015 3939
											131	1182			88	786	
Grand Total	525	4299	999	5823		3451		5661	1063		601	4788	496		308	4312	20584
Apprch % Total %	9.0 2.6	73.8 20.9	17.2	20.2	13.7	61.0	25.3	27.5	22.2	65.2	12.6		11.5	81.4	7.1	20.0	
10(4) 70	2.0	20.9	4.9	28.3	3.8	16.8	7.0	27.5	5.2	15.2	2.9	23.3	2.4	17.0	1.5	20.9	
		HAZELT	INE AVE	Ξ. Ι	R	IVERSI	DE DRIV	/Ε	1	IAZFI T	INE AVE	: 1	R	IVERSI	DE DRIV	E	
	HAZELTINE AVE. Southbound						oound		·		bound				ound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From			09:45		k 1 of 1		L	Total	1			rotar i				Total	Total
Intersection			467	1000	4-5-	045											
Volume	64	827	197	1088	133	812	331	1276	116	349	59	524	63	712	23	798	3686
Percent 08:00	5.9	76.0	18.1		10.4	63.6	25.9		22.1	66.6	11.3		7.9	89.2	2.9		
Volume	17	241	58	316	32	216	77	325	28	93	14	135	24	194	2	220	996
Peak Factor																	0.925
	08:00	AM			07:45	AM			07:30	AM			07:45	ΔM			0.525
Volume	17	241	58	316	27	236	85	348	40	79	17	136	18	195	7	220	
Peak Factor				0.861				0.917				0.963	•	-	•	0.907	

File Name : 355507 Site Code : 00355507 Start Date : 11/16/2005 Page No : 2

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			INE AV	Ē.	F		DE DRIV	/E	i		INE AVI	Ξ.	F		DE DRIV	/E	
		South	nbound			vves	tbound			North	nbound			⊨ast	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Froi	m 03:0	D PM to	05:45		k 1 of 1			rotai			<u> </u>	10(0)	!			rotai	Total
Intersection	05:00	PM															
Volume	89	764	162	1015	172	564	220	956	239	812	131	1182	111	587	88	786	3939
Percent	8.8	75.3	16.0		18.0	59.0	23.0		20.2	68.7	11.1		14.1	74.7	11.2		
05:45	23	212	48	283	37	146	69	252	59	203	32	294	26	138	22	186	1015
Volume					υ,		0,5			200	32	2.5		150		100	1015
Peak Factor																	0.970
High Int.	05:45	05:45 PM				PM			05:30	PM			05:00	PM			
Volume	23	212	48	283	37	146	69	252	62	204	33	299	34	156	21	211	
Peak Factor				0.897				0.948				0.988				0.931	

Groups Printed-Turning Movement

File Name : 355508 Site Code : 00355508 Start Date : 11/16/2005

Page No : 1

			INE AV	E.	Fa	shion S	roups Pr Square Li tbound	inted- Lu ane	rning ivid	VENTU	t RA BLVE hbound	Э.			-Entrance	!	
Ctart Time	Diebt		1	App.	D!-LL			App.	Di-L-			App.	Dielek	T		App.	Int.
Start Time Factor	Right 1.0	Thru 1.0	Left 1.0	Total	Right	Thru	Left 1.0	Total	Right	Thru	Left	Total	Right	Thru	Left 1.0	Total	Total
07:00 AM	2	190	1.0	205	1.0	1.0	0	0	1.0	1.0 67	1.0	73	1.0	1.0	0	0	278
07:00 AM 07:15 AM	4	222	4	230	0	1	0	1	4	83	2	89	1	0	1	2	322
07:30 AM	3	240	10	253	0	ō	0	ō	6	129	6	141	0	0	1	1	395
07:45 AM	0	302	12	314	ő	0	0	0	3	118	3	124	2	0	Ô	2	440
Total	9	954	39	1002	0	1	0	1		397	13	427	3	0	2	5	1435
08:00 AM	2	316	15	333	1	0	1	2	5	129	0	134	4	0	0	4	473
08:15 AM	2	277	5	284	1	0	0	1	8	127	3	138	1	0	1	2	425
08:30 AM	1	252	12	265	1	0	1	2	3	113	2	118	0	0	1	1	386
08:45 AM	2	220	13	235	1	0	1	2	5	125	2	132	1	0	3	4	373
Total	7	1065	45	1117	4	0	3	7	21	494	7	522	6	0	5	11	1657
09:00 AM	2	241	12	255	з	0	1	4	4	98	0	102	3	2	1	6	367
09:15 AM	0	196	24	220	3	0	3	6	8	117	1	126	0	1	1	2	354
09:30 AM	1	188	46	235	2	0	3	5	8	103	5	116	1	0	1	2	358
09:45 AM	3	189	42	234	3	0	0	3	14	113	1	128	0	0	1	1	366
Total	6	814	124	944	11	0	7	18	34	431	7	472	4	3	4	11	1445
*** BREAK **	*																
03:00 PM	1	169	67	237	24	1	12	37	19	211	1	231	0	0	1	1	506
03:15 PM	0	212	41	253	19	1	24	44	25	201	6	232	3	Ō	3	6	535
03:30 PM	0	181	39	220	20	0	30	50	20	207	1	228	1	0	3	4	502
03:45 PM	0	137	59	196	10	0	16	26	23	223	0	246	4	0	2	6	474
Total	1	699	206	906	73	2	82	157	87	842	8	937	8	0	9	17	2017
04:00 PM	2	194	32	228	19	3	19	41	24	218	0	242	2	0	1	3	514
04:15 PM	2	203	42	247	12	0	16	28	20	211	3	234	3	1	7	11	520
04:30 PM 04:45 PM	0	184	49	233	16	0	19	35	19	210	0	229	3	0	4	7	504
Total	<u>0</u>	225 806	38 161	263 971	16 63	<u>0</u> 3	20 74	36 140	22 85	232 871	<u>1</u> 4	255 960	1 9	0 1	8 20	9 30	563 2101
	7		101	9/1	. 03	3	/4	140	. 65	0/1	-1	900	9	1	20	30	2101
05:00 PM	1	207	41	249	14	0	24	38	20	243	2	265	2	0	2	4	556
05:15 PM	3	209	52	264	10	0	16	26	20	246	4	270	2	0	0	2	562
05:30 PM	0	185	47	232	11	0	13	24	9	255	1	265	3	1	3	7	528
05:45 PM		243	58	302	16	1	20	37	18	230	4	252	2	0	1	3	594
Total	5	844	198	1047	51	1	73	125	67	974	11	1052	9	1	6	16	2240
Grand Total		5182	773	5987	202	7	239	448	311	4009	50	4370	39	5	46	90	10895
Apprch %	0.5			rr 0	45.1	1.6		4 4	7.1	91.7	1.1	40.4	43.3	5.6			
Total %	0.3	47.6	7.1	55.0	1.9	0.1	2.2	4.1	2.9	36.8	0.5	40.1	0.4	0.0	0.4	0.8	
		HAZELT	INE AVI	<u> </u>	Fa	shion S	quare La	ne		VENTU	RA BLVD	l		P.LOT-	Entrance		
			bound				bound				bound				bound		-
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Froi			09:45	AM - Pea	k 1 of 1		,						,				
Intersection				=	_											_	
Volume		1135	42	1184	2	0	1	3	22	503	12	537	7	0	2	9	1733
Percent	0.6	95.9	3.5		66.7	0.0	33.3		4.1	93.7	2.2		77.8	0.0	22.2		
08:00	2	316	15	333	1	0	1	2	5	129	0	134	4	0	0	4	473
Volume Peak Factor																	0.916
High Int.	08:00	ΔМ			08:00	ΛM			07:30	ΛM			00.00	۸۸۸			0.310
Volume	2	AM 316	15	333	1	A™ 0	1	2	6	129	6	141	08:00 4	ΑΙΨΙ 0	0	4	
Peak Factor	4	210	1.0	0.889		U	1	0.375		123	U	0.952		U	U	0.563	
i cun i accol				0.009	I			0.5/5	I			0.252	1			0.505	

File Name : 355508 Site Code : 00355508 Start Date : 11/16/2005 Page No : 2

	l		INE AV	E.	Fa		quare La	ane	,		RA BLVE).			Entrance bound	•	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00) PM to	05:45	PM - Pea	k 1 of 1												
Intersection	05:00	PM															
Volume	5	844	198	1047	51	1	73	125	67	974	11	1052	9	1	6	16	2240
Percent	0.5	80.6	18.9		40.8	0.8	58.4		6.4	92.6	1.0		56.3	6.3	37.5		
05:45 Volume	1	243	58	302	16	1	20	37	18	230	4	252	2	0	1	3	594
Peak Factor																	0.943
High Int.	05:45	05:45 PM			05:00	PM			05:15	PM			05:30	PM			
Volume	1	243	58	302	14	0	24	38	20	246	4	270	3	1	3	7	
Peak Factor				0.867				0.822				0.974				0.571	

Groups Printed- Turning Movement

File Name : 355509 Site Code : 00355509 Start Date : 11/15/2005 Page No : 1

		HAZELT		Ε.	P	MOORP/		Έ.			INE AVE	Ξ.		MOORP	ARK AV	E.	
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	*****	1.0	1.0	1.0		
07:00 AM	80	104	16	200	14	129	22	165	2	24	6	32	7	42	21	70	467
07:15 AM	75	122	20	217	11	156	20	187	5	32	8	45	4	53	19	76	525
07:30 AM	64	177	22	263	18	179	16	213	3	46	6	55	16	84	27	127	658
07:45 AM	90	201	39	330	33	169	19	221	2	66	5	73	10	95	23	128	752
Total	309	604	97	1010	76	633	77	786	12	168	25	205	37	274	90	401	2402
MA 00:80	83	239	35	357	16	172	28	216	8	44	10	62	13	83	21	117	752
08:15 AM	84	213	42	339	18	191	18	227	7	49	15	71	6	98	20	124	761
08:30 AM	79	216	45	340	31	152	18	201	8	48	11	67	21	101	25	147	755
08:45 AM	86	207	41	334	18	150	31	199	7	57	10	74	19	102	24	145	752
Total	332	875	163	1370	83	665	95	843	30	198	46	274	59	384	90	533	3020
09:00 AM	70	153	39	262	20	157	14	191	6	49	16	71	15	100	24	139	663
09:15 AM	74	128	53	255	22	149	20	191	5	49	14	68	21	78	23	122	636
09:30 AM	75	92	37	204	15	133	17	165	7	58	12	77	20	79	26	125	571
09:45 AM	57	88	36	181	24	109	13	146	8	69	18	95	13	101	34	148	570
Total	276	461	165	902	81	548	64	693	26	225	60	311	69	358	107	534	2440
*** BREAK **																	
03:00 PM	55	74	36	165	23	95	14	132	6	96	15	117	29	133	69	231	645
03:15 PM	55	99	32	186	30	108	17	155	13	97	30	140	22	134	61	217	698
03:30 PM	70	93	35	198	29	102	22	153	13	108	22	143	14	161	58	233	727
03:45 PM	77	92	36	205	32	126	15	173	7	100	18	125	16	149	63	228	731
Total	257	358	139	754	114	431	68	613	39	401	85	525	81	577	251	909	2801
04:00 PM	53	89	43	185	27	110	18	155	15	105	24	144	15	157	70	242	726
04:15 PM	54	122	46	222	36	112	16	164	15	114	27	156	14	179	64	257	799
04:30 PM	64	91	37	192	37	133	14	184	9	106	35	150	17	161	76	254	780
04:45 PM	81	98	42	221	32	132	11	175	13	97	25	135	20	175	74_	269	800
Total	252	400	168	820	132	487	59	678	52	422	111	585	66	672	28 4	1022	3105
05:00 PM	68	92	33	193	25	114	25	164	16	128	23	167	29	138	72	239	763
05:15 PM	74	117	41	232	39	135	18	192	10	107	34	151	21	178	64	263	838
05:30 PM	78	106	43	227	42	122	19	183	11	129	12	152	19	152	81	252	814
05:45 PM	69	102	43	214	46	132	28	206	6	92	35	133	13	152	72	237	790
Total	289	417	160	866	152	503	90	745	43	456	104	603	82	620	289	991	3205
Grand Total	1715	3115	892	5722	638	3267	453	4358	202	1870	431	2503	394	2885	1111	4390	16973
Apprch %	30.0	54.4	15.6		14.6	75.0	10.4		8.1	74.7	17.2		9.0	65.7	25.3		
Total %	10.1	18.4	5.3	33.7	3.8	19.2	2.7	25.7	1.2	11.0	2.5	14.7	2.3	17.0	6.5	25.9	

		HAZEL1	INE AV	E.	V	JOORP	ARK AVE	Ξ.	l	HAZELT	INE AVE	•	N	MOORP.	ARK AV	E.	
		Souti	nbound			Wes	tbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
				Total	, Kigilic	11110	LCIT	Total	i vigile	mu	LCIT	Total	Right	, iii u	LCIC	Total	Total
Peak Hour Fro	m 07:00	AM to	09:45	AM - Pea	k 1 of 1	_				-							
Intersection	07:45	AM															
Volume	336	869	161	1366	98	684	83	865	25	207	41	273	50	377	89	516	3020
Percent	24.6	63.6	11.8		11.3	79.1	9.6		9.2	75.8	15.0		9.7	73.1	17.2		
08:15	84	213	42	339	18	191	18	227	7	49	15	71	6	98	20	124	761
Volume	07	213	72	333	10	171	10	221	,	45	15	/ 1	"	90	20	147	701
Peak Factor																	0.992
High Int.	08:00	08:00 AM			08:15	AM			07:45	AM			08:30	AM			
Volume	83	239	35	357	18	191	18	227	2	66	5	73	21	101	25	147	
Peak Factor				0.957				0.953				0.935				0.878	

File Name : 355509 Site Code : 00355509 Start Date : 11/15/2005 Page No : 2

			TINE AV	E.	l		ARK AV lbound	E.	I		INE AV	E.	١		ARK AV	Έ.	
Start Time			Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:0	0 PM to	05:45	PM - Pea	k 1 of 1							***************************************	h	I			
Intersection	04:45	PM															
Volume	301	413	159	873	138	503	73	714	50	461	94	605	89	643	291	1023	3215
Percent	34.5	47.3	18.2		19.3	70.4	10.2		8.3	76.2	15.5		8.7	62.9	28.4		
05:15 Volume	74	117	41	232	39	135	18	192	10	107	34	151	21	178	64	263	838
Peak Factor																	0.959
High Int.	05:15 PM				05:15	PM			05:00	PM			04:45	PM			0.555
Volume	74	117	41	232	39	135	18	192	16	128	23	167	20	175	74	269	
Peak Factor				0.941				0.930				0.906				0.951	

File Name : 355510 Site Code : 00355510 Start Date : 11/15/2005 Page No : 1

							_	–							Page	No :	7
	I	HAZEL	TINE AVE	Ξ.	<u> </u>		roups Pr IRA AVE	inted- Tu	rning Mo	vement			I	VENTI	JRA AVE		1
		Sout	hbound				tbound	-		North	bound				tbound	•	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	In Tot
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	, 0	1.0	1.0	1.0	rotar	100
07:00 AM	72	0	29	101	15	267	0	282	0	0	0	0	0	232	14	246	62
07:15 AM	97	0	36	133	20	312	0	332	0	0	0	0	0	267	20	287	75
07:30 AM	99	0	63	162	28	340	0	368	0	0	0	0	0	247	26	273	80
07:45 AM	123	0	75	198	34	331	0	365	0	0	0	0	0	265	27	292	85
Total	391	0	203	594	97	1250	0	1347	0	0	0	0	0	1011	87	1098	303
MA 00:80	136	0	83	219	26	303	0	329	0	0	0	0	l o	229	21	250	79
08:15 AM	123	0	72	195	27	344	0	371	0	0	0	0	0	271	25	296	86
08:30 AM	111	0	80	191	31	326	0	357	0	0	0	0	0	294	19	313	86
08:45 AM	127	0	79	206	32	273	0	305	0	0	0	0	0	233	22	255	76
Total	497	0	314	811	116	1246	0	1362	0	0	0	0	0	1027	87	1114	328
09:00 AM	91	0	57	148	30	281	0	311	0	0	0	0	0	268	23	291	75
09:15 AM	83	0	55	138	28	291	0	319	0	0	0	0	0	307	24	331	78
09:30 AM	73	0	39	112	42	296	0	338	0	0	0	0	0	293	26	319	76
09:45 AM	57	0	28	85	43	249	0	292	0	0	0	0	0	278	25	303	68
Total	304	0	179	483	143	1117	0	1260	0	0	0	0	0	1146	98	1244	298
** BREAK **	*																
03:00 PM	47	0	44	91	61	280	0	341	0	0	0	0	0	270	36	306	73
03:15 PM	55	0	53	108	53	287	0	340	Ō	Ō	Ō	0	ō	278	44	322	77
03:30 PM	37	0	41	78	48	283	0	331	Ō	Ō	ō	o	Ö	262	50	312	72
03:45 PM	47_	0	47	94	64	300	0	364	0	0	0	0	0	309	46	355	81
Total	186	0	185	371	226	1150	0	1376	0	0	0	0	0	1119	176	1295	304
04:00 PM	45	0	35	80	60	300	0	360	0	0	0	0	0	308	45	353	79
04:15 PM	50	0	53	103	74	262	0	336	0	0	0	0	0	315	34	349	78
04:30 PM	47	0	41	88	61	297	0	358	0	0	0	0	0	372	39	411	85
04:45 PM	42	0	46	88	60	287	0	347	0	0	0	0	0	313	44	357	79
Total	184	0	175	359	255	1146	0	1401	0	0	0	0	0	1308	162	1470	323
05:00 PM	48	0	43	91	72	316	0	388	0	0	0	0	0	352	47	399	87
05:15 PM	53	0	47	100	63	267	0	330	0	0	0	o	0	342	49	391	82
05:30 PM	56	0	39	95	70	296	0	366	0	0	0	0	0	356	50	406	86
05:45 PM	51	0	45	96	54	310	0	364	0	0	0	0	0	367	49	416	87
Total	208	0	174	382	259	1189	0	1448	,O	0	0	0	0	1417	195	1612	344
Grand Total	1770	0	1230	3000	1096	7098	0	8194	0	0	0	0	0	7028	805	7833	1902
Apprch %	59.0	0.0	41.0		13.4	86.6	0.0		0.0	0.0	0.0	-	0.0	89.7	10.3		
Total %	9.3	0.0	6.5	15.8	5.8	37.3	0.0	43.1	0.0	0.0	0.0	0.0	0.0	36.9	4.2	41.2	
	8 1	W 2017	1515 51.														
	H		INE AVE			VENTU	KA AVE.							VENTU	RA AVE.		

			INE AV	Ε.		VENTU	IRA AVE							VENTU	RA AVE		
		South	nbound			Wes	tbound			North	bound			East	bound		
Start Time		Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fron	m 07:00	O AM to	09:45	AM - Pea	k 1 of 1												
Intersection																	
Volume	493	0	310	803	118	1304	0	1422	0	0	0	0	0	1059	92	1151	3376
Percent	61.4	0.0	38.6		8.3	91.7	0.0		0.0	0.0	0.0	_	0.0	92.0	8.0		
08:15	123	0	72	105	27	244		274	_		•	•		274	25	206	062
Volume	123	U	12	195	27	344	0	371	0	U	0	U	0	271	25	296	862
Peak Factor																	0.979
High Int.	08:00	AM			08:15	AM			6:45:0	0 AM			08:30	AM			
Volume	136	0	83	219	27	344	0	371	0	0	0	0	0	294	19	313	
Peak Factor				0.917				0.958								0.919	

File Name : 355510 Site Code : 00355510 Start Date : 11/15/2005 Page No : 2

	ŀ		INE AV	Ē.		VENTU	RA AVE			· · · · · · · · · · · · · · · · · · ·				VENTU	RA AVE		
		South	nbound			Wes	lbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00) PM to	05:45	PM - Pea	k 1 of 1												
Intersection	05:00	PM															
Volume	208	0	174	382	259	1189	0	1448	0	0	0	0	0	1417	195	1612	3442
Percent	54.5	0.0	45.5		17.9	82.1	0.0		0.0	0.0	0.0		0.0	87.9	12.1		
05:00 Volume	48	0	43	91	72	316	0	388	0	0	0	0	0	352	47	399	878
Peak Factor																	0.980
High Int.	05:15	PM			05:00	PM							05:45	PM			
Volume Peak Factor	53	0	47	100 0.955	72	316	0	388 0.933	0	0	0	0	0	367	49	416 0.969	

File Name : 355511 Site Code : 00355511 Start Date : 11/17/2005 Page No : 1

						_									Page	NO :	1
		WOODN	AN AV	=	1	Gr MAGNO		inted- Tui			MAN AVE			MACNO	LIA BLV	<u> </u>	1
			bound		'		bound	υ,			viAis Avi 1bound				bound	U.	
Start Time	Right	The	Left	App.	Dielek		1	App.	D. 1.			App.	<u> </u>		T T	App.	Int.
Start Time	Right	Thru	reit	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0	***************************************	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		10001
07:00 AM	19	303	25	347	21	134	25	180	8	108	14	130	11	85	4	100	757
07:15 AM	22	331	49	402	19	177	20	216	20	145	17	182	19	130	11	160	960
07:30 AM	19	349	46	414	36	240	28	304	42	203	27	272	20	186	17	223	1213
07:45 AM	30	302	45	377	50	257	28	335	29	214	30	273	16	232	15	263	1248
Total	90	1285	165	1540	126	808	101	1035	99	670	88	857	66	633	47	746	4178
MA 00:80	27	303	38	368	24	223	26	273	14	164	21	199	15	205	21	241	1081
08:15 AM	38	262	30	330	16	189	21	226	13	131	14	158	29	215	9	253	967
08:30 AM	19	288	37	344	13	165	16	194	7	139	8	154	19	195	11	225	917
08:45 AM	13	304	36	353	17	179	27	223	17	139	14	170	17	171	10	198	944
Total	97	1157	141	1395	70	756	90	916	51	573	57	681	80	786	51	917	3909
09:00 AM	17	245	21	283	20	124	23	167	18	120	21	159	16	173	10	199	808
09:15 AM	20	251	18	289	16	111	13	140	22	139	14	175	21	150	9	180	784
09:30 AM	12	264	28	304	12	94	21	127	13	132	12	157	14	134	7	155	743
09:45 AM	11	228_	29	268	15	109	26	150	12	133	11	156	15	111	9	135	709
Total	60	988	96	1144	63	438	83	584	65	524	58	647	66	568	35	669	3044
*** BREAK ***	*																
03:00 PM	13	210	30	253	26	129	26	181	44	244	21	309	17	182	16	215	958
03:15 PM	23	250	24	297	48	162	27	237	30	277	26	333	16	193	22	231	1098
03:30 PM	18	184	26	228	34	147	29	210	25	249	27	301	16	180	17	213	952
03:45 PM	10	210	28	248	28	145	23	196	22	242	10	274	13	182	15	210	928
Total	64	854	108	1026	136	583	105	824	121	1012	84	1217	62	737	70	869	3936
04:00 PM	15	177	27	219	39	133	17	189	17	209	30	256	18	171	11	200	864
04:15 PM	13	184	20	217	21	154	20	195	21	247	27	295	16	217	14	247	954
04:30 PM	15	202	23	240	24	144	16	184	17	230	18	265	19	205	27	251	940
04:45 PM	18	206	28	252	27	150	23	200	23	252	10	285	17	187	13	217	954
Total	61	769	98	928	111	581	76	768	78	938	85	1101	70	780	65	915	3712
0E-00 BM	20	246		1				1									
05:00 PM	29	218	13	260	14	122	23	159	19	239	14	272	25	218	18	261	952
05:15 PM	20	211	21	252	21	155	29	205	23	276	24	323	31	245	21	297	1077
05:30 PM	22	183	20	225	24	128	24	176	28	224	32	284	33	209	20	262	947
05:45 PM	16	190	19	225	22	119	24	165	24	263	25	312	21	229	25	275	977
Total	87	802	73	962	81	524	100	705	94	1002	95	1191	110	901	84	1095	3953
Grand Total	459	5855	681	6995	587	3690	555	4832	508	4719	467	5694	454	4405	352	5211	22732
Apprch %	6.6	83.7	9.7		12.1	76.4	11.5	,002	8.9	82.9	8.2	3054	8.7	84.5	6.8	2211	22132
Total %	2.0	25.8	3.0	30.8	2.6	16.2	2.4	21.3	2.2	20.8	2.1	25.0	2.0	19.4	1.5	22.9	
				23.0			'	-1.5		20.0	٠.1	ا ۱۰۰۵	2.0	13.7	1.0	22.3	

	1		MAN AVI	Ξ.	Į,		LIA BLV	D.	1		VAN AV	Ξ.	N		LIA BLV	D.	
		South	ibouna			vves	lbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:0	λM to	00:45		k 1 of 1			TOLAI	L			TULAI				TOLAI	TOLAI
			U5.43	An - rea	KTOIT	_			1								
Intersection	07:30	AM															
Volume	114	1216	159	1489	126	909	103	1138	98	712	92	902	80	838	62	980	4509
Percent	7.7	81.7	10.7		11.1	79.9	9.1		10.9	78.9	10.2		8.2	85.5	6.3		
07:45	30	302	45	377	50	257	20	225	20	24.4	20	272		222		262	4240
Volume	30	302	40	3//	50	257	28	335	29	214	30	273	16	232	15	263	1248
Peak Factor																	0.903
High Int.	07:30	AM			07:45	AM			07:45	AM			07:45	AM			
Volume	19	349	46	414	50	257	28	335	29	214	30	273	16	232	15	263	
Peak Factor				0.899				0.849				0.826				0.932	

File Name : 355511 Site Code : 00355511 Start Date : 11/17/2005 Page No : 2

			MAN AV	E.	N		LIA BLV tbound	D.			MAN AVI	Ξ.	N		LIA BLV bound	D.	
Start Time	Right	Thru	Left	lotal	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From	m 03:00) PM to	05:45	PM - Pea	k 1 of 1									!		Total	1 Ottal
	05:00												1				i
Volume	87	802	73	962	81	524	100	705	94	1002	95	1191	110	901	84	1095	3953
Percent 05:15	9.0	83.4	7.6		11.5	74.3	14.2		7.9	84.1	8.0	:	10.0	82.3	7.7	1030	3333
Volume	20	211	21	252	21	155	29	205	23	276	24	323	31	245	21	297	1077
Peak Factor																	0.918
High Int.	05:00			1	05:15	PM			05:15	PM			05:15	PM			0.918
Volume Peak Factor	29	218	13	260 0.925	21	155	29	205 0.860	23	276	24	323 0.922	31	245	21	297 0.922	

File Name : 355512 Site Code : 00355512 Start Date : 11/16/2005 Page No : 1

						G	roups Pri	nted- Tu	rnina Ma	wamani	,				Page	e No∷	
		WOODI	MAN AV	E.			IDE DRIV				MAN AV	F	F	RIVERS	DE DRI	VF	1
		Sout	hbound			Wes	tbound				hbound				bound	•-	
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	In
Factor	1.0	1.0	1.0	Total	1.0	1.0	1.0	Total	1.0	1.0	1.0	Total	_			Total	Tota
07:00 AM	15	262	34	311	24	148	95	267	39	134	25	198	1.0 25	1.0 42	1.0	78	85
07:15 AM	19	289	36	344	59	205	93 84	348	43	145	25 16	204	25	117	22		
07:30 AM	25	285	48	358	88	245	73	406	57	215						167	106
07:45 AM	30	272	64	366	35	261			f		40	312	21	170	47	238	131
Total	89	1108	182	1379	206	859	64	360	68	199	47	314	20	235	28	283	132
10001	03	1100	102	13/9	200	029	316	1381	207	693	128	1028	94	564	108	766	455
MA 00:80	18	280	58	356	30	256	72	358	44	143	46	233	39	239	10	288	123
08:15 AM	34	283	50	367	25	209	71	305	42	149	43	234	32	202	6	240	114
08:30 AM	27	270	64	361	22	169	72	263	30	125	39	194	26	237	2	265	108
08:45 AM	31	234	53	318	25	167	62	254	45	134	53	232	23	226	8	257	106
Total	110	1067	225	1402	102	801	277	1180	161	551	181	893	120	904	26	1050	452
09:00 AM	38	257	58	353	ا عد	165	r2	244	l 54	150	F-4	262	2.0	24.0	20	262	
09:00 AM 09:15 AM	23	243	58 65	353 331	26	165	53	244	54	158	51	263	30	210	29	269	1129
09:30 AM	27	202	65 44	273	16	165	48	229	28	129	66	223	21	178	15	214	997
09:45 AM	41	202	39	285	19	158	57	234	56	120	58	234	34	158	17	209	950
Total	129	907	206	1242	25 86	148 636	61 219	234	49	106	58	213	23	142	13	178	91
10001	123	507	200	1272	00	030	219	941	187	513	233	933	108	688	74	870	398
** BREAK **	*																
03:00 PM	27	196	41	264	87	182	53	322	30	205	75	310	49	206	48	303	1100
03:15 PM	43	193	44	280	72	244	51	367	52	203	75 86	379	65	200	59	348	1199 137
03:30 PM	46	225	25	296	61	242	58	361	49	218	77	344	03 77	207	59 51	348	1336
03:45 PM	45	185	37	267	54	204	60	318	37	200	84	321	58	233	52	343	1249
Total	161	799	147	1107	274	872	222	1368	168	864	322	1354	249	<u>233</u> 870	210	1329	515
												,					
04:00 PM	47	188	38	273	57	182	61	300	51	223	82	356	47	217	43	307	1236
04:15 PM	50	186	34	270	56	199	59	314	41	199	65	305	46	178	49	273	1162
04:30 PM	38	187	29	254	49	213	57	319	45	197	70	312	54	232	52	338	1223
04:45 PM	52	188	21	261	62	208	68	338	40	244	71	355	45	209	56	310	1264
Total	187	749	122	1058	224	802	245	1271	177	863	288	1328	192	836	200	1228	488
05:00 PM	46	177	30	253	53	217	63	333	31	230	84	345	60	191	47	298	1229
05:15 PM	44	190	39	273	47	217	71	335	35	253	84	372	55	204	48	307	128
05:30 PM	41	178	39	258	77	226	61	364	32	222	72	326	49	204	56	311	1259
05:45 PM	50	171	31	252	50	225	67	342	39	221	97	357	37	237	49	323	127
Total	181	716	139	1036	227	885	262	1374	137	926	337	1400	201	838	200	1239	5049
	_							'									
Grand Total	857	5346		7224		4855		7515	1037			6936	964		818	6482	28157
Apprch %		74.0	14.1	25 -	14.9	64.6	20.5		15.0	63.6	21.5		14.9		12.6		
Total %	3.0	19.0	3.6	25.7	4.0	17.2	5.5	26.7	3.7	15.7	5.3	24.6	3.4	16.7	2.9	23.0	
<u> </u>		WOODN	IAN AVE	· 1		II/EDell	DE DRIV	E	1	MOODA	1AN AVE	· · · · · · · · · · · · · · · · · · ·		VEDOU	אר טטוי	/= 1	
			nbound		17		bound	-	\		ian ave bound		R		DE DRIV ound	L	
Start Time	Riaht	Thru	Left	App.	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App.	Int.
		11111	الاست	Total	NULL	IIII											Total

			nbound				tbound	VE			ibound	: .	1		bound	/E	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
Peak Hour Fro	m 07:0	D AM to	09:45		k 1 of 1			Total				Total				Total	Total
Intersection	07:30	AM															
Volume	107	1120	220	1447	178	971	280	1429	211	706	176	1093	112	846	91	1049	5018
Percent	7.4	77.4	15.2		12.5	67.9	19.6		19.3	64.6	16.1		10.7	80.6	8.7		
07:45	30	272	64	366	35	261	64	360	68	199	47	314	20	235	28	283	1323
Volume	50	2,2	01	300))	201	. 07	300	00	133	7/	214	20	233	20	203	1323
Peak Factor																	0.948
High Int.	08:15	AM			07:30	AM			07:45	AM			08:00	AM			
Volume	34	283	50	367	88	245	73	406	68	199	47	314	39	239	10	288	
Peak Factor				0.986				0.880				0.870				0.911	

File Name : 355512 Site Code : 00355512 Start Date : 11/16/2005 Page No : 2

	1		MAN AV	Ξ.	F		DE DRI	/E	,		MAN AVI	Ξ.	F		DE DRI	VΕ]
		South	nbound			West	tbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App.	Int.
Dook House From	~ ^2.0	7 004 4-	05.45		1- 4 - 6 4			TOLAI			}	TOLA			l	Total	Total
Peak Hour From			U5:45	PM - Pea	K T OL 1												
Intersection	03:15	PM															
Volume	181	791	144	1116	244	872	230	1346	189	882	329	1400	247	881	205	1333	5195
Percent	16.2	70.9	12.9		18.1	64.8	17.1		13.5	63.0	23.5		18.5	66.1	15.4		
03:15 Volume	43	193	44	280	72	244	51	367	52	241	86	379	65	224	59	348	1374
Peak Factor																	0.945
High Int.	03:30	РМ			03:15	РМ			03:15	DM			03:15	DM			0.943
Volume	46	225	25	296	72	244	51	367	52	241	86	379	65	224	59	348	
Peak Factor				0.943				0.917				0.923	-	- - ·		0.958	

File Name : 355513 Site Code : 00355513 Start Date : 11/15/2005 Page No : 1

						C.	ouna Dr	inted Tu	rnina M	nuom ont					Page	No :	1
	T	WOODN	AN AV	E.	US		OFF R	inted- Tu AMP			MAN AV	F	119	3 101 NE	3 ON RA	MP	7
			bound	_,			bound				hbound				bound	(1411	
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Diabt	7-1	1 -64	App.	D:-L-			App.	Int.
				Total	Rigit	HIII	Leit	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	137	202	0	339	57	0	62	119	0	155	76	231	0	0	0	0	689
07:15 AM	156	236	0	392	41	0	54	95	0	180	94	274	0	0	0	0	761
07:30 AM	147	249	0	396	53	1	60	114	0	237	78	315	0	0	0	0	825
07:45 AM	143	262	0	405	58	2	62	122	0	229	84	313	0	0	0	0	840
Total	583	949	0	1532	209	3	238	450	0	801	332	1133	0	0	0	0	3115
MA 00:80	111	255	0	366	82	0	82	164	0	201	87	288	0	0	0	0	818
08:15 AM	123	255	0	378	62	1	98	161	0	189	86	275	0	0	0	0	814
08:30 AM	119	240	0	359	90	0	84	174	2	194	67	263	0	0	0	0	796
08:45 AM	91	246	0	337	93	0	117	210	0	192	72	264	0	0	0	0	811
Total	444	996	0	1440	327	1	381	709	2	776	312	1090	0	0	0	0	3239
09:00 AM	86	210	0	296	76	0	107	183	0	195	69	264	0	0	0	0	743
09:15 AM	91	207	0	298	58	0	78	136	0	201	78	279	0	0	Ō	0	713
09:30 AM	105	204	0	309	57	0	98	155	0	196	75	271	0	Ō	Ō	0	735
09:45 AM	98	183	0	281	60	0	79	139	0	220	56	276	0	ō	Ö	Ō	696
Total	380	804	0	1184	251	0	362	613	0	812	278	1090	0	0	0	0	2887
*** BREAK **	*																
03:00 PM	144	230	0	374	114	0	49	163	l o	240	102	342	l 0	0	0	0	879
03:15 PM	151	224	Ö	375	89	0	79	168	0	253	78	331	0	0	0	0	874
03:30 PM	126	223	Ö	349	52	0	80	132	0	274	107	381	0	0	0	0	862
03:45 PM	109	209	Ö	318	63	Ő	94	157	ő	265	77	342	0	0	0	0	817
Total	530	886	0	1416	318	0	302	620	0	1032	364	1396	0	0	0	0	3432
04:00 PM	98	206	0	304	78	0	74	152	l o	240	83	323	0	0	0	0	779
04:15 PM	111	225	Ő	336	67	0	82	149	0	244	85	329	0	0	Ö	0	814
04:30 PM	107	216	Ō	323	69	ő	92	161	0	258	90	348	0	0	Ö	0	832
04:45 PM	104	220	ō	324	91	Õ	79	170	ő	283	70	353	0	Ö	ő	0	847
Total	420	867	0	1287	305	0	327	632	0	1025	328	1353	0	0	0	0	3272
05:00 PM	113	203	0	316	98	0	88	186	0	285	91	376	0	0	0	0	878
05:15 PM	109	240	0	349	70	Ō	96	166	ō	291	83	374	ő	Ö	ő	Ö	889
05:30 PM	122	215	0	337	89	0	100	189	0	302	69	371	Ō	ŏ	Ö	Ō	897
05:45 PM	123	224	0	347	92	0	103	195	0	262	59	321	Ō	ō	Õ	ō	863
Total	467	882	0	1349	349	0	387	736	0	1140	302	1442	0	0	Ō	0	3527
Grand Total	2824	5384	0	8208	1759	4	1997	3760	2	5586	1916	7504	0	0	0	0	19472
Apprch %	34.4	65.6	0.0		46.8	0.1	53.1		0.0	74.4	25.5		0.0	0.0	0.0		
Total %	14.5	27.6	0.0	42.2	9.0	0.0	10.3	19.3	0.0	28.7	9.8	38.5	0.0	0.0	0.0	0.0	
		MOODM	A N I A N //	÷		404 110	055 51										
	,	WOODM South		=-	US	101 NB Westl	OFF RA	WP.	'		IAN AVE	. .	US	101 NB Eastb	ON RAI	MP	
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
Peak Hour Fro	m 07:00) AM to	09:45	Total AM - Pea	k 1 of 1			Total			<u></u>	Total				Total	Total
Intersection																	
Volume		1021	0	1545	255	4	302	561	0	856	335	1191	0	0	0	0	3297
Percent	33.9	66.1	0.0		45.5	0.7	53.8		0.0	71.9	28.1		0.0	0.0	0.0		
07:45	143	262	0	405	58	2	62	122	0	229	84	313	0	0	0	0	840
Volume	110	202	J	705	50	2	UΖ	144	U	223	04	212	U	U	0	U	
Peak Factor																	0.981
High Int.					08:00				07:30				6:45:00	MA (
Volume	143	262	0	405	82	0	82	164	0	237	78	315					
Peak Factor				0.954				0.855				0.945					

File Name : 355513 Site Code : 00355513 Start Date : 11/15/2005 Page No : 2

			MAN AV hbound	E.	ÜS		OFF R	AMP			MAN AV	E.	US		3 ON RA	MP	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00	OPM to	05:45	PM - Pea	k 1 of 1												
Intersection	05:00	PM			ĺ											i	
Volume	467	882	0	1349	349	0	387	736	0	1140	302	1442	0	0	0	0	3527
Percent	34.6	65.4	0.0		47.4	0.0	52.6		0.0	79.1	20.9		0.0	0.0	0.0	J	3327
05:30	122	215	0	337	89	n	100	189	0	302	69	371	0	0			007
Volume		-10	U	337	05	U	100	103	U	302	09	3/1	U	0	0	U	897
Peak Factor																	0.983
High Int.	05:15	PM			05:45	PM			05:00	PM							0,500
Volume	109	240	0	349	92	0	103	195	0	285	91	376					
Peak Factor				0.966				0.944				0.959					

Groups Printed-Turning Movement

File Name : 355514 Site Code : 00355514 Start Date : 11/15/2005 Page No : 1

	· · · · · · · · · · · · · · · · · · ·							iiileu- iu	ming with	Meillein	<u> </u>						
			MAN AV	Έ.	U		B ON RA				MAN AV	Έ.	US	3 101 S	B OFF F	RAMP	1
		Sout	hbound			Wes	tbound			Nort	hbound			Eas	tbound		
Start Time	Right	Thru	Left	App.	Diaht	Th	Loft	App.	Diabe	Thomas	1 - 64	App.	D:-1-1		1 . 6	App.	Int.
Start Time	Right	111111	Leit	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	700	1000
07:00 AM	0	178	95	273	0	0	0	0	88	149	0	237	57	0		141	651
07:15 AM	ő	187	108	295	0	0			74					-			1
07:30 AM					1		0	0	1	171	0	245	51	0	107	158	698
	0	212	96	308	0	0	0	0	90	209	0	299	58	0	108	166	773
07:45 AM	0	237	77	314	0	0	0	0	81	228	0	309	66	1	90	157	780
Total	0	814	376	1190	0	0	0	0	333	757	0	1090	232	1	389	622	2902
												•					
MA 00:80	0	260	70	330	0	0	0	0	109	209	0	318	79	2	74	155	803
08:15 AM	0	277	82	359	0	0	0	0	78	194	0	272	85	0	75	160	791
08:30 AM	0	239	90	329	0	Ö	Ö	0	81	175	0	256	107	0	94	201	786
08:45 AM	Ö	275	85	360	0	0	0	-					1	-			
Total	0	1051					~~~	0	77	178	0	255	106		83	190	805
TOLAT	U	1051	327	1378	0	0	0	0	345	756	0	1101	377	3	326	706	3185
	_																
09:00 AM	0	256	64	320	0	0	0	0	68	181	0	249	124	1	92	217	786
09:15 AM	0	223	62	285	0	0	0	0	61	182	0	243	126	0	101	227	755
09:30 AM	0	235	61	296	0	0	0	0	73	167	Ō	240	129	0	108	237	773
09:45 AM	0	191	73	264	ō	0	Ö	0	75	165	ő	240	113	0	105	218	722
Total	0	905	260	1165	0	0	0	0	277	695	0		1	1			
iotai	U	203	200	1103	, U	U	U	U	2//	093	U	972	492	1	406	899	3036
*** BREAK **	*																
03:00 PM	0	186	87	273	0	0	0	0	92	226	0	318	75	0	110	185	776
03:15 PM	Ő	225	83	308	ő	0	0	0	89	227		316	69			1	
03:30 PM	0	220	68								0			2	95	166	790
				288	0	0	0	0	95	266	0	361	78	0	117	195	844
03:45 PM	0	253	69	322	0	0	0	0	93	229	0_	322	72	2	118	192	836
Total	0	884	307	1191	. 0	0	0	0	369	948	0	1317	294	4	440	738	3246
04:00 PM	0	204	71	275	0	0	0	0	96	228	0	324	70	0	93	163	762
04:15 PM	0	238	60	298	0	0	0	0	78	257	0	335	74	0	73	147	780
04:30 PM	0	256	55	311	0	0	Ō	0	69	264	Ō	333	79	Ŏ	88	167	811
04:45 PM	0	240	63	303	Ö	Ö	0	ő	82	237	Ö	319	77	1	110	188	810
Total	0	938	249	1187	0	0	0	0	325	986	0	1311	300	<u></u>	364	665	3163
	_					•	_		323		U		. 300	1	304		3103
05:00 PM	0	224	70	294	0	0	0	0	87	281	0	368	80	0	99	179	841
05:15 PM	0	255	74	329	0	0	0	0	76	266	0	342	89	0	119	208	879
05:30 PM	0	252	69	321	0	0	0	0	105	268	0	373	97	0	121	218	912
05:45 PM	0	248	68	316	0	0	0	0	76	211	0	287	95	2	106	203	806
Total	0	979	281	1260	0	0	0	0	344	1026	0	1370	361	2	445	808	3438
	Ū	3,3	201	1200	U	Ü	J	0 1	2.1.1	1020	U	13/0	301	2	כדד	000	2420
Grand Total	0	5571	1800	7371	0	0	0	0	1993	5168	0	7161	2056	12	2370	4438	18970
Apprch %	0.0	75.6	24.4		0.0	0.0	0.0	Ĭ	27.8		0.0	. 101	46.3	0.3		55	
Total %	0.0	29.4	9.5	38.9		0.0	0.0	0.0				27.7				22.4	
70141 70	0.0	23.4	3.5	20.5	0.0	0.0	0.0	0.0	10.5	27.2	0.0	37.7	10.8	0.1	12.5	23.4	
			MAN AV	Ξ.	US	101 SE	ON RAI	MP		NOODN	1AN AVE	Ξ.	US	101 SB	OFF R	AMP	
		South	nbound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Froi	m 07:00) AM to	09:45		k 1 nf 1				!					!		, 5	· Juli
Intersection								1				1				1	
Volume		1051	327	1378	Δ.		^	_	245	756	•		277	_	226	700	2105
				12/0	0	0	0	0	345	756	0	1101	377	3	326	706	3185
Percent	0.0	76.3	23.7		0.0	0.0	0.0		31.3	68.7	0.0		53.4	0.4	46.2		
08:45	_																

08:45

High Int. 08:45 AM

Volume Peak Factor

Volume

Peak Factor

0

275

0 275

85

85

360

360

0.957

0

6:45:00 AM

0

0

0

0

0

0

0

77

08:00 AM

109

178

209

0

255

318

0.866

106

107

08:30 AM

83

94

1

0

190

201

0.878

805

0.989

File Name : 355514 Site Code : 00355514 Start Date : 11/15/2005 Page No : 2

	1		MAN AVI	Ξ.	US		3 ON RA	MP.			MAN AVI	Ξ.	US		OFF RA	AMP	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App.	Int.
Peak Hour From	m 03:00) PM to	05:45		k 1 of 1			iotai			1	TULai	_			Total	Total
	04:45																
Volume	0	971	276	1247	0	0	0	0	350	1052	0	1402	343	1	449	793	3442
Percent	0.0	77.9	22.1		0.0	0.0	0.0		25.0	75.0	0.0		43.3	0.1	56.6	, , , ,	
05:30	0	252	69	321	0	Λ	0	0	105	268	0	373	07	0	171	240	013
Volume	U	232	09	341	U	U	U	U	105	200	U	3/3	97	0	121	218	912
Peak Factor																Ì	0.944
High Int.	05:15	PM							05:30	PM			05:30	PM			
Volume	0	255	74	329	0	0	0	0	105	268	0	373	97	0	121	218	
Peak Factor				0.948								0.940				0.909	

Groups Printed- Turning Movement

File Name : 355515 Site Code : 00355515 Start Date : 11/15/2005 Page No : 1

			MAN AVE	Ξ.		MOORE	PARK ST				MAN AVE	Ξ.			PARK ST.		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
Factor	1.0	1.0	1.0	Total		1.0		Total	_			Total				Total	Total
07:00 AM	31	157	1.0	222	1.0	123	1.0	100	1.0	1.0	1.0	117	1.0	1.0	1.0	404	622
07:15 AM	32	148	44	224	72	141	18 17	198 230	9	104 99	3 6	112 114	5 4	67 64	29 33	101	633 669
07:30 AM	36	183	52	271	61	174	29	264	7	102	4	113	9	112	33 30	101	
07:45 AM	42	188	72	302	71	180	23	274	4	136	3	143	11	134	22	151 167	799 886
Total	141	676	202	1019	261	618	87	966	25	441	<u>3</u> 	482	29	377	114	520	2987
															11.	320	. 2507
08:00 AM	48	208	68	324	68	189	17	274	6	138	2	146	8	131	27	166	910
08:15 AM	53	213	68	334	81	169	18	268	5	107	5	117	10	136	23	169	888
08:30 AM	55	209	78	342	45	160	24	229	8	90	2	100	9	129	35	173	844
08:45 AM	66	195	79	340	48	147	18	213	9	81	9	99	6	120	30	156	808
Total	222	825	293	1340	242	665	77	984	28	416	18	462	33	516	115	664	3450
09:00 AM	56	202	76	334	46	119	16	181	4	92	8	104	4	118	49	171	790
09:15 AM	64	185	78	327	47	128	13	188	6	97	12	115	10	111	43	164	794
09:30 AM	76	212	68	356	40	106	12	158	10	113	9	132	11	83	34	128	774
09:45 AM	42	164	72	278	52	88	8	148	9	109	4	122	18	118	25	· 161	709
Total	238	763	294	1295	185	441	49	675	29	411	33	473	43	430	151	624	3067
*** BREAK **	*																
03:00 PM	46	144	56	246	57	89	13	159	11	180	10	201	13	127	57	197	803
03:15 PM	55	154	55	264	56	96	16	168	16	154	14	184	15	117	49	181	797
03:30 PM	57	158	50	265	71	102	14	187	13	186	15	214	13	156	51	220	886
03:45 PM	65	164	63	292	55	113	11	179	11	150	10	171	17	147	52	216	858
Total	223	620	224	1067	239	400	54	693	51	670	49	770	58	547	209	814	3344
04:00 PM	52	137	56	245	68	105	15	188	8	166	6	180	10	151	55	216	829
04:15 PM	65	150	65	280	59	120	17	196	12	162	11	185	11	175	53	239	900
04:30 PM	67	173	67	307	55	111	11	177	9	166	16	191	16	155	56	227	902
04:45 PM	49	162	63	274	67	126	12	205	10	155	12	177	11	183	52	246	902
Total	233	622	251	1106	249	462	55	766	39	649	45	733	48	664	216	928	3533
05:00 PM	54	171	54	279	69	122	21	212	14	227	19	260	17	157	47	221	972
05:15 PM	70	186	64	320	63	150	13	226	11	194	12	217	16	181	40	237	1000
05:30 PM	58	173	60	291	59	121	14	194	13	170	18	201	25	170	57	252	938
05:45 PM	59	176	75	310	57	137	20	214	15	151	15	181	21	151	53	225	930
Total	241	706	253	1200	248	530	68	846	53	742	64	859	79	659	197	935	3840
Grand Total	1298	4212	1517	7027	1424	3116	390	4930	225	3329	225	3779	290	3193	1002	4485	20221
Apprch %	18.5	59.9	21.6		28.9	63.2	7.9		6.0	88.1	6.0		6.5	71.2	22.3		
Total %	6.4	20.8	7.5	34.8	7.0	15.4	1.9	24.4	1.1	16.5	1.1	18.7	1.4	15.8	5.0	22.2	
	1077	MOG = 1			,					****							
	,		MAN AVE				ARK ST.	.	1		IAN AVE	. 7			PARK ST.		
CL LT	D	Jouli	ibouriu	App.		vvest	bound	App.		INORN	bound	Ann		East	bound	Ann.	Int.

	,		VA NAN	Ξ.		MOORE	PARK ST			WOODN	MAN AVE	Ξ.		MOORF	PARK ST	-	
		South	nbound			Wes	tbound			North	nbound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:00	O AM to	09:45	AM - Pea	k 1 of 1											, , , , , ,	1 2 3 3 1 1
Intersection	07:45	AM														1	
Volume	198	818	286	1302	265	698	82	1045	23	471	12	506	38	530	107	675	3528
Percent	15.2	62.8	22.0		25.4	66.8	7.8		4.5	93.1	2.4		5.6	78.5	15.9		
08:00 Volume	48	208	68	324	68	189	17	274	6	138	2	146	8	131	27	166	910
Peak Factor																	0.969
High Int.	08:30	AM			07:45	AM			08:00	AM			08:30	AM			
Volume	55	209	78	342	71	180	23	274	6	138	2	146	9	129	35	173	
Peak Factor				0.952				0.953				0.866				0.975	

File Name : 355515 Site Code : 00355515 Start Date : 11/15/2005 Page No : 2

	1		MAN AV	E.			PARK ST	•	١		MAN AVE	Ξ.			PARK ST	Γ.	
		South	nbound			vves	tbound			Norti	nbound			East	bound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
			li	Total				Total	,			Total				Total	Total
Peak Hour Froi	m 03:00) PM to	05:45	PM - Pea	k 1 of 1												
Intersection	05:00	PM															
Volume	241	706	253	1200	248	530	68	846	53	742	64	859	79	659	197	935	3840
Percent	20.1	58.8	21.1		29.3	62.6	8.0		6.2	86.4	7.5		8.4	70.5	21.1		
05:15	70	186	64	320	63	150	13	226	11	194	12	217	16	181	40	237	1000
Volume	70	100	04	320	03	130	13	220	11	194	12	217	10	101	40	237	1000
Peak Factor																	0.960
High Int.	05:15	PM			05:15	PM			05:00	PM			05:30	PM			
Volume	70	186	64	320	63	150	13	226	14	227	19	260	25	170	57	252	
Peak Factor				0.938				0.936				0.826				0.928	

File Name : 355516 Site Code : 00355516 Start Date : 11/15/2005 Page No : 1

						0-	oune D-	nted- Tur	ning Ma	wamant					rage	INO . I	
	,	WOODN	AN AVE	<u> </u>			oups Pri RA BLVD				AN AVE	Ξ.		VENTU	RA BLVI	D	
			bound				bound	••			bound				tbound		
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App.	Int.
				Total				Total	, ,			Total				Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	76	38	45	159	24	209	2	235	3	37	10	50	1	193	51	245	689
07:15 AM	71	36	54	161	24	241	6	271	2	37	12	51	3	233	43	279	762
07:30 AM	84	51	49	184	16	300	8	324	3	40	9	52	6	259	51	316	876
07:45 AM	88	51	50	189	20	250	10	280	2	52	12	66	5	261	44	310	845
Total	319	176	198	693	84	1000	26	1110	10	166	43	219	15	946	189	1150	3172
08:00 AM	77	57	51	185	35	266	11	312	4	62	11	77	6	240	28	274	848
08:15 AM	101	56	54	211	27	276	12	315	3	42	13	58	8	242	32	282	866
08:30 AM	82	52	53	187	28	257	10	295	9	37	13	59	6	297	33	336	877
08:45 AM	78	56	41	175	29	218	4	251	10	40	18	68	7	231	24	262	756
Total	338	221	199	758	119	1017	37	1173	26	181	55	262	27	1010	117	1154	3347
09:00 AM	76	45	64	185	27	211	7	245	7	34	14	55	4	266	29	299	784
09:15 AM	66	48	58	172	35	251	9	295	2	41	19	62	5	295	31	331	860
09:30 AM	81	41	53	175	28	182	4	214	5	45	10	60	9	224	38	271	720
09:45 AM	63	41	58	162	38	214	4	256	2	42	13	57	7	250	38	295	770
Total	286	175	233	694	128	858	24	1010	16	162	56	234	25	1035	136	1196	3134
*** BREAK **	*																
03:00 PM	45	40	30	115	46	226	11	283	8	49	15	72	6	234	50	290	760
03:15 PM	49	40	34	123	31	225	8	264	2	33	7	42	8	228	49	285	714
03:30 PM	52	33	35	120	40	221	7	268	6	65	8	79	6	231	47	284	751
03:45 PM	62	32	27	121	24	218	7	249	0	36	14	50	9	240	39_	288	708
Total	208	145	126	479	141	890	33	1064	16	183	44	243	29	933	185	1147	2933
04:00 PM	39	28	34	101	31	216	7	254	6	33	11	50	19	228	55	302	707
04:15 PM	45	21	34	100	50	222	6	278	3	34	14	51	13	261	45	319	748
04:30 PM	58	24	36	118	38	215	5	258	1	28	16	45	10	263	33	306	727
04:45 PM	57	32	. 29	118	35	226	6	267	0	44	20	64	77	253	57	317	766
Total	199	105	133	437	154	879	24	1057	10	139	61	210	49	1005	190	1244	2948
05:00 PM	49	34	27	110	26	208	5	239	3	58	8	69	6	293	43	342	760
05:15 PM	54	47	28	129	26	228	7	261	3	62	12	77	19	254	53	326	793
05:30 PM	68	42	36	146	28	213	5	246	0	41	17	58	7	251	45	303	753
05:45 PM	67	35	35	137	21	202	2	225	2	42	11	55	12	272	44	328	745
Total	238	158	126	522	101	851	19	971	8	203	48	259	44	1070	185	1299	3051
Grand Total	1588	980	1015	3583	727	5495	163	6385	86	1034	307	1427	189	5999	1002	7190	18585
Apprch %	44.3	27.4	28.3		11.4	86.1	2.6		6.0	72.5	21.5		2.6	83.4	13.9		
Total %	8.5	5.3	5.5	19.3	3.9	29.6	0.9	34.4	0.5	5.6	1.7	7.7	1.0	32.3	5.4	38.7	
	1	WOODN	ΛΑΝ Δ\/F	•	Γ	VENTUE	RA BLVE)	,	WOODI	JAN AVE		,	VENTU	RA BLVI	D.]	
	1	WOODMAN AVE.							l .							1	

	\		/IAN AVI abound	Ξ.	,		RA BLVI tbound	Э.	,		/IAN AVE Ibound	Ξ.	,		RA BLVI bound),	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:00) AM to	09:45	AM - Pea	k 1 of 1												
Intersection	07:45	AM															
Volume	348	216	208	772	110	1049	43	1202	18	193	49	260	25	1040	137	1202	3436
Percent	45.1	28.0	26.9		9.2	87.3	3.6		6.9	74.2	18.8		2.1	86.5	11.4		
08:30 Volume	82	52	53	187	28	257	10	295	9	37	13	59	6	297	33	336	877
Peak Factor																	0.979
High Int.	08:15	AM			08:15	AM			08:00	AM			08:30	AM			
Volume	101	56	54	211	27	276	12	315	4	62	11	77	6	297	33	336	
Peak Factor				0.915				0.954				0.844				0.894	

File Name : 355516 Site Code : 00355516 Start Date : 11/15/2005 Page No : 2

	1		MAN AV nbound	E.			RA BLVE),	,		MAN AVE	Ξ.			RA BLVI	D.	
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00) PM to	05:45	PM - Pea	k 1 of 1		1				······································						L
Intersection	04:45	PM															
Volume	228	155	120	503	115	875	23	1013	6	205	57	268	39	1051	198	1288	3072
Percent	45.3	30.8	23.9		11.4	86.4	2.3		2.2	76.5	21.3		3.0	81.6	15.4		
05:15 Volume	54	47	28	129	26	228	7	261	3	62	12	77	19	254	53	326	793
Peak Factor																	0.000
High Int.	05:30	PM			04:45	PM			05:15	PM			05:00	PM			0.968
Volume Peak Factor	68	42	36	146 0.861	35	226	6	267 0.949	3	62	12	77 0.870	6	293	43	342 0.942	

File Name : 355507-1 Site Code : 00355507 Start Date : 11/29/2005 Page No : 1

						C	Dula	had Tur	mina Ma	womon	+				raye	140 . 1	
	1	IAZELTI	NIE AV	_	D		DE DRIV	ted- Tur			INE AVE	:	R	TVFRST	DE DRIV	/E	
	-		ine Avi bound	Ξ.	K		bound				bound	••	, ,		bound		
	1	20001		App.				Арр.				App.	5: 14			App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	16	166	33	215	9	106	43	158	14	34	16	64	10	47	2	59	496
07:15 AM	21	153	34	208	21	158	66	245	22	72	13	107	6	76	8	90	650
07:30 AM	19	178	46	243	34	203	92	329	37	70	9	116	8	105	6	119	807
07:45 AM	13	226	54	293	31	171	100	302	36	81	6	123	19	124	4	147	865
Total	69	723	167	959	95	638	301	1034	109	257	44	410	43	352	20	415	2818
					ı								۱ ،	4 4 4	_	101	932
MA 00:80	18	228	38	284	25	216	97	338	23	108	15	146	18	141	5 6	164 154	932 844
08:15 AM	15	206	37	258	33	184	77	294	30	89	19	138	14	134 158	5 5	177	772
08:30 AM	17	176	46	239	26	131	67	224	28	83	21	132 135	14 19	135	3	157	760
08:45 AM	15	188	52	255	20	125_	68	213	25	93	<u>17</u> 72	551	65	568	19	652	3308
Total	65	798	173	1036	104	656	309	1069	106	373	12	221	03	200	15	032	3300
00 00 414		176	r- 4	238	1 20	98	54	181	31	86	20	137	22	130	6	158	714
09:00 AM	11	176 199	51 45	258	29 25	96 86	5 4	163	45	75	22	142	16	128	5	149	715
09:15 AM	17 12	170	45 48	230	23	96	40	157	27	74	14	115	20	108	12	140	642
09:30 AM 09:45 AM	29	156	46	231	24	89	36	149	33	85	18	136	22	109	7	138	654
Total	69	701	190	960	99	369	182	650	136	320	74	530	80	475	30	585	2725
*** BREAK **											47	204	1 24	4 4 77	12	154	876
03:00 PM	22	148	42	212	38	137	51	226	75	162	47	284	24 25	117 116	13 10	151	878
03:15 PM	32	168	41	241	43	115	49	207	77 69	168 175	34 42	279 286	30	114	24	168	890
03:30 PM	32	163	40	235	48	103	50 64	201 219	82	190	38	310	22	136	25	183	952
03:45 PM	19	178 657	43 166	240 928	33 162	122 477	214	853	303	695	161	1159	101	483	72	656	3596
Total	105	05/	100	920	102	4//	214	ررن	505	023	101	1133	1 101	.00		,	
04:00 PM	30	168	43	241	41	113	46	200	76	204	49	329	16	143	22	181	951
04:15 PM	23	146	41	210	46	113	60	219	83	207	46	336	26	138	28	192	957
04:30 PM	24	164	42	230	40	102	48	190	75	206	33	314	26	145	24	195	929
04:45 PM	20	168	40	228	47	145	45	237	66	188	45	299	19	137	21	177	941
Total	97	646	166	909	174	473	199	846	300	805	173	1278	87	563	95	745	3778
																404	٥٦٣
05:00 PM	29	173	42	244	56	125	53	234	58	191	37	286		132	32	191	955 995
05:15 PM	21	197	41	259	42	146	59	247	65	224	39	328	24	119	18 31	161 215	1054
05:30 PM	29	183	42	254	43	184	52	279	63	204	39	306	32	152 141	29	194	989
05:45 PM	32	158	54	244	54	172	50_	276	59	178 797	38 153	275 1195	24 107	544	110	761	3993
Total	111	711	179	1001	195	627	214	1036	245	/9/	123	1132	1 107	244	110	701	ت ر ر د
Cue v 3 T-1-1	F16	4776	1041	5793	829	3240	1419	5488	1199	3247	677	5123	483	2985	346	3814	20218
Grand Total	516 8.9	4236 73.1	1041 18.0	5/93	15.1	59.0	25.9	JTOO	23.4	63.4	13.2		12.7	78.3	9.1		
Apprch % Total %	2.6	21.0	5.1	28.7	1	16.0	7.0	27.1	1	16.1	3.3	25.3	2.4	14.8	1.7	18.9	
10(a) 70	2.0	21.0	٠.١	20.7	1 1.1	10.0	,.5		,								

	1	HAZELT	INE AV	E.	R	IVERSI	DE DRI	√E	ŀ	HAZELT	INE AVE	Ξ.	R	IVERSI	DE DRI\	/E	
			bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From	m 07:00	O AM to	09:45	AM - Pea	k 1 of 1	l										1	
	07:30								425	240	40	F37	59	504	21	584	3448
Volume	65	838	175	1078	123	774	366	1263	126	348	49	523	10.1	86.3	3.6	JU-1	3110
Percent	6.0	77.7	16.2		9.7	61.3	29.0		24.1	66.5	9.4		10.1	00.5	5.0		
08:00	18	228	38	284	25	216	97	338	23	108	15	146	18	141	5	164	932
Volume	10	220	50	201			٠.										0.925
Peak Factor									Ì								0.923
High Int.	07:45	AM			08:00	AM			08:00	AM			08:00		_		
Volume	13	226	54	293	25	216	97	338	23	108	15	146	18	141	5	164	
Peak Factor				0.920				0.934				0.896	İ			0.890	

File Name : 355507-1 Site Code : 00355507 Start Date : 11/29/2005 Page No : 2

	ŀ	HAZELT	INE AVE	Ξ.	R	IVERSI	DE DRI	VE	ŀ	HAZELT	INE AV	Ε,	R		DE DRI'	VE	
		South	nbound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour From	m 03:00) PM to	05:45 F	PM - Pea	k 1 of 1												
Intersection	05:00	PM															
Volume	111	711	179	1001	195	627	214	1036	245	797	153	1195	107	544	110	761	3993
Percent	11.1	71.0	17.9		18.8	60.5	20.7		20.5	66.7	12.8		14.1	71.5	14.5		
05:30 Volume	29	183	42	254	43	184	52	279	63	204	39	306	32	152	31	215	1054
Peak Factor																	0.947
High Int.	05:15	PM			05:30	PM			05:15	PM			05:30	PM			
Volume	21	197	41	259	43	184	52	279	65	224	39	328	32	152	31	215	
Peak Factor				0.966				0.928				0.911				0.885	

APPENDIX B-2

WEEKEND CONDITIONS

File Name : HazelRiver Site Code : 00000000 Start Date : 3/17/2007

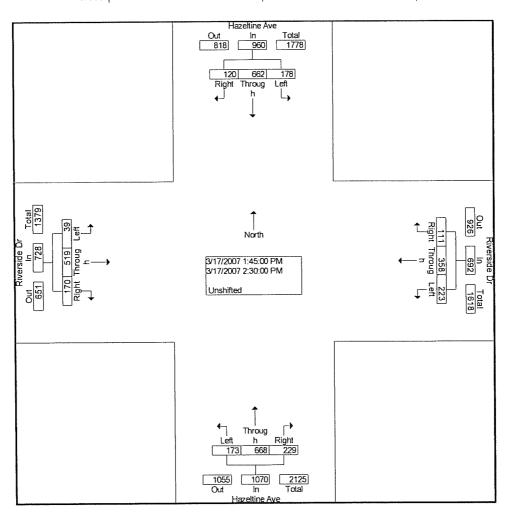
Page No : 1

Groups Printed- Unshifted

	Н	azeltine Ave		R	liverside Dr		Н	azeltine Ave	9		Riverside Dr		
	5	Southbound		ν	Vestbound		١	Northbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	54	145	27	39	92	31	52	152	55	9	85	45	786
01:15 PM	41	139	32	57	91	37	45	147	61	19	90	29	788
01:30 PM	53	172	20	52	95	35	40	161	42	17	119	34	840
01:45 PM	40	205	41	54	88	28	43	156	67	9	116	37	884
Total	188	661	120	202	366	131	180	616	225	54	410	145	3298
02:00 PM	43	145	21	53	100	26	41	167	51	5	126	44	822
02:15 PM	43	148	24	51	71	33	45	164	61	15	141	44	840
02:30 PM	52	164	34	65	99	24	44	181	50	10	136	45	904
02:45 PM	39	159	29	59	83	31	46	193	61	13_	124	30	867
Total	177	616	108	228	353	114	176	705	223	43	527	163	3433
Grand Total	365	1277	228	430	719	245	356	1321	448	97	937	308	6731
Apprch %	19.5	68.3	12.2	30.8	51.6	17.6	16.8	62.2	21.1	7.2	69.8	23.0	
Total %	5.4	19.0	3.4	6.4	10.7	3.6	5.3	19.6	6.7	1.4	13.9	4.6	

File Name: HazelRiver Site Code: 00000000 Start Date: 3/17/2007

			ine Ave				side Dr tbound				ine Ave				side Dr bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	n 01:00 l		2:45 PM		of 1	9		, 0.0.				, , , , ,		<u> </u>			
Intersection	01:45 F	PM															
Volume	178	662	120	960	223	358	111	692	173	668	229	1070	39	519	170	728	3450
Percent	18.5	69.0	12.5		32.2	51.7	16.0		16.2	62.4	21.4		5.4	71.3	23.4		
02:30 Volume	52	164	34	250	65	99	24	188	44	181	50	275	10	136	45	191	904
Peak Factor																	0.954
High Int.	01:45	PM			02:30 F	PM			02:30 1	PM			02:15 l	PM			
Volume	40	205	41	286	65	99	24	188	44	181	50	275	15	141	44	200	
Peak Factor				0.839				0.920				0.973				0.910	



File Name : HazelFashion Site Code : 00000000

Start Date : 3/17/2007

Page No : 1

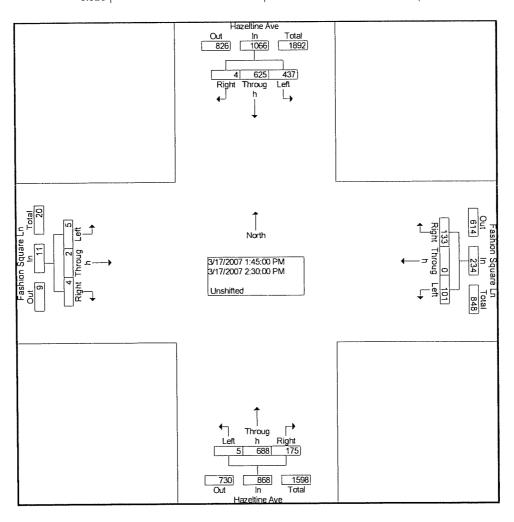
Groups Printed- Unshifted

	Н	azeltine Ave		Fash	nion Square	. Ln	Н	azeltine Ave			nion Square	Ln	
	5	Southbound		٧	Vestbound		<u> </u>	Vorthbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	87	156	1	15	0	26	0	174	35	2	1	1	498
01:15 PM	84	141	0	10	0	19	1	186	39	0	0	0	480
01:30 PM	98	165	0	26	0	21	0	176	47	1	0	1	535
01:45 PM	113	172	2	23	0	40	0	189	44	1	1	1	586
Total	382	634	3	74	0	106	1	725	165	4	2	3	2099
02:00 PM	98	147	1	26	0	28	4	172	51	2	0	0	529
02:15 PM	102	145	1	23	0	36	1	167	41	1	1	1	519
02:30 PM	124	161	0	29	0	29	0	160	39	1	0	2	545
02:45 PM	107	153	1	34	1	32	1	180	49	1	0	0	559
Total	431	606	3	112	1	125	6	679	180	5	1	3	2152
Grand Total	813	1240	6	186	1	231	7	1404	345	9	3	6	4251
Apprch %	39.5	60.2	0.3	44.5	0.2	55.3	0.4	80.0	19.6	50.0	16.7	33.3	
Total %	19.1	29.2	0.1	4.4	0.0	5.4	0.2	33.0	8.1	0.2	0.1	0.1	

File Name : HazelFashion Site Code : 00000000

Start Date : 3/17/2007

		Hazelt	ine Ave		F	ashion	Square L	ח		Hazeli	ine Ave		F	ashion	Square L	.n	
		South	bound			West	bound			North	bound			East	bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	1 01:00 J		2:45 PM	- Peak 1	of 1												
Intersection	01:45 F	PM															
Volume	437	625	4	1066	101	0	133	234	5	688	175	868	5	2	4	11	2179
Percent	41.0	58.6	0.4		43.2	0.0	56.8		0.6	79.3	20.2		45.5	18.2	36.4		
01:45	113	172	2	287	23	0	40	63	0	189	44	233	1	1	1	3	586
Volume	113	112		201		Ů	, ,	•									
Peak Factor																	0.930
High Int.	01:45 F	PM			01:45	2М			01:45	PM -			01:45	PM			
Volume	113	172	2	287	23	0	40	63	0	189	44	233	1	1	1	3	
Peak Factor				0.929	1			0.929				0.931				0.917	



File Name: WoodRiver Site Code: 00000000

Start Date : 3/17/2007

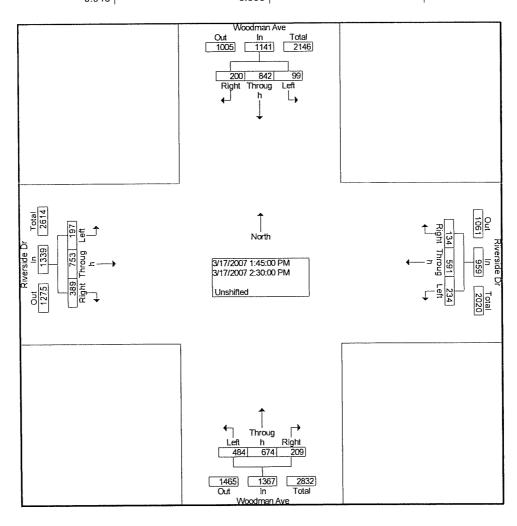
Page No : 1

Groups Printed- Unshifted

	W	oodman Ave	2	R	liverside Dr		Wo	odman Ave	9	R	iverside Dr		
	S	outhbound		٧	Vestbound		N	lorthbound		E	astbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	24	223	63	51	158	34	134	212	39	42	135	74	1189
01:15 PM	27	229	58	60	146	22	118	169	38	52	155	88	1162
01:30 PM	28	204	40	64	176	32	118	193	31	39	168	74	1167
01:45 PM	28	219	54	63	147	32	124	167	48	41	164	95	1182
Total	107	875	215	238	627	120	494	741	156	174	622	331	4700
02:00 PM	27	213	50	65	168	35	110	149	57	49	198	106	1227
02:15 PM	16	216	52	58	139	32	122	180	49	56	184	93	1197
02:30 PM	28	194	44	48	137	35	128	178	55	51	207	95	1200
02:45 PM	33	189	60	54	135	29	122	182	76	42	169	81	1172
Total	104	812	206	225	579	131	482	689	237	198	758	375	4796
Grand Total	211	1687	421	463	1206	251	976	1430	393	372	1380	706	9496
Apprch %	9.1	72.7	18.2	24.1	62.8	13.1	34.9	51.1	14.0	15.1	56.1	28.7	
Total %	2.2	17.8	4.4	4.9	12.7	2.6	10.3	15.1	4.1	3.9	14.5	7.4	

File Name: WoodRiver Site Code: 00000000 Start Date: 3/17/2007

			nan Ave				side Dr tbound		***		nan Ave ibound				side Dr bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	1 01:00		2:45 PM	- Peak 1	of 1												
Intersection	01:45 F	PM															
Volume	99	842	200	1141	234	591	134	959	484	674	209	1367	197	753	389	1339	4806
Percent	8.7	73.8	17.5		24.4	61.6	14.0		35.4	49.3	15.3		14.7	56.2	29.1		
02:00	27	213	50	290	65	168	35	268	110	149	57	316	49	198	106	353	1227
Volume	21	213	30	230	03	100	00	200	,,,		0,	0.0	, ,				
Peak Factor																	0.979
High Int.	01:45	⊃M			02:00 F	PM			02:30	PM			02:00 1	PM			
Volume	28	219	54	301	65	168	35	268	128	178	55	361	49	198	106	353	
Peak Factor				0.948				0.895				0.947				0.948	



File Name: WoodNB101 Site Code: 00000000

Start Date : 3/17/2007

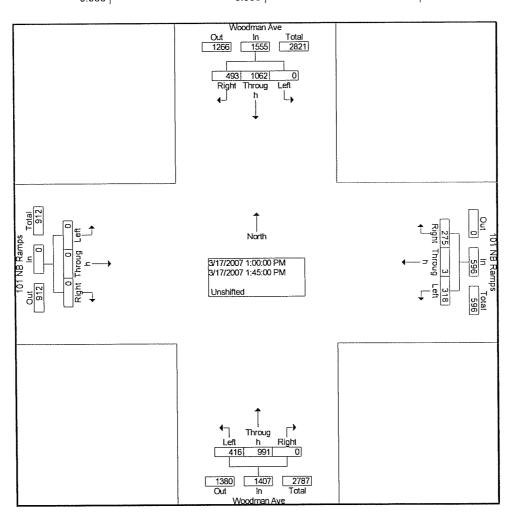
Page No : 1

Groups Printed- Unshifted

	W	oodman Av	e	10	1 NB Ram	os	W	oodman Av	re	10	1 NB Ramp	s	
	S	Southbound		ν	Vestbound		1	Vorthbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	0	255	122	74	2	82	106	268	0	0	0	0	909
01:15 PM	0	271	126	83	1	80	118	232	0	0	0	0	911
01:30 PM	0	287	118	73	0	63	87	250	0	0	0	0	878
01:45 PM	0	249	127	88	0	50	105	241	0	0	0	0	860
Total	0	1062	493	318	3	275	416	991	0	0	0	0	3558
00.00.714	0	252	122	54	2	73	100	215	0	0	0	0	818
02:00 PM	0	252			2	60	100	251	0	0	0	o l	903
02:15 PM	0	278	118	95	!				- 1	0	0	0	892
02:30 PM	0	269	149	78	1	65	68	262	0	Ü	-	- 1	
02:45 PM	0	237	115	104	11	69	91	326	0	0	00	0	943
Total	0	1036	504	331	5	267	359	1054	0	0	0	0	3556
Grand Total	0	2098	997	649	8	542	775	2045	0	0	0	0	7114
Apprch %	0.0	67.8	32.2	54.1	0.7	45.2	27.5	72.5	0.0	0.0	0.0	0.0	
Total %	0.0	29.5	14.0	9.1	0.1	7.6	10.9	28.7	0.0	0.0	0.0	0.0	

File Name: WoodNB101 Site Code: 00000000 Start Date: 3/17/2007

			man Ave				Ramps				nan Ave				Ramps bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	01:00		2:45 PM	- Peak 1	of 1						income in the second						
Intersection	01:00 F	PM															
Volume	0	1062	493	1555	318	3	275	596	416	991	0	1407	0	0	0	0	3558
Percent	0.0	68.3	31.7		53.4	0.5	46.1		29.6	70.4	0.0		0.0	0.0	0.0		
01:15 Volume	0	271	126	397	83	1	80	164	118	232	0	350	0	0	0	0	911
Peak Factor																	0.976
High Int.	01:30 I	PM			01:15 F	PM			01:00 F	PM			12:45:	00 PM			
Volume	0	287	118	405	83	1	80	164	106	268	0	374					
Peak Factor				0.960				0.909				0.941				1	



File Name: WoodSB101 Site Code: 00000000

Start Date : 3/17/2007

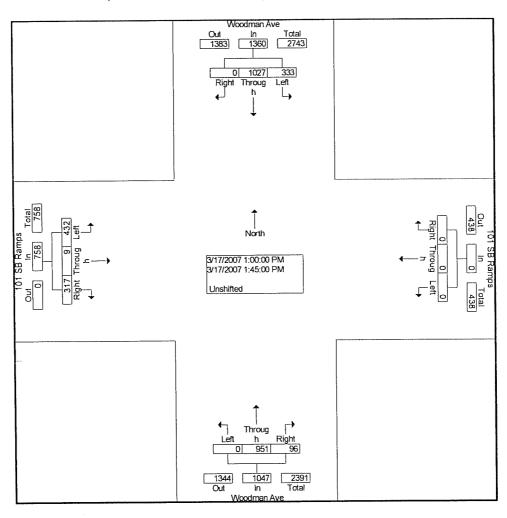
Page No : 1

Groups Printed- Unshifted

	W	oodman Av	e	10	1 SB Ramp	s	Wo	odman Ave	9	10	1 SB Ramp	S	
	S	Southbound		٧	Vestbound		N	lorthbound			Eastbound		1000000000
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	88	224	0	0	0	0	0	229	32	140	3	95	811
01:15 PM	71	286	0	0	0	0	0	238	14	109	2	80	800
01:30 PM	91	270	0	0	0	0	0	236	17	93	3	69	779
01:45 PM	83	247	0	0	0	0	0	248	33	90	11	73	775
Total	333	1027	0	0	0	0	0	951	96	432	9	317	3165
02:00 PM	90	221	0	0	0	0	0	205	14	92	2	79	703
02:15 PM	80	288	0	0	0	0	0	246	12	99	2	102	829
02:30 PM	73	267	0	0	0	0	0	221	15	106	3	91	776
02:45 PM	73	263	0	0	0	0	0	261	10	147	11	84	839
Total	316	1039	0	0	0	0	0	933	51	444	8	356	3147
Grand Total	649	2066	0	0	0	0	0	1884	147	876	17	673	6312
Apprch %	23.9	76.1	0.0	0.0	0.0	0.0	0.0	92.8	7.2	55.9	1.1	43.0	
Total %	10.3	32.7	0.0	0.0	0.0	0.0	0.0	29.8	2.3	13.9	0.3	10.7	

File Name: WoodSB101 Site Code: 00000000 Start Date: 3/17/2007

			nan Ave				Ramps bound				man Ave bound				Ramps bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	01:00	PM to 02	:45 PM	- Peak 1	of 1												
Volume	01:00 F 333	1027	0	1360	0	0	0	0	0	951	96	1047	432 57.0	9 1.2	317 41.8	758	3165
Percent	24.5	75.5	0.0		0.0	0.0	0.0		0.0	90.8	9.2		37.0	1.2	41.0	1	
01:00 Volume	88	224	0	312	0	0	0	0	0	229	32	261	140	3	95	238	811
Peak Factor High Int.	01:30 I	PM		:	12:45:0	00 PM			01:45 F	PM			01:00	PM			0.976
Volume Peak Factor	91	270	0	361 0.942	0	0	0	0	0	248	33	281 0.931	140	3	95	238 0.796	



File Name: MatRiver Site Code: 00000000

Start Date : 3/17/2007

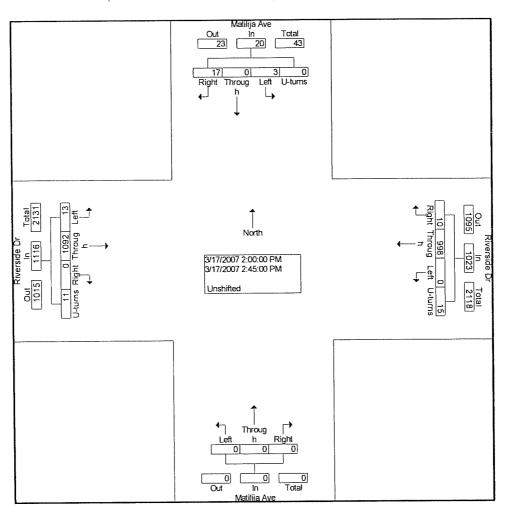
Page No : 1

Groups Printed- Unshifted

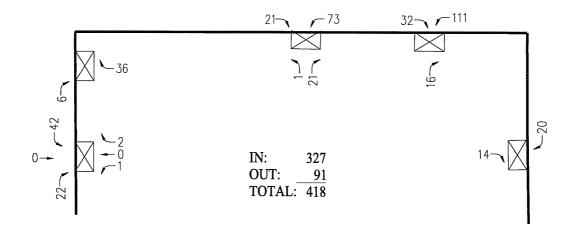
		Matilija	a Ave			Rivers	ide Dr		N	latilija Av	е		Riversi			
		South	oound			West	oound		N	orthboun	d		Eastb	ound		
Start Time	Left	Throu g h	Right	U- turns	Left	Throu g h	Right	U- turns	Left	Throu g h	Right	Left	Throu g h	Right	U- turns	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	1	0	15	2	0	256	4	1	0	0	0	7	229	0	2	517
01:15 PM	1	0	6	2	0	250	3	2	0	0	0	5	222	0	0	491
01:30 PM	0	0	8	2	0	257	3	1	0	0	0	5	229	0	3	508
01:45 PM	1	0	1	0	0	239	2	2	0	0	0	2	279	0	3	529
Total	3	0	30	6	0	1002	12	6	0	0	0	19	959	0	8	2045
02:00 PM	2	0	4	0	0	256	0	3	0	0	0	1	261	0	4	531
02:15 PM	1	0	9	0	0	244	3	0	0	0	0	4	295	0	3	559
02:30 PM	0	0	3	0	0	234	4	9	0	0	0	4	251	0	3	508
02:45 PM	0	0	1	0	0	264	3	3	0	0	0	4	285	0	1	561
Total	3	0	17	0	0	998	10	15	0	0	0	13	1092	0	11	2159
Grand Total Apprch % Total %	6 10.2 0.1	0 0.0 0.0	47 79.7 1.1	6 10.2 0.1	0.0 0.0	2000 97.9 47.6	22 1.1 0.5	21 1.0 0.5	0.0 0.0	0.0 0.0	0 0.0 0.0	32 1.5 0.8	2051 97.6 48.8	0.0 0.0	19 0.9 0.5	4204

File Name: MatRiver Site Code: 00000000 Start Date: 3/17/2007

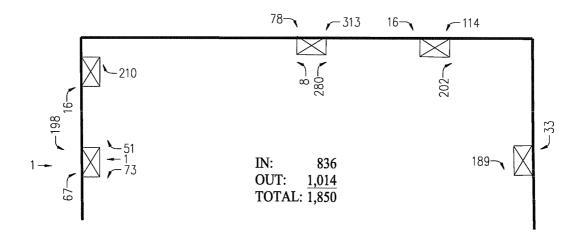
			Aatilija A					iverside Vestbou		••••			ija Ave nbound				verside astbou			
Start Time	Left	Thro ug h	1 -	U- turn s	App. Total	Left	Thro ug h	Righ t	U- turn s	App. Total	Left	Thro ug h	1 9 1	App. Total	Left	Thro ug h	Righ t	U- turn s	App. Total	Int. Total
Peak Hour Fr	om 01:	00 PM	to 02:4	5 PM -	Peak 1	of 1														
Intersection	02:00	PM																		
Volume	3	0	17	0	20	0	998	10	15	1023	0	0	0	0	13	109 2	0	11	1116	2159
Percent	15.0	0.0	85.0	0.0		0.0	97.6	1.0	1.5		0.0	0.0	0.0		1.2	97.8	0.0	1.0	1	
02:45 Volume Peak	0	0	1	0	1	0	264	3	3	270	0	0	0	0	4	285	0	1	290	561 0.962
Factor																				
High Int.	02:15	PM				02:45	PM				12:45	:00 PM	1		02:15	PM				
Volume	1	0	9	0	10	0	264	3	3	270	0	0	0	0	4	295	0	3	302	
Peak Factor					0.500					0.947									0.924	



APPENDIX C
FASHION SQUARE DRIVEWAY TRAFFIC COUNTS



AM PEAK HOUR



PM PEAK HOUR



FIGURE C-1 EXISTING SITE TRAFFIC VOLUMES

NOVEMBER 16, 2005

WESTFIELD FASHION SQUARE EXPANSION PROJECT

o:\job_file\3606\dwg\appnd-c1.dwg LDP 07:55:03 08/06/2008 rodriquez

LINSCOTT, LAW & GREENSPAN, engineers

File Name : 355501Dwy Site Code : 03555001 Start Date : 11/16/2005

															, ago		
	<u>:</u>							nted-Tur	ning Mo	vement			,			 -	
.: .:	٠ ١	(AZELT)	NÉ AVE	Ξ.	DWY		ashion S	quare		HAZELT	INE AVE						
1	i '	South		•			n.		·		bound	-		Éast	bound ·	1	
a						vvest	bound	<u></u>	,	,	r	Ann			-	App.	Int.
Start Time	Right	Thru	Left	App.	Right	Thru	Left	App.	Right	Thru	Left	App. Total	Right	Thru	Left	Total	Total
		 -		Total	¦ ·;			Total		1 7 7		" i orai	1.0	1.0	1.0	1000	- <u>10001</u>
<u>Factor</u>	1.0	1.0 !	1.0		1.0 ;	1.0	1.0		1.0	[1.0 i	1.0			0	1.01	0	1
07:00 AM	0	0	0	0	1	ō	0	1		0	0	Ó	0	-		ő	
07:15 AM	0	0	0	0	2	Ö	0	2	0	0	0	0	0	0	0		2 2
07:30 AM	0	Ō	0	0	2	0	Ò	2	0	٥	0	0	0	0	Ō	0	
07:45 AM	0	0	0	0	4	0	0	4	1	0	0	1	0	0.	0	0	5_
Total	Ō	0	0	0	9	0	0	9	1	Ö	0	1	0	0	0	0	10
08:00 AM	0	0	0	0	8	0	0	8 أ	0	0	0	0	0	0	0	0	8
08:15 AM	0	0	0	0	2	0	0	2	1	0	0	1 :	0	0	٥	0	3
MA 05:80	0	0	0	0	3	0	0	3	1	ø	0	1	Ö	0	Q	0	4
08:45 AM	. 0	Ó	O	0	6	0	0	6	0	0	0	0	٥	0	0	0	6
Total	0	0	0	0	19	0	0	19	2	0	Q	2	0	0	Ō	0	21
09:00 AM	0	0	0	Ó	6	Ó	0	6	1	0	0	1	O	0	0	0	7
09:15 AM	0	0	0	0	12	0	0	12	1	0	0	1	0	0	0	0;	13
09:30 AM	ŏ	ŏ	ŏ	ŏ	11	ŏ	ŏ	11	2	ō	ō	2	Ô	0	0.	0	13
09:45 AM	ã	ŏ	٥	ő	7	Ö	ŏ	7	2	õ	ŏ	2	ó	ō	ō	ō	9
Total	0	ō	Ö	0	36	Ö	ŏ	36	6	ŏ	Ö	6			Ö,	Ŏ İ	42
*** BRFAK **	· eb				•				•								
*** BREAK **	·· ··																
03:00 PM	0	0	0	0	43	0	Q	43 !	2	0	0	2	0	0	0	0	45
03:15 PM	0	0	0	0	53	O	Ó	53	3	0	0	3⁺	0	0	0	0	56
03:30 PM	ō	ō	ō	ō	54	ō	ō	54	6	ō	Ō	6	0	0	0	0	60
03:45 PM	Õ	ŏ	Ö	Ö		ŏ	ŏ	31	7	,ŏ	0	7	0	0	.0	0 :	38
Total	Ō	0	0	Ö	181	Ō	0	181	18	Ö	0	18	0	0	0	0]	199
04:00 PM	0	0	0	o	46	o	O	46	6	0	0	6	0	0	0	0	52
04:15 PM	ŏ	ŏ	ŏ	Õ	42	õ	Ö	42	3	ŏ	ŏ	3	ŏ	ō	ō	0	45
04:30 PM	ő	ő	ŏ	0	50	Õ	ŏ	50	4	ő	õ	4	ŏ	ŏ	ŏ	ō	54
04:45 PM	ő	ŏ	ŏ	ő	72	.0	ŏ	72	;	ŏ	ŏ	3	Ö,	ō	ō	0	75
Total	0	ő	0	0	210	0	0	210		0	0	16	. ö	Ö	0	0	226
05:00 PM	. 0	0	0	0	39	0	0	39	5	0	0	5	٥ ا	0	0	0 !	44
05:15 PM	Ö	ő	ŏ	0	38	0	0	38	6	Ö	Ö	6	ő	ő	o.	ō	44
05:30 PM	Ö	ő	0	0		Ö	0	38	3	0	Ö	3	ŏ	ő	ŏ	Ö	41
	_						-		1	_			_		ŏ.	. 0	49
05:45 PM Total	0	<u>0</u>	0	0	48 163	0	0	48	1 15	0	0	1. 15	0	. <u> 0</u>	0.	. 0	178
rotar	U	U	Ų	Ü	103	U	U	163	15	U	U	13	į	U	υ.	U [. 1/0
Grand Total	Ò	0	. 0	0	,	0	0	618		0	0	58	0	O	. 0	.0	676
Apprch %	0.0	0.0	0.0		100.	0.0	0.0		100.	0.0	0.0		0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0		0.0	0.0	91.4	8.6	0.0	0.0	8.6	0.0	0.0	0.0	0.0 !	

	- 1 m 2 m 2 m 2 m 1 m 1	HAZELT South	INE AVI	=	DWY		ashion S n. bound	Square	ş	AZELT North	INE AV			Eastl	ound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 07:0	O AM to	09:45	AM - Pea	k 1 of 1										*	•	
Intersection		AM							i								
· · Volume	. 0.	ø	Q.	0	36	O	Ó	36	6	Ö	0	6	0	0	0	0	42
Percent	0.0	0.0	0.0		100. 0	0.0	0.0		100. 0	0.0	0.0		0.0	0.0	0.0.		
09:30 Volume		0	O'	0	11	0	0	11	2	0	0	2	o	0	Q .:	0	13
Peak Factor															•		0.808
High Int.	6:45:0	MA OC			09:15	AM			09:30	AM			6:45:0	MA O			
Volume	0	O	0	0	12	0	0	12	2	¢	O	2	1				
Peak Factor	i							0.750				0.750					

FROM : Accutek 909 595 6022

Nov. 22 2005 06:31PM P35

<< ACCUTEK >> << 21114 TRIGGER LANE >> << DIAMOND BAR, CA 91765 >> << (909) 595-6199 FAX: (909) 595-6022 >

File Name : 355501Dwy Site Code : 03555001 Start Date : 11/16/2005 Page No : 2

	· HAZELTINE AVE. Southbound					DWY 1-n/o Fashion Square Ln. Westbound					INE AVE						
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. <u>Total</u>	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00	PM to	05:45	PM Pea	k 1 of 1												
Intersection	04:00	PM															
Volume	0	Q.	O	0	210	0	Q	210	16	Ò	0	16	0	0	0	0	226
Percent	0.0	0.0	0.0		100.	0.0	0.0		100.	0.0	0.0		0.0	0.0	0.0		
04:45 Volume	0	0	0	0	72	0	0	72	3	0	0	3	0	0	٥.	0	75
Peak Factor					[į	0.753
High Int.	•				04:45	PM			04:00	PM			İ				
Volume	Ö	0	0	0	72	Q	Q	72	6	0	0	6					
Peak Factor								0.729				0.667					

File Name : 355502DWY Site Code : 03555002 Start Date : 11/16/2005 Page No : 1

• • • • • • • • • • • • • • • • • • • •	•					Gro	ouos Pric	ited- Tu	roina Mi	ovemer	nt .				Page r	10 : 1	
	Southbound				F	RIVERSI	DE DRI		DWY	/ 2-e/o Parking	Ranchite Structur		R				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left.	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	• :	
07:00 AM	0	0	0	Ó	0	0	4	4	1	0	0	1	1	Ō.	0	1:	6
07:15 AM	0	0	0	Q.	0	0	7	7	1	0	1	2	4	0	0	4	13
07:30 AM	0	0	0	Ó	0	0	5	5	0	0	Q	Q	9	Ò	0	9	14
07:45 AM	O O	0	0	0	0	0	.6	6.	0_	0	Q	0	3	Ō	. 0_	3	9
Total	0	0	0	0	0	0	22	22	2	0	1	. 3	17	0	0	17	. 42
08:00 AM	0	0	0	0	٥	0	8	8	2	0	0	2	2	0	0	2	· 12
08:15 AM	0	0	0	Q	0	0	5	5	0	0	1	1	4	0	Ò	4	10
08:30 AM	Ō	ō	0	0	0	0	4	4	1	0	0	1	2	Ō	O	2	7
08:45 AM Total	0_	0	0	0	0	<u> </u>	16	16	1	0	0	1	1	0.	0		18
	U	Ų	v	0	. 0	Ü	33	33	4	0	1	5	9	0	0	. 9 j	47
MA 00:00	Q	0	0	0	0	Ö	21	21	1	0	0	1	5	0	Ò	5	27
09:15 AM	0	0	0	0 ;	0	Q	11	11	9	0	O	9	7	0	0	7	27
09:30 AM	0	0	0	0 :	•	0	25	25	10	0	1	11	8	0	0	8	44
09:45 AM	0	0	0	. 0;	0	Ò	54	54	10	<u>0</u>	0	10	14	0	0.	14	78
Total	0	Ó	0	0	0	0	111	111	30	0	1	31	34	0	0	34	176
*** BREAK **	*																
03:00 PM	0	0	0	0	Q	Q	62	62	66	0	2	68	28	0	0	28	158
03;15 PM	0	0	0	0	٥	Ó	82	82	75	0	4	79	20	0	0	20	181
03:30 PM	Ó	0	O	0	0	0	73	73	81	O	0	81 .	18	0	0	18	172
03:45 PM	0	0	0	0	0	0_	79	79	60	0	2	62	. 19	0	0	19	·160
Total	0	0	0	0	0	0	296	296	282	0	8	290	85	Ó	0	. 85	671
04:00 PM	0	0	0	0	0	0	79	79	64	0	2	66	21	Q	0	21	166
04:15 PM	Ò	0	0	0	0	0	60	60	54	0	0	54	18	0	Q.	18	132
04:30 PM	O.	0	0	0	0	0	54	54	60	0	0	60	18	0	0.	18 .	132
04:45 PM Total	<u> 0</u>	0	0	0 !	<u>, 0</u>	0	73	. 73	69	0	. 2	71	1.7	0	0	17	161
iotai	V	U	U	0 ;	U	0	266	266	247	O	4	251 ¦	74	0	0	74	591
05:00 PM	0	0	Ō	0	0	0	73	73	54	0	2	56	24	O	Ó	24	153
05:15 PM	0	0	0	0	0	0	60	60	62	0	3	65	22	0	0.	22	147
05:30 PM	Ó	0	Ō	0	0	0	57	57	61	0	3	64	16	0	O.	16	137
05:45 PM	<u> </u>	0	0	<u>o l</u>	0	0		77	63	0	0	63	<u>25</u> 87	0	0	25	165
Total	0	0	0	0	0	0	267	267	240	0	8	248	87	0	0	87	602
Grand Total	O	Ō	0	0	Ō	0	995	995	805	0	23	828	306	0	0	306	2129
Apprch %	0.0	0.0	0.0		0.0	0.0	100. 0	į	97.2	0.0	2.8		100. 0	0.0	0.0	į	
Total %	0.0	0.0	0.0	0.0	0.0	0.0	46.7	46.7	37.8	0.0	1.1	38.9	14.4	0.0	0.0	14.4	

	FITANIAN	South	nbound		R		DE DRIV	'E		arking.	Ranchito Structure bound		Ŕ				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Rlght	Thru	Left	App. Total	Right	Thru	Left	App. :- Total :	Int. Total
Peak Hour From	m 07:00	AM to	09:30	AM - Pea	k 1 of 1						**** **** **** ****						
Intersection									!			-				1	
Volume	0	. 0	0	0	0	0	73	73	21	0	1	22	21	0	0	21	116
Percent '	0.0	0.0	0.0		0.0	0.0	100. 0		95.5	0.0	4.5		100. 0	0.0	0.0		
09:30 Volume	0	0	Ō	0	Q	0	25	25	10	0	1	11	8	0	0	8 ;	44
Peak Factor	•															į,	0.659
High Int.	6:45:00 AM				09:30 AM				99:30 AM				09:30				
. Volume .	. ' 0	0	Q.	Q	0	0	25	25	10	0	1	11	8	0	0	8	

FROM : Accutek 909 595 6022

PHONE NO. : 19095956022

Nov. 22 2005 06:32PM P37

Peak Factor				; i		0.730 ;						0.500	0.656 ;				
Peak Hour From	n 02:45 03:15 f		05:45 P	M - Peal	k 1 of 1			!					ļ				1
Volume	0	0	0	o	Q	Q	313	313	280	0	8	288	78	0	0	78	679
Percent	0.0	0.0	0.0		0.0	0.0	100. 0		97.2	0.0	2.8		100. O	0.0	0.0		
03:15 Volume Peak Factor	0	0	O	0	0	0	82	82	75	0	· 4	79	20	0	0	20	181 0.938
High Int.				İ	03:15 F	M			03:30	PΜ		İ	04:00 F	M		į	0.936
Volume Peak Factor	0	0	O	0	0	0	82	82 0.954	81	0	0	81 0.889	21	0	0.	21 0.929	

File Name: 355503DWY Site Code : 03555003 Start Date : 11/16/2005 Page No : 1

****WL2 = Traffic to the W/Side Driveway, NR1 = Traffic from the W/Side Driveway to the Riverside, NR2 = Traffic from the W/Side Driveway to the E/Side Driveway***

. Balta and annual		South	pond		F		DE DRIV	/E	DWY 3-e/o Parking Structure Northbound				F				
Start Time	Right	Thru	Left;	App.	Right	WL2	Left	App.	Right	NR1	NR2	App. Total	Right	Thru	bound Left.	App. Total	Int. Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	0	0	0	0	"Ò"	14	4	18	0	0	0	0	1	0	0	1	19
07:15 AM	0	O	٥	Ò	0	7	5	12	0	Q	0	Q	8	Ö	ō.	8 !	20
07:30 AM	0	0	Ò	0 ;	0	14	9	23	2	Ó	Ó	2	8	0	٥	. 8	33
07:45 AM	0	0	0	0	Q.	11	8	19	2	1	0	3	5	Q	Ó	5	27
Total	0	ø	O.	0	0	46	26	72	4	1	0	5	22	0	Ō	22 ;	99
08:00 AM	0	0	0	O ;	0	18	13	31	1	3	0	4	3	0	0.	3 :	38
08:15 AM	0	O	0	0	Ò	17	6	23	2	1	1	4	4	Ó	O	4	31
MA 08:80	O	٥	0	Ò ;	Ó	11	3	14	0	1	0	1	3	¢	Ó	3	18
08:45 AM	. 0	0	0	. 0	0	20	8	28	3	2	0	5	3	0	0	3	36
Total	0	0	0	0	Q	66	30	96	6	7	1	14	13	0	0	13	123
09:00 AM	ō	0	0	0	0	22	15	37	4	0	3	7		0	0	5	49
09:15 AM	0	Ō	0	0	Q	23	22	45	5	6	2	13	7	0	0	7	65
09:30 AM	0	Ó	O	0	0	22	19	41	4	5	0	9	8	0	0	8	58
09:45 AM Total	. <u>0</u>	<u> </u>	. <u>0</u>	0	0	44	18	62	3	5	0	8	12	0	0	12 !	82
	_	Ų	U	0	0	111	74	185	16	16	5	37	32	0	0	32 ;	254
** BREAK ***	•															•	
03:00 PM	0	O	0	0	0	26	26	52	16	43	2	61	6	0	0.	6	119
03:15 PM	0	0	0	O i	0	23	35	58	20	70	3	93	4	ŏ	Ŏ	4	155
03:30 PM	0	Ó	0	0 !	0	34	32	66	24	50	6	. 80	2	Ö	ō	2	148
03:45 PM	Ò	0	0	0 !	Ò	31	23	54	25	39	3	67	4	Q	0	4	125
Total	0	0	0	0	0	114	116	230	85	202	14	301	16	0	0.	. 16	547
04:00 PM	0	Ó	0	0	0	26	19	45	13	40	2	55	3	0	0	3 !	103
04:15 PM	0	0	0	0 !	O	35	22	57	11	26	1	38	5	Ó	0 .	5	100
04:30 PM	0	Ò	0	0 }	0	38	28	66	23	42	2	67	10	٥	0.	10	143
04:45 PM	0	0	00	0	0	. 31	. 19	50	21	33	0	54	6	0	0	6	110
Total	. 0	0	0	0	0	130	88	218	68	141	5	214	24	0	0	24	45 6
05:00 PM	0	0	0	0	0	34	29	63	19	39	1	59	6	0	0	6	128
05:15 PM . 05:30 PM	. 0	0	0	0	0	35	28	63	23	29	3	55	2	Q	O,	2	120
05:30 PM 05:45 PM	. 0	0	0	0	0	26	20	46	21	43	2	66	4	0	<u>o :</u>	4	116
Total	0	-0	0	<u>o</u>	0	40	26	66	12	38	2	52.	10	0	0	10 .	128
•	•	-	U	0	Ų	135	103	238	75	149	8	232	22	0	0.	. 22	492
Grand Total	0	0	0	0	0	602	437	1039	254	516	33	803	129	0	0	129	1 9 71
Apprch %	0.0	0.0	0.0		0.0	57.9	42.1		31.6	64.3	4.1	:	100. 0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	30.5	22.2	52.7	12.9	26.2	1.7	40.7	6.5	0.0	0.0	6.5	

		Souti	nbound		F		DE DRIV	Ē	WY.		arking Str bound	ucture	R] :			
			Left	App. Total	Right	WL2	Left	App. Total	Right	NR1	NR2	App. Total	Right		Left	App. Total	Int. Total
Peak Hour Fron	n 07:00	AM to	09:45	AM - Pea	k 1 of 1				· · · · · · · · · · · · · · · · · · ·				···································	··	, , , , , , , , , , , , , , , , , , , ,		•
Intersection	09:00	AM.											1		-		
Volume	. 0	. 0	0	0	Q.	111	74	185	16	16	5	37	32	O	0;	32	. 254
Percent	0.0	0.0	0.0		0.0	60.0	40.0		43.2	43.2	13.5		100.	0.0	0.0		
09:45. Volume		0	0	0	٥	44	18	62	3	5	٥	8	12	0	0	12	82
Peak Factor		.											:				0.774
Hìgh Int.	6:45:0	U AM			09:45	AM		:	: 09:15 .	AM			09:45 .	AM			İ

PHONE NO.: 19095956022

Nov. 22 2005 06:33PM P39

Volume Peak Factor	0	0	0	o :	0	44	18	62 0.746	5	6	2	13 0.712	12	0	0	12 0.667	
Peak Hour From Intersection	n 03:00 l 03:00 Pl		05:45 PI	M - Peal	< 1 of 1			ļ					ļ			į	
Volume	0	0	0	0	0	114	116	230	85	202	14	301	16	0	٥	16	547
Percent	0.0	0.0	0.0		0.0	49.6	50.4	;	28.2	67.1	4.7		100. 0	0.0	0.0		
03:15 Volume	0	0	0	0	0	23	35	58	20	70	3	93	4	0	0.	4	155
Peak Factor High Int. Volume	0	0	Q	Q	03:30	PM 34	32	66	03:15 20	РМ 70	3	93	03:00 F 6	M Q	0	6	0.882
Peak Factor				!				0.871				0.809				0.667	

07:30

High Int.: 07:30 AM

Volume

Volume

Peak Factor

Peak Factor

0

0

0

8

8

0

0

0

6:45:00 AM

0

8

8

0.625

0

0

0

0

6:45:00 AM

0

0

0

0

0

0

0

7

07:30 AM 7

0

0

0

0

7

7

0.500

15

0.567

<< ACCUTEK >> << 21114 TRIGGER LANE >> << DIAMOND BAR, CA 91765 >> << (909) 595-6199 FAX: (909) 595-6022 >

File Name : 355504Dwy Site Code : 03555004 Start Date : 11/16/2005

															Start Page	Date:1 No:1	1/16/200
						Gro	ups Prin	ted- Tu		ovemen							
`` · `` · i'	ļ · '' ''	MOODI		/E	i						1AN AVE	,	DWY		101 NB I	Ramps	•
	<u> </u>		iponüğ	Ann	 -	West	oound	۸۸۸	 -	North	bound	Ann	1	East	bound	Ann.	Int.
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left :	App. Total	Right	Thru	Left.	App. Total	Total
Factor	1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1 1 1 1
07:00 AM	4	0	0	4	. 0	0	0	0	0	0	0	0		0	Ö	2	6
07:15 AM	6	0	0	6	0	0	O	0	. 0	0	0	0	: 4	0	0	4	10
07:30 AM 07:45 AM	8 2	0	0	8	•	0	0	0	0	0	0	0	7	0	0	7	15
Total	<u>_</u>	<u>0</u>	0	20	. <u>0</u>	0	0	0 0	0	0	0	0	14	0	0	14	<u>3</u> .
						_	_	_		•	•	_		_			
MA 00:80	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
08:15 AM 08:30 AM	2	0	0	2	0	0	0	0	0	0	0	0	4	0	1	5	. 7
08:45 AM	4 3	0 0	0	4 3	0	0	0	0	0	0	0	0	3	0	0	3 2	7 5
Total	9		0	<u>د</u> 9		<u>0</u> .	0	0	0	0 0	. 0	. 0	11	. 0	01	12	21
						-	•	Ū		•	·	•	,	_			
09:00 AM	1	0	0	1		0	0	0	0	0	0	0	4	0	0	4	5
09:15 AM 09:30 AM	4 6	0	0	4	0	0	0	0	0	0	0	0	2	0	0	2	6
09:45 AM	7	0	0	6 7	0	0	0	0	0	0	0	0 0.	5 5	0	0	5 5	11 12
Total	18	0	0	18	Ö		ō	0		0	0	0	16	0 -	0	16	
*** BREAK **	*																
00-00 PM	4.0	_	_		l _				1				ı	_			
03:00 PM 03:15 PM	10	0	0	10	0	0	0	0	0	0	0	0	61	0	0	61	71
03:30 PM	2 15	0	0	2 15	0	0	0	0	0	0	0 0	0	37 39	0	0	37 39	39 54
03:45 PM	9	0	0	15	0	0	0	0	0	<u>Q</u>	., <u>"</u> 0	0	44	0	Ö	39 ; 44 :	53
Total	36	0	0	36		0	0	0	Ö	<u>ö</u> .	<u>0</u>	0	181	0	0	181	217
04:00 PM	16	0	0	16	0	0	0	0	0	0	0	0	48	0	0.	48	64
04:15 PM	6	0	0	6	0	Ö	Ŏ	Ö	o	ō	Ö	Ö	33	Ö	Ö	33	39
04:30 PM	4	0	0	4	0	0	0	0	0	0	0	0	47	0	0.	47	51
04:45 PM Total	30	0	0	4 30	0	0	0	0	0	0	0	0	55 183	0	0	. 55 183	<u>59</u> 213
, IULAI	. 30	U	U	טכ	ı v	U	U	0	U	U	0	0	183	U	U	702	213
05:00 PM	12	0	0	12	0	0	0	0	0	0	0	0	40	0	0	40	52
05:15 PM	13	0	0	13	0	0	0	0	0	0	0	0	47	0	0	47	.60
05:30 PM 05:45 PM	15	0	0	15	0	0	0	0	0	0	0	0	25	0	0.	25 37	40 45
Total	- <u>8</u> 48	0	0	8 48	0	0	<u> </u>	0	0	. O.	0.	0	37 149	0	0	149	45 197
Grand Total	161	٥	-				-	-		_	-						
	100.	_	0	161	!	0	0	0	•	0	0	0		0	1	555	716
Apprch %	0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		99.8	0.0	0.2		
Total %	22.5	0.0	0.0	22.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	77.4	0.0	0.1	77.5	
		WOODN	AANI AT		··					Mach			P1474		01 100 0	Inmes	
	`		bound	L	<u> </u>	Westh	ound				IAN AVE	•	DVV Y4		01 NB F		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	i	Left	App. Total	Int. Total
Peak Hour From	m 07:00	AM to		AM - Pea	k 1 of 1			i Utal	L		•	i U(di	•	!		INDI	, 0.01
Intersection	07:00				i								•			1	
Volume	20	0	0	20	0	0	0	0	0	0	0	0	14	0	0	14	34
Percent	100. O	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		100.	0.0	0.0	Ì	

PHONE NO.: 19095956022

Nov. 22 2005 06:34PM P41

<< ACCUTEK >> << 21114 TRIGGER LANE >> << DIAMOND BAR, CA 91765 >> << (909) 595-6199 FAX; (909) 595-6022 >

File Nama : 355504Dwy Site Code : 03555004 Start Date : 11/16/2005 Page No : 2

•							** * **********************************										I
	, ,	WOOD	MAN AV	/E					'	NOODM	1AN AVE	<u>.</u>	DWY4		01 NB I	kamps	
1			nbound			West	bound			North	bound			Easti	ound		
Start Time	Rìght	Thru	Left	l Δnn	Right	~~	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:01	PM to	05:45	PM - Pea	k 1 of 1											:	
Intersection	04:30	PM															
Volume	33	0	0	33	0	0	0	0	0	0	0	0	189	0	0	189	222
Percent	100.	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		100. 0	0.0	0.0		
05:15 Volume	13	0	0	13	0	0	0	0	0	0	0	0	47	0	0	47	60
Peak Factor High Int. Volume Peak Factor	05:15 13	PM 0	0	13 0.635	0	0	0	0	0	0	0	0	04: 4 5 55	PM 0	0	55 0.859	0,925

PHONE NO. : 19095956022

<< ACCUTEK >> << 21114 TRIGGER LANE >> << DIAMOND BAR, CA 91765 >> << (909) 595-6199 FAX: (909) 595-6022 >

File Name : 355508 Site Code : 00355508 Start Date : 11/16/2005 Page No : 1

•						٥.									Page	e NO :	l .
-1	!	HAZELI	INE AVE	=			guare L	inted-Tur	ning ivid		RA BÜVÜ	5	·	PIOT-	Entrance		
İ			hbound				bound	arie.	;		bound	- 1	į		bound	٠.	. '
Charl The C		1		App,	:		·T	App.	·			App.				App.	Int.
Start Time	Right	Thru	Left	Total	Right	Thru	Left	Total	Right	(Thru	Left	Total	Right	Thru	Left	Total	Total
Factor	1,0	1.0	1.0		1.0	1.0 ;	1.0		1.0	1.0	1.0		1.0	1.0	1.0		
07:00 AM	2	190	13	205	0	0	Ö	0	4	67	2	73	0	0	0	0	278
07:15 AM	4	222	4	230	0	1	ō	1:	. 4	83	Ž	89	i	ō	i	2	322
07:30 AM	3	240	10	253	0	٥	ō	ō	6	129	6	141	ő	ŏ	1	1	395
07:45 AM	0	302	12	314	Ō	ŏ		ō	3	118	3	124	2	ō	Ō	2	440
Total	9	954	39	1002	0	1	<u>0</u>	<u>0</u> 1	17	397	13	427	3	Ō	2	5	1435
MA 00:80	2	316	15	333	1	0	1	2 !	_	129	0	134 i	4	0	0	4!	473
08:15 AM	ž	277	5	284	1	ő	Ó	1	5 8	127	3	134	1	ő	1	2	425
08:30 AM	1	252	12	265	1	ő	1	2	3	113	2	118	Ď	ő	1	1	386
08:45 AM	2	220	13	235	1	Ô	1	2	5	125	2	132	1	0	_	4	373
Total	<u>-</u> -	1065	45	1117			3	7	21	494	7	522		0	3	11	1657
· rotar	,	1003	-7.3	+ + 4 + / ;	- 7	U	J	7 ;	2.1	727	,	322	U	J	,	7.1	1057
09:00 AM	2	241	12	255	3	0	1	4 :	4	98	Q	102	3	2.	1	б	367
09:15 AM	0	196	24	220	3	0	3	6	8	117	1	126	0	1	1	2	354
09:30 AM	1	188	46	235	2	Q	3	5	8	103	5	116	1	0	1	2	358
09:45 AM	3	189	42	234	3	Ö	0	3	. 14	113	1	128	0	0	_i_	1	366
Total	6	814	124	944	11	ġ-	7	18	34	431	7	472	4	3	4	11	1445
*** BREAK **	*																
03:00 PM	1	169	67	237	24	1	12	37	19	211	1.	231	0	0	1	1	506
03:15 PM	0	212	41	253	19	1	24	44	25	201	6	232	3	0	3	6	535
03:30 PM	0	181	39	220	20	0	30	50	20	207	1.	228	1	0	3	4 :	502
03:45 PM	0	137	59	196	10	0	16	. 26	23	223	Ō	246	. 4	<u>Q</u>	2	6	474
Total	1	699	206	906	73	2	82	157	87	842	8	937	8	Q	9	17	2017
04:00 PM	2	194	32	228	19	3	19	41	24	218	Q	242	2	0	1	3	514
04:15 PM	2	203	42	247	12	0	16	28	20	211	3	234	3	1	7	11	520
04:30 PM	. 0	184	49	233	16	0	19	35	19	210	Ō	229	3	0	4	7	504
04:45 PM	0	225	38	263	16	_ 0	20	36 '	22	232	1	255	1	0	8	9	563
Total	4	806	161	971	63	3	74	140 j	85	871	4	960	9	1	20	30	2101
05:00 PM	1	207	41	249	14	0	24	38	20	243	2	265	2	0	2	4 ¦	556
05:15 PM	3	209	52	264	10	ŏ	16	26	20	246	4	270	2 2	ŏ	ō	2 !	562
05:30 PM	Ò	185	47	232	11	ő	13	24	9	255	i	265	3	1	3	7:	528
05:45 PM	ĩ	243	58	302	16	1	20	37	18	230	4	252		Ô	i	3	594
Total	5	844	198	1047	51	1	73	125	67	974	11	1052	. 2	1	6	16	2240
Grand Total	32	5182	773	5987	202	7	239	448 i	311	4009	50	4370	39	5	46	90	10895
Apprch %	0.5	86.6	12.9		45.1	1.6	53.3		7.1	91.7	1.1		43.3	5.6	51.1		
Total %	0.3	47.6	7.1	55.0	1.9	0.1	2,2	4.1	2.9	36.8	0.5	40.1	0.4	0.0	0.4	0.8	

	- 1	HAZEL	TINE AV	Έ,	FE	shìon \$	quare La	ane	·	VENTU	RA BLÜÜ) <u>,</u>	i	P.LOT-	Entrance		
<u></u>		Sout	hbound		i	Wes	lbound			North	pound		:	East	bound		
Start Time	Rìght	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. ! Total
Peak Hour From	m 07:00	AM to	09:45	AM - Pea	k 1 of 1				L. 1				•••				
Intersection	07:30	AM															
Volume	7	1135	42	1184	. 2	O	1	3	22	503	12	537	7	Q	2	9	1733
Percent	0.6	95.9	3.5		66.7	0.0	33.3		4.1	93.7	2.2		77.8	0.0	22.2		
08:00 Volume		316	15	333	1	0	1	2	5	129	0	134	4	0	o :	4	. 473
Peak Factor					İ]			•	0.916
High Int.	08:00	AM.			08:00	AM			07:30	AM			08:00	AM			
Volume	2	316	15	333	1	0	1	2	6	129	6	141	4	0	0	4	
Peak Factor	٠.			0.889				0.375				0.952	ļ			0.563	

PHONE NO.: 19095956022

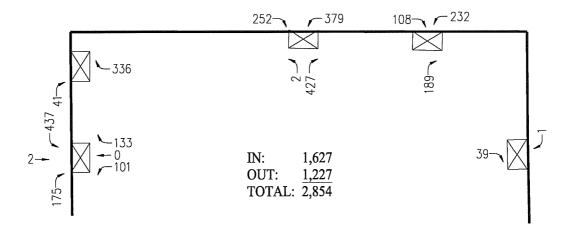
Nov. 22 2005 06:21PM P17

<< ACCUTEK >> << 21114 TRIGGER LANE >> << DIAMOND BAR, CA 91765 >> << (909) 595-6199 FAX: (909) 595-6022 >

File Name : 355508 Site Code : 00355508 Start Date : 11/16/2005 Page No : 2

äge l	No	:	2

		HAZELT	INE AV	E.	Fë		quare Li	ane			RA BLVE).	<u> </u>		Entrance bound]
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right		Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Fro	m 03:00	PM to	05:45										<u> </u>			······································	
Intersection	05:00	PM			İ								1				
Volume	5	844	198	1047	51	1	73	125	67	974	11	1052	9	1	6	16	2240
Percent	0.5	80.6	18.9		40.8	8.0	58.4		6.4	92.6	1.0		56.3	6.3	37.5		
05:45 Volume	1	243	58	302	16	1	20	37	18	230	4	252	2	0	1	3	594
Peak Factor																	0.943
	05:45				05:00	PM			05:15				05:30	ÞΜ			
Volume	1	243	58	302	14	0	24	38	20	246	4	270	3	1	3	7	
Peak Factor				0.867				0.822				0.974				0.571	



SATURDAY MIDDAY PEAK HOUR



APPENDIX FIGURE C-2
EXISTING SITE TRAFFIC VOLUMES
WEEKEND CONDITIONS

LINSCOTT, LAW & GREENSPAN, engineers

WESTFIELD FASHION SQUARE EXPANSION PROJECT

File Name : Drive1 Site Code : 00000000 Start Date : 3/17/2007

Page No : 1

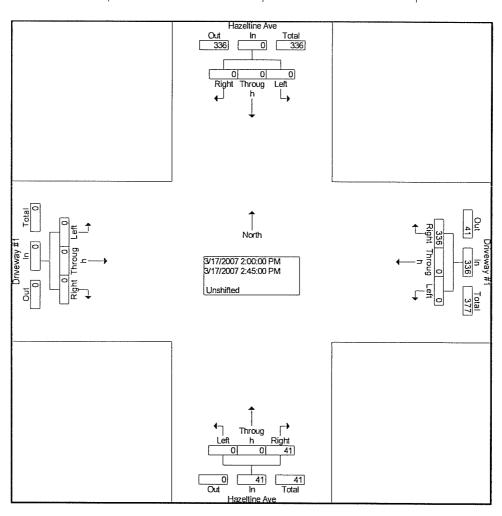
Groups Printed- Unshifted

	Н	azeltine Ave			riveway #1		H	azeltine Ave	9		Driveway #1		
	5	Southbound		V	Vestbound		١	orthbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	0	0	0	0	0	59	0	0	5	0	0	0	64
01:15 PM	0	0	0	0	0	60	0	0	7	0	0	0	67
01:30 PM	0	0	0	0	0	50	0	0	2	0	0	0	52
01:45 PM	0	0	0	0	0	48	0	0	8	0	0	0	56
Total	0	0	Ó	0	0	217	0	0	22	0	0	0	239
02:00 PM	0	0	0	0	0	69	0	0	18	0	0	0	87
02:15 PM	0	0	0	0	0	73	0	0	6	0	0	0	79
02:30 PM	0	0	0	0	0	96	0	0	2	0	0	0	98
02:45 PM	0	0	0	0	0	98	0	0	15	0	0	0	113
Total	0	0	0	0	0	336	0	0	41	0	0	0	377
Grand Total	0	0	0	0	0	553	0	0	63	0	0	0	616
Apprch %	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	100.0	0.0	0.0	0.0	
Total %	0.0	0.0	0.0	0.0	0.0	89.8	0.0	0.0	10.2	0.0	0.0	0.0	

File Name : Drive1 Site Code : 00000000 Start Date : 3/17/2007

Page No : 2

		Hazeli	ine Ave			Drive	way #1			Hazel	tine Ave			Drive	way #1		
			nbound			Wes	tbound			North	nbound			East	bound		
Start Time	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Left	Throu g h	Right	App. Total	Int. Total
Peak Hour From	า 01:00 เ	PM to 02	2:45 PM	Peak 1	of 1												
Intersection	02:00 F	PM															
Volume	0	0	0	0	0	0	336	336	0	0	41	41	0	0	0	0	377
Percent	0.0	0.0	0.0		0.0	0.0	100.0		0.0	0.0	100.0		0.0	0.0	0.0		
02:45	0	0	0	0	0	Λ	98	98	0	0	15	15	_	0	_		440
Volume	U	U	U	U	U	U	90	90	U	U	15	15	U	0	0	0	113
Peak Factor																	0.834
High Int.	12:45:0	00 PM			02:45 F	PM			02:00 F	PM			12:45:0	00 PM		-	
Volume	0	0	0	0	0	0	98	98	0	0	18	18					
Peak Factor								0.857				0.569					



File Name: Drive2 Site Code: 00000000

Start Date : 3/17/2007

Page No : 1

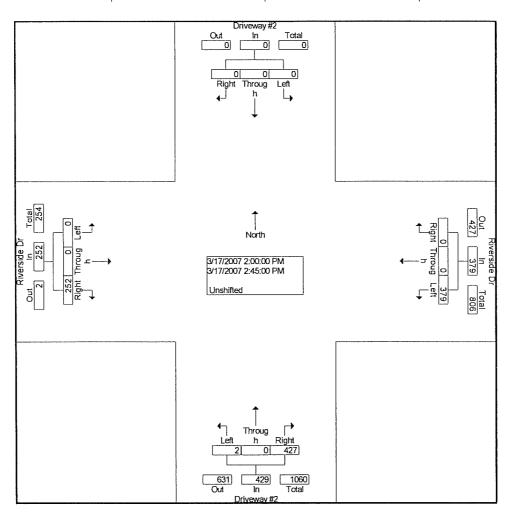
Groups Printed- Unshifted

)riveway #2		F	Riverside Dr			riveway #2			Riverside Dr		
	5	Southbound			Vestbound		N	lorthbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	0	0	0	110	0	0	7	0	80	0	0	37	234
01:15 PM	0	0	0	100	0	0	1	0	84	0	0	64	249
01:30 PM	0	0	0	89	0	0	4	0	70	0	0	64	227
01:45 PM	0	0	0	101	0	0	0	0	98	0	0	67	266
Total	0	0	0	400	0	0	12	0	332	0	0	232	976
02:00 PM	0	0	0	94	0	0	0	0	102	0	0	63	259
02:15 PM	0	0	0	110	0	0	0	0	117	0	0	46	273
02:30 PM	0	0	0	87	0	0	1	0	98	0	0	64	250
02:45 PM	0	0	0	88	0	0	1	0	110	0	0	79	278
Total	0	0	0	379	0	0	2	0	427	0	0	252	1060
Grand Total	0	0	0	779	0	0	14	0	759	0	0	484	2036
Apprch %	0.0	0.0	0.0	100.0	0.0	0.0	1.8	0.0	98.2	0.0	0.0	100.0	
Total %	0.0	0.0	0.0	38.3	0.0	0.0	0.7	0.0	37.3	0.0	0.0	23.8	

File Name: Drive2 Site Code: 00000000 Start Date: 3/17/2007

Page No : 2

			way #2				side Dr				way #2		·		side Dr		
		South	nbound			Wes	lbound			North	nbound			East	bound		
Start Time	Left	Throu	Right	App.	Left	Throu	Right	Арр.	Left	Throu	Right	App.	Left	Throu	Right	Арр.	Int.
Start Hille	LUII	gh	Right	Total	Len	gh	Right	Total	Len	gh	Migrit	Total	Cen	g h	Nigrit	Total	Total
Peak Hour From	1 01:00 l	PM to 02	2:45 PM	- Peak 1	of 1												
Intersection	02:00 F	PM															
Volume	0	0	0	0	379	0	0	379	2	0	427	429	0	0	252	252	1060
Percent	0.0	0.0	0.0		100.0	0.0	0.0		0.5	0.0	99.5		0.0	0.0	100.0		
02:45	0	Ω	Λ	O	88	Λ	0	88	4	0	110	111	0	0	79	79	278
Volume	U	U	U	U	00	U	U	00	1	U	110	111	U	U	75	19	210
Peak Factor																	0.953
High Int.	12:45:0	00 PM			02:15 F	PM			02:15	PM			02:45	PM			
Volume	0	0	0	0	110	0	0	110	0	0	117	117	0	0	79	79	
Peak Factor								0.861				0.917				0.797	



File Name : Drive3

Site Code : 00000000 Start Date : 3/17/2007

Page No : 1

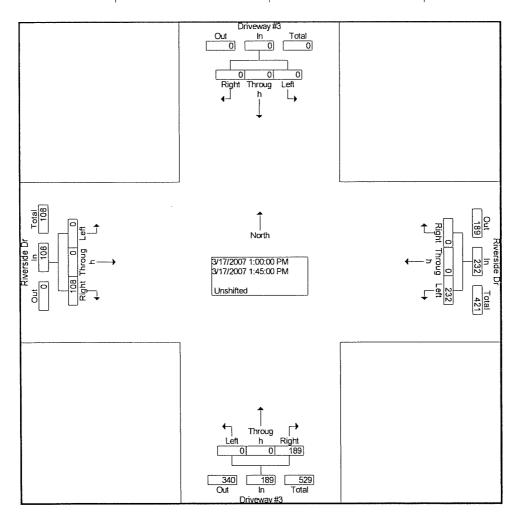
Groups Printed- Unshifted

		7 niveway #3		R	liverside Dr		E	riveway #3		F	Riverside Dr		
	5	Southbound		٧	Vestbound			orthbound			Eastbound		
Start Time	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Left	Throug h	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
 01:00 PM	0	0	0	78	0	0	0	0	52	0	0	32	162
01:15 PM	0	0	0	63	0	0	0	0	47	0	0	24	134
01:30 PM	0	0	0	53	0	0	0	0	51	0	0	16	120
01:45 PM	0	0	0	38	0	0	0	0	39	0	0	36	113
Total	0	0	0	232	0	0	0	0	189	0	0	108	529
02:00 PM	0	0	0	42	0	0	0	0	59	0	0	21	122
02:15 PM	0	0	0	62	0	0	0	0	49	0	0	34	145
02:30 PM	0	0	0	44	0	0	0	0	61	0	0	26	131
02:45 PM	0	0	0	40	0	0	0	0	41	0	0	37	118
Total	0	0	0	188	0	0	0	0	210	0	0	118	516
Grand Total	0	0	0	420	0	0	0	0	399	0	0	226	1045
Apprch %	0.0	0.0	0.0	100.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	100.0	
Total %	0.0	0.0	0.0	40.2	0.0	0.0	0.0	0.0	38.2	0.0	0.0	21.6	

File Name: Drive3
Site Code: 00000000
Start Date: 3/17/2007

Page No : 2

		Drive	way #3			River	side Dr			Drive	way #3			River	side Dr		
		South	bound			Wes	tbound			North	bound			East	bound		
C1 - 1 Ti	Left	Throu	Right	App.	Left	Throu	Right	App.	Left	Throu	Right	App.	Left	Throu	Right	Арр.	Int.
Start Time	Leit	gh	ragin	Total	Len	g h	Rigitt	Total	Len	gh	Night	Total	Len	gh	Ngm	Total	Total
Peak Hour From	101:00 F	PM to 02	2:45 PM	- Peak 1	of 1												
Intersection	01:00 F	PM														1	
Volume	0	0	0	0	232	0	0	232	0	0	189	189	0	0	108	108	529
Percent	0.0	0.0	0.0		100.0	0.0	0.0		0.0	0.0	100.0		0.0	0.0	100.0		
01:00	0	0	0	0	78	0	0	78	0	0	52	52	0	0	32	32	162
Volume	U	U	U	U	70	U	U	70	U	U	JZ	JZ	0	U	JZ	32	102
Peak Factor																	0.816
High Int.	12:45:0	00 PM			01:00 I	PM			01:00 F	PM			01:45 F	PM			
Volume	0	0	0	0	78	0	0	78	0	0	52	52	0	0	36	36	
Peak Factor								0.744				0.909				0.750	



File Name: Drive4 Site Code: 00000000

Start Date : 3/17/2007

Page No : 1

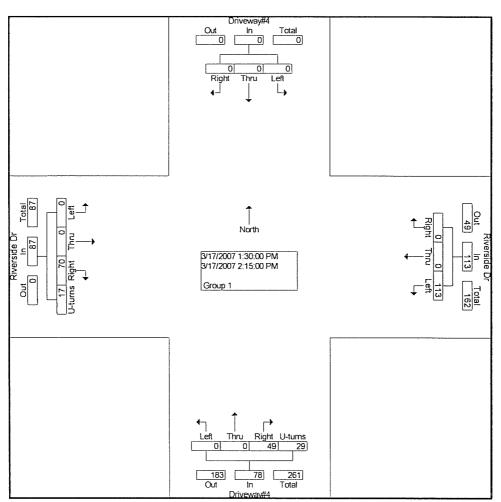
Groups Printed- Group 1

	1	riveway#			verside [Drivev				Rivers			
	So	outhboun	ıd	W	'estboun	d		North	oound			Eastb	ound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U- tums	Left	Thru	Right	U- turns	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	0	0	0	24	0	0	0	0	10	7	0	0	18	0	59
01:15 PM	0	0	0	24	0	0	0	0	8	8	0	0	15	4	59
01:30 PM	0	0	0	28	0	0	0	0	10	10	0	0	16	5	69
01:45 PM	0	0	0	32	0	0	0	0	18	5	0	0	12	1	68
Total	0	0	0	108	0	0	0	0	46	30	0	0	61	10	255
02:00 PM	0	0	0	25	0	0	0	0	11	3	0	0	22	3	64
02:15 PM	0	0	0	28	0	0	0	0	10	11	0	0	20	8	77
02:30 PM	0	0	0	13	0	0	0	0	13	1	0	0	21	2	50
02:45 PM	0	0	0	25	0	0	0	0	10	7	0	0	20	5	67
Total	0	0	0	91	0	0	0	0	44	22	0	0	83	18	258
Grand Total	0	0	0	199	0	0	0	0	90	52	0	0	144	28	513
Apprch %	0.0	0.0	0.0	100.0	0.0	0.0	0.0	0.0	63.4	36.6	0.0	0.0	83.7	16.3	
Total %	0.0	0.0	0.0	38.8	0.0	0.0	0.0	0.0	17.5	10.1	0.0	0.0	28.1	5.5	

File Name : Drive4 Site Code : 00000000 Start Date : 3/17/2007

Page No : 2

			way#4				side Dr				rivewa	•				verside			
Start Time	Left	Thru	Righ t	App. Total	Left	Thru	Righ t	App. Total	Left	Thru	Righ t	U- turn s	App. Total	Left		Righ t	U- turn s	App. Total	Int. Total
Peak Hour Fr	om 01:	00 PM	to 02:4	15 PM -	Peak 1	of 1								•					
Intersectio n	01:30	РМ																	
Volume	0	0	0	0	113	0	0	113	0	0	49	29	78	0	0	70	17	87	278
Percent	0.0	0.0	0.0		100. 0	0.0	0.0		0.0	0.0	62.8	37.2		0.0	0.0	80.5	19.5		
02:15 Volume Peak	0	0	0	0	28	0	0	28	0	0	10	11	21	0	0	20	8	28	77 0.903
Factor	40.45	.00 DN			01:45	DM			01:45	DN4				00.45	DNA				0.303
High Int. Volume	12:45	:00 PN 0	0	0	32	PIVI 0	0	32	01:45	0 0	18	5	23	02:15 0	PIVI 0	20	8	28	
Peak Factor	U	U	U	U	32	U	U	0.883		U	10	5	0.848		U	20	0	0.777	



File Name: Drive5 Site Code: 00000000

Start Date : 3/17/2007

Page No : 1

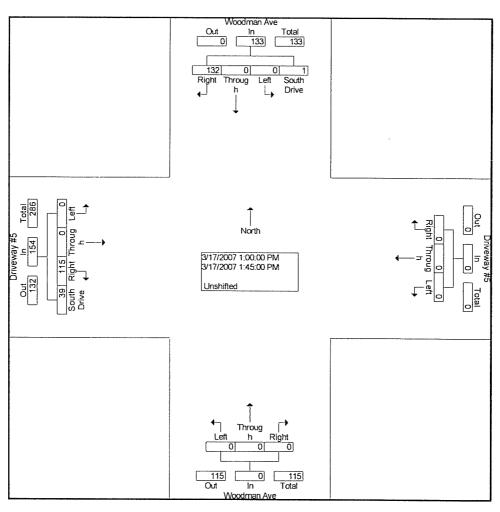
Groups Printed- Unshifted

							, , ,,,,,cu		~~~~						
		Woodm				riveway #			oodman A			Drivew	<i>ı</i> ay #5		
		South	oound		V	Vestbound	d	١	lorthbound	d		Eastb	ound		
Start Time	Left	Throug h	Right	South Drive	Left	Throug	Right	Left	Throug h	Right	Left	Throug h	Right	South Drive	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
01:00 PM	0	0	36	00	0	0	0	0	0	0	0	0	28	7.0	71
01:00 FM	0	0	25	0	0	0	0	0	0	0	0	0	31	,	
01:30 PM	0	0	29	4	0	0	- 1	0	0	0	0	0		8	64
	_	_		,	0	0	0	0	0	U	0	U	28	12	70
01:45 PM	0	0	42	0	U	0	0	0	0	0	0	0	28	12	82
Total	0	0	132	1	0	0	0	0	0	0	0	0	115	39	287
02:00 PM	0	0	28	0	0	0	0	0	0	0	0	0	28	9	65
02:15 PM	0	0	21	0	0	0	0	0	0	0	0	0	31	10	62
02:30 PM	0	0	17	0	0	0	0	0	0	0	0	0	22	3	42
02:45 PM	0	0	19	2	0	0	0	0	0	0	0	Õ	34	3	58
Total	0	0	85	2	0	0	0	0	0	0	0	0	115	25	227
							•			,				'	
Grand Total	0	0	217	3	0	0	0	0	0	0	0	0	230	64	514
Apprch %	0.0	0.0	98.6	1.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	78.2	21.8	
Total %	0.0	0.0	42.2	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	44.7	12.5	

File Name: Drive5 Site Code: 00000000 Start Date: 3/17/2007

Page No : 2

			oodman outhbol					way #5 tbound				man Ave				riveway			
Start Time	Left	Thro ug h	Righ t	Sout h Driv e	App. Total	Left	Thro ug h	Righ t	App. Total	Left	Thro	Righ t	App. Total	Left	Thro	Righ t	Sout h Driv	App. Total	Int. Total
Peak Hour Fro	m 01:0	0 PM to	02:45	PM - P	eak 1 of	1								•	·		1		
Intersection	01:00	PM																1	
Volume	0	0	132	1	133	0	0	0	0	0	0	0	0	0	0	115	39	154	287
Percent	0.0	0.0	99.2	8.0		0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	74.7	25.3		
01:45 Volume	0	0	42	0	42	0	0	0	0	0	0	0	0	0	0	28	12	40	82
Peak Factor																			0.875
High Int.	01:45	PM				12:45:	00 PM			12:45:	00 PM			01:30	PM				
Volume Peak Factor	0	0	42	0	42 0.792	0	0	0	0	0	0	0	0	0	0	28	12	40 0.963	



APPENDIX D

CMA AND LEVELS OF SERVICE EXPLANATION
CMA DATA WORKSHEETS
WEEKDAY AM & PM PEAK HOURS
WEEKEND MID-DAY PEAK HOUR

CRITICAL MOVEMENT ANALYSIS (CMA) DESCRIPTION

Level of Service is a term used to describe prevailing conditions and their effect on traffic. Broadly interpreted, the Level of Service concept denotes any one of a number of differing combinations of operating conditions which may take place as a roadway is accommodating various traffic volumes. Level of Service is a qualitative measure of the effect of such factors as travel speed, travel time, interruptions, freedom to maneuver, safety, driving comfort and convenience.

Six Levels of Service, A through F, have been defined in the 1965 *Highway Capacity Manual*. Level of Service A describes a condition of free flow, with low traffic volumes and relatively high speeds, while Level of Service F describes forced traffic flow at low speeds with jammed conditions and queues which cannot clear during the green phases.

Critical Movement Analysis (CMA) is a procedure which provides a capacity and level of service geometry and traffic signal operation and results in a level of service determination for the intersection as a whole operating unit.

The per lane volume for each movement in the intersection is determined and the per lane intersection capacity based on the Transportation Research Board (TRB) Report 212 (*Interim Materials on Highway Capacity*). The resulting CMA represents the ratio of the intersection's cumulative volume over its respective capacity (V/C ratio). Critical Movement Analysis takes into account lane widths, bus and truck operations, pedestrian activity and parking activity, as well as number of lanes and geometrics.

The Level of Service (abbreviated from the *Highway Capacity Manual*) are listed here with their corresponding CMA and Load Factor equivalents. Load Factor is that proportion of the signal cycles during the peak hour which are fully loaded; i.e. when all of the vehicles waiting at the beginning of green are not able to clear on that green phase.

Critical M	Iovement Analysis Characte	ristics
Level of Service	Load Factor	Equivalent CMA
A (free flow)	0.0	0.00 - 0.60
B (rural design)	0.0 - 0.1	0.61 - 0.70
C (urban design)	0.1 - 0.3	0.71 - 0.80
D (maximum urban design)	0.3 - 0.7	0.81 - 0.90
E (capacity)	0.7 - 1.0	0.91 - 1.00
F (force flow)	Not Applicable	Not Applicable

SERVICE LEVEL A

There are no loaded cycles and few are even close to loaded at this service level. No approach phase is fully utilized by traffic and no vehicle waits longer than one red indication.

SERVICE LEVEL B

This level represents stable operation where an occasional approach phase is fully utilized and a substantial number are approaching full use. Many drivers begin to feel restricted within platoons of vehicles.

SERVICE LEVEL C

At this level stable operation continues. Loading is still intermittent but more frequent than at Level B. Occasionally drivers may have to wait through more one red signal indication and backups may develop behind turning vehicles. Most drivers feel somewhat restricted, but not objectionably so.

SERVICE LEVEL D

This level encompasses a zone of increasing restriction approaching instability at the intersection. Delays to approaching vehicles may be substantial during short peaks within the peak hour, but enough cycles with lower demand occur to permit periodic clearance of queues, thus preventing excessive backups. Drivers frequently have to wait through more than one red signal. This level is the lower limit of acceptable operation to most drivers.

SERVICE LEVEL E

This represents near capacity and capacity operation. At capacity (CMA = 1.0) it represents the most vehicles that the particular intersection can accommodate. However, full utilization of every signal cycle is seldom attained no matter how great the demand. At this level all drivers wait through more than one red signal, and frequently through several.

SERVICE LEVEL F

Jammed conditions. Traffic backed up from a downstream location on one of the street restricts or prevents movement of traffic through the intersection under consideration.

APPENDIX D-1

CMA DATA WORKSHEETS WEEKDAY AM & PM PEAK HOURS

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA1 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Van Nuys Boulevard @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 E.	2007 EXIST. TRAFFIC	FFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	E	2012 \	W OTHE	2012 W/ OTHER PROJECTS	:TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
	_	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	ı	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. L-T		0	•			0	•			0	ı			0	,			0	,
NB Thru	1410	2	652	141	1551	2	717	96	1647	2	757	0	1647	2	759	0	1647	2	759
Comb. T-R		Ψ-	652			•	717			-	757			-	759			-	759
NB Right	546	0	,	22	601	0	1	23	624	0		5	629	0	ı	0	629	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	158	1	158	16	174	-	174	8	182	1	182	5	187	-	187	0	187	-	187
Comb. L-T		0	,			0	,			0				0	1			0	,
SB Thru	1227	m	409	123	1350	ო	450	37	1387	ო	462	0	1387	ო	462	0	1387	က	462
Comb. T-R		0	,			0				0				0				0	1
SB Right	0	0		0	0	0	,	0	0	0	,	0	0	0		0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0	,	0	0	0	,	0	0	0		0	0	0	1	0	0	0	,
Comb. L-T		0	ı			0				0	,			0				0	,
EB Thru	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. T-R		0	•			0	,			0				0				0	,
EB Right	0	0	ı	0	0	0	,	0	0	0		0	0	0	,	0	0	0	1
Comb. L-T-R -		0				0				0				0				0	
WB Left	488	2	268	49	537	2	295	16	553	2	304	4	557	2	306	0	557	2	306
Comb. L-T		0	,			0				0	,			0	,			0	
WB Thru	0	0		0	0	0		0	0	0 +		0	0	0		0	0	0	
Comb. T-R	ļ	ο.		;		ο.		:	;	0			;	0				0	
WB Right	257	-	257	56	283	τ- 1	283	Ξ	294		294	ဖ	299	-	599	0	299	-	599
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	810			N-S:	891			N-S:	939			N-S:	946			N-S:	946
	_	E-W:	268			E-W.	295			E-W.	304			E-W:	306			E-W:	306
******	-,	SUM:	1078			SUM:	1186			SUM:	1243			SUM:	1252			SUM:	1252
No of Dhases:			C*				c				c				·				6
NO. OF PIERSES.			0				2				n				2				ກ
Volume / Capacity:	city:	[1]	0.687			[1]	0.762			[1]	0.802			[1]	0.808			[2]	0.778
Level of Service:	'ni		æ			_	ပ				_				۵				U

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wc ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA 1 Accutek N-S St: E-W St: Project: File Name:

Counts by:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Riverside Drive Peak Hour: PM 2.00% Annual Growth:

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	SOUT EXIST TRAFFIC	SEELC	2012	W/ AMPIE	2012 W/ AMBIENT GROWTH		2012	V OTHER	2012 W/ OTHER PROJECTS	TS	2012	N PROP	2012 W/ PROPOSED PRO JECT	TOH!	2012	MOITABITION W 210C	NO.	
		No of	2 -	Addad	F CF	No of		Added Total	Total	No of		Added Total	100	No of	900	Addod	100	, o	000
Movement	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume		Lanes	Volume		Volume	Lanes	Volume
NB Left	0	00		0	0	00		0	0	00		0	0	00	, ,	0	0	00	s
NB Thru	1592	· ~	719	159	1751	2 7 7	797	7	1822	. 64	822	0	1822	0 77 +	828	0	1822	o 01 +	828
NB Right Comb. L-T-R -	564	.00	1	99	620	-00		24	644	-00	,	18	662	.00	3	0	662	- 0 0	2
SB Left	216	- 0	216	22	238	- 0	238	6	247	- 0	247	19	266	- c	266	0	266	F C	266
SB Thru	1431	o m c	477	143	1574	o m c	525	82	1656		552	0	1656	. m c	552	0	1656		552
SB Right Comb. L-T-R -	0	000		0	0	000		0	0	000		0	0	000		0	o	000	1 1
EB Left Comh 1-T	0	0 0	. ,	0	0	0 0		0	0	0 0	. ,	0	0	0 0	. ,	0	0	0 0	
EB Thru	0	000	i	0	0	000	í	0	0	000	,	0	0	000	,	0	0	0.0	•
EB Right Comb. L-T-R -	0	000		0	0	000		0	0	000		0	0	000		0	0	000	: 1
WB Left Comb. L-T	475	2 0	261	48	523	0 2	288	29	552	2 0	303	25	577	2 0	317	0	577	2 0	317
WB Thru Comb, T-R	0	000	1 1	0	0	000		0	0	00	1 1	0	0	000		0	0	000	
WB Right Comb. L-T-R -	234	- 0	234	23	257	- 0	257	7	264	-0	264	20	314	-0	314	0	314	-0	314
Crit. Volumes:		N-S: E-W: SUM:	935 261 1196			N-S: E-W: SUM:	1028 288 1316			N-S: E-W: SUM:	1069 303 1373			N-S: E-W: SUM:	1094 317 1411			N-S: E-W: SUM:	1094 317 1411
No. of Phases:			က				ო				က				m				3
Volume / Capacity: Level of Service:	acity:	[1]	0.770 C			[1]	0.854 D			[1]	0.893 D			[1]	0.920 E			[2]	0.890 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA2 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps Peak Hour: AM Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	7000	CIEDACT TOIVE TOOC	VEEL	2 6406	aldway.	UTWOOD THEIR AMPLE	1	2042	W OTHER	2012 W/ OTHER DRO IECTS	Į.	2012	ACAG //	2012 W/ PROPOSED PROJECT	D.IECT	2012	2012 W/ MITIGATION	NOITA	
	7007	7	2	7 7 7			-	1 70 70		,			10.5	No of		Polotic	Total	No.	- C
	Veluma	No. of	Lane	Added lotal	lotal	No. of	Lane	Volume	Volume	No. of	Volume	Volume Volume	lotal Volume	No. or	Volume		Volume	NO. OI	Volume
Movement Volume Lanes	Volume	Lanes	Acidana	ADIDIA	AOIGINE	Lailes	ACIONICA		Acidine	Lanco	all line		2						
NB Left	354	7	194	35	389	2	214	23	412	7	227	0	412	5	227	0	412	2	227
Comb. L-T		0				0				0				0				0	
NB Thru	1423	က	474	142	1565	ო	522	=======================================	1676	ო	559	വ	1681	က	260	0	1681	က	260
Comb. T-R		0	•			0				0	,			0	•			0	
NB Right	0	0	ì	0	0	0		0	0	0	ı	0	0	0		0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T		0	1			0	ı			0				0	,			0	
SB Thru	1093	2	459	109	1202	2	505	48	1250	2	522	7	1252	2	522	0	1252	7	522
Comb. T-R		-	459			-	505			-	522			-	522			-	522
SB Right	630	-	347	83	693	-	381	S	869	-	384	7	700	-	385	0	700	-	385
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. L-T		0	,			0	į			0	1			0	,			0	ı
EB Thru	0	0	,	0	0	0	•	0	o	0		0	0	0	•	0	0	0	ı
Comb. T-R		0	,			0	į			0				0				0	,
EB Right	0	0	,	0	0	0	,	0	0	0	1	0	0	0		0	0	0	•
Comb. L-T-R -		0				0				0				0				0	
WB Left	291	-	160	58	320	-	176	2	322	-	177	0	322	1	177	0	322	-	177
Comb. L-T		0	,			0				0	,			0				0	ı
WB Thru	œ	0	380	-	თ	0 (418	0	თ	0 (422	0	ത	0 0	422	0	ത	0	422
Comb. T-R	!	0		į	1	ο ·	,	•	i	۰ د	,	(Č	> •		(i	o 1	
WB Right	535	•	294	83	288		323	∞	296		328	0	296		328	0	296	, - ,	328
Comb. L-T-R -		-				-				-				-				_	
Crit. Volumes:		N-S:	653			N-S:	719			N-S:	748			Ņ-S:	749			N-S:	749
		Ε-Κ:	380			E-W.	418			Ē-W:	422			E-W:	422			щ. Х.	422
		SUM:	1033			SUM:	1137			SUM:	1171			SOM:	1171			SUM:	1171
No. of Phases;			ဗ				9				3				ന				8
Volume / Canacity:	city:	Ξ	0.655			2	0.698			[2]	0.721			[2]	0.722			[2]	0.722
Volunte / Cape	. di	Ξ	3							Ţ	; ;			Ī	;			<u>[</u>	
Level of Service.	ž.		۵				٥				,				,	***************************************			

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wo ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA2 Accutek N-S St: E-W St: Project: File Name: Counts by:

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps Peak Hour. PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	1 7006	CONT EXIST TRAFFIC	O E E IC	2012	W/ AMRI	2012 W/ AMBIENT GROWTH	Ħ	2012 V	W OTHE	2012 W/ OTHER PROJECTS	TS.	2012 /	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	ane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	-	Volume	Lanes	Volume		Volume	Lanes	Vоlите	Volume	_	Lanes	Volume
NB Left	724	2	398	72	796	2	438	21	817	2	449	0	817	2	449	0	817	2	449
Comb. L-T		0	•			0	,			0	,			0				0	•
NB Thru	1698	က	266	170	1868	ო	623	8	1958	ო	653	18	1976	ო	629	0	1976	က	629
Comb. T-R		0				0				0				0	,			0	1
NB Right	0	0		0	0	0		0	0	0	,	0	0	0		0	0	0	•
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0	,	0	0	0		0	0	0	,	0	0	0	,	0	0	0	,
Comb. L-T		0				0				0	•			0				0	
SB Thru	1101	7	483	110	1211	7	531	101	1312	2	266	12	1324	2	572	0	1324	2	572
Comb. T-R		•	483			-	531		,	-	566		;	-	572	•	į		572
SB Right	770	-	423	77	847	-	466	9	857	-	471	12	869		478	0	869	1	478
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0		0	0	0		0	0	0	,	0	0	0	-
Comb. L-T		0				0				0	,			0	ň			0	
EB Thru	0	0		0	0	0		0	0	0	1	0	0	0	i	0	0	0	
Comb. T-R		0				0	ı			0	ı			0	;			0	
EB Right	0	0	,	0	0	0	•	0	0	0		0	0	0	,	0	0	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	304	-	167	30	334	-	184	11	345	-	190	0	345	-	190	0	345	-	190
Comb. L-T		0				0	,			0	1			0				0	
WB Thru	2	0 0	341	0	7	0 0	375	0	7	0 0	383	0	2	0 0	383	0	7	00	383
WB Right	449	o	247	45	494	c	272	9	200	- c	275	0	200	-	275	0	200	~ ○	275
Comb. L-T-R -		-	!			-				~				-				-	
Crit. Volumes:		N-S:	881			N-S:	696			N-S:	1015			N-S:	1021			N-S:	1021
		Ę-W:	341			E-W:	375			Ę-W:	383			Ë-₩.	383			Ę-Ķ	383
		SUM:	1222			SUM:	1344			SUM:	1398			SUM:	1404			SUM:	1404
No. of Phases:]; <u>.</u>		က				3				8				93				က
Volume / Capacity:	acity:	Ξ	0.787			[2]	0.843			[2]	0.881			[2]	0.885			[2]	0.885
Level of Service:	, , ,	:	O				٥			•	D				D				D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 55% of volume is assigned to exclusive lane. For one excl. and one opt. turn lane, 56% of overlapping left turn. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA3 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps Peak Hour: AM Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 EXIST. TRAFFIC	PAFFIC	2012 V	V/ AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 ₩	V/ OTHER	2012 W/ OTHER PROJECTS	:TS	2012 \	N/ PROP(2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	\TION	
	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement V	Movement Volume Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0		0	0	00		0	0	00		0	0	00		0	0	00	
NB Thru	1050 3	318 318	105	1155	ე რ -	350	127	1282	o w ←	383	ю	1285	o m -	384	0	1285	o ო ←	384
Comb. L-T-R -	223 0	,	22	245	-00	3	ω	251	-00	3	0	251	. 0 0	}	0	251	.00	,
SB Left	310 1	310	31	341	 C	341	4	345	F- c	345	0	345	c	345	0	345	- 0	345
SB Thru	1276 2	, 638	128	1404	0 00 0	702	46	1450	0 00 0	725	2	1452	0 10 0	726	0	1452	0 00 0	726
Comb. I-K SB Right Comb. L-T-R -	0		0	0	000		0	0	000	1 1	0	0	000		0	0	000	1 1
EB Left	607 1	334	61	899	- 0	367	80	9/9	- 0	372	2	678	← c	373	0	678	- 0	373
EB Thru	-	592	0	-	000	651	0	Υ	000	. 663	0	τ	000	664	0	τ-	000	664
Comb. I-K EB Right Comb. L-T-R -	706 1	388	71	777		427	19	796) 	438	0	796	O	438	0	796) 	438
WB Left	0		0	0	0 0		0	0	0 0		0	0	0 0		0	0	0 0	, ,
WB Thru	0	, ,	0	0	000		0	0	000		0	0	00		0	0	00	1 1
WB Right Comb. L-T-R -	0		0	0	000	•	0	0	000		0	0	000		0	0	000	1
Crit. Volumes:	N-S: E-W: SUM:	638 592 1230			N-S: E-W; SUM:	702 651 1353			N-S: E-W; SUM;	728 663 1392			N-S: E-W: SUM:	729 664 1393			N-S: E-W: SUM:	729 664 1393
No. of Phases:		၈				က				9				6				8
Volume / Capacity: Level of Service:	ity: [1]	0.793 C			[2]	0.850 D			[2]	0.877 D			[2]	0.878 D			[2]	0.878 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA3 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps Peak Hour: 2.00% Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 EX	2007 EXIST. TRAFFIC	\FFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	王	2012 V	W OTHER	2012 W/ OTHER PROJECTS	:TS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	JUECT	2012	2012 W/ MITIGATION	NOIL	
	Z	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume L	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0 0	1	0	0	00	1	0	0	00	i	0	0	00	,	0	0	00	ı :
NB Thru	1707	⊃ m +	507	171	1877	o w +	558 558	103	1980	o m -	586 586	Ξ	1991	o e> ←	589	0	1991	o w ⊷	589
Comb. L-T-R -	322	-00	3	32	355	-00	8 ,	O	364	- 0 0	}	0	364	. 0 0	}	0	364	.00	1
SB Left	362	- (362	36	398	← (398	6	407	- 0	407	0	407	- 0	407	0	407	← 0	407
SB Thru	1054	9 C	527	105	1159) N	579	102	1261	0 7 0	630	12	1273	0 70 0	- 636	0	1273	0 67 0	- 636
Comb. T-R SB Right Comb. L-T-R -	0	000		0	0	000		0	0	000		0	0	000		0	0	000	
EB Left	713	-	392	71	785	- (432		792	- (435	7	799	- 0	439	0	799	- '	439
Comb. L-1 EB Thru	9	00	591	-	7	00	650	0	7	00	- 665	0	7	00	- 668	0	7	000	899
Comb. T-R EB Right Comb. L-T-R -	587	0	323	59	645	0	355	25	670	0	369	0	670	o	369	0	670	0	369
WB Left	0	0		0	0	0	-	0	0	0	,	0	0	0		0	0	0	
Comb. L-T	c	0 0		c	c	00		c	c	0 0		c	c	0 0		c	c	00	
Comb. T-R	•	00		0	0	0		•	o	0		•	o	00		•	0	0	
WB Right Comb. L-T-R	0	00		0	0	00	,	0	0	00		0	0	00		0	0	00	,
Crit. Volumes:		N-S: E-W: SUM:	869 591 1460			N-S: E-W: SUM:	956 650 1606			N-S: E-W: SUM:	993 665 1658			N-S: E-W: SUM:	996 668 1664			N-S: E-W: SUM:	996 668 1664
No. of Phases:	34		က				ဗ				m				က				3
Volume / Capacity:	acity:	Ξ	0.955 E			[2]	1.027 F			[2]	1.063 F			[2]	1.068 F			[2]	1.068 F

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSACATCS system installation.

Tyrone Avenue Moorpark Street Westflield Fashion Square /1-05-3606-1 CMA4 Accutek N-S St: E-W St: Project: File Name: Counts by:

Tyrone Avenue @ Moorpark Street Peak Hour: AM Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST TRAFFIC	FFIC	2012	W/ AMRIE	2012 W/ AMRIENT GROWTH	HE	2012 V	WOTHER	2012 W/OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	DIECT	2012	2012 W/ MITIGATION	NOITY		Г
	1	2	2	1 .		,		1 .	i :	, , , , , , , , , , , , , , , , , , ,	, .) .	1 .		,		
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane			No. of	Lane	Added	Total	No. of	Lane	
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	
NB Left	34	0	,	ო	38	0	1	~	39	0	3	0	39	0	,	0	39	0		
Comb. L-T			40			-	43			-	63			-	63			-		63
NB Thru	2	0		-	9	0	,	19	25	0		0	25	0		0	22	0		
Comb. T-R		0				0	,			0				0				0		
NB Right	239	-	239	24	263	-	263	2	265	-	265	က	268	-	268	0	268	-	Ō	268
Comb. L-T-R -		0				0				0				0				0		
SB Left	80	0	,	-	6	0		-	10	0		0	10	0		0	10	0		Τ
Comb. L-T		0				0	,			0	1			0	,			0		
SB Thru	32	0	51	က	35	0	29	20	22	0	79	0	55	0	79	0	55	0		79
Comb. T-R		0				0	,			0				0				0		
SB Right	10	0		_	=	0	,	2	13	0	1	0	13	0	1	0	13	0	ı	
Comb. L-T-R -		-				-				-				-				-		
EB Left	3	1	9	0	8	-	3	2	5	-	5	0	5	٢	5	0	5	-		2
Comb. L-T		0				0	1			0	ı			0	,			0		
EB Thru	284	,	284	28	312	-	312	33	351	-	351	0	351	-	351	0	351	-	m	351
Comb. T-R		0				0	,			0	,			0	,			0		
EB Right	37	-	37	4	4	-	4	-	45	₩	42	0	42	-	42	0	42	-		42
Comb. L-T-R -		0				0				0				0				0		
WB Left	297	-	297	30	327	-	327	2	329	-	329	2	331	-	331	o	331	-	3	331
Comb. L-T		0				0	,			0				0				0	,	
WB Thru	759	0		9/	832	0		18	853	0	•	0	853	0		0	853	0	,	
Comb. T-R		-	766			-	843			-	863			-	863			-	æ	863
WB Right	7	0		-	∞	0		2	5	0	•	0	10	0		0	9	0	,	
Comb. L-T-R -		0				0				0				0				0		
Crit. Volumes:		N-S:	66			N-S:	109			S-N	118			N-S:	118			N-S:		<u></u>
		E-W.	770			E-W:	847			Ē-₩.	869			E-W:	869			E-W:	æ	869
		SUM:	868			SUM:	955			SUM:	986			SUM:	986			SUM:	ਨ	986
No of Dhases.			r				6				6				۲,					\[\(\)
NO. OF PITASES.			י				ס				o				o					·
Volume / Capacity:	ity:	Ξ	0.539			Ξ	0.600			Ξ	0.622			Ξ	0.622			[2]	0.5	0.592
Level of Service:			⋖				V				В				Ю				V	

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. Assumptions:

N-S St: E-W St: Project: File Name: Counts by:

Tyrone Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA4 Accutek

Tyrone Avenue @ Moorpark Street Peak Hour; 2.00% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/04/2008 2007 2012

YOUN PROMOTER IN		<u>ַ</u>		1	200 E	F	3042 18	はいけつと		U.F.	2040	0000	0000	FCD		TACIFACIFIED VALUE CACC	301		_
		<u> </u>	71.07	<u>n</u>	בו אסציים	<u> </u>	7107		ZUIZ W/ UINER PROJECIS	0	71.07	בי היי	ZUIZ WI PROPOSED PROJECT	מונים בי	71.07) 	2		
	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	•
ı	Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Je.
NB Left 152	0		15	167	0	,	2	169	0		0	169	0	,	0	169	0	1	
Comb. L-T NB Thru 34		186	ო	38	- 0	- 205	20	28	- 0	227	0	28	- 0	227	0	58	- 0	i	227
Comb. T-R NB Right 612 Comb. L-T-R -	0 - 0	612	61	673	0 - 0	673	ო	929	0 - 0	676	10	686	0-0	- 686	0	686	0 - 0	í	989
SB Left 23		,	2	25	0 0		2	27	0 0	-	0	27	0 (0	27	0 (
SB Thru 35		. 75	4	39	000	- 82	59	89	000	115	0	89	000	115	0	89	000	1	115
Comb. I-R SB Right Comb. L-T-R -	00-	1 3	0	18	- 00		2	20	00-	, i	0	20	- o c	1 1	0	20	- o c	, ,	
EB Left 4	- 0	4	0	5	- 0	5	3	80	- 0	89	0	8	- 0	80	0	8	- (88
EB Thru 500	o - - c	500	20	550	o ← c	550	33	583	o ← c	583	0	583	o c	583	0	583	o ← 0		583
Comb. I-R EB Right 45 Comb. L-T-R -	0-0	45	4	49	0 - 0	49	2	51	0-0	51	0	51	0 - 0	. 51	0	51	0 - 0		51
WB Left 386	- c	386	39	424	- c	424	9	427	- c	427	12	439	- 0	439	0	439	- 0		439
WB Thru 607 Comb. T-R WB Right 17 Comb. L-T-R -		624	61	668	00-00	989	33	701	00-00	721	0 0	701	00-00	721	0 0	701	00-00		721
Crit. Volumes:	N-S: E-W: SUM:	441 886 1328			N-S: E-W: SUM:	486 975 1460			N-S: E-W: SUM:	489 1011 1500			N-S: E-W: SUM:	493 1023 1516			N-S; E-W; SUM;	7 7 7	493 1023 1516
No. of Phases:		ო				ო				က				e					ო
Volume / Capacity: Level of Service:	[5]	0.862 D			Ξ	0.955 E			Ξ	0.983 E			E	0.994 E			[2]	П	0.964

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

70% of voralapping left tum.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA5 N-S St; E-W St; Project: File Name: Counts by:

Accutek

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour:

AM
Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

			9111			1000		1		011	-	1000	0000	0 100	FOL	1000	COLT & CITIES WAY OF CO.	1011	
	7007	ZUU/ EXIST. IKAPPIC	יוייי	2102	W/ AMBIL	ZU1Z W/ AMBIENI GROWIN	_	7107	ב ב ב	ZUIZ W/ UINER PROJECIS	0	71.07	אר אי	ZUIZ W/ PROPOSED PROJECT	כאנכ	71.07) 	20	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	ı	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	87	-	87	ത	96	-	96	4	110	τ-	110	0	110	-	110	0	110	-	110
Comb. L-T		0	,			0	1			0				0	1			0	
NB Thru	163		117	16	180	τ	129	Ψ-	181		9 6 6 6	က	184		141	0	184		141
Comb. 1-X	7	- c		7	78	- c	6 <u>7</u>	5	ά	- c	-	c	ğ	- c	- -	c	ď	- c	4
Comb. L-T-R -	-	00	•	-	2	0	,	3	3	00		o	3	0		•	8	0	
SB Left	11	-	=	-	13	-	13	13	26	-	26	0	26	-	26	0	26	-	26
Comb. L-T		0				0	,			0	,			0	1			0	•
SB Thru	206	-	206	21	227	-	227	-	228	-	228	7	230	-	230	0	230	-	230
Comb. T-R	;	ο ·	;	;		۰ ،	:	(1	0 1	,	(,	0	,	(۰ ،	,
SB Right	144	· (144	4	158	- c	128	თ	167	- c	167	0	167	, c	167	0	167	- - c	167
Comb. L-1-K		0				5				>				>				5	
EB Left	6/	- (6/	80	87	- (87	16	103	- (103	0	103	- 0	103	0	103	- 0	103
Comb. L-1	1194	⊃ -	656	0.7	1313	→	- 722	118	1431	- c	- 792	-	1432) (797	c	1432	> ~	797
Comb. T-R	!	-	656	•	?	· -	722		!	-	792		!	-	792)	!		792
EB Right	119	0		12	130	0		22	152	0		0	152	0		0	152	0	į
Comb. L-T-R -		0				0				0				0				0	
WB Left	75	-	75	7	82	-	82	13	95	-	95	0	95	-	95	0	92	-	96
Comb. L-T		0	,			0	ı			0				0				0	
WB Thru	1146		578	115	1261		635	14	1338	- τ	676 272	4	1339	- -	677	0	1339	·- ·	677
WB Right	σ	- 0	0 0	-	10	- 0	3	ĸ	15	- 0	2	0	5	- 0	,	o	15	- c	· /D -
Comb. L-T-R -		0				0				0				0			!	0	
Crit. Volumes:		N-S:	293			N-S:	323			N-S:	338			N-S:	340			N-S:	340
		Ē-₩:	731			E-W.	804			E-W:	887			E-W.	888			E-W:	888
		SUM:	1024			SUM:	1127			SUM:	1225			SUM:	1227			SUM:	1227
No. of Phases:			2				2				2				2				2
Volume / Capacity:	city:	Ξ	0.613			[2]	0.651			[2]	0.717			[2]	0.718			[2]	0.718
Level of Service:	ie.		В				В				C				С				O

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

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[2] wor ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 : CMA5 Accutek N-S St: E-W St: Project: File Name: Counts by:

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour: PM Annual Growth: 2.00%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/04/2008 2007 2012

	2007	2007 FXIST TRAFFIC	AFFIC	2012 V	2012 W/ AMBIE	ENT GROWTH	H	2012	W OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012 W	2012 W/ MITIGATION	NOIL	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	-	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume V	Volume	Lanes	Volume
NB Left	190	-	190	19	209		209	28	237	₩.	237	0	237	← (237	0	237	₩ (237
Comb. L-T NB Thru	557	0 - 1	347	56	613	0 - 1	382	-	614	0 7	391	10	624) T	396	0	624	o	396
Comb. I-K NB Right Comb. L-T-R	137	- 0 0	. 34/	4	151	-00	385	16	167	- 0 0	- 2000 -	0	167	- 0 0	0.000	0	167	-00	1
SB Left	36	- -	36	4	40	-	40	10	20	- (50	0	20	← 0	50	0	20	- 0	50
Comb. L-T SB Thru	297	0 - 0	297	30	327	0 - 0	327	-	328	o c	328	12	340	⊃ - 0	340	0	340	o c	340
Comb. I-R SB Right Comb. L-T-R -	. 56	0 - 0	. 56	φ	62	0 + 0	. 62	23	85	0 - 0	85	0	82	0 + 0	. 85	0	82	0 0	85
EB Left	109	- (109	11	120	- 0	120	17	137	← c	137	0	137	c	137	0	137	- 0	137
Comb. L-1 EB Thru	1099	o ,	579	110	1209	o - •	637	118	1327) - -	707	ഗ	1332) -	709	0	1332	o •	709
Comb. 1-K EB Right Comb. L-T-R -	58	-00	6/c -	ω	64	-00	28 ,	22	86	-00	è,	0	86	-00	,	o	86	-00	
WB Left	146	-	146	15	160	- (160	12	172	- 0	172	0	172	- 0	172	0	172	- (172
Comb. L-1 WB Thru Comb. T-R	1124	o	576 576	112	1237	D	- 634 634	126	1363) - -	700	S.	1368) - -	703	0	1368	o	703
WB Right Comb. L-T-R -	- 28	-00) ;	ო	31	-00	;	^	38	.00	,	0	38	00		0	38	00	1
Crit. Volumes:		N-S: E-W: SUM:	488 724 1212			N-S: E-W: SUM:	537 797 1333			N-S: E-W: SUM:	566 879 1444			N-S: E-W: SUM:	578 881 1459			N-S: E-W: SUM:	578 881 1459
No. of Phases:			2				2				2	- The state of the			2				2
Volume / Capacity: Level of Service:	acity:	Ξ	0.738 C			[2]	0.789 C			[2]	0.863 D			[2]	0.873 D			[2]	0.873 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavler lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 Project: File Name: Counts by: N-S St: E-W St:

Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Magnolia Boulevard Peak Hour: AM AM 2.0% Annual Growth:

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	W/ AMBIE	2012 W/ AMBIENT GROWTH	ИTH	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	78	-	78	8	98	-	86	ß	9	-	91	-	92	-	92	0	92	-	92
Comb. L-T		0				0				0				0	,			0	,
NB Thru	441	_	252	44	485	-	277	14	499	Ψ-	285	က	505	-	287	0	502	-	287
Comb. T-R		-	252			-	277			-	285			_	287			τ-	287
NB Right	62	0		9	69	0	,	ო	72	0		0	72	0		0	72	0	•
Comb. L-T-R -		0				0				0				0				0	
SB Left	126	-	126	13	138	-	138	-	137	-	137	0	137	-	137	0	137	-	137
Comb. L-T		0	,			0	,			o				0				0	•
SB Thru	813	-	453	81	895	-	498	13	908	-	505	7	915	_	208	0	915	-	508
Comb. T-R		_	453			-	498			-	505			-	508			-	208
SB Right	93	0		6	102	0	,	0	102	0	,	0	102	0		0	102	0	,
Comb, L-T-R -		0				0				0				0				0	
EB Left	57	-	57	9	63	-	63	-	64	-	64	0	64	-	64	0	64	-	64
Comb. L-T		0				0	,			0				0	,			0	,
EB Thru	822		479	82	904	-	527	23	927	-	541	0	927	-	541	0	927	-	541
Comb. T-R		-	479			,	527			-	541			•	541			-	541
EB Right	136	0		4	150	0	,	ß	155	0	1	-	156	0		0	156	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	147	-	147	15	161	-	161	2	163	-	163	-	164	-	164	0	164	-	164
Comb. L-T		0	•			0	,			0				0	,			0	,
WB Thru	964	-	541	96	1060	-	595	ო	1063	_	595	0	1063	-	292	0	1063	τ	292
Comb. T-R		-	541			τ	595			-	595			-	295			τ-	295
WB Right	118	0	•	12	129	0		-5	127	0		0	127	0	,	0	127	0	•
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	531			is-N	584			N-S:	596			S-N	900			N-S:	009
		Ę-₩	929			E-W:	688			E-W:	704			Ę-W.	206			E-W:	902
		SUM:	1156			SUM:	1272			SUM:	1300			SUM:	1306			SUM:	1306
No. of Phases:			2				2				2				2				2
Volume / Capacity:	ıcity:	[1]	0.701			[2]	0.748			[2]	0.766			[2]	0.770			[2]	0.770
Level of Service:	, , ,		O				ပ			•	ပ				O				

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wc ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Hazeltine Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Magnolia Boulevard Peak Hour: PM Annual Growth: 2.00%

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	Ŧ	2012 \	W OTHE	2012 W/ OTHER PROJECTS	:TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	140	₩.	140	4	154	₩.	154	5	159	₩.	159	9	165	 1	165	0	165	₩.	165
Comb. L-T NB Thru	858	o -	502	98	944	0	553	12	956	o -	559	31	986	0 -	574	0	986	0 -	574
Comb. T-R	3		502	3		-	553	!			559	i		Ψ-	574)			574
NB Right	147	0		15	161	0		-	162	0		0	162	0		0	162	0	•
Comb. L-T-R -		0				0				0				0				0	
SB Left	66	-	66	10	109	-	109	-	108	-	108	0	108	-	108	0	108	F	108
Comb. L-T		0				0				0	•			0				0	•
SB Thru	657	-	363	99	723	-	399	16	739	, ,	408	29	768	-	422	0	768	-	422
SB Right	69	- 0	505.	7	76	- 0	665 -	-	77	- 0	408	0	77	- 0	422	0	77	- 0	422
Comb. L-T-R -		0				0				0				0				0	
EB Left	111	-	111	17	122	-	122	-	123	-	123	0	123		123	0	123	-	123
Comb. L-T	0	0 1	,	Š	4400	0 1	,	,	7	0 +	,	c	,	0 1		c	7	0 1	
Comb T-R	9001		622 622	5	901.		685 685	4	021		694 694)	021		/69 /69	o	0211	- •	769
EB Right	239	- 0		24	263	. 0	3	5	268	. 0	3	9	274	. 0	i '	0	274	. 0	}
Comb. L-T-R -		0				0				0				0				0	
WB Left	102	- (102	10	112	- 1	112	9	115	- (115	9	121	- (121	0	121	- (121
Comb. L-1	i	-	,	i		э.	,	;	i	٠ د	,	(į	э.		(1	ο ·	,
WB Thru	512		285 285	51	263		314	-	5/4		319	0	5/4		319 319	0	5/4	· ·	319
WB Right	29	- 0	}	9	65	- 0	,	7	64	. 0)	0	64	- 0	;	0	9	- 0	2
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	601			N-S:	661			N-S:	299			N-S:	682			N-S:	682
		Ę-₩.	724			У	797			E-W.	808			E-¥	818			Ē-₩:	818
		SUM:	1325			SUM:	1458			SUM:	1476			SUM:	1500			SUM:	1500
No. of Phases:			2		***************************************		2				2				2				2
Volume / Capacity:	ıcity:	Ξ	0.814			[2]	0.872			[2]	0.884			[2]	0.900			[2]	0.900
Level of Service:			D				۵				۵				۵				٥

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

819th turns on red from excl. lanes = 50% of overlapping left turn.

11 wic ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

12) wic ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 Accutek N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Riverside Drive Peak Hour:

AM 2.0%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year;

08/05/2008 2007 2012

	1,000	CITTAGE FOLVE FOOD	Ciddi	1000	TICORA "A	THE COOL TIME CANAL CANAL	F	2040	127170 %	STORY OF CENTER OF THE	O.F.	2042	0000 //8	TOTI ODD CHOOSED WY CFOCK	TOH C	2042	MOITA CITIM WAY CACE	NOIL I	
	7007) ב	2012		MONE IN	<u> </u>	7107		1302	2	7107		7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	5	7107	AA 1811 192		
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	ı	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	61	~	61	ω	29	← .	29	0	29	₩.	29	2	69	₩.	69	0	69	- -	69
Comb. L-T NB Thru	363	0 0 0	181	36	388	0 7 0	200	51	414	0 7 0	207	-	415	0 7 0	208	0	415	0 70 0	208
Comb. 1-K NB Right Comb. L-T-R -	121	0 - 0	121	12	133	0 - 0	133	7	140	0 - 0	140	0	140	0 - 0	140	0	140	0 - 0	140
SB Left	205	-	205	20	225	-	225	ဖ	231	-	231	5	236	-	236	0	236	-	236
Comb. L-T SB Thru	860	0 - 1	463	86	946	0 - 4	510	15	961	0 - 1	518	7	896	0 - 1	522	0	896	0 - 1	522
Comb. I-K SB Right Comb. L-T-R -	29	-00	404	7	73	-00	ore .	8	75	-00	20.0	0	75	-00	77c	0	75	-00	775 -
EB Left	24	← c	24	2	26	- 0	26	-	27	- 0	27	0	27	- 0	27	0	27	- 0	27
EB Thru	740	o v	403	74	815	· - ·	443	30	845	, - -	458	80	853) +	464	0	853) +	464
Comb. L-T-R-Comb. L-T-R-	99	-00	,	7	72	-00	,	0	72	-00	, 000	ю	75	-00	, 0 1	0	75	-00	-
WB Left	344	- c	344	34	379	- 0	379	9	385	← c	385	0	385	- c	385	0	385	€	385
WB Thru	844	0 70 0	422	84	929	2 10 0	464	24	953	0 70 0	476	80	960	0 00 0	480	0	960	0 70 0	480
WB Right Comb. L-T-R -	138	0 - 0	138	4	152	0-0	152	9	158	0-0	158	w	163	0 ~ 0	163	0	163	0 0	163
Crit. Volumes:		N-S: E-W: SUM:	525 747 1272			N-S: E-W: SUM:	577 822 1399			N-S: E-W: SUM:	586 843 1429			N-S: E-W: SUM:	591 848 1440			N-S: E-W: SUM:	591 848 1440
No. of Phases:			2				2				2				2			NATIONAL PROPERTY.	2
Volume / Capacity: Level of Service:	acity:	Ξ	0.778 C			Ξ	0.863 D			[1]	0.882 D			Ξ	0.890 D			[2]	0.860 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. You need from excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. You want lane excl. and one opt. turn lane excl. and one opt. turn lane excl. and one opt. It is a for exclusive lane. You want land excl. an

Hazelfine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 Accutek Project: File Name: Counts by: N-S St: E-W St:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	W AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SUECT	2012	2012 W/ MITIGATION	VIION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume		Volume	Volume Volume	Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	136	-	136	14	150	-	150	0	150	 1	150	17	167	← (167	0	167		167
Comb. L-T NB Thru	844	0 70 0	422	84	929	0 0 0	464	15	944	0 0 0	472	σ	953	0 77 0	476	0	953	o 0 c	- 476
Comb. I-K NB Right Comb. L-T-R -	249	0 - 0	249	25	273	0 0	273	4	277	0 - 0	277	0	277	0 - 0	277	0	277	0 0	277
SB Left	168	- 0	168	17	185	- 0	185	5	190	- 0	190	19	209	- 0	509	0	209	·- c	209
SB Thru	795	7 C	444	79	874) - -	488	48	892) - -	497	27	919) - -	511 114	0	919	>	511
SB Right Comb. L-T-R -	. 63	-00	,	ത	102	- 0 0	}		103	-00	<u>,</u>		103	-00	;	0	103	-00	1
EB Left	92	-	92	6	101	- (101	2	103	- (103	0	103	← 0	103	0	103	- 0	103
Comb. L-1 EB Thru	610	o — •	363	9	672	o	389	30	702	o •	414	31	733	o	436	0	733	o	436
Comb. 1-K EB Right Comb. L-T-R -	115	-00	ç ,	12	127	-00	n n n n	0	127	-00	, 1	12	139	-00	,	0	139	- 0 0	
WB Left	229	- 0	229	23	252	- 0	252	2	257	- c	257	0	257	F- C	257	0	257	c	257
WB Thru	587	0 70 0	293	59	645	0 70 0	323	35	680	0 70 0	340	63	743	0 N C	372	0	743	0 N C	372
WB Right Comb, L-T-R -	179	0 0	179	18	197) - 0	197	ഹ	202	0 - 0	202	47	249	0 - 0	249	0	249	0 - 0	249
Crit. Volumes:		N-S: E-W: SUM:	591 592 1182			N-S: E-W: SUM:	650 651 1301			N-S: E-W: SUM:	662 671 1333			N-S: E-W: SUM:	686 692 1378			N-S: E-W: SUM:	686 692 1378
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity: ce:	Ы	0.718 C			[1]	0.797 C			Ε	0.819 D			[1]	0.849 D			[2]	0.819 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

50% of overlapping left turn.

1/3 w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

1/2 w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 CMA8 N-S St: E-W St: Project: File Name:

Accutek Counts by:

Date: Date of Count: Projection Year:

Hazeitine Avenue @ Fashion Square Lane Peak Hour: AM Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	2012 W/ AMBIE	ENT GROWTH	ТН	2012 V	V/ OTHER	2012 W/ OTHER PROJECTS	TS	2012 V	V/ PROP(2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	/olume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	12	-	12	-	4	+-	4	0	4	•	4	0	4	-	4	o	4	-	14
Comb, L-T		0	ı			0				0				0	•	ı		0	,
NB Thru	523	-	273	52	575		300	22	297	-	311	•	298	Ψ-	314	0	598	-	314
Comb. T-R		τ-	273			-	300			-	311			-	314			-	314
NB Right	23	0 (2	25	0 (0	52	0	į	ល	30	0		0	39	0	
Comb. L-1-K-		0				0				0				0				0	
SB Left	44	-	44	4	48	-	48	0	48	-	48	10	58	٢	58	0	58	-	58
Comb. L-T		0	,			0				0				0	,			0	,
SB Thru	1180	~	294	118	1298	-	653	22	1320	-	664	0	1320	-	664	0	1320	-	664
Comb. T-R	i	-	594			-	653				664			-	664			-	664
SB Right	_	0		•	ω,	0	1	0	œ	0	1	0	ω	0	,	0	ω	0	
Comb. L-T-R -		0				0				0				0				0	
EB Left	2	-	2	0	2	-	2	0	2	-	2	0	2	-	2	0	2	-	2
Comb. L-T		0	,			0	,			0				0				0	•
EB Thru	0	0		0	0	0	,	0	0	0		0	0	0	,	0	0	0	
Comb. T-R		-	7			-	80			-	80			-	ω			-	80
EB Right	7	0	1	_	œ	0		0	80	0		0	89	0		0	ю	0	,
Comb. L-T-R -		0				0				0				0				0	
WB Left		-	-	0	-	-	-	0	1	-	ŀ	6	4	-	4	С		-	7
Comb. L-T		0	,			0	,			0				0		,		0	,
WB Thru	0	0	ı	0	0	0	,	0	0	0		0	0	0	•	0	0	0	,
Comb. T-R			2			-	2			-	7			0				0	•
WB Right	2	0	1	0	2	0		0	2	0		2	4		4	0	4	-	4
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	909			N-S:	299			N-S:	678			N-S:	678			N-S	678
		E-W:	80			E-W:	თ			E-W:	6			E-W:	12			Щ-W.	12
		SUM:	615			SUM:	9/9			SUM:	687			SUM:	069			SUM:	069
No. of Phases:			m				8				ო				က				e e
			1000							:	1								
Volume / Capacity:	<u>∷</u> .	Ξ	0.361			Ξ	0.404			Ξ	0.412			Ξ	0.414			[2]	0.384
Level of Service,			۲				τ.				¥				₹				⋖

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1600, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

If I wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

I? Wc ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 CMA8 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Date: Date of Count: Projection Year:

Hazelline Avenue @ Fashion Square Lane Peak Hour: PM Annual Growth: 2.00%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	E	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012 \	N/ PROP(2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	=	-	=	-	13	₩.	5	0	13	- (13	0	<u>6</u>	₩.	1 3	0	13		13
Comb. L-T NB Thru	1013	o	541	101	1114	o -	595	19	1133	o -	. 605	rt.	1138	0 -	- 617	C	1138	0 -	- 617
Comb. T-R)		541	į			595	2			605)		٠,-	617	•	2	-	617
NB Right	20	0	,	7	77	0		0	77	0	•	20	46	0		0	97	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	206	-	206	21	227	-	227	0	227	-	227	39	266	-	266	0	266	-	266
Comb. L-T		0				0	,			0				0				0	
SB Thru	878		441	88	996	-	486	23	686	Ψ-	497	0	989	-	497	0	686	-	497
Comb. T-R	t	 (441	•	c	 c	486	c	Ċ	- 0	497	c	(- (497	c	C	← (497
SE RIGHT	n	> c		-	٥	> C	•	>	٥	> C		5	٥	> c		5	٥	> 0	
Comb. L-1-K		>				>				>				>				Þ	
EB Left	9	-	9	-	7	-	7	0	7	-	7	0	7	-	7	0	7	-	7
Comb. L-T		0	,			0				0	1			0				0	•
EB Thru	-	0		0	-	0	,	0	-	0		0	-	0		0	_	0	•
Comb. T-R		-	5			-	Ξ			-	-				Ξ			-	-
EB Right	ത	0		Υ-	9	0		0	6	0		0	5	0	,	0	우	0	•
Comb. L-T-R -		0				0				0				0				0	
WB Left	76	-	76	8	84	-	84	0	84	-	84	20	104	-	104	0	104	-	104
Comb. L-T		0	,			0				0	,			0				0	•
WB Thru	-	0		0	-	0		0	-	0		0	-	0		0	•	0	
Comb. T-R			25			-	28			τ	29			0				0	,
WB Right	23	0		2	28	0		0	28	0	ļ	14	72		72	0	72	Ψ-	72
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	747			N-S:	822			N-S:	831			N-S:	883			N-S:	883
		E-W:	98			Ę. Ķ	92			E-W:	92			E-W:	115			E-W.	115
		SUM:	834			SUM:	917			SUM:	926			SUM:	866			SUM:	866
No. of Phases:			6				က				3				က				8
Volume / Canacity:	city.	Ξ	0.515			Ξ	0.573			[1]	0.580			Ξ	0.630			[5	0.800
Votable / Cape	acity.	Ξ								Ξ				Ξ				[7]	
בפעפו חו ספו עורפי	ان						<				τ.				n				¥

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual tum lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane. 80% of overlapping left tum. 131 for critical context lanes = 80% of overlapping left tum. 131 for critical culties a 0.07 reduction due to installation of ATTSAC as part of the Victory System No. 6. [2] wor ratio reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA9 N-S St: E-W St: Project: File Name:

Accutek

Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Moorpark Street Peak Hour: AM Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	7000	CIDACT TOVE TACE	0000	7 0000	AND WATER	TAYOOO THE	חב	2042 V	STORI ORGENIES INC.	A Cad	STS	2012	W/ PROP	2012 W/ PROPOSED PRO IECT	O.IFCT	2012	2012 W/ MITIGATION	MOITA	
	7007		2 6	Addod		, o	928	Added	Total E	No of	9 -	Added Total	Total	No.	lane	Added	Total	No. of	Tane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume			Lanes	Volume
l		-	43	4	47	-	47	0	47	-	47	0	47	-	47	0	47	1	47
Comb 1-T	2	- 0	?	•	:	0	:	,		0				0	,			0	,
NB Thru	215	· —	121	22	237	·	133	22	259	-	145	2	261	-	146	0	261	-	146
Comb. T-R		-	121				133			-	145			-	146			-	146
NB Right	56	0		ო	59	0		ო	32	0	•	O	32	0	,	0	32	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	167	1	167	17	184	-	184	0	184	-	184	0	184	-	184	0	184	+	184
Comb. L-T		0	,			0				0				0	į			0	,
SB Thru	904	-	627	90	994	-	689	21	1015	-	700		1016		701	0	1016	-	701
Comb. T-R		-	627			•	689			-	700				701			Ψ-	701
SB Right	349	0	1	32	384	0		0	384	0	,	7	386	0	•	0	386	0	,
Comb. L-T-R -		0				0				0				0				0	
EB Left	93	F	93	6	102	-	102	0	102	-	102	3	105	-	105	0	105	-	105
Comb. L-T		0	,			0	ı			0	,			0	•			0	•
EB Thru	392	-	392	39	431	Ψ-	431	42	473	-	473	0	473	-	473	0	473	-	473
Comb. T-R		0	,			0	ı			0	ı			0	1			0	•
EB Right	25	-	52	ഹ	27	-	22	0	22	Ψ-	22	0	27	_	22	0	27	•	22
Comb. L-T-R -		0				0				0				0				0	
WB Left	86	-	86	တ	95	-	95	4	66	-	66	0	66	-	66	0	66	-	66
Comb. L-T		0				0	i			0	,			0	1			0	,
WB Thru	711	-	407	71	782	-	447	22	804	_	458	0	804	-	459	0	804	-	459
Comb. T-R			407			-	447			-	458			-	459			-	459
WB Right	102	0	į	9	112	0	1	0	112	0	•	-	113	0		0	113	0	
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	699			N-S:	736			Ŋ-Ś:	747			N-S:	748			N-S:	748
		E-W:	499			E-W:	549			E-W:	572			E-W	572			Ē-₩.	572
		SUM:	1168			SUM:	1285			SUM:	1319			SUM:	1320			SUM:	1320
No. of Phases:			2				2				2				2				2
Volume / Capacity:	city:	Ξ	0.709			[2]	0.757			[2]	0.779			[2]	0.780			[2]	0.780
Level of Service:	Э		O				O				C				O				C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeitine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA9 Accutek N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Moorpark Street Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/04/2008 2007 2012

							l i				9				1011		CITIES NO.		
	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	2012 W/ AMBIEI	ENT GROWTH	H.	2012 \	W OTHER	2012 W/ OTHER PROJECTS	2	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OPECI	2012	2012 W/ MILIGATION	20	
		No. of	Lane	Added Total	Total	No. of	Lane	Added Total		No. of	Lane	Added Total		No. of	Lane	Added	Total	No, of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	86	-	86	10	108	-	108	0	108	-	108	0	108	-	108	0	108	-	108
Comb. L-T		0	,			0	,			0				0	1			0	
NB Thru	479	-	266	48	527	Ψ.	292	18	545	Ψ,	302	6	554		307	0	554		307
Comb. T-R	;		566	•	1	- (292	•	í	- (302		í	- (307	•	í	- (/05
NB Right	25	0	ı	ഹ	27	0		2	29	0	1	0	20	0		0	29	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	165	-	165	17	182	1	182	0	182	-	182	0	182	-	182	0	182	-	182
Comb. L-T		0				0	,			0				0	,			0	,
SB Thru	430	-	371	43	472	-	408	21	493	-	419	5	498	-	427	0	498	-	427
Comb. T-R		-	371			-	408			-	419			-	427			-	427
SB Right	313	0		31	344	0		0	344	0		12	356	0	,	0	356	o	•
Comb. L-T-R -		0				0				0				0				0	
EB Left	303	-	303	30	333	-	333	0	333	-	333	10	343	-	343	0	343	-	343
Comb. L-T		0				0				0	,			0	ı			0	,
EB Thru	699	-	699	29	736	-	736	38	774	-	774	0	774	Ψ-	774	0	774	-	774
Comb. T-R		0				0	,			0	,			0	•			0	
EB Right	93	-	93	ത	102	-	102	0	102	-	102	0	102	Ψ-	102	0	102	-	102
Comb. L-T-R -		0				0				0				0				0	
WB Left	9/	1	76	80	84	1	84	2	98	-	98	0	98	-	98	0	86	-	86
Comb. L-T		0	•			0	,			0	•			0				0	,
WB Thru	523	-	333	25	575	τ-	367	38	613	-	386	0	613	-	387	0	613	-	387
Comb. T-R		τ	333			-	367				386			τ-	387			_	387
WB Right	144	0		4	158	0	,	0	158	0	1	ო	161	0	,	0	161	0	1
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	469			N-S:	516			N-S:	526			N-S:	535			N-S:	535
		E-W:	745			E-W:	819			E-W:	859			E-W.	859			Ē-₩.	859
		SUM:	1214			SUM:	1335			SUM:	1386			SUM:	1394			SUM:	1394
No of Dhases.			6				6				6				6				6
140. Of 1 18363.			1			*	1			3	1				1				1
Volume / Capacity:	city:	[1]	0.739			[2]	0.790			[2]	0.824			[2]	0.829			[2]	0.829
Level of Service:	e)		ပ				U				۵				۵				٥

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Hazeltine Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA10 Acottek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Ventura Boulevard Peak Hour: AM 2.0% Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 EXIST. TRAFFIC	TRAFFIC	2012	2012 W/ AMBIEN	ENT GROWTH	Ŧ	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Movement Volume Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume Lanes	/olume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	1	0	0	O		0	0	0	,	O	0	0		0	0	0	
Comb. L-T	Ų	'			0				0	•			0				0	
NB Thru	0		0	0	0 (1	o	0	0 (0	0	0 0		0	0	0 0	
Comb. T-R			c	c	0 0		c	c	o c		c	c	o c		c	c	> c	
NB Kignt Comb. L-T-R -		,	>	5	00	1	0	•	00	1	•	Þ	0	ı	•	•	00	
SB Left	322 2	177	32	355	2	195	-	356	2	196	0	356	2	196	0	356	2	196
Comb. L-T		•			0	,			0				0				0	•
SB Thru	0	,	0	0	0 (0	0	0 (,	0	0	0 0		0	0	00	
Comb. T-R	513	513	52	564	o	564	21	585	- ℃	585	-	586	-	586	0	586	→ ⊂	586
Comb. L-T-R -					0				0				0				0	
EB Left	96 1	96	10	105	-	105	24	129	-	129	-	130	-	130	0	130	-	130
Comb. L-T	1101		7	1211	٥٥	- 606	154	1365	٥٨	- 683	C	1365	٥ ۸	- 683	O	1365	0 0	- 683
Comb. T-R		'	-		10	3	5		0	,	,	!	0				0	
EB Right	0		0	0	0		0	0	0	1	0	0	0		0	0	0	,
Comb. L-T-R -		•			0				0				0				0	
WB Left	0 0	-	0	0	0 0	-	0	0	0 0		0	0	0 0		0	0	0 0	
Comb. L-I WB Thru	1356	739	136	1492	- c	813	93	1585	-	860	0	1585	→	861	0	1585	→	861
Comb. T-R		739			-	813			-	860			-	861			•	861
WB Right	123 0		12	135	0		-	136	0	,	-	137	0	,	0	137	0	
Comb. L-T-R -		_			0				0				0				0	
Crit. Volumes:		465			N-S:	511			N-S:	520			S-S:	521			N-S:	521
	E-W: SUM:	835 1300			E-W: SUM:	919 1430			E-W: SUM:	990 1510			SUM:	991 1512			SUM:	1512
No. of Phases:		2				2				2				2				2
						1			3	1000			2	000			3	
Volume / Capacity:	acity: [1]] 0.797 C			[2]	0.853			<u>Z</u>	0.30 L			[7]	808.0 H			[7]	л 908.0
Level of Service.	, ,	اد												,				

Maximum Sum of Ortical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wo ratios reflect additional 0.03 reduction due to the future citywide ATSACATCS system installation. Assumptions:

Hazeitine Avenue Ventura Boulevard Westfliel Fashion Square /1-05-3606-1 CMA10 Acoutek N-S St: E-W St:

Project: File Name: Counts by:

Hazeltine Avenue @ Ventura Boulevard Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/04/2008 2007 2012

	1500	1015	Citt	2500	PICTER A 1/A	PACO TINDICARA WAY CASO	1	2040	DATUES !!	STUDIO DE CARE	T.	2042	0000 //	2042 WI BEOBOSED BEO IECT	TOBIC	2042	MOITABITION IN CARO	NOIT	
	7007	ZUU! EAISI. IRAFFIC)	2012		NO INI	<u> </u>	7107		7	2	2 .		1 1 1		2 .			
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	:	0	0	0	,	0	0	0	,	0	0	O	1	0	0	0	
Comb. L-T		0				0				0	1			0	ı			0	,
NB Thru	0	0		0	0	0	t	0	0	0		0	0	0	i	0	0	0	,
Comb. T-R		0	,			0				0	•			0				0	•
NB Right	0	0	,	0	0	0		0	0	0	•	0	0	0		0	0	0	ı
Comb. L-T-R -		0				0				0				0				0	
SB Left	181	2	100	18	199	2	109	г	202	7	111	0	202	2	111	0	202	2	111
Comb. L-T		0	•			0	1			0	,			0	ı			0	1
SB Thru	0	0		0	0	0	,	0	0	0	,	0	0	0	,	0	0	0	
Comb. T-R		0	,			0	i			0		٠		0	•			0	
SB Right	216	-	216	22	238	τ-	238	20	258	-	258	2	263	-	263	0	263	-	263
Comb. L-T-R -		0				0				0				0				0	
EB Left	203	-	203	20	223	-	223	16	239	+	239	9	245	-	245	0	245	-	245
Comb. L-T		0	,			0	•			0				0	1			0	
EB Thru	1474	7	737	147	1621	2	811	139	1760	2	880	0	1760	2	880	0	1760	7	880
Comb. T-R		0				0				0	,			0	į			0	,
EB Right	0	0		0	0	0	í	0	0	0		0	0	0		0	0	0	1
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0		0	0	0		0	0	0		0	0	0		0	0	0	<u> </u>
Comb. L-T		0	,			0	,			0				0				0	,
WB Thru	1237	-	753	124	1360	-	828	151	1511	-	905	0	1511	-	906	0	1511	-	906
Comb. T-R		-	753			-	828			Ψ-	902			Ψ-	906			•	906
WB Right	569	0		27	296	0	,	m	299	0		2	301	0		0	301	0	,
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	115			N-S:	126			N-S:	138			S-S:	140			N-S:	140
		E-W:	926			E-W:	1051			Ē. Ķ	1144			E-W	1151			E-W:	1151
		SUM:	1071			SUM:	1178			SUM:	1283			SUM:	1292			SUM:	1292
No of Dhases.			6				6				6				,				6
10. Or 110363			7				ı				1				1				ı
Volume / Capacity:	ıcity:	Ξ	0.644			[2]	0.685			[2]	0.755			[2]	0.761			[2]	0.761
Level of Service:	je j		В				B				C				ပ				U

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Woodman Avenue Magnolia Boulevard Westlield Fashion Square /1-05-3606-1 CMA11 Accutek Project: File Name: Counts by: N-S St: E-W St:

Woodman Avenue @ Magnolia Boulevard

CRITICAL MOVEMENT ANALYSIS

AM 2.0% Peak Hour: Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	7000	2007 EVICT TDAEEIC	Ciaa	2042	TICHNA //A	MAMBIENT GBOWTH	1	2012 V	W OTHER	2012 W/ OTHER PROJECTS	<u>ل</u> ا	2012	W PROP	2012 W/ PROPOSED PROJECT	J.IFCT	2012	2012 W/ MITIGATION	NOITA	
	7007	2 .	2 .	7 .						7	, <u> </u>		,	31		100000	100	, J. J.	
		No. of	Lane	Added	Total	No. of	Lane			NO. 01	Lane	Added		NO. 01	rane	Added	Otal	NO. OI	- Cane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	96	₩.	96	10	105	₩.	105	0	105	₩ (105	0	105	← (105	0	105	← 0	105
Comb. L-T NB Thru	740	0 -	421	74	815	0 -	463	28	843	o -	- 477	0	843	o ←	478	0	843	- 0	- 478
Comb. T-R		_	421			-	463			-	477			-	478				478
NB Right	102	0		10	112	0		0	112	0 (-	113	0 0		0	113	0 0	1
Comb. L-T-R -		0				0				0				0				5	
SB Left	165	-	165	17	182	-	182	7	181	-	181	0	181	-	181	0	181	-	181
Comb. L-T		0	,			0	1			0				0	1			0	
SB Thru	1265	 1	692	126	1391	τ− τ	761	21	1412		171	လ	1417	τ- τ	774	0	1417		774
SB Right	119	- 0	760	12	130	- 0	ē ,	0	130	- 0		0	130	- 0	;	0	130	- 0	<u> </u>
Comb. L-T-R -		0				O				0				0				0	
EB Left	64	-	64	9	71	-	71	0	71	 -	71	0	7	₩ (71	0	71	(7.1
Comb. L-T	2,2	0 +	. 777	70	0	۰ -	. 525	70	000	⊃ ~	537	c	0	> +	, 537	c	g	⊃ ←	- 537
Comb T-R	7/0		477	0	000		525	t V	200		537	0	2		537	•	3		537
EB Right	83	0	·,	80	92	0		0	95	0		0	95	0	,	0	95	0	ı
Comb. L-T-R -		0				0				0				0				0	
WB Left	107	- 0	107	11	118	- 0	118	-	119	 c	119	0	119	~ C	119	0	119	 C	119
Comb. L-1	976	> +	, 2,	9	1040	→	. 502	Cr.	1043	> 	. 593	-	1044	۰ ح	, 593	c	1044	-	503
Comb. T-R	6		538	3	2		592 592	•	2		593	-			593)			593
WB Right	131	0		13	144	0		-5	142	0	ı	0	142	0	,	0	142	0	,
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	787			N-S:	866			N-S:	877			S-S:	879			N-S:	879
		Е-W.	603			М.	663			М. ф.	663			М	664			Х	664
		SOM:	1390			SOM:	6Z9L			SON CON	1540			SOM:	1543			SOIN:	543
No. of Phases:	ļ		2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.857			[2]	0.919			[2]	0.927			[2]	0.929			[2]	0.929
Level of Service:	.e:		٥				E				Ш				Ш				E

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes. 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Woodman Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA11 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Magnolia Boulevard Peak Hour: 2.00% Annual Growth: Peak Hour:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	7002	CIEST TOACE	VIE V	2042	VAI ARGUE	2012 W AMBIENT CEOMEN	15	2012	V OTHER	2012 W/ OTHER PROJECTS	E.	2012	W PROP	2012 W/ PROPOSED PRO JECT	D.IECT	2012	2012 W/ MITIGATION	NOITA		Γ
	200		2 1	1010 H 1010 W		, J			100	No	-	Addod	Total	No of	000	Added Total	Total	No.	010	
		NO. 01	Lane	Added	Added Total No. of	5 5	Velumo			5 5	Volumo			50.00	Volume	Volume Volume	Volume	20.00	Volumo	
Movement	volume	Lanes	Volume	volume	Aoinme	Lanes	Voluntie	ACIUILIE	Aoinile	Lailes	VOIDING			Falles	Aciditie	o dining	all line	Fallos		Ţ
NB Left	66	-	66	10	109	•	109	0	109		109	0	109	 (109	0	109	₩ (=	109
Comb. L-T		0	,			0	ì			0	,			0		,		o ·	,	-
NB Thru	1042		570	104	1146		627	22	1168	⊷ -	638	0	1168	- -	641 641	0	1168		വ് വ	641
NB Right	80	- 0	2	10	108	- 0	30 '	0	108	- 0	8	9	114	- 0	;	0	114	- 0	,	
Comb. L-T-R -		0				0				0				0				0		
SB Left	76	-	76	8	84	-	84	F	83	1	83	0	83	-	83	0	83	-		83
Comb. L-T		0				0	,			0	1			0				0	ı	
SB Thru	834	•	462	83	917	-	509	24	941	-	521	18	929		530	0	929	-	iń i	530
Comb. T-R		•	462			-	209		;	 (521	•		- (530	,		- (เก๋	 06 06
SB Right	6	0		ത	100	0	,	0	100	0 1	ı	0	100	0 (0	100	0 (
Comb. L-T-R -		0				0				0				0				0		
EB Left	87	F	87	6	96	-	96	o	96	-	96	0	96	-	96	0	96	-		96
Comb. L-T		0	,			0				0				0				0	,	
EB Thru	937	-	526	94	1031	-	578	14	1045	-	585	0	1045	-	585	0	1045		เด็	585
Comb. T-R			526			-	578			τ-	585			-	585	1		-	เกิ	385
EB Right	114	0	,	-	126	0	,	0	126	0	1	0	126	0	,	0	126	0 (
Comb. L-T-R -		0				0				0				0				0		
WB Left	104	-	104	10	114	-	114	-	115	1	115	0	115	-	115	0	115	-	-	115
Comb. L-T		0				0	,			0	,			0	,			0	,	
WB Thru	545	_	315	54	299	-	346	=	610	-	351	Ф	616	-	354	0	616	-	ന്	354
Comb. T-R		-	315			-	346			-	351			-	354			_	ന്	354
WB Right	84	0		ω	93	0	,	Υ,	92	0	,	0	95	0	,	0	92	0		
Comb. L-T-R -		0				0				0				0				0		
Crit. Volumes:		N-S:	646			N-S:	710			N-S:	720			N-S:	723			N-S:	7	723
		E-W:	630			E-W:	693			E-W.	701			E-W:	701			Ë-W:	7	5
		SUM:	1276			SUM:	1403			SUM:	1421			SUM:	1424			SUM:	14	124
No. of Phases:			2				2				2				2					2
																				T
Volume / Capacity:	acity:	Ξ	0.780			[2]	0.835			[2]	0.847			[2]	0.849			[2]	0.8	0.849
Level of Service:	ce:		ပ				۵				۵				D				٥	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA12 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year;

	7000	COOT TOYER	7132	2042	CATO VAV AREIE	TAL GOOD TAL	1	2042 V	V. OTHER	2012 W/ OTHER DRO JECTS	ا پر	2012	M PBOB	TOTI OBO CENTRO INC.	TOE!	2012 1	NOITA MITIGATION	NOIL	
	7007		ב ב	7107		AOUS IN	<u> </u>	2012		LUCATO	2	7107				7 .			
	~	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume L	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	183	2.0	101	18	201	80	111	0	201	01.0	11	18	219	8	121	0	219	8	121
Comb. L-1 NB Thru	734) N C	. 367	73	808	0 77 0	404	20	828	0 77 0	414	0	828	0 70 0	414	0	828	0 10 0	414
Comb. T-R NB Right [2] Comb. L-T-R -	219	0 - 0	219	22	241	0 - 0	241	52	293	0 - 0	293	0	293	0 0	293	0	293	0-0	293
SB Left	229	- 0	229	23	252	← 0	252	7	259	F- 0	259	0	259	- 0	259	0	259	- 0	259
SB Thru	1165	9 N C	582	116	1281	o 00 c	641	4	1295	0 00 0	648	m	1298	0 10 0	649	0	1298	- 73 C	475
Comb. 1-K SB Right [2] Comb. L-T-R -	111	0 - 0	E	1	122	0 0	122	0	122	0 - 0	122	ო	125	0 - 0	125	0	125	-00	Ĉ
EB Left	95	- (98	6	104	- 0	104	-	105	- 0	105	0	105	- 0	105	0	105	- 0	105
Comb. L-1 EB Thra	880	0 77 0	440	88	968) N C	484	4	1009	0 00 0	504	ß	1014	0 70 0	507	0	1014	0 00 0	507
Comb. T-R EB Right Comb. L-T-R -	116	0-0	116	12	128	o c	128	-	129	0 - 0	129	30	159	0 - 0	159	0	159	0 + 0	159
WB Left	291	۲- (291	29	320	- 0	320	35	355	- 0	355	0	355	- 0	355	0	355	- 0	355
WB Thru	1010) N C	505	101	1111	0 10 0	. 555	35	1146	0 00 0	573	12	1158	0 00 0	579	0	1158	0 00 0	629
Comb. I-R WB Right Comb. L-T-R -	185	0 - 0	185	61	204	o — o	204	7	211	0 - 0	211	0	211	o ← o	211	0	211	0 0	211
Crit. Volumes:		N-S: E-W: SUM:	683 731 1414			N-S: E-W: SUM:	751 804 1556			N-S: E-W: SUM:	758 860 1618			N-S: E-W: SUM:	770 862 1632			N-S: E-W: SUM:	673 862 1535
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	ity:	Ξ	0.959 E			Ξ	1.061 F			Ξ	1.107 F			Ξ	1.117 F			[3]	1.016 F

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of volume just turn.

Fight turns on red from excl. lanes = 50% of overlapping left turn.

Fight turns as a 0.07 reduction due to installation of ATSA.

Figh vortabound right turn has an overlapping phase with the westbound left-turn movement and southbound right turn has an overlapping phase with the eastbound left-turn movement.

Fight vortabound right turn has an overlapping phase with the westbound left-turn movement.

Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA12 Accutek Woodman Avenue N-S St: E-W St: Project: File Name: Counts by:

Date: Date of Count: Projection Year:

08/04/2008 2007 2012

Woodman Avenue @ Riverside Drive Peak Hour: 2.00% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	2012 W/ AMBIEN	ENT GROWTH	H	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	JJECT	2012	2012 W/ MITIGATION	VIION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume		Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	342	2.0	188	34	376	2.0	207	-	377	2.0	208	71	448	<i>(</i> 7)	247	0	448	20.0	247
Comb. L-1 NB Thru	917) N C	459	95	1009	0 00	505	18	1027	O 00 C	514	0	1027	O 70 C	514	0	1027	0 70 0	514
Comb. I-K NB Right [2] Comb. L-T-R -	197	0 - 0	197	20	216	0 + 0	216	_	223	0 + 0	. 223	0	223	0 - 0	223	0	223	0 - 0	223
SB Left	150	- 0	150	15	165	- 0	165	5	170	- 0	170	0	170	- 0	170	0	170	← c	170
SB Thru	823) N C	411	82	905	O 70 C	452	50	925	0 10 0	462	10	935	0 00 0	467	0	935	- 10 0	385
SB Right [2] Comb. L-T-R -	188	0 - 0	188	6	207	0 0	207	-	208	0 0	208	12	220	0 - 0	220	0	220	- 0 0	1
EB Left	213	- 0	213	21	235	- €	235	-	236	- 0	236	0	236	0	236	0	236	- c	236
EB Thru	916	0 70 0	458	92	1008	0 10 0	504	38	1046	0 101 0	523	36	1082	0 10 0	541	0	1082	2 14 0	541
Comb. I-R EB Right Comb. L-T-R	257	0 0	257	56	283	0 0	283	τ-	284	0 0	284	300	584	0 - 0	584	0	584	0-0	584
WB Left	239	- 0	239	24	263	- 0	263	9	269	- 0	269	0	269	- 0	269	0	269	- 0	269
WB Thru	206	O 70 C	453	9	966	0 00 0	499	42	1040	0 101 0	520	46	1086	0 00 0	543	0	1086	0 10 0	543
Comb. I-R WB Right Comb. L-T-R -	254	0 - 0	254	25	279	0 0	279	4	283	0 0	283	0	283	0 0	283	0	283	0 - 0	- 283
Crit. Volumes:		N-S: E-W: SUM:	608 697 1306			N-S: E-W: SUM:	669 767 1436		=	N-S: E-W: SUM:	683 792 1475			N-S: E-W: SUM:	714 810 1524			N-S: E-W: SUM:	683 810 1493
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	acity:	[1]	0.880 D			[1]	0.975 E			[1]	1.003 F			[1]	1.038 F			[3]	0.986 E

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lane = 50% of overlapping left turn.

Fight turns on red from excl. lane = 50% of overlapping left turn.

Fight turns on red from excl. lane = 50% of overlapping left turn.

Fight vertices a 0.07 reduction due to installation of ATSAC and to the Victory System No. 6. 12! Northbound right turn has an overlapping phase with the wastbound left-turn movement and southbound right turn has an overlapping phase with the wastbound left-turn movement [3] v/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps AM 2.0% Peak Hour: Annual Growth:

08/04/2008 2007 2012

Date: Date of Count: Projection Year:

	2007	2007 EXIST, TRAFFIC	AFFIC	2012 W	// AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	2012 W/ OTHER PROJECTS	PROJE	CTS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	VTION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume		Volume		/olume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	348	-	348	35	383	-	383	-	384	, -	384	0	384	-	384	0	384	Ψ-	384
Comb. L-T		0				0				0				0	,			0	1
NB Thru	890	ო	297	89	979	က	326	22	1036	က	345	80	1044	m	348	0	1044	က	348
Comb. T-R		0		,		0		•	•	0 (•	ć	0 0	•	(•	0 0	
NB Right	0	0		0	0	0		0	0	0 (,	0	0	o (,	0	9) (
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0	,	0	0	0		0	0	0	-	0	0	0		0	0	0	,
Comb. L-T		0	,			0				0	•			0	•			0	,
SB Thru	1062	4	265	106	1168	4	292	42	1210	4	303	우	1220	4	305	0	1220	4	305
Comb. T-R	,	ο ·		i	i	o ·	,	(Ċ	0 1	,	L	č	0 1	,	c	č	o ,	
SB Right	545	σ	545	55	299	- c	266	ກ	808	- 0	809	n	613	- c	613	0	613	- c	5 0
Comb. L-1-k		0				>				>				>				0	
EB Left	0	0	-	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T		0				0	,			0	,			0	ı			0	,
EB Thru	0	0	1	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. T-R		0	1			0	,			0	,			0				0	
EB Right	0	0		0	0	0		0	0	0	,	0	0	0	,	0	0	0	
Comb. L-T-R -		0				0				0				0				Ö	1
WB Left	314	-	173	31	345	-	190	18	363	-	200	0	363	-	200	0	363	-	200
Comb. L-T		0				0				0	1			0			!	0	,
WB Thru	4	0	265	0	C)	0 (291	0	co	0 (306	0	2	0 0	309	0	2	0 0	309
Comb. T-R	Ċ	۰ ۰		1	Ċ	۰ د		,	ć	Э т		c	7	> +	, 5,	c	770	Э т	- 170
vva kigni Comb. L-T-R -	607		5	7	767		8	Ţ	905		9	0	<u> </u>		2	•	<u>†</u>		2
Crit. Volumes:		N-S:	893			N-S:	983			N-S:	993			N-S:	966			N-S:	866
		E-W: SUM:	265 1158			E-W: SUM:	291 1274			E-W: SUM:	306 1298			E-W: SUM:	309 1307			E-W: SUM:	309
No. of Phases:			3				9				3				ო				က
Volume / Canacity	- Aio	Ξ	0.743			[2]	0.824			=	0.841			Ξ	0.847			2	0.817
Volunte / Capaci		Ξ	} ;							Ξ				Ξ.	2			<u>[</u>	
	į		,																

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. Assumptions:

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps Peak Hour: PM Annual Growth: 2.00%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	E	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	NOIT	
		No of	eue	Arided Total	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	_	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	314	-	314	31	345	-	345	2	347		347	0	347	-	347	0	347	-	347
Comb. L-T NB Thru	1186	0 6	395	119	1304	0 11	435	26	1330	0 m	- 443	31	1361	O 10	- 454	0	1361	0 10	454
Comb. T-R	2	0	}	-		0	}	ì		0	!	;		0)		0	
NB Right	0	00		0	0	0 0	•	0	0	00		0	0	00	,	0	0	00	
00110.		9				•))				,	
SB Left	0	0 0		0	0	0 0	,	0	0	0 0	,	0	0	0 0	,	0	0	00	1
SB Thru	917	o 4	229	92	1009	0.4	252	28	1037	0 4	259	29	1104	0.4	276	0	1104	2 4	276
Comb. T-R SR Right	486	o	486	49	534	0 -	534	۲,	533	o 	533	33	566	o -	566	0	566	o ~	- 566
Comb, L-T-R -		0				0				0				0				0	
EB Left	0	0	-	0	0	0		0	0	0		0	0	0	,	0	0	0	
Comb. L-T EB Thru	0	00		0	0	00		0	0	00	, ,	0	0	0 0	, ,	0	0	00	
Comb. T-R	ı	0	1	•	•	0	,	•	•	0	,	•	(0 (1	•	•	0	
EB Right Comb. L-T-R -	0	0 0		5	0	00	1	0	0	00	i	5	o	00		o o	o	00	ı
WB Left Comb. I-T	402	۰ 0	221	40	443	1 0	244	16	429	- 0	252	0	429	- 0	252	0	459	۰ 0	252
WB Thru	0	0	344	0	0	0	379	0	0	0	387	0	0	0	401	0	0	0	401
Comb. T-R WB Right Comb. L-T-R	363	0	200	36	388	0	220	-	400	o	. 220	33	433	⊃ -	238	0	433	0	238
Crit. Volumes:		N-S: E-W: SUM:	800 344 1144			N-S: E-W: SUM:	880 379 1259			N-S: E-W: SUM:	881 387 1267			N-S: E-W: SUM:	914 401 1315			N-S: E-W: SUM:	914 401 1315
No. of Phases;			3				က				က				ო				3
Volume / Capacity:	city:	Ξ	0.733			E	0.813			Ξ	0.819			[]	0.853			[2]	0.823
Level of Service:	ë.		U				D				۵				D				٥

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes 50% of overlapping left turn.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. Assumptions:

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour: AM AM 2.0% Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	7000	CHEAT TOYET	CIBBL	2012	W/ AMRIE	2012 WI AMBIENT GROWTH	H	2012	"OTHER	2012 W/ OTHER PROJECTS	15	2012	W PROP(2012 W/ PROPOSED PROJECT	D.JECT	2012	2012 W/ MITIGATION	NOIT	
	7007	2 .	2 .							30 -14	· -	10000	F	No of		7777	Total	No of	our l
		No. of	Lane	Added lotal	otal	No. of	Lane	Added		NO. 01	Lane	Added			ב ב	Yanen	100	5.0	רמו ב
Movement	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	ı	0	0	0	ı	0	0	0	,	0	0	0	,	0	0	0	
Comb. L-T		0	1			0				0	1			0				0	•
NB Thru	786	m +	262	79	865	o -	288	44	606	ო -	303 413	ო	912	eo ←	304 413	0	912	m ←	304 413
NB Right	359	- 0	3	36	395	- 0	3	18	413	0	,	0	413	0	!	0	413	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	340	- '	340	34	374	- 0	374	10	384	- 0	384	5	389	- c	389	0	389	- 0	389
Comb. L-1 SB Thru	1093	o 0	547	109	1202	о «	601	20	1252	р К	- 626	ß	1257	0 0	629	0	1257	9 0	629
Comb. T-R		0				0	•			0				0	1			0	1
SB Right	0	0 (1	0	0	00		0	0	0 0	Ī	0	0	00	1	0	0	00	,
Comb. L-1-K-		5				5				>				o				>	
EB Left	339	- 0	186	34	373	- 0	205	14	387	- c	213	ις	392	- с	216	0	392	 C	216
EB Thru	က	00	332	0	m	00	365	0	ဗ	0	372	0	ო	0	374	0	က	0	374
Comb. T-R	202	0 +	316	90	131	O +	- 227	•	432	0 -	- 238	c	432	o -	- 238	c	432	0 -	238
Comb. L-T-R -			2	3	P		į	-	į	· 		•		-		!	!	-	
WB Left	0	0	1.	0	0	0		0	0	0	,	0	0	0	1	0	0	0	
Comb. L-T		0	,			0				0				0	1			0	•
WB Thru	0	0	,	0	0	0		0	0	0 (0	0	0 (0	0	0 (
Comb. T-R	(0 0		•	ć	0 0		c	c	o c		c	c	> C		c	c)	
We kignt Comb, L-T-R -	o	00		5	>	00	ı	>	0	0 0	ı	0	0	0 0	ı	•	o	00	1
Crit. Volumes:		N-S:	669			N-S:	769			N-S:	797			N-S:	802			N-S:	802
		E-W: SUM:	332 1031			E-W: SUM:	365 1134			E-W: SUM:	372 1169			E-W: SUM:	374 1176			E-W: SUM:	374 1176
No. of Phases:	l		က				3				е				3				ო
Volume / Capacity:	acity:	Ξ	0.654			[2]	0.696			[2]	0.720		***************************************	[2]	0.725			[2]	0.725
Level of Service:	ce:		В				В				ပ				S				S

Maximum Sum of Ortical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour: PM Annual Growth: 2.00%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 E	2007 EXIST TRAFFIC	AFFIC	2012 \	2012 W/ AMBIEI	ENT GROWTH	II.	2012 V	V/ OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	TION	
		No. of	Lane	Added	Total	No, of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume		Volume	Lanes	Volume			Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
		0	1	0	0	0	1	0	0	0	,	0	0	0		0	o	0	,
Comb. L-T		0				0				0	i			0	,			0	1
NB Thru	1094	დ -	365 365	109	1203	e +	404 401	28	1231	ω ←	410 414	=	1242	ო ←	4 4 4 4 4	0	1242	e ←	414 414
NB Right	364	- 0	,	36	400	. 0		14	414	0		0	414	0	ı	0	414	0	,
Comb. L-T-R -	,	0				0				0				0				0	
SB Left	287	-	287	29	316	-	316	0	316	- 0	316	36	352	- 0	352	0	352	- 0	352
Comb. L-T	1010	0 0	505	10	1111	o ~	555	45	1156	o 0	578	30	1186	N C	593	0	1186	N C	593
Comb. T-R	2	10	}	2		0				0				0	,			0	
SB Right	0	0 0		О	0	00		0	0	00	ı	0	0	0 0		0	0	00	•
Comb. L-1-K-		>				>				5)				•	
EB Left	467	- (257	47	514	- 0	283	0	514	- 0	283	19	533	- 0	293	o	533	- 0	293
Comb. L-1 EB Thru		00	372	0	-	00	409	0	-	00	410	0	-	0	418	0	~	0	418
Comb. T-R	7 11 1	0 +	100	ä	303	0 +	, atc	c	707	۰ -	. 217	c	394	o -	217	C	394	0	217
Comb. L-T-R -			200	8	8		2	1	3		<u>.</u>	•	3	Ψ-	i	•		· -	
9	c	c		c	c	-		c	c	c		c	c	c		c	c	c	
Comb. L-T	>	0		>	0	0		•	•	0		•	•	0			•	0	ı
WB Thru	0	0		0	0	0		0	0	0		0	0	0	1	0	0	0 (1
Comb. T-R		0		•	•	0 (•	•	0 0		ć	c	0 0		•	c	5 6	•
WB Right Comb. L-T-R -	0	00		0	0	00	,	0	>	00	ı	>	0	00		5	o	00	1
Crit. Volumes:		N-S:	652			N-S:	717			N-S:	730			N-S:	766			N-S:	766
		E-W:	372 1023			E-W: SUM:	409 1126			E-W: SUM:	410 1140			E-W: SUM:	418 1184			SUM:	418 1184
No. of Phases:			ю				8				ю				က				၉
Volume / Capacity:	acity:	Ξ	0.648			[2]	0.690			[2]	0.700			[2]	0.731			[2]	0.731
Level of Service:	ce:		8				8				8				O				O

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15 Accutek Project: File Name: Counts by: N-S St: E-W St:

Woodman Avenue @ Moorpark Street AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	HE	2012 \	W OTHE	2012 W/ OTHER PROJECTS	TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	- 1	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	12	- (12	-	4	₩.	14	0	14	₩ (4	0	14	← (14	0	14	 (14
Comb. L-1 NB Thru	490) N C	245	49	539) N C	. 269	22	596	9 20 0	298	2	598) N C	299	0	598) N C	- 299
Comb. I-K NB Right Comb. L-T-R -	24	0 0	24	2	26	0-0	- 26	2	28	0-0	- 28	0	28	0-0	- 28	0	28	0 - 0	- 28
SB Left	297	- 0	297	30	327	- 0	327	2	329	- c	329	-	330	← C	330	0	330	- 0	330
SB Thru	851) - -	528	85	936	o	581 581	48	984) 	605 605	က	286) 	607	0	286) - -	607
SB Right Comb. L-T-R -	206	-00		21	227	-00	,	0	227	-00	3	0	227	-00	· .	0	227	- 0 0	<u> </u>
EB Left	111	- c	111	=	122	- 0	122	0	122	- 0	122	-	123	← c	123	0	123	- 0	123
EB Thru	551	o — 0	551	55	909	> ← (909	28	634	o c	634	0	634	o ← 0	634	0	634	o 0	634
Comb. I-R EB Right Comb. L-T-R -	40	0 - 0	40	4	43	0 - 0	43	o	43	0 - 0	43	0	43	0 - 0	43	0	43	0 + 0	- 43
WB Left	85	- 0	85	6	94	- 0	94	2	96	- 0	96	0	96	 c	96	0	96	- 0	96
WB Thru	726	o ← c	726	73	799	o c	799	43	842	o ← c	842	~	843	o c	843	0	843	o — c	843
WB Right Comb, L-T-R -	276	0 - 0	276	28	303	0 0	303	4	307	0 - 0	307	0	307	0-0	307	0	307	0 - 0	307
Crit. Volumes:		N-S: E-W: SUM:	542 837 1380			N-S: E-W: SUM:	597 921 1518			N-S: E-W: SUM:	627 964 1591			N-S: E-W: SUM:	629 966 1595			N-S: E-W: SUM:	629 966 1595
No. of Phases:	ļ.		2				2				2				2				2
Volume / Capacity: Level of Service:	city: e:	Ξ	0.850 D			[1]	0.942 E			[1]	0.991			Ξ	0.993 E			[2]	0.963 E

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

80% of volume is assigned to exclusive lane.

70% of vordapping laft turn.

81 Wich ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] Wich ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Assumptions:

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Woodman Avenue @ Moorpark Street Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	\FFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	H	2012	N/ OTHE	2012 W/ OTHER PROJECTS	STS	2012	W PROP	2012 W/ PROPOSED PROJECT	JUECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	29	-	29	7	73	₩ 1	73	0	73	₩ (73	0	73	← (73	0	73	← (73
Comb. L-T NB Thru	772	0 70 0	386	77	849	0 77 0	424	44	893	0 0 0	446	ω	901) N C	450	0	901	o 70 c	450
Comb. I-R NB Right Comb. L-T-R -	. 55	o c	. 55	9	61	o + 0	61	7	63	0 - 0	63	0	. 63	0 - 0	63	0	63	0 - 0	63
SB Left	263	- 0	263	26	289	- 0	289	o	289	- 0	289	10	299	- 0	299	0	299	- 0	299
SB Thru	734) - -	492	73	808	o	542	47	855	→ →	565 885	20	875) - -	575 575	0	875	o — +	575
SB Right Comb, L-T-R -	251	-00	1	25	276	-00	ž '	0	276	-00	,	0	276	-00	,	0	276	-00	
EB Left	205	- 0	205	20	225	- 0	225	0	225	- 0	225	2	227	- 0	227	0	227	- 0	227
EB Thru	685	⊃ (. 685	69	754	o	754	35	789	o	789	0	789	o c	789	0	789	o - 0	789
Comb. 1-K EB Right Comb. L-T-R -	- 82	o + c	82	ω	06	0 0	06	0	06	0 - 0	06	0	06	0 - 0	06	0	06	0 0	
WB Left	71	- 0	71	7	78	- 0	78	က	81	- 0	81	0	81	F- C	81	0	84	- 0	81
WB Thru	551	o	551	55	909	o ← c	909	45	651	o ← c	651	2	653	o c	. 653	0	653	o c	. 653
œ	258	0 - 0	258	26	284	0 - 0	284	0	284	0 - 0	284	0	284	0 - 0	284	0	284	0-0	284
Crit. Volumes		N-S: E-W: SUM:	649 756 1405			N-S: E-W: SUM:	714 832 1546			N-S: E-W: SUM:	736 877 1613			N-S: B-W: SUM:	750 881 1631			N-S: E-W: SUM:	750 881 1631
No. of Phases:	نف		2				2				2				2				2
Volume / Capacity:	acity: ce:	E	0.867 D			Ξ	0.960 E			Ξ	1,005 F			Ξ	1.017 F			[2]	0.987 E

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Assumptions:

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Boulevard Peak Hour: AM 2.0% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	VE 7000	CONT. TO VETICAL	CIZZ	2042	SO42 WAY ARREST	HTWOOD IN	ī	2042	AV OTHE	2012 W/ OTHER DRO IECTS)TC	2012	QC00 //W	2012 W/ PROPOSED PRO IECT	TORIC	2012	2012 W/ MITIGATION	NOIT	
		No of	200	Added Total	Total	No of	908	Added Total	Total	No.	ane	Added	Total	Jo ON	lane	Added	Total	Į,	ane
Movement	Volume L	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	-	Lanes	Volume		Volume	Lanes	Volume
NB Left	51	-	51	5	56	₩.	56	8	64	- 1	64	0	64		64	0	64	← (64
Comb. L-T NB Thru	201	00 -	, ,	20	221	00+	1 1	9	227	00+		-	228	00+		0	228	00+	י י
Conflight NB Right Comb, L-T-R -	19	-00	67	2	21	-00		4	25	-00		0	25	-00	707	O	25	-00	
SB Left	216	← ¢	216	22	238	- 0	238	7	245	- 0	245	2	247	- 0	247	0	247	- 0	247
SB Thru	225	o ← c	225	22	247	o ← c	247	ഹ	252	o ← c	. 252	-	253	o ← c	253	0	253	> ← C	253
SB Right Comb. L-T-R -	362	0 - 0	362	36	398	0-0	398	38	436	0-0	436	0	436	0-0	436	0	436	0 - 0	436
EB Left	142	- 0	142	14	157	- 0	157	41	198	- c	198	0	198	- c	198	0	198	- 0	198
EB Thru	1082) - -	554	108	1190) 	609	96	1286) 	665	0	1286)	665	0	1286) 	665
EB Right Comb. L-T-R -	56	-00	}	ო	59	.00	}	16	45	.00	}	0	45	.00	}	0	45	-00)
WB Left	45	- c	45	4	49	F C	49	-	20	⊢ c	90	O	20	← c	20	0	20	- c	90
WB Thru Comb. T-R WB Right Comb. L-T-R -	114	00	603	109	1200	000	663	52	1252	000	695	← ←	1253	000	969	0 0	1253) - - 0 0	969 -
Crit. Volumes:	2 4 6	N-S: E-W: SUM:	436 745 1181			N-S: E-W: SUM:	479 820 1299			N-S: E-W: SUM:	496 893 1389			N-S: E-W: SUM:	499 894 1393			N-S: E-W: SUM:	499 894 1393
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	oity:	Ε	0.717 C			[2]	0.766 C			[2]	0.826 D			[2]	0.829 D			[2]	0.829 D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.
For dual turn lanes, 55% of volume is assigned to heavier lane.
70% of volume is assigned to exclusive lane.
70% of volume is assigned to exclusive lane.
8 Right turns on red from excl. Ianes = 50% of voerlapping left turn.
11 yor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
[2] wor ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Assumptions:

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 N-S St: E-W St: Project: File Name:

Accutek Counts by:

Woodman Avenue @ Ventura Boulevard CRITICAL MOVEMENT ANALYSIS

ğ 2.00%

Annual Growth: Peak Hour:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	W/ AMBIE	2012 W/ AMBIENT GROWTH	H.	2012	W OTHE	2012 W/ OTHER PROJECTS	TS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012 \	2012 W/ MITIGATION	TION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	59	- c	59	Ф	65	 0	65	24	89	← 0	88	0	89	← 0	89	0	83	← 0	89
NB Thru	213	00-		21	235	o -	241	9	241	0 0 -	- 249	ဖ	247) O -	255	0	247	00 -	 255
NB Right Comb. L-T-R -	· .	.00	1	-	7		;	7	σ	- 0 0) 	0	σ	- 0 0	3	0	თ	-00	2
SB Left	125	- c	125	12	137	c	137	-	136	- c	136	12	148	- 0	148	0	148	- 0	148
SB Thru	161	o ← c	161	16	177	o c	177	4	181	o c	181	9	187) - c	187	0	187	o ← c	187
SB Right Comb. L-T-R -	237	0 - 0	237	24	261	0 - 0	261	47	308	0 0	308	Ψ-	309	o – o	309	0	309	0 - 0	309
EB Left	206	- 0	206	21	227	- c	227	40	267	- c	267	0	267	C	267	0	267	- 0	267
EB Thru	1093	· -	567	109	1202) 	623	84	1286) + +	674	0	1286) - -	674	0	1286) 1	674
EB Right Comb. L-T-R -	4	- 0 0	<u>.</u>	4	45	-00	3	18	63	-00	t i	0	63	-00	<u>.</u>	0	63	-00	† D
WB Left Comb 1-T	24	- 0	24	2	26	۰ ۵	. 26	-	27	- c	27	0	27	- c	27	0	27	- c	27
WB Thru	910		515	9	1001	, - -	566	86	1087) - -	609	2	1089) 	611	0	1089)	611
WB Right Comb. L-T-R -	120	00	•	12	132	00		0	132	00		7	134	00	:	0	134	-00	,
Crit. Volumes:		N-S: E-W: SUM:	344 721 1065			N-S: E-W: SUM:	379 793 1171			N-S: E-W: SUM:	386 876 1261			N-S: E-W: SUM:	404 878 1281			N-S: E-W: SUM:	404 878 1281
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	[1]	0.640 B			[2]	0.681 B		ŀ	[2]	0.741 C			[2]	0.754 C			[2]	0.754

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

70% of vortiapping left turn.

11 wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wc ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA17 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2 700C	2007 EXIST TRAFFIC	SEFIC	2012	W/ AMB	2012 W/ AMBIENT GROWTH	F	2012 \	2012 W/ OTHER PROJECTS	PROJE	CTS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	JUECT	2012	2012 W/ MITIGATION	ATION	
		No of	2 6	Added Total	Total	No. of	- ane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume		Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
					٠			c				rt	ц	,-	ư	c	ц	-	L.
NB Left	0	o c		>	>	o c		0	0	0		ס	•	- 0	,)	o	- 0	,
NB Thru	0	0	1	0	0	0		0	0	0		0	0	0	•	0	0	0	
Comb. T-R)	0				0	,			0				0	,			0	
NB Right [3]	0	0		0	0	0		0	0	0	1	52	52	2	28	0	25	5	28
Comb. L-T-R -		0				0				0				0				0	
SBleff	35	0		4	39	0	-	0	39	0	\rightarrow \rightarrow -37	2	0	ŀ	0	2	0		
Comb. L-T	;	0				0				0	,			0				0	
SB Thru	0	0	29	0	0	0	65	0	0	0	65	0	0	0	1	0	0	0	
Comb. T-R		0				0		•	;	0 (•	Ċ	0 1		C	ć	0 •	,
SB Right	24	o +	Ú	2	56	0 +	,	0	26	0 -	,	0	56	- 0	97	5	97	- 0	97
ביוים. ביוים		-				-				-				•					
EB Left	9	-	9	1	7	-	7	0	7	-	7	<i>L-</i>	Q	0		0	Ģ	0	•
Comb, L-T		0	,			0				0		,		0 1		•		0 (,
EB Thru	1017	7	209	102	1119	5	229	43	1162	5	281	-	1173	~ ~	286	0	11/3	01 0	986
Comb. T-R		0		•	(0 ((C	0 0		Ĺ	ť	⊃ •		c	ŭ	5 7	, 1
EB Right	0	0	ı	0	0	o (,	>	>	5 6	1	c c	n n	- (co o	0	3	- c	2
Comb. L-T-R -		0				0				>				>				0	
WB Left	0	0		0	0	0		0	0	0		204	204	2	112	0	204	2	112
Comb. L-T		0	•			0	1			0				0	,			0	•
WB Thru	1103		226	110	1214	•	611	36	1250	-	629	13	1263	-	929	0	1263	Ψ-	636
Comb. T-R		-	556			-	611			-	629				636	•	•	- (636
WB Right	œ	0		-	თ	0	•	0	თ	0		0	თ	0		0	ത	0 (
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	59			N-S:	65			N-S:	65			N-S:	31			S-S:	31
		E-W.	562			Е-W.	618			Ë. .≺	636			.: Х	669			E-W:	669
		SUM:	621			SUM:	684			SUM:	702			SUM:	730			SOM:	730
No. of Phases:			D				ח				ס				3				е
Volume / Capacity:	acity:		0.518				0.570				0.585			[1], [2]	0.412			[1]. [2]	0.412
Level of Service:	.;;		⋖				∢				Ą				∢				A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1406, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of voverhapping left turn.

It is intersection will be span of the proposed project.

[2] Worthbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Worthbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Worthbourd right turn has an overlapping phase with the westbound left-turn movement.

[3] Worthbound right turn has an overlapping phase with the westbound left-turn movement.

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA17 Project: File Name: Counts by: N-S St: E-W St:

Accutek

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	E	2012 \	W/ OTHER	2012 W/ OTHER PROJECTS	TS.	2012 \	W PROP	2012 W/ PROPOSED PROJECT	JUECT	2012 \	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	ŀ		Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0		0	0	0		0	0	0	,	46	46	-	46	0	46	~	46
Comb. L-T		0		•	•	0 ((C	0 0		C	c	0 0		c	c	0 0	ı
NB Thru	0	o c		0)	0		>	>	00		>	5	00		0	0	0	
NB Right [3]	0	000	ı	0	0	00		0	0	00	i	573	573	2 0	315	0	573	8 0	315
. L-1-1		0				o				•)				,	
SB Left	27	0		ო	30	0 (0	30	0 0		-29	٢	0 0	·	0	-	0 0	
Comb. L-T SB Thru	0	00	- 46	0	0	0	- 50	0	0	0	20	0	0	0		0	0	0	. ,
Comb. T-R		0				0				0				0	1			0	,
SB Right	19	0 -	ı	2	21	0 -		0	21	o	1	0	21	- 0	21	0	21	0	21
2		-																	
EB Left	21	- (21	2	23	- 0	23	0	23	- 0	23	-22	-	0 0	1	0	-	0 0	,
Comb. L-1 EB Thru	1046	9 C	523	105	1151	о С	575	39	1190	N C	595	75	1265	N C	632	0	1265	7	632
Comb. T-R	Ċ	00	•	c	c	00		c	c	0 0	•	7.0	70	۰ ۲	- 76	c	27	0 -	. 70
Comb. L-T-R -		0	ı	o	Þ	00	•	0)	0		ì	i	0	ī)	i	• о	ì
																		ľ	
WB Left Comb I -T	0	00	, ,	0	0	0 0		0	0	00	i 1	306	306	0 0	. 168	0	306	0 0	168
WB Thru	1168	· –	599	117	1285	-	629	44	1329	-	681	53	1382	-	707	0	1382	Ψ.	707
Comb. T-R	Č	← ¢	299	۳	33	~ ⊂	629	c	ć,	- c	681	c	33	- c	, 707	c	33	- c) ₀ /
Comb. L-T-R -		00	ı	o	3	0 0)	3	0		•	:	0		,	:	0	
Crit. Volumes:		N-S:	46			N-S:	20			N-S:	50			S-N	147			N-S:	147
		E-W: SUM:	620 666			SUM:	682 732			E-W: SUM:	704 754			SUM:	801 948			SUM:	801 948
No. of Phases:			⊃				D				n				က				3
Volume / Capacity:	acity:		0.555				0.610				0.628			[1]. [2]	0.565			[1]. [2]	0.565
Level of Service:	.e.		∀ I				n				۵				1				1

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55%

Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of vowdapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Wor ratio includes a 6.10 relation due to installation of ATSAC/ATCS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Fashion Square New Project Driveway (Tunnel Access) Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA18 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Fashion Square New Project Driveway (Tunnel Access) @ Riverside Drive Peak Hour: AM 2.0% Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	H	2012 \	N/ OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0		0	0	0	1	0	0	0	ı	89	ω	-	æ	0	80	-	80
Comb. L-T		0				0	,			0	,			0	,			0	
NB Thru	0	0		0	0	0	1	0	0	0		0	0	0	,	0	0	0	
Comb. T-R		0	r			0	•			0	1			0	ì			0	1
NB Right	0	0	,	0	0	0	,	0	0	0	1	21	21	-	21	0	21	-	21
Comb. L-T-R -		0				0				0				0				0	
SBLeft	0	0		0	0	0		0	0	0	-	0	0	0		0	0	0	
Comb. L-T		0	,			0				0	,			0				0	
SB Thru	0	0	•	0	0	0		0	0	0	ı	0	0	0		0	0	0	•
Comb. T-R		0	,			0	,			0	1			0				0	,
SB Right	0	0		0	o	0	,	0	0	0		0	0	0		0	0	0	
Camb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0	-	0	0	0	,	0	0	0	,	0	0	0	
Comb. L-T		0	1			0				0	,			0				0	•
EB Thru	1023	2	512	102	1126	2 (563	43	1169	0,1	584	9	1175	7	287	0	1175	2	287
Comb. T-K	c	0 0		c	c	o c		c	c	o c		Č	Ç	O +	,	c	ć	O +	
Comb T.D.		o c	•	•	•	o c	•	•	>	o c		3	5	~ c	3	•	3	- c	กั
COLIE: F-1-1		>				0				•				0				5	
WB Left	0	0		0	0	0	,	o	o	0	ı	98	98		98	0	86	-	98
Comb. L-T		0	,			0			į	0				0	,			0	,
WB Thru	1127	2 0	264	113	1240	7	620	36	1276	2 0	638	_	1277	2 0	639	0	1277	2 '	639
Comb. I-K	c	> c		c	c	o c		c	c	> c	,	c	c	> c		c	c	o c	
Comb 1-T-R		0 0		•	•	0 0	į	•	•	0	ı	•	•	0 0	į	•	•	o c	i
: : : : : : : : :		•				1				l				,)	
Crit. Volumes:		N-S:	0			N-S:	0			N-8:	0			N-S:	8			N-S:	
		E-W.	564			Ē-W:	620			Ē-₩.	638			E-V:	673			E-W.	673
		SUM:	564			SUM:	620			SUM:	638			SUM:	681			SUM:	.89
No. of Phases:			ס				_	:			D				ო				3
							1				62.0			5	010			100	
volume / Capacity:	acity:		0.470				715.0				0.532			[7], [2]	0.378			[1], [2]	0.378
Level of Service:	 		⋖				¥				⋖				4				∢

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual tum lanes, 55% of volume is assigned to heavier lane.
For one excl. and one opt, tum lane, 70% of volume is assigned to exclusive lane.
Right tums on red from excl. lanes = 50% of overlapping left tum.
[1] Intersection will be signalized as part of the proposed project.
[2] we ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.
Note: Pass-by reductions not applied to this intersection per LADOT standards.

Fashion Square New Project Driveway (Tunnel Access) Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St:

Project: File Name:

CMA18

Accutek Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square New Project Driveway (Tunnel Access) @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

08/04/2008 2007 2012

Date: Date of Count; Projection Year;

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	2012 W/ AMBIENT	ENT GROWTH	ďΉ	2012	W OTHE	2012 W/ OTHER PROJECTS	CTS	2012 V	2012 W/ PROPOSED PROJECT	SED PRO	DJECT	2012 \	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0		0	0	0	•	0	0	0		77	77	-	77	0	77	-	77
Comb. L-T		0	•			0				0				0				0	
NB Thru	0	0	,	0	0	0	,	0	0	0	•	0	0	0	•	0	0	0	,
Comb. T-R		0	•			0				0	ı			0				0	•
NB Right	0	0		0	0	0		0	0	0		206	206	.	206	0	206	-	206
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0		0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T		0	•			0				0				0				0	•
SB Thru	0	0	•	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. T-R		0	,			0	,			0				0				0	
SB Right	0	0	•	0	0	0		0	0	0	į	0	0	0	ı	0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0	ı	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T		0	ı			0				0	,			0				0	
EB Thru	1067	2	534	107	1174	2	287	39	1213	2	909	39	1252	2	626	0	1252	7	626
Comb. T-R		0 0	1	C	•	0 0		•	ć	0 (,	;	;	o ,	,	•	;	ο.	
EB Kigni	0	Э (1	0	0	> (1	0	>	5 (ı	114	114	- (114	0	114	 (114
Comb. L-1-K		0				o				5				0				0	
WB Left	0	0 (0	0	0 (0	0	0 (,	364	364	- (364	0	364	- 1	364
Comb. L-1	107	5 (,	7	1004	۰ د		*	270	o (,	r	7	o ('	c	,	0 (,
Comb T-R		v C	0 1	<u> </u>	coci	v C	3	‡	5	V C	20 1	3	7001	v C	0/0	>	7001	V C	0/0
WB Right	c	o c	ı	c	c	· c	,	С	c	c	. 1	C	c	o c		c	c	o c	. ,
Comb. L-T-R -		0		ı	•	0		ı		0		•	r	0		1	•	0	
Crit. Volumes:		N-S:	0			N-S:	0			N-S:	0			N-S:	77			N-S:	77
			593				653			E-W:	675			E-W:	990			Ë-W.	066
		SCINI SCINI	283			 NO	603			SON C	6/9			SOM:	106/			SOM:	106
No. of Phases:			D				ם								m				6
Volume / Capacity:	ıcity:		0.494				0.544				0.562			[1]. [2]	0.649			(1), (2)	0.649
Level of Service	, , ,		∢				⋖				∢				В				æ

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.
Folly turns on red from excl. lanes = 50% of overlapping left turn.
[1] Intersection will be signalized as part of the proposed project. [2] vc ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.
Note: Pass-by reductions not applied to this intersection per LADOT standards. Assumptions:

APPENDIX D-2

CMA DATA WORKSHEETS WEEKEND MID-DAY PEAK HOUR

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 City Traffic Counters N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/05/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	2012 W/ AMBIE!	ENT GROWTH	 ‡	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS	2012	W PROP	2012 W/ PROPOSED PROJECT	SJECT	2012 \	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	173	← ¢	173	17	190	 c	190	0	190	- 0	190	20	210	~ 0	210	0	210	 c	210
NB Thru	668	0 10 0	334	29	735	0 64 6	367	23	758	0 10 0	379	=	769	0 10 0	384	0	492	0 00 0	384
Comb. L-T-R -	. 229	0 0	229	23	252	0-0	252	0	262	0 - 0	262	0	262	0 - 0	262	0	262	0 - 0	- 262
SB Left	178	- 0	178	18	196	- 0	196	5	201	- 0	201	27	228	← c	228	0	228	- 0	228
SB Thru	662) -	391	99	728	o 	430	56	754) -	444	39	793	o	463	0	793	o	463
SB Right Comb. L-T-R -	120	-00	3	12	132	-00	}	-	133	-00	,	0	133	-00	}	0	133	-00	2
EB Left Comb 1-T	33	c	39	4	43	F C	43	7	45	- c	45	0	45	- c	45	0	45	- 0	45
EB Thru	519) + +	345	52	571) -	379	52	623) - -	405	45	899	· · ·	436	0	999) - - (436
EB Right Comb. L-T-R -	170	- 0 0	,	17	187	-00	, ,	0	187	-00	,	18	205	-00	,	0	205	-00	2
WB Left	223	c	223	22	245	F- C	245	13	258	- 0	258	0	258	~ c	258	0	258	- 0	258
WB Thru	358	2 17 0	179	36	394	0 70 0	197	43	437	0 N C	218	52	488	0 10 0	244	0	488	0 00 0	244
WB Right Comb. L-T-R -	÷ ±	0 - 0		E	122	0 - 0	122	ĸ	127	0 - 0	127	32	159	0 0	159	0	159	0 - 0	159
Crit. Volumes:		N-S: E-W: SUM:	564 568 1132			N-S: E-W: SUM:	620 624 1245			N-S: E-W: SUM:	634 663 1297			N-S: E-W: SUM:	673 695 1368			N-S: E-W: SUM:	673 695 1368
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	[1]	0.684 B			[1]	0.760 C			[1]	0.795 C			Ξ	0.842 D			[2]	0.812 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 55% of volume is assigned to exclusive lane. 70% of volume is assigned to exclusive lane. 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA8 City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Fashion Square Lane Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	W AMBIE	2012 W/ AMBIENT GROWTH	TH	2012 V	W OTHER	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	VIION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	5	-	c)	-	9	-	9	0	9	-	9	0	9	-	9	0	9	-	9
Comb. L-T	;	0	,	;	1	0	,	;	i	0			;	0		•		0	•
NB Thru Comb. T-R	688	- -	432 432	69	757		475 475	ဗ္ဗ	790		491	_	797		203 203	0	797		809 809
NB Right	175	. 0	! !	18	193	. 0	,	0	193	0		29	222	0	}	0	222	- 0	}
Comb. L-T-R -		0				0				0				0				0	
SB Left	437	F	437	44	481	-	481	0	481	-	481	57	538	-	538	0	538	-	538
Comb. L-T		0	,			0				0				0				0	ı
SB Thru	625	τ-	315	63	688	-	346	39	727		365	0	727	-	365	0	727	-	365
Comb. T-R	•	 ⊂	315	c	*	- c	346	c	•	~ c	365	c	•	~ ¢	365	c	•	- c	365
Comb. L-T-R -	r	0	,	0	t	0	,	•	r	0	,	o	t	00		•	t	0	•
EB Left	5	-	S	Ψ-	9	-	9	0	9	-	9	0	9	-	9	0	9	-	9
Comb. L-T		0	•			0	•	,	,	0				0	ı			0	ı
EB Thru	2	۰ م	,	0	7	۰ ۰	,	0	2	0 1	,	0	2	o ,	,	0	7	o ·	,
Comb. 1-K	_	- c	و	c	7	c	,	c	~	- c	_ '	c	*	- c	,	c	•	c	,
Comb 1-T-R-	r	o c		•	t	o c		•	•	o c	•	•	•	o c	ı	•	r	o c	
)				,)				•				•	
WB Left	101	+	101	10	111	1	111	0	111	-	111	24	135	-	135	0	135	-	135
Comb. L-T	•	0 1		•	•	0 (•	٠	0 (•	•	0 1	į	1		0	,
WB Thru	0	O +	1 23	0	0	O +	146	0	0	۰ -	,	0	0	00		0	0	0 0	
WB Right	133	- 0	3	13	146	- 0	2	0	146	- 0	<u>.</u>	17	163		163	0	163	·	163
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	869			N-S:	955			N-S:	972			N-S:	1047			N-S:	1047
		E-W:	138			E-W:	152			E-W:	152			E-W:	142			Ē-Қ .∵	142
		SUM:	1007			SUM:	1107			SUM:	1124			SUM:	1189			SUM:	1189
No. of Phases:			က				m				ო				3				3
Volume / Canacity:	city:	Ξ	0.636			Ξ	707.0			Ξ	0.719			[2]	0 764			161	0.734
Volume / Capaci	diy.	Ξ	2				<u>.</u>			Ξ	3				5			4	t
רפעפו חו ספו עור	Ď,		۵				ر				د				ر				 ر

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

If I w/c ratio includes a 0.07 readuction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Westfield Fashion Square /1-05-3606-1 Woodman Avenue Riverside Drive CMA12 Project: File Name: N-S St: E-W St:

City Traffic Counters

Counts by:

Date: Date of Count: Projection Year;

08/04/2008 2007 2012

Woodman Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	2007 E	2007 EXIST. TRAFFIC	FFIC	2012 \	V/ AMBIE	2012 W/ AMBIENT GROWTH	표	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	SUECT	2012	2012 W/ MITIGATION	VOITA	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	484	8 6	266	48	532	87.0	293	-	533	20	293	102	635	20	349	0	635	2.0	349
NB Thru	674	0 10 0	337	29	741	0 10 0	371	32	773	0 00 0	387	0	773	0 10 0	387	0	773	0 00 0	387
Comb. 1-K NB Right [2] Comb. L-T-R -	509	0-0	- 209	21	230	0 - 0	230	17	247	0-0	247	0	247	0 - 0	247	0	247	0-0	247
SB Left	66	- 0	66	10	109	F- C	109	S	114	- 0	114	0	114	- c	114	0	114	0	114
SB Thru	842	0 10 0	421	84	926	o 70 C	463	33	929) N C	480	5	974	0 70 0	487	0	974	· /3 C	405
SB Right [2] Comb. L-T-R -	. 200	0-0	200	20	220	0 - 0	220	ო	223	0 - 0	223	8	241	0 - 0	241	0	241	-00	2
EB Left	197	- 0	197	20	217	← C	217	4	221	- c	221	11	232	← c	232	0	232	- 0	232
EB Thru	753	0 10 0	377	75	828	0 10 0	414	64	892	0 00 0	446	44	936	0 00 0	468	0	936	0 10 0	468
Comb. 1-K EB Right Comb. L-T-R -	389	0 0	389	39	428	o – o	428	-	429	0 0	429	174	603	0 - 0	. 603	0	603	0 - 0	. 603
WB Left	234	- c	234	23	257	 C	257	28	285	- c	285	0	285	- c	285	0	285	- 0	285
WB Thru	591	2 7 0	296	59	650	2 22 0	325	56	706	0 N C	353	29	773	0 N C	387	0	773	o ~ c	387
WB Right Comb. L-T-R -	134	0 - 0	134	55	147	o 0	147	ო	150	0 - 0	150	0	150	0 - 0	150	0	150	0 0	150
Crit. Volumes:		N-S: E-W: SUM:	687 611 1298			N-S: E-W: SUM:	756 672 1427			N-S: E-W: SUM:	773 732 1505			N-S: E-W: SUM:	837 754 1590			N-S: E-W: SUM:	755 754 1508
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	acity:	[1]	0.874 D			Ξ	0.968 E			[5]	1.024 F			Ξ	1.086 F			[6]	0.997 E
							***************************************	-											1000

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

Fight turns on red from excl. lanes = 50% of overlapping left turn.

Fight vortable as a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[3] Wor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 N-S St: E-W St: Project: File Name: Counts by:

City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007 EXIST. TRAFFIC	IST. TRA	FFIC	2012 V	V/ AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	и отнея	2012 W/ OTHER PROJECTS	TS	2012 V	V/ PROPC	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
	Ž	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Movement Volume Lanes		Volume	Volume Volume		Lanes	Volume	Volume	Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	416		416	42	458	•	458	7	460	-	460	0	460	(460	0	460	← (460
Comb. L-T NB Thru	991	OM	330	66	1090	O M	363	48	1138	0 %	379	44	1182	၁၈	394	. 0	1182	၁ ო (394
Comb. T-R NB Right	0	00	1 1	0	0	000	1 1	0	0	000	1 1	0	0	000		0	0	000	1 1
Comb. L-T-R -		0				0				Э				>				>	
SB Left	0	0 0		o	0	0 0		0	0	0 0		0	O	00		0	o	0 0	
SB Thru	1062	O 4 (266	106	1168	24 (292	57	1225	040	306	82	1307	4 0	327	0	1307) 4 C	327
Comb. T-R SB Right	493	0 -	493	49	542	- c	542	ß	547	→ C	547	41	588	o (588	0	588	→	588
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0 0	. ,	0	0	0 0		o	0	0 0		0	0	0 0		0	0	0 0	
EB Thru	0	00		0	0	00	1	0	0	000	,	0	0	000	•	0	0	00	ı
Comb. T-R EB Right	0	00		0	0	0 0		0	0	00		0	0	0		0	0	0	
Comb. L-T-R		0				0				o				0				0	
WB Left	318	⊢ c	175	32	350	- c	192	32	382	- c	210	0	382	- c	210	0	382	- 0	210
WB Thru	ĸ	00	270	0	ь	0	297	0	က	00	312	0	ო	0	334	0	ო	0	334
Comb. T-R WB Right Comb. L-T-R	275	0	151	28	303	0	166	7	305	0	167	48	353	D	194	0	353	D	194
Crit. Volumes:		N-S: E-W: SUM:	909 270 1179			N-S: E-W: SUM:	1000 297 1297			N-S: E-W: SUM:	1007 312 1319			N-S: E-W: SUM:	1048 334 1382			N-S: E-W: SUM:	1048 334 1382
No. of Phases:			3				က				60				က				ဗ
Volume / Capacity:	acity:	Ξ	0.757			Ξ	0.840			E	0.856 D			Ξ	0.900			[2]	0.870 D
רבאבו חו ספואו																			

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] V/C ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Assumptions:

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 City Traffic Counters N-S St: E-W St: Project: File Name: Counts by:

Date: Date of Count; Projection Year:

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/04/2008 2007 2012

	7000	CIEDAGT TOYOU	VEEL	2040	DO42 IAII AREDIC	UTAICOS TIAS		2042	W OTUE	2012 W. OTHER PROJECTS	 	2042	,acaa ,,,	2013 W/ PBOBOSED BBO IECT	1011	2042	MOITA SITING WAY	NOIE		
	7007	EAI31. IN) [.	7 .		יייי פאר פאר פאר פאר פאר פאר פאר פאר פאר פאר		7 .			· ·	7 .		, j	į .	· ·		, `		
		No. of	Lane	Added	Total	No. of	Lane	Added	lotal	No. of	Lane	Added	lotal	No. of	Lane	Added	lotal	No. of	Lane	_ :
Movement	volume	Lanes	volume	Volume Volume	хонише	Lanes	Aoinme	volume	Volume	Lanes	volume		Volume	Lanes	Volume	Volume	Aciditie	ranes	Aoign	
NB Left	0	0	,	0	0	0	,	0	0	0		0	0	0		0	0	0		
Comb. L-T		0				0	,			0				0	,			0		
NB Thru	951	ო -	262	92	1046	თ -	288	48	1094	ო .	306	16	1110	ო .	310	0	1110	ო .	.,,	310
Comb. T-R		_	262			τ-	288	1		-	306	•		, ,	310			_	.,	310
NB Right	96	0		9	106	0		56	132	0	,	0	132	0	1	0	132	0		
Comb. L-T-R -		0				0				0				0				0		
SB Left	333	-	333	33	366	-	366	9	372	-	372	45	417	-	417	0	417	-		417
Comb. L-T		0				0	1			0				0	,			0		
SB Thru	1027	7	514	103	1130	7	265	82	1212	7	909	37	1249	2	624	0	1249	7	_	624
Comb. T-R		0	1			0				0	,			0				0	,	
SB Right	0	0	,	0	0	0		0	0	0		0	0	0	,	0	0	0		
Comb. L-T-R -		0				0				0				0				0		
EB Left	432	-	238	43	475	-	261		476	-	262	28	504	-	277	0	504	-		277
Comb. L-T		0				0	,			0	,			0	,			0	,	
EB Thru	თ	0	346	-	9	0	381	0	9	0	382	0	9	0	395	0	9	0	• •	395
Comb. T-R	,	ο,	,	ć		ο,	,	c	i	۰ ,	,	•	i	0 1	,	•	i	0 ,	•	
EB Right	317		1/4	35	349		192	7	351	- ,	193	0	351	, ,	193)	351		•	193
Comb. L-1-K -		-				-				-				-				_		
WB Left	0	0		0	0	0		0	0	0		0	0	0	,	0	0	0	,	
Comb. L-T		0	1			0	•			0				0	1			0		
WB Thru	0	0		0	0	0		0	0	0		0	0	0 1		0	0	0	1	
Comb. T-R		0	1			0	•			0				0	,			0		
WB Right	0	0		0	0	0 (0	0	0 (1	0	0	0 (0	0	0 (
Comb. L-T-R -		0				0				0				0				0		
Crit. Volumes:		N-S:	595			N-S:	654			N-S:	629			N-S:	728			N-S:		728
		E-∀.	346			E-W.	381			Ē-W:	382			Е-V:	395			E-W.	.,	395
		SUM:	941			SUM:	1035			SUM	1061			SUM:	1122			SUM:	÷	122
No. of Phases:			က				က				ю				3					3
			0			5				5				5	0			5		
Volume / Capacity:	acity:	Ξ	0.590			7	0.626			7	0.644			[7]	0.688			7		0.688
Level of Service:	je:		٧				В				m				В				m	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes.

55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane,

55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA17 N-S St: E-W St: Project: File Name:

City Traffic Counters Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Peak Hour: Annual Growth:

08/04/2008 2007 2012 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBI	2012 W/ AMBIENT GROWTH	MTH	2012	N/ OTHE	2012 W/ OTHER PROJECTS	STS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION		
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane		Total	No. of	Lane	Added	Total	No. of	Lane	
Movement	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	
NB Left	0	0	1	0	0	0	,	0	0	0	,	28	58	-	28	0	28	-		58
Comb. L-T		0				0	,			0				0	,			0		
NB Thru	0	0	•	0	0	0	,	0	0	0	,	0	0	0		0	0	0		
Comb. T-R		0	,			0	•			0	,			0	1			0		
NB Right [3]	0	0	•	0	0	0		0	0	0		520	520	2	286	0	520	2	7	286
Comb. L-T-R -		0				0				0				0				0		
SB Left	က	0	,	0	က	0		0	3	0	,	6-	0	0	,	0	0	0		
Comb. L-T		0	,			0				0	,			0	,			0		
SB Thru	0	0	20	0	0	0	22	0	0	0	22	0	0	0		0	0	0	,	
Comb. T-R	!	0				0			!	0			!	0			:	0		
SB Right	17	ο.		7	19	ο.		0	5	э.	,	0	3	(19	0	5	- (<u>.</u>
Comb. L-T-R -		-								-				0				0		
EB Left	24	-	24	2	26	-	26	0	26	-	56	-26	0	0	1	0	0	0		
Comb. L-T		0	•			0				0	,			0				0	1	
EB Thru	1092	21	546	109	1201	7	601	29	1268	2 '	634	95	1360	2 (980	0	1360	7	9	089
Comb. I-K	c	0 0		c	c	o c		c	c	o c	, ,	124	124	⊃ ←	124	C	124	⊃ -	,	124
Comb. L-T-R -		0		•)	0)	0		!	!	0	!	1	!	0	•	·
WB Left	0	00		0	0	00		0	0	0 0	1	455	455	0 0	250	0	455	0 0	2	250
Comb. L-1	27.0	> +	, 1	Š	**	o +	-	ě	1175	> *	-	F	7.00	o +	,	c	4.00.0	o 7	,	ç
Comb T-R	2		512	2	<u>+</u>		263	ō	2		593		7671		632 632	0	7071		o cc	552
WB Right	9	- 0	,	•	Ξ	. 0	}	0	7	0	}	0	Ţ	. 0	1	0	Ξ	. 0	,	
Comb. L-T-R -		0				0				0				0				0		
Crit. Volumes:		N-S:	20			N-S:	22			Š.	22			N-S:	76			N-S:		76
		E-W	546			E-W:	601			Ē-₩:	634			E-W:	930			E-W:	6	930
		SUM:	999			SUM:	623			SUM:	656			SUM:	1007			SUM:	무	
No. of Phases:))								n				3		<u>.</u>			m
																				Ī
Volume / Capacity:	acity:		0.472				0.519				0.547			[1], [2]	909.0			[1]. [2]		909.0
Level of Service:	ě.		4				A				А				В				В	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Nor raiso includes a 0.10 reduction due to installation of ATSACATOS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Fashion Square New Project Driveway Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA18 N-S St: E-W St: Project: File Name:

Accutek

Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square New Project Driveway @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Peak Hour: Annual Growth:

08/04/2008 2007 2012

Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	MEFIC	2012 V	2012 W/ AMBIENT	ENT GROWTH	MTH	2012 \	W OTHE	2012 W/ OTHER PROJECTS	CTS	2012 \	N/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total		Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB I off	c	c		C	C	c		o	0	o	,	96	96	-	96	0	96	-	96
Comb. L-T	•	0	•	•	•	0		•	,	0		:		0	,			0	,
NB Thru	0	0	ı	0	0	0		0	0	0	1	0	0	0	,	0	0	0	ŧ
Comb. T-R		0	ì			0				0				0				0	
NB Right	0	0	•	0	0	0		0	0	0		258	258	-	258	0	258	-	258
Comb, L-T-R -		0				0				0				0				0	
SBLeft	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T		0	•			0				0				0	,			0	
SB Thru	0	0	ı	0	0	0		0	0	0	,	0	0	0	•	0	0	0	1
Comb. T-R		0	•			0	•			0				0	,			0	
SB Right	0	0		0	0	0	1	0	0	0		0	0	0		0	0	0 (1
Comb. L-T-R	,	0				0				0				0				0	
EB Left	0	0	,	0	0	0	,	0	0	0		0	0	0		0	0	0	
Comb. L-T		0	,			0	1			0				0				0	,
EB Thru	1116	2	558	112	1228	2	614	29	1295	7	647	47	1342	5	671	0	1342	2	671
Comb. T-R		0	ı			0	,			0				0	,			0	,
EB Right	0	0	ı	0	0	0		0	0	0		303	303	-	303	0	303	Ψ-	303
Comb. L-T-R -	,	0				0				0				0				0	
WB Left	0	0		0	0	0		0	0	0		452	452	-	452	0	452	-	452
Comb. L-T		0	1			0	,			0				0				0	
WB Thru	1030	2	515	103	1133	2	267	61	1194	2	297	4	1198	2	299	0	1198	7	299
Comb. T-R		0	,			0				0				0				0	1
WB Right	0	0		0	0	0	,	0	0	0		0	0	0	1	0	0	0	•
Comb. L-T-R	i	0				0				0				0				0	
Crit. Volumes:	ļ	N-S:	0			N-S:	0			N-S:	0			N-S:	96			N-S:	96
		Ē-₩:	558			E-W:	614			E-W:	647			E-W.:	1123			E-W.	1123
		SUM:	558			SUM:	614			SUM:	647			SUM:	1219			SUM:	1219
No. of Phases:	1,5		Э))) D				က				က
Volume / Capacity:	acity:		0.465	William			0.512				0.539			141. [2]	0.755			[11]. [2]	0.755
40000																			
Level of Service:	 Ge:		< 1				A				¥				ر				د

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lane act. I anne act. I anne act. I anne act. I anne act. I anne act. I anne act. I anne I appropriate a part of the proposed project. [2] we ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. Note: Pass-by reductions not applied to this intersection per LADOT standards. Assumptions:

APPENDIX E
TRAFFIC SIGNAL WARRANT WORKSHEETS

LLG Ref. 1-053606-1 Westfield Fashion Square Expansion Project

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Appendix Table E-1 WEEKDAY HOURLY TRAFFIC VOLUMES [1] Matilija Avenue-Proposed Fashion Square Project Driveway/Riverside Drive Intersection

	-					- 1					
	eway	Minor Street Approach	Total [5]	554	969	644	574	590	564	610	542
Street	are Project Driv	Minor Street Approach	Total [4]	1012	1183	1213	1110	1142	1071	1146	1080
Minor Street	Proposed Fashion Square Project Driveway		NB Right [3]	457	487	569	536	551	506	536	538
	Propo		NB Left [3]	36	39	46	43	44	40	. 43	43
		Major Street Approach	Total [2]	1660	1809	1775	1843	2090	2106	2192	2037
		WB	Approach	1346	1546	1475	1396	1668	1634	1795	1659
			WB Right	12	6	14	11	15	17	21	4
Iajor Street	rerside Drive	WB	Through	816	880	863	854	1107	1093	1207	1156
Maj	River		WB Left	518	657	598	531	546	524	567	499
		EB	Αp	832	920	868	978	968	966	964	877
			Through EB Right	133	169	86	106	104	110	110	104
		EB	Through	669	751	800	872	864	988	854	773
·	Begin	D	Time	11:00 AM	12:00 PM	1:00 PM	2:00 PM	3:00 PM	4:00 PM	5:00 PM	6:00 PM

[1] Traffic count data collected by City Traffic Counters on Thursday, January 17, 2007.

[2] Based on the California MUTCD document, the major street volume is the sum of both approaches for the major street (Riverside Drive) excluding

the higher volume of the major street left-turn volume (i.e., the westbound left-turn volume).

[3] Reflects a portion of the vehicles currently utilizing the existing Fashion Square Project Driveways and Riverside Shopping Center Driveway

on Riverside Drive.

[4] Based on the California MUTCD document, the minor street volume is the higher volume of the major street left-turn volume (i.e., the westbound left-turn volume) plus the higher volume minor street approach.

[5] Based on the California MUTCD document, the minor street volume is the higher volume of the major street left-turn volume (i.e., the westbound

left-turn volume) plus the higher volume minor street approach. In addition, per the LADOT Manual of Policies and Procedures document, the minor street approach volume does not include right-turn volumes (i.e., the northbound right-turn volumes).

Proposed Fashion Square Westerly Project Driveway/Riverside Drive Intersection (Tunnel Access) WEEKDAY HOURLY TRAFFIC VOLUMES [1] Appendix Table E-2

					Major Street	eet			Minor Street	
Begin					Riverside Drive	rive		Proposed Fas	Proposed Fashion Square Project Driveway	ject Driveway
	EB		EB		WB	WB	Major Street Approach			Minor Street Approach
Time	Through	Through EB Right	Approach	WB Left	Through	Approach	Total [2]	NB Left [3]	NB Right [3]	Total [4]
11:00 AM	683	114	797	364	786	1150	1583	51	150	415
12:00 PM	732	114	846	364	849	1213	1695	56	164	420
1:00 PM	789	114	903	364	827	1191	1730	71	204	435
2:00 PM	853	114	296	364	813	1177	1780	65	188	429
3:00 PM	852	114	996	364	1086	1450	2052	67	196	431
4:00 PM	698	114	983	364	1901	1425	2044	59	174	423
5:00 PM	850	114	964	364	1176	1540	2140	65	188	429
M4 00:9	757	114	871	364	1117	1481	1988	65	189	429

- [1] Traffic count data collected by City Traffic Counters on Thursday, January 17, 2007.
- [2] Based on the California MUTCD document, the major street volume is the sum of both approaches for the major street (Riverside Drive) excluding the higher volume of the major street left-turn volume (i.e., the westbound left-turn volume).
- westbound left-turn volume) plus the higher volume minor street approach. In addition, per the LADOT Manual of Policies and Procedures [3] Reflects a portion of the vehicles currently utilizing the existing Fashion Square Project Driveways.
 [4] Based on the California MUTCD document, the minor street volume is the higher volume of the major street left-turn volume (i.e., the document, the minor street approach volume does not include right-tum volumes (i.e., the northbound right-turn volumes).

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Appendix Table E-3 SUMMARY OF CONTROL DELAY AND LEVELS OF SERVICE

				[1] EXIST WITH PR	ING
NO.	INTERSECTION	HOUR	MOVEMENT	DELAY	LOS
1	Matilija Avenue-	11 AM	NB Approach	122.2	F
	Easterly Project Driveway/		NB Left	1390.0	F
	Riverside Drive		NB Right	22.4	С
					_
		12 PM	NB Approach	446.2	F
			NB Left	5671.0	F
			NB Right	27.8	D
		1 PM	ND Ammragah	358.7	F
1		1 1 1 1 1 1	NB Approach NB Left	4183.0	r F
			NB Right	4163.0	F E
			ND Right	49.3	E
		2 PM	NB Approach	294.0	F
			NB Left	3338.0	F
			NB Right	49.8	E
			C		
		3 PM	NB Approach	413.4	F
			NB Left	4915.0	F
		ł	NB Right	53.9	F
1		4 PM	NB Approach	313.0	F
1	·		NB Left	3731.0	F
			NB Right	42.8	E
		5 PM	NID Ammoodh	100 5	r
		J PIVI	NB Approach NB Left	498.5 6126.0	F F
1				1	r E
			NB Right	47.1	£
		6 PM	NB Approach	220.8	F
		0 1 1.1	NB Left	2513.0	F
			NB Right	37.6	Ë
			1,12,10,6,11	37.0	2

^[1] Control delay reported in seconds based on HCM method of analysis.

Appendix Table E-3 (Continued) SUMMARY OF CONTROL DELAY AND LEVELS OF SERVICE

				[1] EXIST WITH PR	ING
NO.	INTERSECTION	HOUR	MOVEMENT	DELAY	LOS
1101			1120 / 22/22/12		
2	Westerly Project Driveway/	11 AM	NB Approach	104.4	F
	Riverside Drive		NB Left	376.1	F
			NB Right	12.1	В
		12 AM	NB Approach	148.5	F
			NB Left	546.6	F
			NB Right	12.6	В
		1 PM	ND Ammagah	233.1	F
		1 1 11/1	NB Approach NB Left	863.1	г F
l			NB Right	13.8	В
			11D Kight	15.0	D
		2 PM	NB Approach	254.5	F
			NB Left	949.7	F
			NB Right	14.1	В
		3 PM	NB Approach	339.1	F
	·		NB Left	1289.0	F
			NB Right	14.3	В
		4 PM	ND 4 1	2140	Τ.
		4 PM	NB Approach NB Left	314.8 1202.0	F F
			NB Left NB Right	1202.0	r B
			No Kight	13.9	ע
		5 PM	NB Approach	373.2	F
		511.1	NB Left	1412.0	F
			NB Right	14.1	В
			Ŭ		
		6 PM	NB Approach	252.9	F
			NB Left	949.7	F
,			NB Right	13.2	В
<u> </u>					

^[1] Control delay reported in seconds based on HCM method of analysis.

File Name: matriver Site Code : 00000000 Start Date : 1/17/2007 Page No : 1

Groups Printed-Unshifted

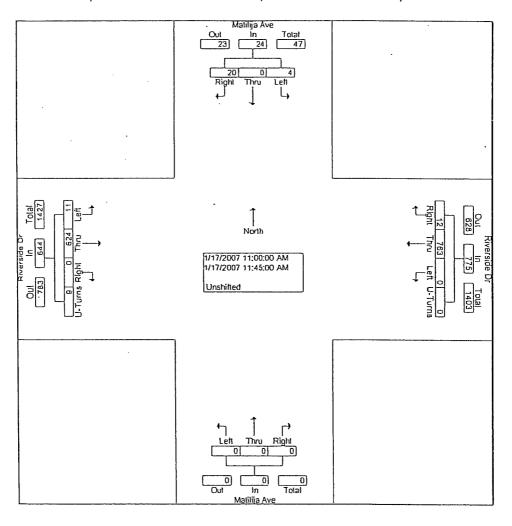
_			- 4*1** - A -			Rivers		Printed-	Unshifte	o Vatilija Av			D:	:J- D-		
			atilija Avo			West				ianija Avi Iorihboun			Rivers Eastb			
-		T	1					U-						1	υ-	
	Start Time	Left	Thru	Right	Left	Thru	Right	Turns	Left	Thru	Right	Left	Thru	Right	Turns	Int. Total
\vdash	Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
	11:00 AM	2	0	10	0	197	3	0	0	0	0	3	156	0	0	371
	11:15 AM	1	0	4	0	185	3	0	0	0	0	4	160	0	2	359
	11:30 AM	0	0	1	0	185	3	0 أ	0	0	0	2	159	0	4	354
	11:45 AM	1	0	5	0	196	3	0	0	0	0	2	149	0	3	359
	Total	4	0	20	0	763	12	0	0	0	0	11	624	0	9	1443
			_	- 1	_		_			_				_		
	12:00 PM *	0	. 0	2	0	209	2	0	0	0	0	4	175	0	2	394
	12:15 PM	0	0	5	0	202	3	0	0	0	0	0	176	0	0	386
	12:30 PM	2	0	6	0 0	196	2 2	0	0	0 0	0 0	5 3	148 177	0	2	361
_	12:45 PM Total	3	0	19	0	220 827	9	0	0	0	0	12	676	0	5	410 1551
	TOTAL	J	U	19	U	021	9		U	U	υĮ	12	070	U	3	1331
	01:00 PM	1	0	5	0	218	3	0	0	0	01	3	179	0	. 3	412
	01:15 PM	1	0	2	0	209	4	1	0	0	0	0	188	Ō	3	408
	01:30 PM	2	0	3	0	171	4	0	0	0	0	4	178	0	4	366
_	01:45 PM	1	0	4	0	211	3	0	0	0	0	4	180	0	4	407
	Total	5	0	14	0	809	14	1	0	0	0	11	725	0	14	1593
	02:00 PM	0	0	o l	0	201	5	0	0	0	0	1	205	0	1	413
	02:00 PM 02:15 PM	0	0	2	0	201	2	0	0	0	0	2	186	0	1	395
	02:30 PM	0	0	5	0	203	2	0	0	0	ő	4	195	0	3	412
	02:45 PM	1	0	2	0	195	2	Ö	Ö	0	ŏ	3	211	0	2	416
	Total	1	0	9	0	801	11	0		0	0	10	797	0	7	1636
								_		_	- 1					
	03:00 PM	0	0	8	0	222	4	0	0	0	0	8	218	0	2	462
	03:15 PM	2	0	12	0	270 263	7 1	0	0	0	0	2	193 207	0 0	4	490 480
	03:30 PM 03:45 PM	2 1	0	3 6	0	299	3	0	0	0	0	2 3	207 171	0	2	484
-	Total	5	0	29	- 0	1054	15			0	0	15	789	0	9	
		•	_	,	_			_	, -	_	- 1			_		•
	04:00 PM	0	0	5	0	249	4	1	0	0	0	2	194	0	1	456
	04:15 PM	2	0	5	0	275	4	0	0	0	0	6	204	0	1	497
	04:30 PM	0	0	4	0	255	6	0	0	0	0	3	207	0	4	479
_	04:45 PM	2	0	4	0	258	3	2	0	0	0	0	206	0	2	477
	Total	4	0	18	0	1037	17	3	0	0	0	11	811	0	8	1909
	05:00 PM	0	0	7	0	286	7	0	1 0	0	0]	7	191	0	3	501
	05:15 PM	1	0	4	0	273	6	ő	Ö	Ö	Ö	7	184	ő	5	480
	05:30 PM	0	Ö	4	Ō	313	4	Ō	0	0	0	5	210	0	2	538
	05:45 PM	1	0	4	0	282	4	0	0	0	0	1	194	0	2	488
_	Total	2	0	19	0	1154	21	0	0	0	0	20	779	0	12	2007
									,							1
	06:00 PM	0	0	3	0	288	1	0	I	0	0	6	174	0	1	1
	06:15 PM	0	0	4	0	247	2	0		0	0	3	187	0	3	446
	06:30 PM	0	0		0	282	0	0	1	0	0	3	171	0	3	
_	06:45 PM	0	0		0	286	1	0			0	0	166	0	<u>1</u> 8	
	Total	0	0	11	0	1103	4	0	l 0	0	0	12	698	0	8	1030
	Grand Total	24	0	139	0	7548	103	4	1 0	0	0	102	5899	0	72	13891
	Appreh %	14.7	0.0		0.0		1.3	0.1			0.0	102	97.1	0.0	1.2	
	Total %	0.2	0.0		0.0		0.7	0.0			0.0		42.5	0.0	0.5	
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File Name: matriver Site Code: 00000000

Start Date : 1/17/2007

Page No : 2

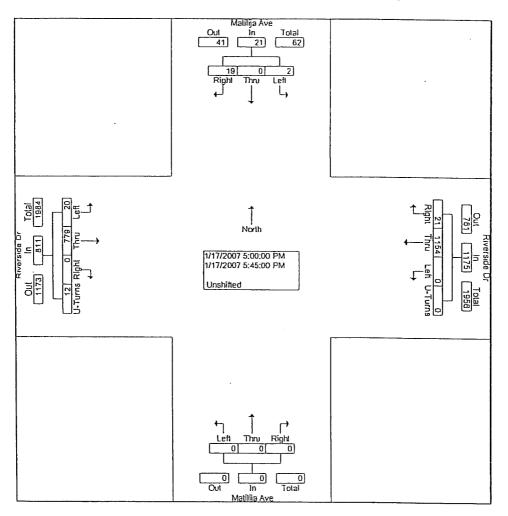
			lija Ave hbound				iverside Vestboi					ija Ave nbound				iverside aslbou			
Start Time	Left	Thru	Righ t	App. Total	Left	Thru	Righ I	U- Turn s	App. Total	Left	Thru	Righ 1	App. Total	Left	Thru	Righ t	U- Turn s	App. Total	Int. Total
Peak Hour Fro	m 11:C	MA O	lo 11:45	AM - Pe	ak 1 of	11											· · · · · · · · · · · · · · · · · · ·		
Intersection	11:00	AM																1	
Volume	4	0	20	24	0	763	12	0	775	0	0	0	0	11	624	0	9	644	1443
Percent	16.7	0.O	83.3		0.0	98.5	1.5	0.0		0.0	0.0	0.0		1.7	96.9	0.0	1.4		
11:00 Volume	2	0	10	12	0	197	3	0	200	0	0	0	0	3	156	0	0	159	371
Peak																			0.972
Factor High Int.	11:00	AM			11:00	AM				10:45:	MA 00			11:15	AM				
Volume	2	О	10	12	0	197	3	0	200	0	0	0	0	4	160	0	2	166	
Peak Factor				0.500					0.969									0.970	



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Site Code : 00000000 Start Date : 1/17/2007 Page No : 3

			ija Ave nbound				iversid: Vestbo					ija Ave ibound				iverside astbou			
Start Time	Left	Thru	Righ I	App. Total	Left	Thru	Righ I	U- Turn s	App. Total	Left	Thru	Righ t	App. Total	Left	Thru	Righ I	U- Turn s	App. Total	Int. Total
Peak Hour Fro	om 12:0	00 PM 1	o 06:45	PM - Pe	ak 1 of	1						•		· · · · · · · · · · · · · · · · · · ·					
Intersection	05:00	PM]													1	
Volume	2	0	19	21	0	1154	21	0	1175	0	0	0	0	20	779	0	12	811	2007
Percent	9.5	0.0	90.5		0.0	98.2	1.8	0.0		0.0	0.0	0.0		2.5	96.1	0.0	1.5		
05:30 Volume	0	0	4	4	0	313	4	0	317	0	0	0	0	5	210	0	2	217	538
Peak																		1	0.933
Factor																			
-	05:00	PM			05:30	PM								05:30	PM				
Volume	0	0	7	7	0	313	4	0	317	0	0	0	0	5	210	0	2	217	
Peak Factor				0.750					0.927									0.934	



File Name : drvwy1rivers Site Code : 00000000 Start Date : 1/17/2007 Page No : 1

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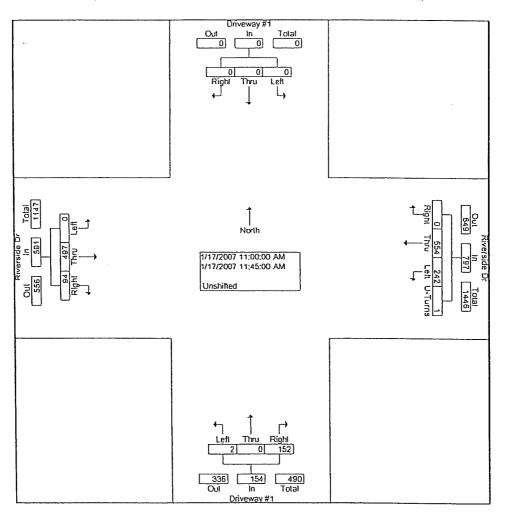
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-	0				T			U-	- No	orthbound	<u> </u>		aslbound		
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L.	11:00 AM	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
	11:15 AM	0	0	0	57 50	155 136	0	0	0	0	37	0	132	21	402
	11:30 AM	0	0	0	58 63	136	0	1	1	0	35	0	138	23	392
	11:45 AM	0	0	0	64	124	0	0	0	0	43	0	103	20	353
	Total	0	0	0	242	139 554	0	0	1	0	37	0	124	30	395
		_					U	1	2	0	152	0	497	94	1542
	12:00 PM	0	0	0	79	129	0	0	0	0	59	0	126	28	421
	12:15 PM	0	0	0	87	125	0	0	0	0	43	Ö	123	27	405
	12:30 PM	0	0	0	70	138	0	0	0	0	44	0	115	32	399
	12:45 PM	0	0	0	84	147	0	0	0	0	63	0	120	27	441
	Total	0	0	0	320	539	0	0	0	0	209	0	484	114	1666
	01:00 PM	0	0	0	82	142	0	0	2	0	74	0	104	15	419
	01:15 PM	0	0	0	72	143	0	1	2	0	75	0	109	24	426
	01:30 РМ 01:45 РМ	0	0	0	61	119	.0	0	4	0	84	0	107	16	391
	Total	0	0	0	64	148	.0	0	3	0	76	0	109	15	415
		U	0	0	279	552	0	1	11	0	309	0	429	70	1651
	02:00 PM	0	0	0)	61	150	0	2	4	0	74	0	129	461	420
	02:15 PM	0	0	0	71	136	0	0	1	Õ	54	0	129	16 27	436
	02:30 PM	0	0	0	73	144	0	0	1	Õ	73	0	121		426 427
	02:45 PM	0	0	0	51	152	0	0	3	ō	62	0	155	15	427
	Total	0	0	0	256	582	0	2	9	0	263	0	542	69	1723
	03:00 PM	0	0	0	55 [°]	181	0	0	1	0	77	0	144	16	474
	03:15 PM	0	0	0	65	221	0	0	1	0	64	ő	152	26	529
	03:30 PM	0	0	0	60	210	0	1	2	0	73	ő	141	11	498
	03:45 PM	0	0	0	78	233	0	0	· 5	0	63	0	115	17	511
	Total	0	0	0	258	845	0	1	9	0	277	0	552	70	2012
	04:00 PM	0	0	0	56	206	0	0 1	1	0	63 j	0	132	13	47.4
	04:15 PM	0	0	0	74	221	0	0	2	Ö	60	0	149	17	47 1 523
	04:30 PM	0	0	0	55	207	0	0	2	Ö	52	0	160	26	502
	04:45 PM	0	0	0	62	204	0	0	2	Ō	49	0	161	12	490
	Total	0	0	0	247	838	0	0	7	0	224	0	602	68	1986
	05:00 PM	0	0	0	57	241	0	1	2	0	54	.0 .	148	21	524
	05:15 PM	0.	0	0	60	221	0	1	1	ō	41	1	141	16	482
	05:30 PM	. 0	0	0	76	243	0	0	3	Ō	68	Ó	155	19	564
	05:45 PM	0	0	0	78	212	0	0	2	1	65	0	133	16	
	Total	0	0	0	271	917	0	2	8	1	228	1	577	72	507 2077
	06:00 PM	0	0	0]	70	218	0	o l	7	0	65 l	0	440	22	500
	06:15 PM	0	0	0	53	206	0	ol	5	ő	51	_	118		500
	06:30 PM	0	0	0	73	210	0	ol	2	ő	66	0 0	137	15	467
	06:45 PM	0	0	0	60	226	0	ō	4	Ö	56		119	15	485
	Total	0	0	Ō	256	860	0	0	18	0	238	0	105 479	66	465 1917
	Grand Total	0	0	01	2129	5687	0	7	64	1	1900			con l	
	Apprch %	0.0	0.0	0.0	27.2	72.7	0.0	0.1	3.3	0.1	96.7	1	4162	623	14574
	Total %	0.0	0.0	0.0	14.6	39.0	0.0	0.0	0.4	0.0	13.0	0.0	87 .0	13.0	
						-			0.7	0.0	10.01	0.0	28.6	4.3	

File Name : drvwy1rivers Site Code : 00000000

Start Date : 1/17/2007

Page No : 2

			way#1 hbound				iverside Vestbou					way #1 abound				side Dr bound		
Slan Time	Left	Thru	Right	App. Total	Left	Thru	Right	Turn s	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	int. Total
Peak Hour Fro	m 11:00	O AM to	11:45	AM - Peal	k 1 of 1													
Intersection	11:00	AM															1	
Volume	0	0	0	0	242	554	0	1	797	2	0	152	154	0	497	94	591	1542
Percent	0.0	0.0	0.0		30.4	69.5	0.0	0.1		1.3	0.0	98.7		0.0	84.1	15.9		
11:00 Volume	0	0	0	0	57	155	0	0	212	0	0	37	37	0	132	21	153	402
Peak Factor																		0.959
High Int.	10:45:	MA00			11:00	AM				11:30	MΑ			11:15	AM		ļ	
Volume	0	0	0	0	57	155	0	0	212	0	0	43	43	0	138	23	161	
Peak Factor									0.940				0.895				0.918	

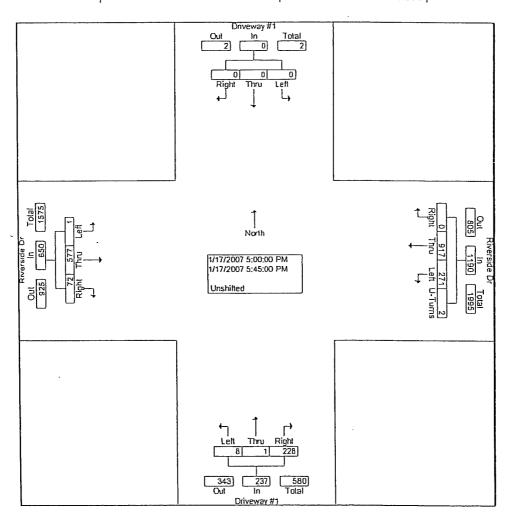


File Name: drvwy1rivers Site Code: 00000000

Start Date : 1/17/2007

Page No : 3

			way#1 bound				iverside Vestbou					way #1 nbound				side Dr bound		
Slart Time	Left	Thru	Right	App. Total	Left	Thru	Right	U- Turn s	App. Tolal	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	int. Total
Peak Hour Fro	m 12:00	D PM to	06:45 I	M - Pea	k 1 of 1											- 4		
Intersection	05:00	РM																
Volume	0	0	0	0	271	917	0	2	1190	8	1	228	237	1	577	72	650	2077
Percent	0.0	0.0	0.0		22.8	77.1	0.0	0.2		3.4	0.4	96.2		0.2	88.8	11.1		
05:30 Volume	0	0	0	0	76	243	0	0	319	3	0	68	71	0	155	19	174	564
Peak Factor																		0.921
High Int.					05:30	PM				05:30	PM			05:30	PM			
Volume	0	0	0	0	76	243	0	0	319	3	0	68	71	0	155	19	174	
Peak Factor									0.933				0.835				0.934	



File Name: drvwy3river Site Code: 00000000 Start Date: 1/17/2007

Page No : 1

Groups Printed- Unshifted

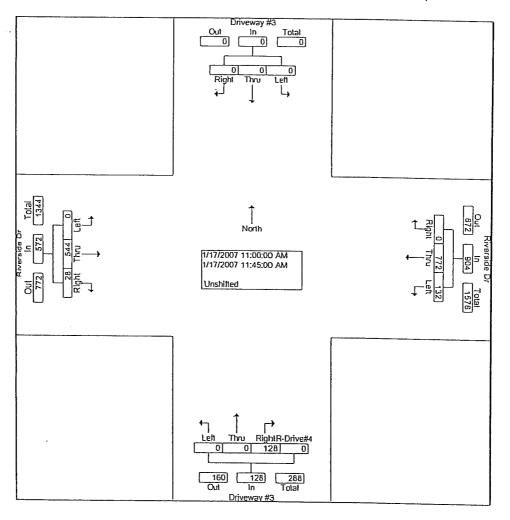
						oups Prin	ted- Unst					Ü		
		iveway #3			verside D			Drivev				verside D		
	Si	outhbound	1		estbound			Northi	oound		E	astbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	R- Drive# 4	Left	Thru	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
11:00 AM	0	0	0	37	194	0	0	0	40	0	0	140	6	417
11:15 AM	0	0	0	29	191	0	0	0	39	0	0	148	5	412
11:30 AM	0	0	0	31	190	0	0	0	20	0	0	128	6	375
11:45 AM	0	0	0	35	197	0	0	0	29	0	0	128	11	400
Total	0	0	0	132	772	0	0	0	128	0	0	544	28	1604
12:00 PM	0	0	0	42	202	. 0	0	0	24	0	0	157	5	430
12:15 PM	0	0	0	48	201	0	0	0	29	0	0	151	10	439
12:30 PM	0	0	0	41	198	0	0	0	36	0	0	130	9	414
12:45 PM Total	0	0	0	40	222	0	0	0	37	2	0	158	20	479
10191	U	U	0	171	823	0	0	0	126	2	0	596	44	1762
01:00 PM	0	0	0	45	223	0	0	0	43	2	0	164	3	480
01:15 PM	0	0	0	35	217	0	0	0	33	2	0	161	5	453
01:30 PM	0	0	0	30	173	0	0	0	37	0	0	172	3	415
01:45 PM Total	0	0	0	37	222	0	0	0	38	2	0	164	6	469
10131	U	U	0	147	835	0	0	0	151	6	0	661	17	1817
02:00 PM	0	0	0	32	208	0	0	0	31	2	0	186	8	467
02:15 PM	0	0	0	28	194	0	0	0	43	2	0	170	4	441
02:30 PM	0	0	0	27	202	0	0	0	28	2	0	186	5	450
02:45 PM Total	0	0	0	25 112	205 809	0	0	0	40 142	6	0	201 743	9 26	480 1838
03:00 PM	0		0				-			•	_			
03:15 PM	0	0 0	0	35 31	233 274	0	0 0	0 0	43 40	0	0	185	6	502
03:30 PM	0	0	0	37	268	0	0	0	40 29	0 3	. 0	187	4	536
03:45 PM	ő	0	o l	27	306	0	0	0	46	1	0 0	182 164	8 5	527 549
Total	0	0	0	130	1081	0	0	0	158	4	0	718	23	2114
04:00 PM	0	0	0	26	254	0]	0	0	30	1	0	180	7	498
04:15 PM	0	0	0	26	271	0	0	0	42	2	ő	180	7	528
04:30 PM	0	0	0	23	255	0	0	0	41	1	0	184	5	509
04:45 PM	0	0	0	29	259	0	0	0	24	0	0	185	12	509
Total	0	0	0	104	1039	0	0	0	137	4	0	729	31	- 2044
05:00 PM	0		0	30	294	0	0	0	47	-2-	0	178	11	562
05:15 PM	0	0	0	27	277	0	0	0	44	3	0	172	3	526
05:30 PM	0	0	0	28	305	0	0	0	48	1	0	184	6	572
05:45 PM Total	0	0	0	34 119	284	0	0	0	38	3	0	179	7	545
Total	U	U	0	119	1160	0	0	0	177	9	0	713	27	2205
06:00 PM 06:15 PM	0	0	0	44	289	0	0	0	44	1	0	185	6	569
06:15 PM 06:30 PM	0	0	0	29	247	0	0	0	50	3	0	165	9	503
06:45 PM	0	0	0	16	273	0	0	0	35	2	0	161	6	493
Total	0	0	0	27 116	285 1094	0	0	0	36 165	7	0 0	14.4 65.5	6 27	499 2064
Grand Total	^			4024							_		·	
Appreh %	0	0 0.0	0	1031	7613	0	0	0	1184	38	0	5359	223	15448
Tolal %	0.0 0.0	0.0	0.0	11.9 6.7	88.1 49.3	0.0	0.0 0.0	0.0	96.9	3.1	0.0	96.O	4.0	
1 CtG1 /B	0.0	U.U	0.0	0.7	C.6 F	0.0]	0.0	0.0	7.7	0.2	0.0	34.7	1.4	

File Name: drvwy3river Site Code: 00000000

Start Date : 1/17/2007

Page No : 2

			way #3 ibound			_	side Dr tbound				riveway Jorthbot	und				side Di bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	R- Drive #4	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	m 11:00	O AM to	11:45	AM - Peal	k 1 of 1							11		1				
Intersection				1	1									I			1	
Volume	0	0	0	0	132	772	0	904	0	0	128	0	128	0	544	28	572	1604
Percent	0.0	0.0	0.0		14.6	85.4	0.0		0.0	0.0	100. 0	0.0		0.0	95.1	4.9		
Volume	0	0	0	0	37	194	0	231	0	0	40	0	40	0	140	6	146	417
Peak Factor High Int.	10:45:0				11:45				11:00	ΔM			į	11:15	MA			0.962
Volume Peak Factor	0	0	0	0	35	197	0	232 0.974	0	0	40	0	40 0.800	0	148	5	153 0.935	

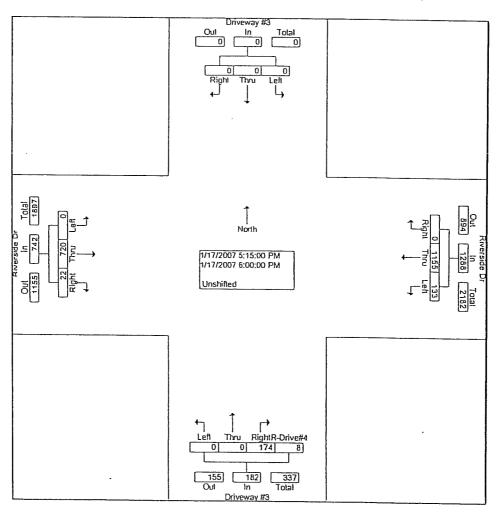


File Name : drvwy3river Site Code : 00000000

Site Code : 00000000 Start Date : 1/17/2007

Page No : 3

Thru O PM to PM	Right 06:45 F	App. Total PM - Peal	Left k 1 of 1	Thru	Right	App. Total	Left	Thru	Right	R- Drive	App.		~.		Арр.	lnt.
	06:45 F	M - Peal	k 1 of 1				j Į			#4	Total	Left	Thru	Right	Total	Total
PM _		1					·					I				
												•			1	
0	0	0	133	1155	0	1288	0	0	174	8	182	0	720	22	742	2212
0.0	0.0	ì	10.3	89.7	0.0		0.0	0.0	95.6	4.4	102	0.0	97.0	3.0	142	2212
0	0	0	28	305	0	333	0	0	48	1	49	0	184	6	190	572
																0.967
			05:30	PM			05:30 I	PM				06:00	⊃М		1	0.907
0	0	0	28	305	0	333	0	0	48	1	49	0	185	6	191	
				0 0 0 28	0 0 0 28 305 05:30 PM	0 0 0 28 305 0 05:30 PM	0 0 0 28 305 0 333 05:30 PM	0 0 0 28 305 0 333 0 05:30 PM 05:30 FM 05:30 F	0 0 0 28 305 0 333 0 0 05:30 PM 05:30 PM 05:30 PM 0 0	0 0 0 28 305 0 333 0 0 48 0 0 0 0 28 305 0 333 0 0 48	0 0 0 28 305 0 333 0 0 48 1 0 0 0 0 28 305 0 333 0 0 48 1	0 0 0 28 305 0 333 0 0 48 1 49 0 0 0 0 28 305 0 333 0 0 48 1 49	0 0 0 28 305 0 333 0 0 48 1 49 0 06:00 F 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 28 305 0 333 0 0 48 1 49 0 184 0 0 0 0 28 305 0 333 0 0 48 1 49 06:00 PM 0 0 0 28 305 0 333 0 0 48 1 49 0 185	0 0 0 28 305 0 333 0 0 48 1 49 0 184 6 0 0 0 0 28 305 0 333 0 0 48 1 49 06:00 PM 0 0 0 28 305 0 333 0 0 48 1 49 0 185 6	0 0 0 28 305 0 333 0 0 48 1 49 0 184 6 190 0 0 0 0 28 305 0 333 0 0 48 1 49 0 185 6 191



File Name: drvwy4river Site Code: 00000000

Site Code : 00000000 Start Date : 1/17/2007

Page No : 1

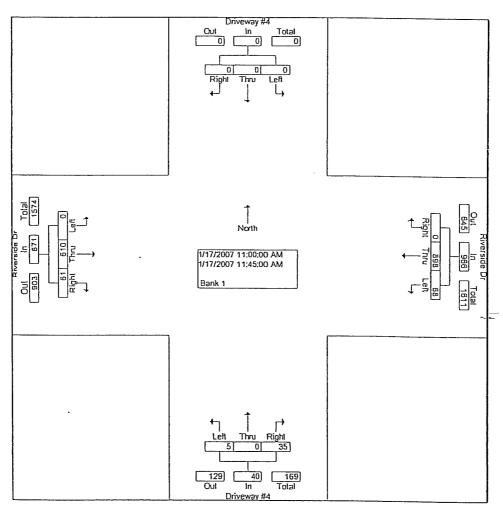
Groups Printed- Bank 1

						Printed-							
		iveway #4			verside Dr			veway #4			verside Dr		
		uthbound			estbound			rthbound			astbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
11:00 AM	0	0	0	18	230	0	1	0	12	0	164	14	439
11:15 AM	0	0	0	18	219	0	2	0	8	0	168	19	434
11:30 AM	0	0	0	16	219	0	2	0	8	0	139	10	394
11:45 AM	0	0	0	16	230	0	0	0	7	0	139	18	410
Total	0	0	0	68	898	0	5	0	35	0	610	61	1677
12:00 PM	0	0	0	19	245	0	0	0	8	0	164	17	453
12:15 PM	0	0	0	25	249	0	0	0	10	0	166	13	463
12:30 PM	0	0	0	22	237	0	2	0	12	0	153	14	440
12:45 PM	. 0	0	0	24	261	0	0	0	21	0	185	10	501
Total	0	0	0	90	992	0	2	0	51	0	668	54	1857
	_	_	- 1			- 1							
01:00 PM	0	0	0	23	266	0	1	0	17	0	196	11	514
01:15 PM	0	0	0	28	253	0	0	0	6	0	177	18	482
01:30 PM	0 ·	0	0	23	199	0	5	0	13	0	199	10	449
01:45 PM	0	0	0	22	257	0	0	0	13	0	188	15	495
Total	0	0	0	96	975	0	6	0	49	0	760	54	1940
02:00 PM	0	0	0 1	23	241	0)	1	0	14	0	207	91	495
02:15 PM	0	0	0	25 25	220	0	2	0	8	0	207 199	15	495 469
02:30 PM	0	0	0	19	229	0	0	0	10	0	203	11	472
02:45 PM	0	0	0	20	228	ő	2	0	11	0	203	11	502
Total	0	0	0	87	918	0	5	0	43	0	839	46	1938
, 0.6.	Ü	Ü	0 1	0,	310	O ,	3	Ū	75 1	U	033	40 }	1330
03:00 PM	0	0	0 1	27	267	0	1	0	12	0	215	13	535
03:15 PM	0	0	0	15	303	0	2	0	17	ō	216	10	563
03:30 PM	0	0	0	24	304	0	0	0	14	Ō	195	16	553
03:45 PM	0	0	0	16	333	0	1	0	5	0	199	12	566
Total	0	0	0	82	1207	.0	4	0	48	0	825	51	2217
						,							
04:00 PM	0	0	0	28	280	0	0	0	10	0	195	16	529
04:15 PM	0	0	0	23	299	0	0	0	23	0	205	16	566
04:30 PM	0	0	0	27	278	0	0	0	14	0	211	13	543
04:45 PM	0	0	0	19	286	0	1	0	14	0	199	11	530
Total	0	0	0	97	1143	0	1	0	61	0	810	56	2168
05:00 PM	0	0	0	27	324	0 l	0	0	13	0	212	13	589
05:15 PM	0	0	0	29	303	ő	1	0	9	0		11	559
05:30 PM	0	0	ő	16	330	ő	1	0	18	0	206 213	19	597
05:45 PM	0	0	ő	29	320	0	Ö	0	7	0	213	6	573
Total	0	0	0	101	1277	0	2	- 0	47	0	842	49	2318
70101	v	U	O Į	101	1211	0 1	2	U	77,	U	04.2	75]	2510
06:00 PM	0	0	0	14	334	0	0	0	16	0	220	7	591
06:15 PM	Õ	0	ő	9	277	ő	Ő	ő	7	ő	204	13	510
06:30 PM	0	0	ő	12	289	ő	Ö	0	6	Ö	187	8	502
06:45 PM	Ö	0	ől	16	310	ŏ	0	0	13	0	171	11	521
Total	0	0	0	51	1210	0	0	0	42	0	782	39	2124
, 5.01	J	·	- 1	٠.		٠,	•	Ū	12-1	U	, 02	55 1	_,_,
Grand Total	0	0	0	672	8620	0	25	0	376	0	6136	410	16239
Apprch %	0.0	0.0	0.0	7.2	92.8	0.0	6.2	0.0	93.8	0.0	93.7	6.3	
Total %	0.0	0.0	0.0	4.1	53.1	0.0	0.2	0.0	2.3	0.0	37.8	2.5	
						,			1		00	1	

File Name: drvwy4river Site Code: 00000000

Start Date : 1/17/2007 Page No : 2

			way #4				side Dr				way #4			River	side Dr	1	
		South	hbound			Wes	lbound			North	bound			East	bound		
Start Time	Left	Thru	Right	App.	Left	Thru	Right	App.	Left	Thru	Right	Арр.	Left	Thru	Right	Арр.	Int.
<u></u> _	44.00		لبتتيا	Total				Total	LL			Total				Total	Total
Peak Hour Fron			1:45 AM	1 - Peak 1	of 1												
Intersection	11:00 /	AΜ													•	1	
Volume	0	0	0	0	68	898	0	966	5	0	35	40	0	610	61	671	1677
Percent	0.0	0.0	0.0		7.0	93.0	0.0		12.5	0.0	87.5		0.0	90.9	9.1		
11:00	0	0	0	0	18	230	0	248	-1	0	12	13	_	-01			
Volume	U	U	U	U	10	230	U	240	,	U	12	13	0	164	14	178	439
Peak Factor																	0.955
High Int.	10:45:0	$M \triangle 00$			11:00 A	MA			11:00 A	M			11:15 A	M		į.	
Volume	0	0	0	0	18	230	0	248	1	0	12	13	0	168	19	187	
Peak Factor								0.974				0.769				0.897	

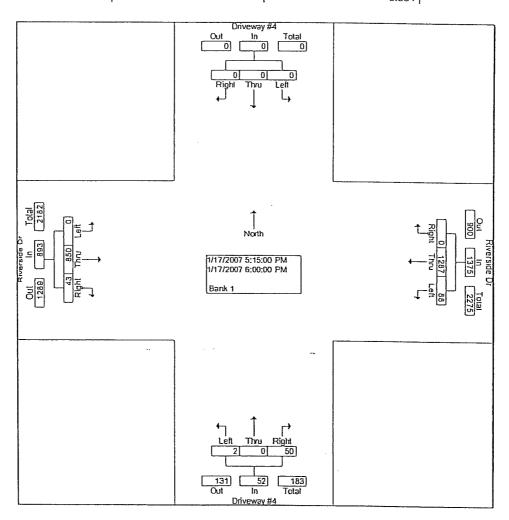


File Name : drvwy4river Site Code : 00000000

Start Date : 1/17/2007

Page No : 3

			way #4 nbound				side Dr tbound				way #4 bound				side Dr bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fron	n 12:00	PM lo C	6:45 PM	1 - Peak 1	of 1		٠					. 0.01			L	TUIDI	10181
Intersection	05:15 1	⊃M							1				1			1	
Volume	0	0	0	0	88	1287	0	1375	2	0	50	52	0	850	43	893	2320
Percent	0.0	0.0	0.0		6.4	93.6	0.0		3.8	0.0	96.2	02	0.0	95.2	4.8	033	2320
05:30 Volume	0	0	0	0	16	330	0	346	1	0	18	19	0	213	19	232	597
Peak Factor																1	0.972
High Int.					05:45 F	M			05:30 F	M			05:30 F	PM		1	0.312
Volume	0	0	0	0	29	320	0	349	1	0	18	19	0	213	19	232	
Peak Factor								0.985				0.684	ŭ	0		0.962	



PAGE 2	DATE 10/2005	SECTION 353	SU	BJECT	WARRAN	IS FOR TRA	FFIC SIGNA	LS		Depa MANUAL O	ortment of Trail OF POLICIES	nsportation & PROCEDU	RES
				TRAF	FIC SI	GNAL	WARI	RANTS	6				
			,								DA	TE <u>5/</u>	7/08
Major St: _ Minor St: _	RIVERSI. EASTERL	DE PR LY PR	IVE QTEC,	Cr <u>T PR</u> IVE	itical A _l	pproacl MA774	h Speed	d VENUE	35	n	nph		
ln b	ical speed uilt up are ation is	a of isc	lated o	commun	ity of <	10,000) pop.	□ RUR	AL or >	☑ URBA 10,000 【URBA	0 🗷 UR	BAN	
WARRAN Condition	A - Minimu	ım Veh	icle Vo	lume	r Volu	1		ATISFIE ATISFIE			No □ No □		
	ļ	SHOWN R	•			Right	Turn Re	eduction a	applicatio	on Minor	Street = .	<u>/00</u> %	
APPROACH LANES		1		OR ORE	1140	12.01	10/ Nd/	1 8	3. PM	To A	5 PM	Moderation	HOUR
Both Appreh	5. 500 (400)	350 (280)	(480)	420 (336)	1660	1809	1775	1843	2090	2/06	2192	2037	
Highest Appr Minor Street	ch. (150 (120)	105 (84)	200 (160)	140 (112)	554	696	644	57 <i>4</i>	690	564	610	542	
Condilion I	3 - Interru	ption of	Contir	nuous T	raffic			ATISFIE ATISFIE			No □ No □		
	1	Minimum R % SHOWN R	•								Street =	<i>100</i> %	
APPROACH LANES		1		OR ORE	1/4/1	12/1/2	ma)	18	/ %	1 / H	59%	6PM	HOUR
Both Approb	1	525 (420)	(900 (720)	630 (504)	1660	1809	1775	1843	2090	2106	2/92	2037	
Highest Appr Minor Street	75 (60)	53 (42)	10 6 (80)	70 (56)	554	696	.644	574	59D	564	610	542	
(Combinat	tion of	Condit	tions A	& B			SATIS	FIED	YES	□ NO		
	REQ	UIREME		1. MINIMU		ARRANT			V	FULFIL	LED		

2. INTERRUPTION OF CONTINUOUS TRAFFIC

Yes □ No □

TWO WARRANTS

SATISFIED 80%

SUBJECT

WARRANTS FOR TRAFFIC SIGNALS

SECTION 353

DATE 10/2005

MATILLIA AVENUE - EASTERLY PROTECT DRIVEWAY/RIVERSIDE DRIVE WARRANT 2 - Four - Hour Vehicular Volume

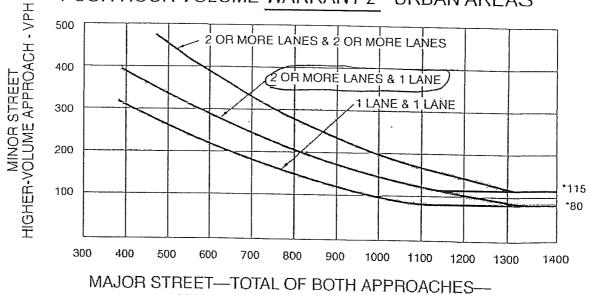
SATISFIED * Yes & No □

Right Turn Reduction application Minor Street = 160 %

Approach Lanes	One	2 or more	70/	, v	y v	er er	HOUR
Both Approaches, Major Street		X	1775	1843	2090	2192	
Highest Approach, Minor Street	X		696	644	574	564	

^{*} Refer to Fig. A (URBAN AREAS) OR Fig. B (RURAL AREAS) to determine if this warrant is satisfied.



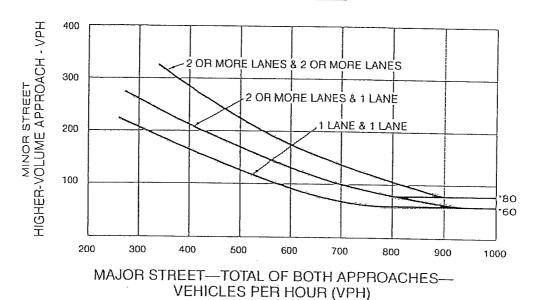


VEHICLES PER HOUR (VPH)

NOTE: 115 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 80 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

MATILITA AVENUE - EASTERLY PROJECT DRIVEWAY/RIVERSIDE DRIVE

Figure B FOUR HOUR VOLUME WARRANT 2 - RURAL AREAS



'NOTE: 80 VPH APPLIES'AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 60 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

WARRANT 3 - Peak Hour	PART A OR PART I	B SATISFIED	Yes□	No □
PART A (All parts 1, 2, and 3 below must be satisfied)		SATISFIED	Yes□	No □
 The total delay experienced for traffic on one minor by a STOP sign equals or exceeds four vehicle-hou and five vehicle-hours for a two-lane approach; <u>AN</u> 	urs for a one-lane approach	Yes □	No 🗆	
The volume on the same minor street approach eq one moving lane of traffic or 150 vph for two movin			No □	
 The total entering volume serviced during the hour for intersections with four or more approaches or 6: three approaches. 	equals or exceeds 800 vph 50 vph for intersections with		No 🗆	
PART B		SATISFIED*	Yes 🕅	Nn □

г	Approach Lanes	One	2 or more	Sh	J. By	HOUR
	Both Approaches, Major Street		X	2096	2192	
	Highest Approach, Minor Street	X		574	564	

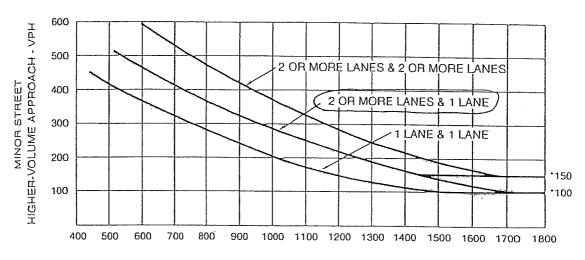
^{*} Refer to Fig. C (URBAN AREAS) OR Fig. D (RURAL AREAS) to determine if this warrant is satisfied

Estimated peak hour volumes based on accepted trip generation rates and traffic assignment may be used when evaluating the installation of a traffic signal to mitigate the impacts of near-term development.

SECTION 353

MATILITA AVENUE-EASTERLY PROTECT DRIVEWAY/RIVERSIDE DRIVE

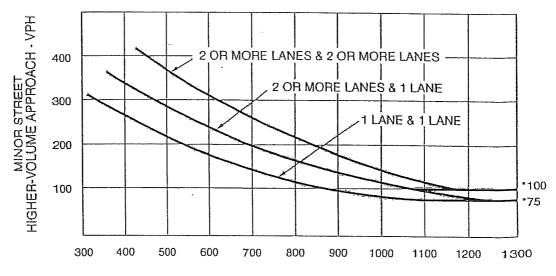
Figure C PEAK HOUR VOLUME WARRANT 3 - URBAN AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

NOTE: 150 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Figure D PEAK HOUR VOLUME WARRANT 3 - RURAL AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

^{*} NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Department of Transpo MANUAL OF POLICIES & PRO	CEDURES	SUBJECT		IANTS FOR TRAFFIC SIGNA		SECTION 353	DATE 10/2005	PAGE 7
<i>MATILITI</i> School Area Traffi	4 AVEX	IUE-E	ASTERL	PROTECTARI	VEWAY/RIUB	RINDE DI		No □
(Both parts must be sa						C) (TIC) II	-D 163 []	110
	Minimu	ım Requir	ements					
PART A		υ	R		HOUR			
Vehicle Volume	Each of 2 hours	500	350					
School Age Pedestrian Crossing	Each of 2 hours	100	70			SATISF	FIED Yes [⊃ No □
Street	or per day	500	350					
				AND				
PART Bs nearest controlled	crossing	more tha	n 600 fee	away?		SATISFI	ED Yes □	No □
WARRANT 6 - C All parls must be			gnal Sys	tem	100%	SATISFI	ED Yes⊠	No □
MINIMUM REQ	UIREMEN	NTS	I	ISTANCE TO NE	AREST SIGNAI	-	FULFIL	LED
> 1000 FEET			N	_ft, Sft,	E <u>600</u> ft, W_	2 <u>000</u> ft	Yes 🗵	No □
ON ISOLATED ONE ADJACENT SIGNAL CONTROL WOULD	S ARE SO	EET OR S FAR APA	STREET W ART THAT	ITH ONE WAY TRA NECESSARY PLAT	FFIC SIGNIFICAN DONING AND SP	NCE EED	IS/	
ON 2-WAY STREET PLATOONING & SP PROGRESSIVE SIG	EED CONT	rol, Pr	T SIGNALS OPOSED	DO NOT PROVIDE SIGNALS COULD CO	NECESSARY ONSTITUTE A		Ø	
WARRANT 7 - C (All parls must be			ce		100%	SATISFI	ED Yes □	No □
REQUIREMENT			W187714WW	WARRANT		V	FULFII	LED
ONE WARRANT	WAF	RRANT 1	- MINIMUN	VEHICULAR VOLU	ME			
SATISFIED 80%	WAF	OR RRANT 2	- INTERRU	PTION OF CONTIN	UOUS TRAFFIC		Yes 🗆	No □
ADEQUATE TRIAL (COLLISION FREQU	OF LESS R ENCY	RESTRICT	TIVE REME	DIES HAS FAILED	TO REDUCE		Yes□	No □
COLLISIONS WITHI CORRECTION	N A 12 MO	NTH PEF	RIOD THAT	ARE SUSCEPTIBL	ЕТО			
MINIMUM REQUIREMENT				BER OF COLLISION			Yes□	No □
3 OR MORE DURIN			DATES OF	QUALIFYING COLL			, 00 0	110

PAGE 2	DATE 10/2005	SECTION 353	SUE	JECT	WARRAN	TS FOR TRA	FFIC SIGNA	LS		Depa MANUAL C	eriment of Tra OF POLICIES	nsportation & PROCEDU	RES
				TRAF	FIC SI	GNAL	WAR	RANT:	5				
											DA	ATE <u>5</u> ATE	/1/08
Major St: _ Minor St: _	RIVERSI; ELSTERL	DE DRI 1 PROT	VE ECT A	Cı RIVEW	ritical A Y-Mi	pproacl 4774/7/	h Spee I AVE	d N ui	35	n	nph		
Criti In b	cal speed uilt up are ation is	l of maj ea of isc	or stree	et traffic ommur	: > 40 n nity of <	nph 🗆 F	RURAL) pop.	or <u><</u> 4(□ RUR) mph [AL or >	&URB/ • 10,00	0 🗷 UR	BAN	
WARRAN Condition	A - Minimu	um Veh	icle Vol	lume	ar Volu	1		ATISFII ATISFII			No □ No □		
	ı		eguiremer IN BRACKI			Right	Tum Re	eduction	applicatio	n Minor	Street =	<u>0</u> %	
	υ	R	υ	R									
APPROACH LANES	OR ORE	11 411	12pm	ra!	2011	3PM	The T	1 20	/ Mg	HOUR			
Both Apprchs Major Street	. 500 (400)	350 (280)	(480)	420 (336)	1660	1809	1775	1843			2192	2037	
Highest Appr Minor Street	ch. 150 (120)	105 (84)	(160)	140 (112)	10/2	1183	1243	1110	1142	1071	146	1080	
Condition E	3 - Interru	ption of	Contin	uous T	raffic			ATISFIE ATISFIE			No 🗆		
	N	linimum R	e quire me r	nts	1						√o □	<i>1</i> -	
	(80°	% SHOWN	IN BRACKI U	ETS)		Right	l Turn Re	eduction	application	on Minor	Street =	%	
APPROACH LANES		1		OR ORE	11AN	Ren] nd/	1 / 28	38	T Ros	5 PM	Mag	HOUR
Both Apprchs Major Street	. 750 (600)	525 (420)	900 (720)	630 (504)	1660	1809	1715	1843	2090	2106	2192	2037	
Highest Appr Minor Street	c h. 75 (60)	53 (42)	(80)	70 (56)	1012	1183	1213	1/10	1142	1071	1146	1080	
(Combinat	ion of	Conditi	ons A	& B	1		SATIS	FIED	YES	□ N O		I

REQUIREMENT	WARRANT	V	FULFILLED
TWO WARRANTS	1. MINIMUM VEHICULAR VOLUME		
SATISFIED 80%	2. INTERRUPTION OF CONTINUOUS TRAFFIC		Yes □ No □

Department of Transportation
MANUAL OF POLICIES & PROCEDURES

SUBJECT
WARRANTS FOR TRAFFIC SIGNALS

SECTION
353

DATE
10/2005
3

MATILITA AVENUE - EASTERLY PROTECT DRIVEWAY RIVERSIDE DRIVE WARRANT 2 - Four - Hour Vehicular Volume

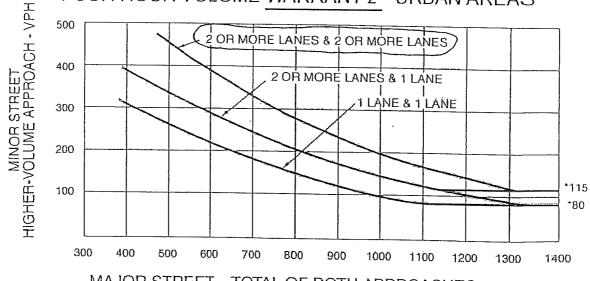
SATISFIED * Yes ☑ No □

Right Turn Reduction application Minor Street = 0 %

r	Approach Lanes	One	2 or more	10	7 3			HOUR
	Both Approaches, Major Street		Х	1775	1843	2090	2192	
	Highest Approach, Minor Street		Х	1213	///0	1142	1146	

^{*} Refer to Fig. A (URBAN AREAS) OR Fig. B (RURAL AREAS) to determine if this warrant is satisfied.

Figure A FOUR HOUR VOLUME WARRANT 2 - URBAN AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

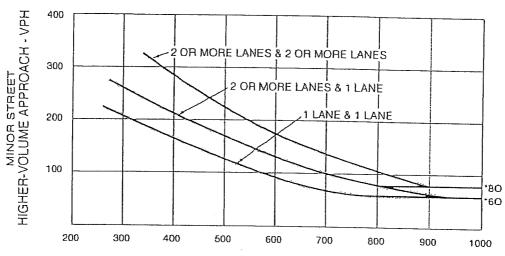
NOTE: 115 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 80 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Department of Transportation
MANUAL OF POLICIES & PROCEDURES

HOUR

MATILIJA AVENUE - EASTERLY PROTECT IN INEWAY/RIVERSIDE ARIVE

Figure B FOUR HOUR VOLUME WARRANT 2 - RURAL AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

'NOTE: 80 VPH APPLIES'AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 60 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

WARRANI	3 - Pea	k Hour
---------	---------	--------

Approach Lanes

Both Approaches, Major Street

Highest Approach, Minor Street

WARRA	NT 3 - Peak Hour	PART A OR PART	B SATISFIE	O Yes □	No □
PART A (All parts	1, 2, and 3 below must be satisfied)		SATISFIED	Yes□	No □
1.	The total delay experienced for traffic on one minor s by a STOP sign equals or exceeds four vehicle-hours and five vehicle-hours for a two-lane approach; AND	s for a one-lane approach	Yes □	No □	
2.	The volume on the same minor street approach equations moving lane of traffic or 150 vph for two moving		r Yes □	No□	
3.	The total entering volume serviced during the hour entering intersections with four or more approaches or 650 three approaches.	quals or exceeds 800 vph ovph for intersections with)	No □	
PART B		or .2\/	SATISFIED*	Yes ⊠	No □

more

2090

One

Estimated peak hour volumes based on accepted trip generation rates and traffic assignment may be used when evaluating the installation of a traffic signal to mitigate the impacts of near-term development.

^{*} Refer to Fig. C (URBAN AREAS) OR Fig. D (RURAL AREAS) to determine if this warrant is satisfied

PAGE

Department of Transportation
MANUAL OF POLICIES & PROCEDURES

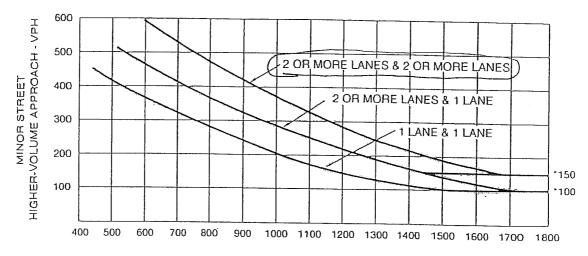
SUBJECT

WARRANTS FOR TRAFFIC SIGNALS

SECTION 353 ATE 10/2005

MATILIJA AVENUE/EASTERLY PROJECT DRIVEWAY/KIVERINDE DRIVE

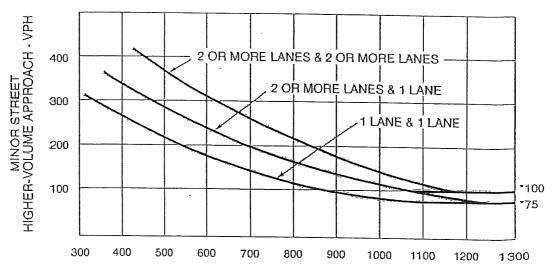
Figure C
PEAK HOUR VOLUME WARRANT 3 - URBAN AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

'NOTE: 150 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Figure D
PEAK HOUR VOLUME WARRANT 3 - RURAL AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

. MAT(U) School Area Traffi Both parls must be sa	ic Signal	JUE- E	ASJÆKLY	PROJECT DRIV	ENAY/RIVE	RSWE ARI SATISFII	<i>VE</i> ED Yes□	I No □
	Minim	um Reguir	ements					
PAIRT A		υ	R		HOUR			
Vehicle Volume	Each of 2 hours	500	350					
School Age	Each of 2 hours	100	70			SATISF	FIED Yes	□ No [
Pedestrian Crossing Street	or per day	500	350					
	-		•	AND				
PART Bs nearest controlled	crossing	more tha	n 600 fee	away?		SATISFI	ED Yes □	No □
VARRANT 6 - C All parts must be			gnal Sy:	tem	100	0% SATISF	IED Yes D	⊠ No □
MINIMUM REQ	UIREME	NTS		ISTANCE TO N	EAREST SIGN	AL	FULFI	LLED
> 1000 FEET			N	_ft, Sft,	E <u>660</u> ft, V	v <u>2000</u> ft	Yes⊠	No □
ON ISOLATED ONE ADJACENT SIGNAL CONTROL WOULD	S ARE SC	EET OR	STREET WART THAT	TH ONE WAY TRA	AFFIC SIGNIFIC FOONING AND	CANCE SPEED	Ø	
ON 2-WAY STREET PLATOONING & SP PROGRESSIVE SIG	EED CON	TROL, PR						u
WARRANT 7 - C All parls must be			ce		100	% SATISFI	IED Yes [□ No E
REQUIREMENT	Г			WARRANT		V	FULFI	LLED
ONE WARRANT	<u> </u>	RRANT 1	- MINIMUN	VEHICULAR VOL	UME			
SATISFIED 80%		OR RRANT 2	- INTERRU	PTION OF CONTI	NUOUS TRAFFI	С	Yes 🗆	No □
ADEQUATE TRIAL (COLLISION FREQU		RESTRICT	TIVE REM	DIES HAS FAILED	TO REDUCE		Yes□	No □
COLLISIONS WITHI	IN A 12 MC	NTH PER	RIOD THAT	ARE SUSCEPTIBI	LE TO			
MINIMUM	I		NUM	ER OF COLLISION	NS			

DATES OF QUALIFYING COLLISIONS

3 OR MORE DURING MOST RECENT 12-MONTH PERIOD; OR 2 PER YEAR DURING

WARRANTS FOR TRAFFIC SIGNALS

Department of Transportation
MANUAL OF POLICIES & PROCEDURES

REQUIREMENT

MOST RECENT 3-YEAR PERIOD

SUBJECT

SECTION 353

DATE 10/2005

Yes 🗆

No □

PAGE 7

	TRAFFIC SIGNAL WARRANTS												
								CALC CHK		tB	_ DA _ DA	TE <u>5/</u> TE	7/08
Major St: RI Minor St: WE	VERSIDE STERL	DRIV Y PRO	E TECT	Cr <u>DRI</u> VEV	itical Ap V A Y	oproach	Speed				ph		
In built	l speed up area on is	a of iso	lated po	ommur	ity of <	10,000	pop. [J RUR.) mph & AL or > . ®) ⊠	10,000	D 🛭 URI	BAN	
WARRANT Condilion A -					ır Volu	10			ED Ye ED Ye		No □ No □		
			quiremen			Right	Tum Re	duction a	applicatio	n Minor :	Street =	100 %	
	υ	R	υ	R							-		
APPROACH LANES	1	l	2 C MO		HOUR REST ST ST ST HOUR								
Both Apprehs. Major Street	500 (400)	350 (280)	(600 (480)	420 (336)	1583	1695	1730	1780	2052	2044	2440	1988	
Highest Apprch. Minor Street	(150)	105 (84)	200 (160)	140 (112)	45	420	435	429	431	423	429	429	
Condition B -	Interrup	otion of	Contin	uous T	raffic				ED Yes		Vo □ Vo □		
	1		e quiremen IN BRACKI U			Righ	t Turn Re	eduction	applicatio	n Minor	Street =	<u>/DD</u> %	
APPROACH LANES		1	2 (OR ORE	11411	Roy Son	My	20m	3/4	464	5pm	/ has	HOUR
Both Apprehs. Major Street	750 (600)	525 (420)	900 (720)	630 (504)	1583	1695	1730	1780	2052	2044	2/40	1988	
Highest Approb. Minor Street	(50)	53 (42)	10u (80)	70 (56)	415	420	.43	429	431	423	429	429	
Сс	Combination of Conditions A & B SATISFIED YES DIOD												

WARRANT

1. MINIMUM VEHICULAR VOLUME

2. INTERRUPTION OF CONTINUOUS TRAFFIC

FULFILLED

Yes □ No □

WARRANTS FOR TRAFFIC SIGNALS

Department of Transportation MANUAL OF POLICIES & PROCEDURES

PAGE 2 DATE 10/2005 SECTION 353

REQUIREMENT

TWO WARRANTS

SATISFIED 80%

SUBJECT

WESTERLY PROJECT DRIVEWAY/ RIVERSIDE DRIVE

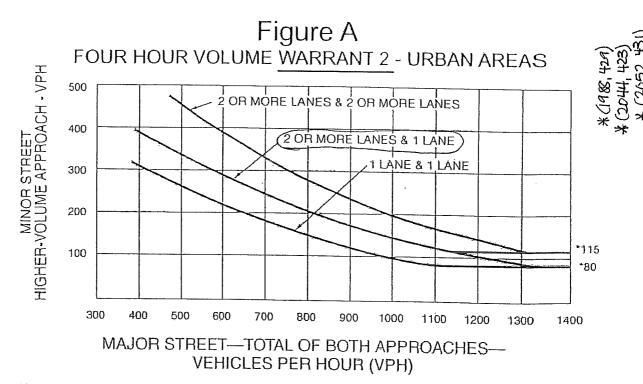
WARRANT 2 - Four - Hour Vehicular Volume

SATISFIED * Yes ₺ No□

Right Turn Reduction application Minor Street = 100 %

Approach Lanes	One	2 or more	8	1	13		HOUR
Both Approaches, Major Street		X	2052	2044	2140	1988	
Highest Approach, Minor Street	X		431	423	429	429	

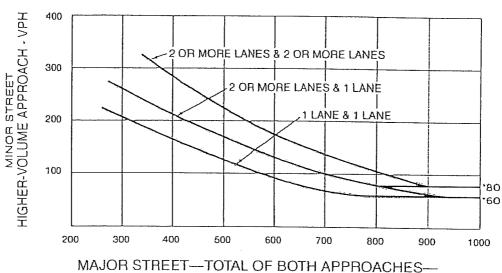
^{*} Refer to Fig. A (URBAN AREAS) OR Fig. B (RURAL AREAS) to determine if this warrant is satisfied.



NOTE: 115 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 80 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

WESTERY PROTECT DRIVEWAY/RIVERSIDE PRIVE

Figure B FOUR HOUR VOLUME WARRANT 2 - RURAL AREAS



VEHICLES PER HOUR (VPH)

NOTE: 80 VPH APPLIES'AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 60 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

WARRANT	3 -	Peal	k H	lour
---------	-----	------	-----	------

PART A OR PART B SATISFIED Yes D No \Box

PART A

(All parts 1, 2, and 3 below must be satisfied)

- 1. The total delay experienced for traffic on one minor street approach controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach; AND
- 2. The volume on the same minor street approach equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; AND
- 3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches.

SATISFIED Yes []

№ П

No □

- Yes □ No 🗆
- Yes 🗆 No 🗆
- Yes 🗆 No 🗆

PART B

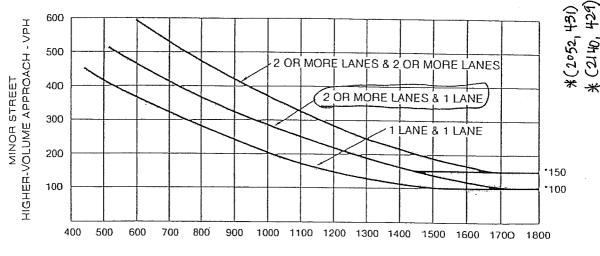
PART B				SATIS	SFIED* Yes⊠
Approach Lanes	One	2 or more	S. S. S. S. S. S. S. S. S. S. S. S. S. S	/ or	HOUR
Both Approaches, Major Street		X	2052	2140	
Highest Approach, Minor Street	X		431	429	

^{*} Refer to Fig. C (URBAN AREAS) OR Fig. D (RURAL AREAS) to determine if this warrant is satisfied

Estimated peak hour volumes based on accepted trip generation rates and traffic assignment may be used when evaluating the installation of a traffic signal to mitigate the impacts of near-term development.

WESTERLY PRITECT DRIVEWAY/RIVERSIDE DRIVE

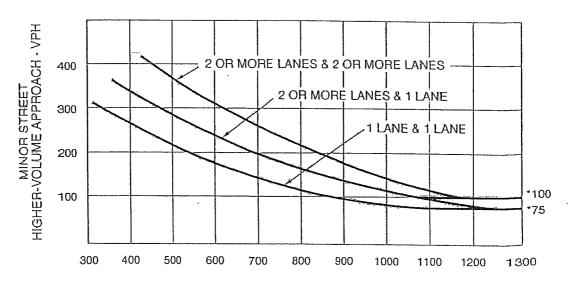
Figure C
PEAK HOUR VOLUME WARRANT 3 - URBAN AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

NOTE: 150 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Figure D
PEAK HOUR VOLUME WARRANT 3 - RURAL AREAS



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

^{*} NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Department of Transpo MANUAL OF POLICIES & PRO	dation OCEDURES	SUBJECT	WARRA	NTS FOR TI	RAFFIC SIGN	NALS	SI	ECTION 353	DATE 10/2005	PAGE 7
	WESTERLY PROTECT DRIVEWAY/RUERSIDE DRIVE chool Area Traffic Signal both parts must be satisfied)							ATISFIE	1	No 🗆
	Minimu	ım Requir	ements							
PART A		υ	R			HOUR				
Vehicle Volume	Each of 2 hours	500	350							
School Age Pedestrian Crossing	Each of 2 hours	100	70				Ş	SATISF	IED Yes [⊃ No □
Street	or per day	500	350							
				Al	ND					
PART B Is nearest controlled	crossing	more tha	n 600 feet a	away?			SA	ATISFIE	D Yes□	No □
WARRANT 6 - C (All parls must be			ınal Syst	em			100% S	ATISFIE	ED Yes □	No □
MINIMUM REQ	UIREMEN	NTS	DI	STANC	E TO N	EAREST S	SIGNAL		FULFIL	LED
> 1000FEET			N	ft, S	ft,	E 2140	ft, w <u>54</u>	O_ft	Yes 🛭	No □
ON ISOLATED ONE ADJACENT SIGNAL CONTROL WOULD I	S ARE SO	EET OR S FAR APA	TREET WIT	H ONE	WAY TR \RY PLA	AFFIC SIG TOONING	NIFICANCE AND SPEE	D	_	
ON 2-WAY STREET PLATOONING & SPI PROGRESSIVE SIG	EED CONT	ROL, PRO							Ø	
WARRANT 7 - C (All parls must be			ce				100% S	ATISFIE	ED Yes □	No □
REQUIREMENT			1	VARR.	ANT			V	FULFIL	.LED
ONE WARRANT	WAF	RRANT 1 -	мимим у	/EHICUL	AR VOL	.UME			-	
SATISFIED 80%	SATISFIED 80% OR WARRANT 2 - INTERRUPTION OF CONTINUOUS TRAFFIC						Yes 🗆	No □		
ADEQUATE TRIAL (COLLISION FREQUI	OF LESS R ENCY	ESTRICT	IVE REMED	IES HAS	S FAILED	TO REDU	CE		Yes□	No □
COLLISIONS WITHII CORRECTION	N A 12 MO	NTH PER	OD THAT A	ARE SUS	SCEPTIB	LE TO				
MINIMUM NUMBER OF COLLISIONS REQUIREMENT DATES OF QUALIFYING COLLISIONS							Yes□	No □		
3 OR MORE DURING MOST RECENT 3-Y	G MOST R EAR PERI	ECENT 1:	2-MONTH P	ERIOD;	OR 2 PE	R YEAR DI	JRING			

	TW	O-WAY STOP	CONTRO	OL SUN	/IMARY	** **		
General Information			Site Ir	ıformat	tion			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 11:00 AM		Interse Jurisdic	Intersection Jurisdiction Analysis Year			re-Riversid s Angeles /ith Projec	le Dr
Project Description 1-05	53606-1/Westfield	l Fashion Square	Project		,			-
East/West Street: Riversi	ide Drive		North/S	outh Stre	eet: <i>Matilija D</i>	r-Project Dri	veway	
Intersection Orientation:	East-West		Study P	eriod (hr	rs): 0.25			
Vehicle Volumes and	d Adjustment:	S						
Major Street		Eastbound				Westbou	nd ,	
Movement	11	2	3		4	5		6
	L	T	R		L	T		R
Volume (veh/h)	4.00	699	133		518	816		12
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	0	699	133		518	816		12
Percent Heavy Vehicles	0				0			
Median Type		Undivided						
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		<u>L</u>	T		TR
Upstream Signal		0 0						
Minor Street		Northbound						
Movement	7	8	9		10	11		12
	L	Т	R		L	Т		R
Volume (veh/h)	36		457					24
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	36	0	457		0	0		24
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		1
Configuration	L		R					R
Delay, Queue Length, an	nd Level of Servi	Ce						
Approach	Eastbound	Westbound	1	Northbou	ınd	S	Southboun	d
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			R
v (veh/h)		518	36		457			24
C (m) (veh/h)		809	13		652			593
v/c		0.64	2.77		0.70			0.04
95% queue length		4.73	5.38		5.70			0.13
Control Delay (s/veh)		17.0	1390		22.4			11.3
LOS		C	F		C C			В
Approach Delay (s/veh)				122.2			11.3	
							B	
Approach LOS				F		1		

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	TW [,]	O-WAY STOP	CONTRO	OL SUM	IMARY			ŀ		
General Information	K. Garage		Site Ir	nformati	ion					
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 12:00 PM					City of Los	Matilija Ave-Riverside Dr City of Los Angeles Existing With Project			
	53606-1/Westfield	Fashion Square								
East/West Street: Riversi					et: <i>Matilija Di</i>	r-Project Dri	iveway			
Intersection Orientation:	THE REPORT OF THE PROPERTY OF	An electrical representation of the project of the company of the	Study P	Period (hrs	s): 0.25		Note that the second se			
Vehicle Volumes and	d Adjustments									
Major Street		Eastbound				Westbou	nd r			
Movement	1	2	3		4	5		6		
\(\frac{1}{2}\)	L L	T 751	R 169		657	T 880		R 9		
Volume (veh/h) Peak-Hour Factor, PHF	1.00	751 1.00	1.00		1.00	880 1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	0	751	169		657	880		9		
Percent Heavy Vehicles	0				0					
Median Type				 Undivide						
RT Channelized			0		1			0		
Lanes	0	2	1		1	2		0		
Configuration		T	R		Ĺ	T		TR		
Upstream Signal		0				0				
Minor Street		Northbound				Southbou	ınd			
Movement	7	8	9		10	11		12		
	L	Т	R		L	Т		R		
Volume (veh/h)	39		487					22		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	39	0	487		0	0		22		
Percent Heavy Vehicles	0	0	0		0	0		0		
Percent Grade (%)		0				0				
Flared Approach		N				N				
Storage		0				0				
RT Channelized			0					0		
Lanes	1	0	1		0	0		1		
Configuration	<u>L</u>		R					R		
Delay, Queue Length, an	1		1				A. Carlotte			
Approach	Eastbound	Westbound	[!	Northboun	nd	[S	Southbound	d		
Movement	1	4	7	8	9	10	11	12		
Lane Configuration	1	L	L		R			R		
v (veh/h)		657	39		487			22		
C (m) (veh/h)	1	750	4		627			567		
v/c		0.88	9.75		0.78			0.04		
95% queue length		10.93	6.59		7.35			0.12		
Control Delay (s/veh)		33.6	5671		27.8	<u> </u>		11.6		
LOS		D D	F		D D			B		
Approach Delay (s/veh)				446.2			11.6			
Approach Delay (3/Ven)			1	770.2		1	11.0			

Approach LOS

В

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F

	TW	O-WAY STOP	CONTR	OL SU	MMARY						
General Information			Site li	nforma	ition						
Analyst FSB Intersection Matilija Ave-Riverside Dr											
Agency/Co.	LLG Engine	eers	1	Jurisdiction			City of Los Angeles				
Date Performed	5/7/2008				Analysis Year			Existing With Project			
Analysis Time Period	1:00 PM					~	-				
Project Description 1-08	53606-1/Westfield	l Fashion Square	Project								
East/West Street: Rivers			North/S	outh Str	eet: <i>Matilija l</i>	Dr-Project Di	riveway				
Intersection Orientation:	East-West		Study F	Period (h	rs): <i>0.2</i> 5						
Vehicle Volumes and	d Adjustments	S				21 27					
Major Street		Eastbound				Westbou	nd				
Movement	11	2	3		4	5		6			
	L	T	R		L	T		R			
Volume (veh/h)	1.20	800	98		598	863		14			
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00			
Hourly Flow Rate, HFR (veh/h)	0	800	98		598	863		14			
Percent Heavy Vehicles	0				0						
Median Type				Undivi			I				
RT Channelized			0					0			
Lanes	0	2	1		1	2		0			
Configuration		T	R		L	T		TR			
Upstream Signal		0				0					
Minor Street		Northbound				Southbou	ınd				
Movement	7	8	9		10	11					
	L	Т	R		L	Т		R			
Volume (veh/h)	46		569					19			
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00			
Hourly Flow Rate, HFR (veh/h)	46	0	569		0	0		19			
Percent Heavy Vehicles	0	0	0		0	0		0			
Percent Grade (%)		0				0					
Flared Approach		N				N					
Storage		0				0					
RT Channelized			0					0			
Lanes	1	0	1		0	0		1			
Configuration	L		R					R			
Delay, Queue Length, an	d Level of Service	ce.									
Approach	Eastbound	Westbound		Northbo	und	5	Southbou	ınd			
Movement	1	4	7	8	9	10	11	12			
Lane Configuration		L	L		R			R			
v (veh/h)		598	46		569			19			
C (m) (veh/h)		765	6		605			572			
v/c		0.78	7.67		0.94			0.03			
95% queue length		7.82	7.35		12.53			0.10			
Control Delay (s/veh)		24.4	4183		49.5			11.5			
LOS		С	F		E			В			
Approach Delay (s/veh)			•	358.7			11.5				
Approach LOS				550.7 F			B				
Capuriant @ 2005 University of Flor							U C	12/2000 1 00 DV			

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	TV	O-WAY STOP	CONTRO)L SU	JMMARY			
General Information			Site In	ıform	ation	1000		
Analyst Agency/Co.	FSB LLG Engin		Intersed Jurisdic			_	ve-Riversid s Angeles	'e Dr
Date Performed	5/7/2008		Analysis	s Year			Vith Project	t
Analysis Time Period	2:00 PM							
		d Fashion Square						
East/West Street: Riversi					treet: <i>Matilija l</i>	Dr-Project Dr	<u>iveway</u>	
Intersection Orientation:			Study Pe	eriod (I	hrs): 0.25	entesta instrumenta in the second second second second second second second second second second second second	over the second consistency to gave	
Vehicle Volumes and	d Adjustment		1. 18. 19.					
Major Street		Eastbound	1			Westbou	nd	
Movement	1 1	2 T	3 R		4 L	5 T		6
Volume (veh/h)		872	106		L 531	854		R 11
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR								
(veh/h)	0	872	106		531	854		11
Percent Heavy Vehicles	0				0			
Median Type				Undiv	⁄ided			
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		L	T		TR
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	ind	
Movement	7	8	9		10	11		12
	L L	T	R		L	T		R
Volume (veh/h)	43		536			1		10
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	43	0	536		0	0		10
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		1
Configuration	L		R					R
Delay, Queue Length, an								
Approach	Eastbound	Westbound		Vorthbo	ound	5	Southbound	<u>t</u>
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			R
v (veh/h)		531	43		536			10
C (m) (veh/h)		714	7		574			577
v/c		0.74	6.14	-	0.93			0.02
95% queue length		6.73	6.85		12.00			0.05
Control Delay (s/veh)		23.2	3338	***************************************	49.8			11.3
LOS		C C	F		F			B B
Approach Delay (s/veh)				294.0			11.3	
Approach LOS				F	<u>U</u>	1	B	
Approacticos								

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	TW	O-WAY STOF	CONTRO	OL SUN	//MARY				
General Information			Site In	nformat	tion				
Analyst	FSB	Interse	ction	of the filment part of the control o	Matilija Ave-Riverside Dr				
Agency/Co.	LLG Engineers		Jurisdi			City of Los Angeles			
Date Performed	5/7/2008		Analysis `			-	Vith Projed	1	
Analysis Time Period	3:00 PM		-			j			
Project Description 1-08	53606-1/Westfield	d Fashion Square	Project						
East/West Street: Rivers	ide Drive		North/S	outh Stre	eet: <i>Matilija D</i>	r-Project Dr	iveway		
Intersection Orientation:	East-West				rs): <i>0.25</i>				
Vehicle Volumes and	d Adjustment	S							
Major Street	Mail 1979 9 277022 (277) 1 Aug 12 (198)	Eastbound	Westboun			nd	the Balling and the state of th		
Movement	1	2	3		4	5		6	
	L	Т	R		L	Т		R	
Volume (veh/h)		864	104		546	1107		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
Hourly Flow Rate, HFR (veh/h)	0	864	104		546	1107		15	
Percent Heavy Vehicles	0				0				
Median Type			· · · · · · · · · · · · · · · · · · ·	Undivid	led				
RT Channelized			0					0	
Lanes	0	2	1 1		1	2		0	
Configuration		T	R		L	T		TR	
Upstream Signal		0				0			
Minor Street		Northbound		So			Southbound		
Movement	7	8	9		10	11		12	
	<u> </u>	Т	R		L	Т		R	
Volume (veh/h)	44		551					34	
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
Hourly Flow Rate, HFR (veh/h)	44	0	551		0	0		34	
Percent Heavy Vehicles	0	0	0		0 0			0	
Percent Grade (%)		0				. 0			
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	1	0	1		0	0		1	
Configuration	L		R					R	
Delay, Queue Length, an	id Level of Servi	ce							
Approach	Eastbound	Westbound	Northbound		nd	S	Southboun	nbound	
Movement	1	4	7	8	9	10	11	12	
Lane Configuration		L	L		R			R	
v (veh/h)		546	44		551			34	
C (m) (veh/h)		720	5		577			476	
v/c		0.76	8.80	•	0.95			0.07	
95% queue length		7.10	7.17		12.84			0.23	
Control Delay (s/veh)		23.9	4915		53.9			13.1	
LOS		C	F		F			В	
Approach Delay (s/veh)			,	413.4		13.1			
Approach LOS				F			B		
Approach Co.	da All Diel i D			Γ*			D	10000	

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	TW	O-WAY STOP	CONTRO	OL SUMI	MARY				
General Information			Site ir	nformatio	on				
Analyst Agency/Co. Date Performed Analysis Time Period	FSB	FSB LLG Engineers 5/7/2008				Matilija Ave-Riverside Dr City of Los Angeles Existing With Project			
		d Fashion Square		·					
East/West Street: Riversi					et: <i>Matilija D</i>)r-Project Dr	iveway		
Intersection Orientation:	Programme and the second secon		Study P	Period (hrs)	s): 0.25				
Vehicle Volumes and	J Adjustments								
Major Street		Eastbound				Westbou	nd		
Movement	1 1	2	3		4	5		6	
	<u> </u>	T	R 110		L	T 1002		R	
Volume (veh/h)	1.00	886	110		524	1093		17	
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	1.00	1.00	1.00		1.00	1.00		1.00	
(veh/h)	0	886	110		524	1093		17	
Percent Heavy Vehicles	0				0			***	
Median Type				Undivide	<u>id</u>				
RT Channelized			0					0	
Lanes	0	2	1		1	2		0	
Configuration		T	R		L	T		TR	
Upstream Signal		0				0			
Minor Street		Northbound				Southbou	und		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
Volume (veh/h)	40		506					22	
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
Hourly Flow Rate, HFR (veh/h)	40	0	506		0	0		22	
Percent Heavy Vehicles	0	0	0	0 0		0		0	
Percent Grade (%)		0							
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	1	0	1		0	0		1	
Configuration	L		R					R	
Delay, Queue Length, an	d Level of Servi						173 173 On 173 173 173		
Approach	Eastbound	Westbound	11	Northbound		\subseteq	Southbound	d	
Movement	1	4	7	8	9	10	11	12	
Lane Configuration		<u>L</u> .	L	f	R			R	
v (veh/h)		524	40	i	506			22	
C (m) (veh/h)		703	6	í	568			480	
v/c		0.75	6.67	í	0.89	1		0.05	
95% queue length		6.75	6.54	i	10.43			0.14	
Control Delay (s/veh)		23.5	3731	i	42.8	1		12.9	
LOS		С	F		E			В	
Approach Delay (s/veh)				313.0			12.9		

Approach LOS

F

В

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	TW	O-WAY STOP	CONTRO	OL SU	MMARY						
General Information			Site Ir	nforma	ation .						
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engineers 5/7/2008 5:00 PM		Jurisdio	Intersection Jurisdiction Analysis Year			Matilija Ave-Riverside Dr City of Los Angeles Existing With Project				
· · · · · · · · · · · · · · · · · · ·	3606-1/Westfield	Fashion Square									
East/West Street: Riverside Drive				North/South Street: Matilija Dr-Project Driveway							
Intersection Orientation: I	East-West		Study P	eriod (h	nrs): <i>0.25</i>						
Vehicle Volumes and Adjustments											
Major Street		Eastbound					Westbound				
Movement	1	2	3		4		5		6		
, , , , , , ,	L	T	R		L	\rightarrow	T 1007		R		
Volume (veh/h)	1.00	854	110		567		1207		21		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00		1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	0	854	110		567		1207		21		
Percent Heavy Vehicles	0				0		B-1- 8-1-				
Median Type				Undivi	ided	т					
RT Channelized			0			_			0		
Lanes	0	2	1		1		2		0		
Configuration		T	R		L		<u>T</u>		TR		
Upstream Signal		0					0				
Minor Street		Northbound					Southbound				
Movement	7	8	9		10		11		12		
	<u> </u>	T	R		L		Т		R		
Volume (veh/h)	43		536				4.00		21		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	0 1.00			1.00		
Hourly Flow Rate, HFR (veh/h)	43	0	536		0		0		21		
Percent Heavy Vehicles	0	0	0		0				0		
Percent Grade (%)		0					0				
Flared Approach		N					N				
Storage		0					0				
RT Channelized			0						0		
Lanes	1	0	1		0		0		1		
Configuration	L		R						R		
Delay, Queue Length, and	d Level of Servic	e									
Approach	Eastbound	Westbound	!	Northbo	und		S	outhbour	ıd		
Movement	1	4	7	8	9		10	11	12		
Lane Configuration		L	L		R				R		
v (veh/h)		567	43		536				21		
C (m) (veh/h)		722	4		582				440		
v/c		0.79	10.75		0.92				0.05		
95% queue length		7.82	7.13	l	11.59				0.15		
		25.7	6126	-	47.1				13.6		
Control Delay (s/veh)				 							
LOS		D	F	:22 /	E			10.0	В		
Approach Delay (s/veh)			498.5		5		13.6				
Approach LOS			F				В				

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	TV	O-WAY STOP	ONTR	JL SUI	MMARY				
General Information			Site Ir	nforma	ition	anger and the second			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 6:00 PM	eers	Interse Jurisdic Analysi			Matilija Ave-Riverside Dr City of Los Angeles Existing With Project			
	053606-1/Westfield	J Fashion Square	Project						
East/West Street: Rivers			North/S	outh Str	eet: <i>Matilija D</i>	r-Project Dr	iveway		
Intersection Orientation:	East-West		Study P	'eriod (h	rs): <i>0.25</i>				
Vehicle Volumes an	d Adjustment								
Major Street		Eastbound				Westbou	nd		
Movement		2	3		4	5		6	
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	LL	T 772	R 104		L 400	T 1450		R	
Volume (veh/h) Peak-Hour Factor, PHF	1.00	773	104		499	1156		4	
Hourly Flow Rate, HFR	1.00		1.00		1.00	1.00		1.00	
(veh/h)	0	773	104		499	1156		4	
Percent Heavy Vehicles	0				0			***	
Median Type			Miles and the second	Undivid	ded				
RT Channelized			0					0	
Lanes	0	2	1		1	2		0	
Configuration		Τ	R		L	T		TR	
Upstream Signal		0				0			
Minor Street		Northbound				Southbou	ınd		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
Volume (veh/h)	43		538					11	
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
Hourly Flow Rate, HFR (veh/h)	43	0	538		0	0		11	
Percent Heavy Vehicles	0	0	0		0	0		0	
Percent Grade (%)		0				0			
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	1	0	1		0	0		1	
Configuration	L		R					R	
Delay, Queue Length, an	<u> </u>								
Approach	Eastbound	Westbound	11	Northbou	bnı	S	Southboun	ıd	
Movement	1	4	7	8	9	10	11	12	
Lane Configuration		L	L		R			R	
v (veh/h)		499	43		538			11	
C (m) (veh/h)		779	9		618			463	
v/c	1.	0.64	4.78		0.87			0.02	
95% queue length		4.71	6.67		10.06			0.07	
Control Delay (s/veh)		17.5	2513		37.6			13.0	
LOS		С	F		E			B	
Approach Delay (s/veh)				220.8			13.0		
Approach LOS				F			B		
Approdon 200	<u></u>		L			L			

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	TV	VO-WAY STOP	CONTF	OL S!	UMN	/IARY				
General Information			Site	Inform	natio	on .	1776			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engin 5/7/2008 11:00 AM		Inters Jurisd	section diction vsis Year		Bandarana Ana	Project D City of Lo Existing V	s Angele	es	Dr
		d Fashion Square	Project							
East/West Street: Rivers				South 5	Street	: Project [Driveway (Tu	 ınnel)	****	
Intersection Orientation:	East-West			Period (
Vehicle Volumes and	d Adjustment	S								
Major Street		Eastbound				2271/22-12-11-1	Westbou	ınd	ASSESSMENT	DER CONTRACTOR OF THE PARTY OF
Movement	1	2	3			4	5			6
	L	T	R			L	Т		F	R
Volume (veh/h)	4**************************************	683	114		ļ	364	786			
Peak-Hour Factor, PHF	1.00	1.00	1.00	<u>)</u> '	<u> </u>	1.00	1.00		1.0	<i>)</i> 0
Hourly Flow Rate, HFR (veh/h)	0	683	114	<i>1</i>		364	786		0)
Percent Heavy Vehicles	0					0	***			
Median Type					ivided	1				
RT Channelized			0						0)
Lanes	0	2	1			1	2		0)
Configuration		Т	R			L	T			
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	und		
Movement	7	8	9			10	11		1	12
	L	Т	R			L	Т		F	R
Volume (veh/h)	51		150							
Peak-Hour Factor, PHF	1.00	1.00	1.00	<u>J</u>		1.00	1.00		1.0	JO
Hourly Flow Rate, HFR (veh/h)	51	0	150)		0	0		0)
Percent Heavy Vehicles	0	0	0			0	0		0)
Percent Grade (%)		. 0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						0)
Lanes	1	0	1			0	0		0	
Configuration	L		R	1						
Delay, Queue Length, an	nd Level of Servi	ice	100 mg 10							Zijane z
Approach	Eastbound	Westbound		Northbo	ound	13392 400 993 7 400 400	5	Southbou	und	Assessment and the second
Movement	1	4	7	8		9	10	11		12
Lane Configuration	1	L	L	 		R			-	
v (veh/h)		364	51	+		150				
C (m) (veh/h)		834	41			660		 		
v/c		0.44	1.24	1				 		
				-		0.23				***************************************
95% queue length	<u> </u>	2.24	5.04		\longrightarrow	0.87		 		
Control Delay (s/veh)	<u> </u>	12.6	376.1			12.1		<u> </u>		
LOS	<u> </u>	В	F			В				
Approach Delay (s/veh)				104.	.4					
Approach LOS				F						

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	TW	O-WAY STOP	CONTRO	OL SUMI	MARY			
General Information			Site Ir	nformatio	on			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 12:00 PM	ers	Intersed Jurisdic Analysi	ction		Project Dwy - Riverside Dr City of Los Angeles Existing With Project		
Project Description 1-05		Fashion Square	······································					
East/West Street: Riversi					et: Project D	riveway (Τι	ınnel)	,
Intersection Orientation:			Study P	eriod (hrs)	<u>): 0.25</u>		- Oracle in the Control of the Contr	
Vehicle Volumes and	d Adjustments	^~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~						
Major Street		Eastbound				Westbou	ınd	
Movement	1 1	2 T	3 R		<u>4</u>	5 T		6 R
Volume (veh/h)	l	732	114		364	849		K
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR								
(veh/h)	0	732	114		364	849		0
Percent Heavy Vehicles	0				0			
Median Type				Undivide	:d			
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	und	
Movement	7	8	9		10	11		12
	L	T	R		L	Т		R
Volume (veh/h)	56	1.00	164			1.00		1.00
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	56	0	164		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		0
Configuration	L		R					
Delay, Queue Length, an	7							Turky allaharak 1
Approach	Eastbound	Westbound	 	Northboun	•		Southbou	
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	56		164			
C (m) (veh/h)		800	35		637			
v/c		0.46	1.60		0.26			
95% queue length	1	2.40	6.08		1.02			
Control Delay (s/veh)		13.2	546.6		12.6		1	
LOS		B	F F		B	<u> </u>	1	
Approach Delay (s/veh)			'	148.5				
Approach LOS				F				
Approacticos			1	I		<u></u>		

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	TW	O-WAY STOP	, CONTRC	L SUI	MMARY			
General Information			Site In	forma	ition			
Analyst	FSB	Mari, 1941	Intersec			Project Dv	-	I 1
Agency/Co.	LLG Engine	eers	Jurisdict			City of Los	_	
Date Performed	5/7/2008		Analysis	s Year		Existing W	√ith Projec	ot [
Analysis Time Period	1:00 PM							
	53606-1/Westfield	Fashion Square						
East/West Street: Riversi					reet: Project D	riveway (Tui	nnel)	
Intersection Orientation:	Conjugate State State (Conjugate Sta		Study Pe	eriod (n	nrs): 0.25	The second second second Secon	The second secon	
Vehicle Volumes and	d Adjustments							
Major Street		Eastbound				Westbour	<u>nd (</u>	
Movement		2	3		4	5		6
I I I II I	L	T 780	R 114		364	T 827		R
Volume (veh/h) Peak-Hour Factor, PHF	1.00	789 1.00	114		364 1.00	827 1.00		1.00
Hourly Flow Rate, HFR								
(veh/h)	О	789	114		364	827		0
Percent Heavy Vehicles	0				0			
Median Type				Undivi	ded			
RT Channelized		<u> </u>	0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	nd	
Movement	7	8	9		10	11		12
	L	T	R		L	Т		R
Volume (veh/h)	71		204			1.00		
Peak-Hour Factor, PHF	1.00	1.00	1.00	-+	1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	71	0	204		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		0
Configuration	L		R			and the state of t	The second secon	- Andrews Angeleder Charles
Delay, Queue Length, an	7				100000000000000000000000000000000000000			
Approach	Eastbound	Westbound	-	Vorthbou		- 	Southboun	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<u> </u>	L	L		R		<u></u>	
v (veh/h)		364	71		204			
C (m) (veh/h)	<u> </u>	761	31		611		<u></u>	
v/c		0.48	2.29		0.33		<u></u>	
95% queue length		2.61	8.23		1.46			
Control Delay (s/veh)		14.0	863.1		13.8			
LOS		В	F		В			
Approach Delay (s/veh)				233.1	ı			
Approach LOS				F				
 								,

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	TΝ	O-WAY STOP	CONTRO	DL SUN	//MARY			
General Information			Site In	format	tion			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engin 5/7/2008 2:00 PM	eers	Intersed Jurisdic Analysi	tion		Project Dv City of Los Existing W	Angeles	
Project Description 1-08		l Fashion Square						
East/West Street: Rivers					et: Project D	riveway (Tui	nnel)	:
Intersection Orientation:			Study P	eriod (hr	s): 0.25			
Vehicle Volumes and	d Adjustment	and an experience and desired each at the section of the contract of the contract of						
Major Street		Eastbound	7		A	Westbour	nd T	
Movement	1	2 T	3 R		4 L	5 T		6 R
Volume (veh/h)	L	853	114		364	813		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	0	853	114		364	813		0
Percent Heavy Vehicles	0				0	****		
Median Type				Undivid	led			
RT Channelized			0					0
Lanes	0	2	1		1	2 .		0
Configuration		T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	nd	
Movement	7	8	9		10	11		12
	L	Т	R		L	Т		R
Volume (veh/h)	65		188					
Peak-Hour Factor, PHF	1.00	1.00	1.00	-	1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	65	0	188		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0	1			0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	11		0	0		0
Configuration	L		R				rosetta kostini rito-kiitaa siisee r	
Delay, Queue Length, an				ا بالمديد ا		T	41- 1	_
Approach	Eastbound	Westbound		Northbou		 	outhboun	
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	65		188			
C (m) (veh/h)		720	27	· · · · · · · · · · · · · · · · · · ·	582			
v/c		0.51	2.41		0.32			
95% queue length		2.88	7.85		1.39			
Control Delay (s/veh)		15.0	949.7		14.1		_	
LOS		В	F		В			
Approach Delay (s/veh)				254.5				
Approach LOS				F				
				TM				10000

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	TV	VO-WAY STOP	CONTRO	OL SUM	MMARY			
General Information			Site Ir	ıforma	tion			1,016,25,25
Analyst	FSB		Interse	ction		Project Di	wy - Rive	rside Dr
Agency/Co.	LLG Engir	neers	Jurisdio	ction		City of Lo	_	
Date Performed	5/7/2008		Analys	is Year		Existing V	Vith Proje	ect
Analysis Time Period	3:00 PM							
		d Fashion Square						
East/West Street: Rivers					eet: <i>Project D</i>	Priveway (Tu	nnel)	
Intersection Orientation:	East-West		Study F	eriod (h	rs): <i>0.25</i>			
Vehicle Volumes and	d Adjustment	S		Subjects				
Major Street		Eastbound				Westbou	nd	
Movement	1	2	3		4	5		6
	L	T	R		L	T		R
Volume (veh/h)		852	114		364	1086		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	0	852	114		364	1086		0
Percent Heavy Vehicles	0				0			
Median Type				Undivid	ded	T	· · · · · · · · · · · · · · · · · · ·	
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	ınd	
Movement	7	8	9		10	11		12
	L	Т	R		L	Т		R
Volume (veh/h)	67		196					
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	67	0	196		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				. 0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		0
Configuration	L		R		-			
Delay, Queue Length, an	nd Level of Serv	ice		Telling to				
Approach	Eastbound	Westbound		Vorthbou	und	9	Southbou	nd
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	67		196			
C (m) (veh/h)		721	22		582			
v/c		0.50	3.05		0.34			
95% queue length		2.87	8.56		1.48			
Control Delay (s/veh)		15.0	1289		14.3			
LOS		В	F		В			
Approach Delay (s/veh)			,	339.1			<u> </u>	
Approach LOS				F				
]			I					

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	ΤV\	O-WAY STOP	CONTRO	OL SUM	MARY			
General Information			Site In	ıformati	on .			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engin 5/7/2008 4:00 PM		Interse Jurisdid Analysi	ction		Project Dwy - Riverside Dr City of Los Angeles Existing With Project		
Project Description 1-05	3606-1/Westfield	d Fashion Square	Project	40.000				
East/West Street: Rivers					et: <i>Project D</i>	Driveway (Tu	nnel)	
Intersection Orientation:	East-West	W	Study F	eriod (hrs	s): <i>0.25</i>	AP-10760		
Vehicle Volumes and	d Adjustment						Jan San San	
Major Street		Eastbound				Westbou	nd	
Movement	1	2	3		4	5		6
Volume (voh/h)	L	T 869	R 114		364	T 1061		R
Volume (veh/h) Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR								
(veh/h)	0	869	114		364	1061		0
Percent Heavy Vehicles	0				0			
Median Type				Undivide	ed			
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration		T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound			,	Southbou	<u>ind</u>	
Movement	7	8 -	9		10	11		12
\	L	T	R		L	Т		R
Volume (veh/h) Peak-Hour Factor, PHF	59 1.00	1.00	174		1.00	1.00		1.00
Hourly Flow Rate, HFR	· · · · · · · · · · · · · · · · · · ·							
(veh/h)	59	0	174		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		0
Configuration	L		R					
Delay, Queue Length, an	d Level of Servi	ce						
Approach	Eastbound	Westbound	I	Vorthbour	nd	S	Southboun	d
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	59		174			
C (m) (veh/h)		711	21		576			
v/c		0.51	2.81		0.30			
95% queue length		2.95	7.64		1.27			
Control Delay (s/veh)		15.3	1202		13.9			
LOS		C	F	***************************************	13.9 B			
Approach Delay (s/veh)			<u>'</u>	314.8	<u>u</u>		L	
Approach LOS				514.0 F	····			
White and FOS				<i>F</i>				

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	TW	O-WAY STOR	CONTROL S	SUMMARY			
General Information			Site Infor	mation			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 5:00 PM	eers	Jurisdiction	Analysis Year Existing With F			
Project Description 1-053 East/West Street: Riversid		Fashion Square		Street: Project	Driveway (Tunnel)	
Intersection Orientation: E				d (hrs): 0.25	Divoway (rannor	/	
Vehicle Volumes and	Adjustments						
Major Street		Eastbound Westbound					
Movement	1	2	3	4	5	6	
	L	Т	R	L	Т	R	
Volume (veh/h)		850	114	364	1176		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	850	114	364	1176	0	
Percent Heavy Vehicles	0			0			
Median Type			Und	divided			
RT Channelized			0			0	
Lanes	0	2	1	1	2	0	
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street		Northbound Southbound					
Movement	7	8	9	10	11	12	
	L	Т	R	L	Т	R	
Volume (veh/h)	65		188				
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR	65	0	188	0	0	0	

Mcdiair Type			Onu	iviueu		
RT Channelized			0			0
Lanes	0	2	1	1	2	0
Configuration		T	R	L	T	
Upstream Signal		0			0	
Minor Street		Northbound			Southbound	
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	65		188			
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	65	0	188	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)		0			0	-
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			
Delay, Queue Length, and	Level of Servic	:e	enterior de la companya de la companya de la companya de la companya de la companya de la companya de la compa			

Delay, Queue Length, an	nd Level of Serv	rice						
Approach	Eastbound	Westbound		Northbound		S	outhbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	65		188			
C (m) (veh/h)		722	20		583			
v/c		0.50	3.25		0.32			
95% queue length		2.87	8.49		1.39			
Control Delay (s/veh)		14.9	1412		14.1			
LOS		В	F		В			
Approach Delay (s/veh)				373.2				
Approach LOS				F				
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	TW	O-WAY STOP	CONTRO	OL SU	MMARY			
General Information			Site In	ıform	ation			
Analyst Agency/Co. Date Performed Analysis Time Period	FSB LLG Engine 5/7/2008 6:00 PM	eers	Intersed Jurisdic Analysi	ction ction		Project Dw City of Los Existing W	s Angeles	s
	53606-1/Westfield	J Fashion Square	Project					
East/West Street: Riversi					treet: <i>Project</i> i	Driveway (Tur	nnel)	
Intersection Orientation:	East-West		Study P	<u>'eriod (</u> l	hrs): <i>0.25</i>			
Vehicle Volumes and	d Adjustments	S						
Major Street		Eastbound				Westbour	nd	
Movement		2	3		4	5		6
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	<u> </u>	T	R		L	T 4447		R
Volume (veh/h)	1.00	757 1.00	114		364 1.00	1117		1.00
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	1.00				1.00	1.00		
(veh/h)	0	757	114		364	1117		0
Percent Heavy Vehicles	0	ana sua			0			
Median Type		 		Undiv	ided			
RT Channelized			0					0
Lanes	0	2	1		1	2		0
Configuration	<u> </u>	. . T	R		L	T		
Upstream Signal		0				0		
Minor Street		Northbound				Southbour	nd	
Movement	7	8	9		10	11		12
	L	Т	R		L	T		R
Volume (veh/h)	65		189					
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	65	0	189		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		0				0		
RT Channelized			0					0
Lanes	1	0	1		0	0		0
Configuration	<u> </u>		R					
Delay, Queue Length, an	id Level of Servi	Ce						
Approach	Eastbound	Westbound	<u> </u>	Northbo	ound	S	outhbour	nd
Movement	1	4	7	8	9	10	11	12
Lane Configuration		L	L		R			
v (veh/h)		364	65		189			
C (m) (veh/h)	i l	783	27		625		1	
v/c	i	0.46	2.41		0.30		1	
95% queue length	1	2.49	7.85		1.27			
Control Delay (s/veh)	1	13.5	949.7		13.2			
LOS	1	B	F		B			
	1			252.				
Approach LOS				252.S F	9			
Approach LOS			1					

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	APPENDIX F
24-Houp I	MACHINE COUNTS

ACCUTEK 21114 TRIGGER LANE DIAMOND BAR, CA 91765 PHONE (909) 595-6199

Site Code : 110135550101 Start Date: 11/16/2005 File I.D. : 35551101NS Page : 1

							PHONE	(909)	595-619	19			Page
Street name	:Ranchit		n/o Riv							اممم الماسي		>	Wednesday
2092	(NB		><-		SB		><-		morned	P.M.		wednesday
Time	A.M.		P.M.		A.M.		P.M.		A.M. 2		22		
12:00 11/16	1		12		1		10		2		17		
12:15	0		11		1		6		1		24		
12:30	1		16		0	_	В	20	i č	6	14	77	
12:45	2	4	8	47	0	2	6	30	2	Ö		, ,	
01:00	0		16		0		9		0		25		
01:15	1		7		1		16		1 2		23		
01:30	1		13		1		13	_	2	_	26	0.0	
01:45	1	3	6	42	0	2	9	47	1	5	15	89	
02:00	0		18		0		19		0		37		
02:15	0		14		0		9		0		23		
02:30	1		15		1		12		2		27		
02:45	1	2	23	70	0	1	11	51	1	3	34	121	
03:00	0		12		0		12		(0		24		
03:15	ī		18		1		13		1 2		31		
03:30	1		15		0		12		1		27		
03:45	0	2	24	69	2	3	19	56	1 2	5	43	125	
04:00	ŏ	_	9		1		13		1		22		
04:15	2		15		1		10		3		25		
04:30	Õ		14		2		6		j 2		20		
04:45	2	4	12	50	0	4	6	35	j 2	8	18	85	
	2	*2	14	50	1 3	7	9		5		23		
05:00	ő		22		1 0		10		i õ		32		
05:15	1		22		1 0		12		1 1		34		
05:30	1	4		7.4	1 1	4	12	43		8	28	117	
05:45		4	16	74	1 4	4	10	43	1 8	0	25	++.	
06:00	4		15				5		3		19		
06:15	0		14] 3		17		7		35		
06:30	1	_	18		1 6	20		20	11	29	23	102	
06:45	2	7	17	64	9	22	6	38	24	25	21	102	
07:00	4		14		20		7				22		
07:15	6		15		30		7		1 36		25		
07:30	4		18		31		7	2.2	35	3 7 3		96	
07:45	8	22	18	65	28	109	10	31		131	28 12	90	
00:00	6		10		30		2		36		20		
08:15	8		11		19		9		27				
08:30	3		13		17		3		20		16		
08:45	7	24	5	39	23	89	3	17	30	113	8	56	
09:00	6		10		13		9		19		19		
09:15	5		7		6		4		1 11		11		
09:30	7		5		11		2		18		7		
09:45	8	26	8	30	11	41	2 6	17	19	67	10	47	
10:00	5		8.		1 11				16		14		
10:15	14		5		15		3		29		8		
10:30	10		4		13		2		1 23		6		
10:45	9	38	4	21	1 7	46	2	13	16	84	6	34	
11:00	9		3		11		3		20		6		
11:15	13		2		16		0		29		2		
11:30	8		3		7		0		1 15		3		
11:45	13	43	2	10	10	44	0	3	23	87	2	13	
Totals	179		581		367		381		546		962		
Day Totals		760	=			748				1508			
Split %	32.7%		60.4%		67.2%		39.6%						
<u> </u>													
Peak Hour	11:00		05:15		07:15		01:15		07:15		03:00		
Volume	43		75		119		57		143		125		
P.H.F.	.82		.85		.95		.75		.99		.72		
	.02												

Site Code : 140235550101 Start Date: 11/16/2005 File I.D. : 35551402NS Page : 1

Street name	:Matilli	ia Ave.	n/o Ri	verside	Drive			,					Page
	(NB		><-		SB		><-	Cc	mbined		>	Wednesday
Time	A.M.		P.M.		A.M.		P.M.		A.M.		P.M.		
12:00 11/16	0	•	9		0		11		0		20		
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Totals	93		325	,	167		186		260		511		
Day Totals		418				353				771			
Split %	35.7%		63.6%		64.2%		36.4%						
Peak Hour	10:45		03:30		07:15		03:15		07:15		03:15		
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Street name :Matillija Ave. n/o Riverside Drive

APPENDIX J

EMPLOYMENT CALCULATIONS



HR&A ADVISORS, INC.

Economic Development, Real Estate Advisory & Public Policy Consultants

Privileged & Confidential Attorney-Client Work Product and/or Communication

March 3, 2008

Mr. Dwight Steinert Planning Associates, Inc. Suite 108 4040 Vineland Ave. Studio City, CA 91604

Re: Direct Employment Estimate for the Fashion Square Expansion Project

Dear Dwight:

At your request, HR&A Advisors has estimated the on-site employment associated with the proposed expansion of the Fashion Square regional shopping center in Sherman Oaks ("Project"). We are familiar with the project from analysis we prepared on the Project's potential to cause "urban decay."

Although there are rules of thumb that are often used to estimate employment of a development project, and a variety of studies of employee densities per land use, we believe that the most accurate estimate is one that is based on the statistical relationship between the specific financial characteristics of a project and the host economy in which it is situated. Accordingly, to estimate the Project's employment we ran the projected annual sales for the Project through the IMPLAN input-output model of the Los Angeles County economy and determined that the direct employment impact is 788 jobs. It should be noted that these include full-time and part-time jobs (not full-time equivalent jobs), which are characteristic of shopping center employment.

Input-output analysis is an economic impact modeling method for understanding the interactions among the industries in a local economy that result from investment in a new development project or planning initiative. In form, it resembles a giant matrix, or spreadsheet, in which the "inflows" of goods and services needed by an industry (i.e., the purchasing sectors) are the columns and the rows consist of the outputs, or selling sectors. This enables analysis of the specific sectors of an area's economy that are affected, and by how much, when a dollar's worth of investment, new employment or other measure of "final demand" is added to a particular sector or sectors. These inter-industry relationships can be expressed in terms of dollar impacts or employment impacts.

2800 28th Street, Suite 325, Santa Monica, California 90405 • Tel: 310.581.0900 • Fax: 310.581.0910

Los Angeles New York

IMPLAN¹ is a widely accepted model that HR&A and many other economics consultants and public agencies use to estimate the economic consequences of new investment in, or other changes to, a local or regional economy.² It explicitly accounts for impact "leakage," or the fact that not all economic impacts are necessarily experienced inside the geographic area under study. The IMPLAN model can be used to generate estimates of direct, indirect and induced employment, compensation (i.e., wages and benefits) and total economic output (i.e., a summary measure of all spending and economic activity), for both the construction phase of a project, and annually, once it has been completed and occupied. The indirect and induced impacts are often referred to collectively as the "multiplier effect" of direct impacts. The economy of a county is typically used in this analysis, because that scale of geography that is usually sufficient to capture all of the transactional flows between and among sectors of a local economy.

In this case, we entered the \$96 million in estimated annual retail sales and \$22 million in estimate annual restaurant sales from the Project as "final demand" into the IMLPAN model, and the model generated the direct and "multiplier effect" employment associated with that scale of retail and restaurant spending in the Los Angeles County economy. These annual sales values are based on analysis included in our report on the Project's potential "urban decay" impacts. The resulting direct employment is the number of jobs that would be expected at the Project site. Attachment A provides the sector-by sector details of the IMPLAN-generated direct employment estimate, which sums to 788 full-time and part-time jobs at the Project site.

Please contact me if you have any questions about this Project employment estimate.

Sincerely,

PAUL J. SILVERN, Partner

HR&A ADVISORS, INC. Page 2

¹ IMPLAN (**IM**pact Analysis for **PLAN**ning), a social accounting and impact analysis software program, was originally developed in 1979 by the U.S. Forest Service in cooperation with the Federal Emergency Management Agency and the U.S. Bureau of Land Management to assist the Forest Service in land and resource planning and management. The IMPLAN accounts closely follow the accounting conventions established for the U.S. economy and the rectangular format recommended by the United Nations. The model is now maintained and updated annually by the Minnesota IMPLAN Group, Inc.

² HR&A has previously used IMPLAN to analyze the economic impacts of a wide range of projects throughout southern California and elsewhere in the U.S., including large residential developments, high-rise office buildings, industrial projects, shopping centers, university buildings, and film and television studio campus expansions.

ATTACHMENT A Employment Estimate for the Fashion Square Expansion Project

	IMPLAN Industry Sector	Direct	Indirect	Induced	Total	Percentage	Cum. %
410	General merchandise stores	411.7	0.8	4.3	416.8	39.8%	39.8%
	Food services and drinking places	376.3	4.6	15.5	396.4	37.9%	77.6%
	Real estate	0.0	11.8	4.4	16.2	1.5%	79.2%
390	Wholesale trade	0.0	7.9	6.7	14.6	1.4%	80.6%
	Employment services	0.0	8.7	3.1	11.8	1.1%	81.7%
	Offices of physicians- dentists- and other health	0.0	0.0	8.6	8.6	0.8%	82.5%
	Management of companies and enterprises	0.0	6.3	1.1	7.3	0.7%	83.2%
	Hospitals Food and beverage stores	0.0 0.0	0.0 0.8	6.9 4.3	6.9 5.1	0.7% 0.5%	83.9% 84.4%
	Social assistance- except child day care services	0.0	0.0	5.0	5.0	0.5%	84.9%
	Private households	0.0	0.0	4.9	4.9	0.5%	85.3%
	Accounting and bookkeeping services	0.0	3.5	1.1	4.6	0.4%	85.8%
458	Services to buildings and dwellings	0.0	3.1	1.5	4.5	0.4%	86.2%
	Advertising and related services	0.0	4.0	0.6	4.5	0.4%	86.6%
	Nursing and residential care facilities	0.0	0.0	4.1	4.1	0.4%	87.0%
	Motor vehicle and parts dealers	0.0	0.6	3.3	3.9	0.4%	87.4%
	Colleges- universities- and junior colleges Nondepository credit intermediation and related a	0.0 0.0	0.3 2.8	3.5 0.9	3.8 3.7	0.4% 0.4%	87.8% 88.1%
	Management consulting services	0.0	2.7	0.9	3.5	0.4%	88.4%
	Legal services	0.0	1.6	1.7	3.3	0.3%	88.8%
	Hotels and motels- including casino hotels	0.0	1.7	1.5	3.2	0.3%	89.1%
	Couriers and messengers	0.0	2.6	0.5	3.2	0.3%	89.4%
394	Truck transportation	0.0	1.9	1.2	3.1	0.3%	89.7%
	Monetary authorities and depository credit interme	0.0	1.2	1.7	2.8	0.3%	89.9%
	Postal service	0.0	2.0	0.8	2.8	0.3%	90.2%
	Nonstore retailers	0.0	0.5	2.3	2.8	0.3%	90.5%
	Clothing and clothing accessories stores Miscellaneous store retailers	0.0 0.0	0.4 0.4	2.3 2.3	2.7 2.7	0.3% 0.3%	90.7% 91.0%
	Spectator sports	0.0	1.7	0.9	2.6	0.3%	91.0%
	Securities- commodity contracts- investments	0.0	0.9	1.6	2.5	0.2%	91.5%
	Performing arts companies	0.0	2.1	0.4	2.5	0.2%	91.7%
493	Civic- social- professional and similar organizati	0.0	0.7	1.8	2.5	0.2%	92.0%
457	Investigation and security services	0.0	1.9	0.6	2.5	0.2%	92.2%
	Child day care services	0.0	0.0	2.4	2.4	0.2%	92.4%
	Automotive repair and maintenance- except car was	0.0	0.5	1.8	2.3	0.2%	92.6%
	Business support services	0.0	1.7	0.6	2.3	0.2%	92.9%
	Insurance carriers Health and personal care stores	0.0 0.0	0.4 0.4	1.8 1.8	2.2 2.2	0.2% 0.2%	93.1% 93.3%
	Warehousing and storage	0.0	1.8	0.3	2.1	0.2%	93.5%
	Building material and garden supply stores	0.0	0.4	1.7	2.1	0.2%	93.7%
	Bread and bakery product- except frozen- manufactu	0.0	1.7	0.3	2.0	0.2%	93.9%
466	Other ambulatory health care services	0.0	0.0	2.0	2.0	0.2%	94.1%
	Other amusement- gambling- and recreation industri	0.0	0.1	1.7	1.8	0.2%	94.2%
	Cut and sew apparel manufacturing	0.0	0.0	1.8	1.8	0.2%	94.4%
	Other State and local government enterprises	0.0	0.8	0.9	1.7	0.2%	94.6%
	Maintenance and repair of nonresidential buildings Elementary and secondary schools	0.0 0.0	1.4 0.0	0.3 1.7	1.7 1.7	0.2% 0.2%	94.7% 94.9%
	Telecommunications	0.0	0.0	0.7	1.6	0.2%	95.1%
	Drycleaning and laundry services	0.0	0.8	0.8	1.6	0.2%	95.2%
	Personal care services	0.0	0.0	1.5	1.5	0.1%	95.4%
439	Architectural and engineering services	0.0	1.0	0.5	1.5	0.1%	95.5%
464	Home health care services	0.0	0.0	1.5	1.5	0.1%	95.6%
	Other educational services	0.0	0.1	1.3	1.4	0.1%	95.8%
	Sporting goods- hobby- book and music stores	0.0	0.2	1.1	1.3	0.1%	95.9%
	Furniture and home furnishings stores	0.0	0.2	1.1	1.3	0.1%	96.0%
	Fitness and recreational sports centers Motion picture and video industries	0.0	0.3 0.8	0.9 0.3	1.2 1.2	0.1%	96.1% 96.3%
	State and local government electric utilities	0.0 0.0	0.8	0.3	1.2	0.1% 0.1%	96.4%
	Office administrative services	0.0	0.8	0.3	1.1	0.1%	96.5%
	Other support services	0.0	0.8	0.3	1.1	0.1%	96.6%
	Computer systems design services	0.0	0.8	0.3	1.0	0.1%	96.7%
	Grantmaking and giving and social advocacy organiz	0.0	0.0	1.0	1.0	0.1%	96.8%
	Meat processed from carcasses	0.0	0.9	0.2	1.0	0.1%	96.9%
	Commercial printing	0.0	0.8	0.2	1.0	0.1%	97.0%
	Radio and television broadcasting	0.0	0.8	0.1	1.0	0.1%	97.1%
	Transit and ground passenger transportation	0.0	0.3	0.6	0.9	0.1%	97.1%
	Electronics and appliance stores	0.0	0.2	0.7	0.9	0.1%	97.2%
	Insurance agencies- brokerages- and related Gasoline stations	0.0 0.0	0.2 0.2	0.7 0.7	0.9 0.9	0.1% 0.1%	97.3% 97.4%
	Scenic and sightseeing transportation and support	0.0	0.5	0.7	0.8	0.1%	97.5%
	All Other Sectors	0.3	14.0	12.2	<u>26.5</u>	2.5%	100.0%
	Totals	788.3	111.9	147.0	1,047.3	100.0%	

Source: IMPLAN Pro ver. 2.0.1025; HR&A, Inc.

APPENDIX K

SUPPLEMENTAL TRAFFIC INFORMATION FOR PROJECT AND ALTERNATIVES

AVERAGE TRIP LENGTH CALCULATIONS TRAFFIC COUNT CONFIRMATIONS ALTERNATIVES ANALYSIS

APPENDIX K

APPENDIX K-1 AVERAGE TRIP LENGTH CALCULATIONS

Westfield Fashion Square Market Area Zip Codes

																												check	
		0.14	0.234	0.142	0.508	0.132	0.992	0.332	0.076	0.464	0.156	0.106	0.112	0.208	0.792	0.3	0.486	0.414	0.248	0.366	0.51	0.45	0.624	9.0	0.54	0.416	0.354	9.702	4.851
																												W. AVG	one-way
		14	23.4	14.2	50.8	13.2	99.2	33.2	7.6	46.4	15.6	10.6	11.2	20.8	79.2	30	48.6	41.4	24.8	36.6	51	45	62.4	09	54	41.6	35.4	9.702	4.851
																												W. AVG	one-way
	Roundfrip	14	23.4	14.2	25.4	13.2	12.4	16.6	3.8	5.8	7.8	10.6	5.6	2.6	8.8	15	16.2	13.8	12.4	12.2	10.2	6	7.8	12	10.8	5.5	11.8	11.56154	
0,0000000000000000000000000000000000000	Distance to Project (mi) Roundtrip	7	11.7	7.1	12.7	9.9	6.2	8.3	1.9	2.9	3.9	5.3	2.8	1.3	4.4	7.5	8.1	6.9	6.2	6.1	5.1	4.5	3.9	9	5.4	2.6	5.9	AVG 5.7807692	
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ioii oquale mainel Alea Lip Coues	ų.	West Hollywood adjacent	Brentwood/Brentwood Heights	3 Hollywood - Southeast of Universal City) Tarzana	I Van Nuys - near Victory & Van Nuys Blvd.	Sherman Oaks, southeast of the 405 & 101	5 Van Nuys - just north of 91401	3 Van Nuys - Van Nuys airport area	Van Nuys - southwest of Van Nuys Blvd. & Victory	Sherman Oaks- south of the 101	Sherman Oaks - Ventura Blvd. area	5 Burbank / Magnolia Park	3 Burbank - west of 91506	Burbank - Disney area	2 Burbank - east of 91521, where Barham turns into Olive	3 Burbank - north of the 134 / NBC area	North Hollywood - Vineland/Chandler	2 North Hollywood - Lankershim/134	Studio City- Laurel Canyon/Ventura Blvd.	i North Hollywood - south of Roscoe, east of the 170	North Hollywood - Lankershim/Victory Blvd.	7 Valley Village	3 Universal City	TOTAL	
Westileiu Fasilioli oquale Mai Net Alea Zip Coues	Zip Code		90049 Brentwood/Brentwood Heights	90068 Hollywood - Southeast of Universal City	Air along	90210 Beverly Hills	91316 Encino - near Woodley Ave. Park	91356 Tarzana	91401 Van Nuys - near Victory & Van Nuys Blvd.				Van Nuys - southwest	Sherman Oaks- south	Sherman Oaks - Veni		91506 Burbank - west of 91506		91522 Burbank - east of 91521, where Barham turns into Olive	91523 Burbank - north of the 134 / NBC area		91602 North Hollywood - Lankershim/134		91605 North Hollywood - south of Roscoe, east of the 170	91606 North Hollywood - Lankershim/Victory Blvd.	91607 Valley Village	91608 Universal City	TOTAL	

9.702

APPENDIX K

APPENDIX K-2 TRAFFIC COUNT CONFIRMATION

MEMORANDUM

To:	Dwight Steinert Planning Associates, Inc.	Date:	August 14, 2008
From:	David S. Shender, P.E. Francesca S. Bravo Linscott, Law & Greenspan, Engineers	LLG Ref:	1-053606-1
Subject:	Westfield Fashion Square Expansion Projec	t – Traffic	Count Comparison

This memorandum outlines our review of the traffic count data utilized in the traffic study and provides a summary of the results of the traffic count comparison conducted for the proposed Westfield Fashion Square Expansion Project.

Summary of Traffic Study Count Data

LLG Engineers prepared a revised traffic impact study (dated August 5, 2008) associated with the proposed Westfield Fashion Square Expansion Project. Manual traffic counts were conducted at the 17 study intersections during the weekday morning and afternoon commuter periods (7:00 to 10:00 AM and 3:00 to 6:00 PM) in November 2005 while local schools are in session. The traffic count data were then increased at a rate of 2.0 percent (2.0%) per year to reflect year 2007 existing conditions.

Summary of 2007 Count Data

Subsequent traffic counts at the 17 study intersections were conducted in November 2007 to determine if the original traffic counts used in the revised traffic impact study are reasonably consistent with the 2007 data and therefore remain appropriate for use in the traffic study. As shown in Table A, the 2007 traffic counts (as aggregated over the 17 study intersections) were 6.6 percent lower during the AM peak hour and 0.5 percent lower during the PM peak hour as compared to the adjusted 2005 traffic counts used in the traffic study. These differences are within the normal variations of peak hour traffic that can typically be expected on a day-to-day basis (i.e., variation of 10% or more can typically be anticipated). No substantial changes in the traffic volumes were observed and thus no new traffic counts or adjustments to the original counts were necessary.

In conclusion, the 2005 adjusted traffic counts are reasonably consistent with the 2007 counts, and fall within a reasonable range of variability. As such, the adjusted 2005 traffic counts used for the traffic study provided a reasonable representation of traffic volumes in the study area.

Please feel free to contact us with any questions or comments.

Attachment

Jonathan Krausche, Westfield Corporation
Ed Casey, Weston Benshoof Rochefort Rubalcava & MacCuish, LLP



Engineers & Planners

Traffic Transportation Parking

Linscott, Law & Greenspan, Engineers

236 N. Chester Avenue Suite 200 Pasadena, CA 91106 **626.796.2322** T 626.792.0941 F www.llgengineers.com

Pasadena Costa Mesa San Diego Las Vegas

Table A
COMPARISON OF 2005 TRAFFIC STUDY AND 2007 TRAFFIC COUNTS
Westfield Fashion Square Expansion Project

PEAK HOUR	[1] 2007 TRAFFIC VOLUMES	[2] 2007 TRAFFIC COUNTS	[2] - [1] VOLUME DIFFERENCE	PERCENT DIFFERENCE
AM	59,846	55,905	(3,941)	-6.6%
PM	64,192	63,856	(336)	-0.5%

^[1] Traffic counts taken at the 17 study intersections in November 2005 and increased by 2% per year to reflect year 2007 conditions.

^[2] Traffic counts taken at the 17 study intersections in November 2007.

APPENDIX K

APPENDIX K-3 ALTERNATIVES ANALYSIS

MEMORANDUM

To:	Dwight Steinert Planning Associates, Inc.	Date:	August 14, 2008
From:	David S. Shender, P.E. Francesca S. Bravo	LLG Ref:	1-053606-1
	Linscott, Law & Greenspan, Engineers		
Subject:	Westfield Fashion Square Expansion Project – Project Alternatives Review		

This memorandum has been prepared to summarize the project alternatives review conducted for the proposed Westfield Fashion Square Expansion Project. In accordance with CEQA Guidelines, each alternative is evaluated to determine whether the overall environmental impacts would be less, similar, or greater than the corresponding impacts of the project. The following alternatives to the proposed Westfield Fashion Square Expansion Project were evaluated:

- A. No Project Alternative
- B. Existing Entitlement Alternative
- C. Reduced Project 1 Alternative (Without Tunnel Access and Subterranean Parking)
- D. Reduced Project 2 Alternative (Without Tunnel Access and Subterranean Parking and With Matilija Avenue Closure at Riverside Drive)
- E. Alternate Site Plan 1 Alternative (Without Tunnel Access and Subterranean Parking)
- F. Alternate Site Plan 2 Alternative (With Pedestrian Mall Entrance)
- G. Open Air Promenade Alternative (Without Tunnel Access and Subterranean Parking)

Each of these alternatives are described and evaluated in the sections that follow. Detailed weekday trip generation forecasts for all of the project alternatives are contained in *Appendix A*. Level of Service and impact analyses were prepared for all study locations for Alternatives E and G to account for the alternative site access schemes for the proposed project. Copies of all supporting impact analyses for the project alternatives are also contained in *Appendix B*.

Alternative A: No Project

The Alternative A project description represents a no project, no development alternative. Alternative A project involves no development and the continued operation of the site (i.e., existing conditions or the status quo). The existing Westfield Fashion Square will continue to operate. The vehicular access associated with the Alternative A project will be consistent with the access currently provided for the site. As there is no change of use proposed under this alternative, no new trip generation is forecast.



Engineers & Planners

Traffic Transportation Parking

Linscott, Law & Greenspan, Engineers

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Pasadena Costa Mesa San Diego Las Vegas Dwight Steinert Planning Associates, Inc. August 14, 2008 Page 2



Alternative B: Existing Entitlement

Under Alternative B, the remaining of approximately 108,000 square feet of gross leasable floor area associated with the center's existing entitlement will be developed. The Alternative B project will include the construction of 108,000 square feet of commercial retail/restaurant space on two floors as an extension at the south end of the existing mall, east of the Bloomingdale's building. As part of the Alternative B project, two levels of parking would be added to the existing grade plus two-level southern parking structure on Hazeltine Avenue. In addition, a new grade plus three-level parking structure would be constructed located east of the Macy's building. The vehicular access associated with the Alternative B project will be consistent with the existing access scheme currently provided for the site.

Weekday Conditions

The weekday trip generation forecast for Alternative B is summarized in *Table A-1*. As presented in *Table A-1*, Alternative B is expected to generate a net increase of 37 vehicle trips (23 inbound trips and 14 outbound trips) during the weekday AM peak hour. During the weekday PM peak hour, Alternative B is expected to generate a net increase of 189 vehicle trips (91 inbound trips and 98 outbound trips). Over a 24-hour period, Alternative B project is forecast to generate a net increase of 1,974 daily trip ends during a typical weekday (987 inbound trips and 987 outbound trips).

A qualitative review was conducted to determine if the Alternative B project would likely result in an increase in project impacts when compared to the proposed project. During the weekday conditions, Alternative B project is expected to generate 58 fewer vehicle trips than the proposed project during the AM peak hour. During the PM peak hour, Alternative B project is expected to generate 287 fewer vehicle trips than the proposed project. Over a 24-hour period, Alternative B project is forecast to generate 2,990 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined that the Alternative B project would likely result in an overall decrease in traffic impacts during the weekday conditions when compared to the proposed project.

Weekend Conditions

The Saturday trip generation forecast for Alternative B is summarized in *Table A-2*. As presented in *Table A-2*, Alternative B is expected to generate a net increase of 250 vehicle trips (131 inbound trips and 119 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative 2 is forecast to generate a net increase of 2,488 daily trip ends during a typical Saturday (1,244 inbound trips and 1,244 outbound trips).

A qualitative review was conducted to determine if the Alternative B project would likely result in an increase in project impacts when compared to the proposed project. During the weekend conditions, Alternative B project is expected to generate 382 fewer vehicle trips than the proposed project during the Saturday mid-day peak hour. Over a 24-hour period, the Alternative B project is forecast to generate 3,764 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined

Dwight Steinert Planning Associates, Inc. August 14, 2008 Page 3



that the Alternative B project would likely result in an overall decrease in traffic impacts during the Saturday mid-day peak hour when compared to the proposed project.

Alternative C: Reduced Project 1

Under Alternative C, a reduced project alternative of 235,000 square feet of gross leasable floor area is proposed. The Alternative C project will include the construction of 235,000 square feet of commercial retail/restaurant space on two levels as an extension at the south end of the existing mall between the Bloomingdale's and Macy's buildings. A new grade plus five parking structure will be constructed extending east past the edge of the Macy's building. It should be noted that this alternative would require the demolition of the existing two-level Macy's parking structure to accommodate the new grade plus five parking structure. As part of the Alternative C project, vehicular access to the site will be provided via four project driveways: two existing driveways on Hazeltine Avenue, one existing driveway on Woodman Avenue, and one new driveway on Riverside Drive at Matilija Avenue (i.e., no tunnel access and subterranean parking). It is anticipated that the Alternative C project would be completed by year 2012.

Weekday Conditions

The weekday trip generation forecast for Alternative C is summarized in *Table B-1*. As presented in *Table B-1*, Alternative C is expected to generate a net increase of 79 vehicle trips (49 inbound trips and 30 outbound trips) during the weekday AM peak hour. During the weekday PM peak hour, Alternative C is expected to generate a net increase of 402 vehicle trips (193 inbound trips and 209 outbound trips). Over a 24-hour period, Alternative C project is forecast to generate a net increase of 4,198 daily trip ends during a typical weekday (2,099 inbound trips and 2,099 outbound trips).

A qualitative review was conducted to determine if the Alternative C project would likely result in an increase in project impacts when compared to the proposed project. During the weekday conditions, the Alternative C project is expected to generate 16 fewer vehicle trips than the proposed project during the AM peak hour. During the PM peak hour, the Alternative C project is expected to generate 74 fewer vehicle trips than the proposed project. Over a 24-hour period, the Alternative C project is forecast to generate 766 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined that the Alternative C project would likely result in an overall decrease in traffic impacts during the weekday conditions when compared to the proposed project.

Weekend Conditions

The Saturday trip generation forecast for Alternative C is summarized in *Table B-2*. As presented in *Table B-2*, Alternative C is expected to generate a net increase of 534 vehicle trips (278 inbound trips and 256 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative C is forecast to generate a net increase of 5,288 daily trip ends during a typical Saturday (2,644 inbound trips and 2,644 outbound trips).

Dwight Steinert Planning Associates, Inc. August 14, 2008 Page 4



A qualitative review was conducted to determine if the Alternative C project would likely result in an increase in project impacts when compared to the proposed project. During the weekend conditions, Alternative C project is expected to generate 98 fewer vehicle trips than the proposed project during the Saturday mid-day peak hour. Over a 24-hour period, the Alternative C project is forecast to generate 964 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined that the Alternative C project would likely result in an overall decrease in traffic impacts during the Saturday mid-day peak hour when compared to the proposed project.

It should be noted that although the Alternative C project is anticipated to result in an overall decrease in traffic impacts when compared to the proposed project, the contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation at seven study intersections as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection will be implemented as part of the Alternative C project.

Alternative D: Reduced Project 2 (With Matilija Avenue Closure at Riverside Drive)

Under Alternative D, a reduced project alternative of 235,000 square feet of gross leasable floor area is proposed. The Alternative D project will include the construction of 235,000 square feet of commercial retail/restaurant space on two levels as an extension at the south end of the existing mall between the Bloomingdale's and Macy's buildings. A new grade plus five parking structure will be constructed extending east past the edge of the Macy's building. It should be noted that this alternative would retain the existing two-level Macy's parking structure. As part of the Alternative D project, vehicular access to the site will be provided via four project driveways: two existing driveways on Hazeltine Avenue. one existing driveway on Woodman Avenue, and one new driveway on Riverside Drive at Matilija Avenue (i.e., no tunnel access and subterranean parking). addition, as part of the Alternative D project, it is proposed that Matilija Avenue be closed for vehicular traffic at Riverside Drive in conjunction with the new main access improvements to be constructed for the center opposite Riverside Drive. It is anticipated that the Alternative D project would be completed by year 2012.

Weekday Conditions

The weekday trip generation forecast for Alternative D is summarized in *Table C-1*. As presented in *Table C-1*, Alternative D is expected to generate a net increase of 79 vehicle trips (49 inbound trips and 30 outbound trips) during the weekday AM peak hour. During the weekday PM peak hour, Alternative D is expected to generate a net increase of 402 vehicle trips (193 inbound trips and 209 outbound trips). Over a 24-hour period, Alternative D project is forecast to generate a net increase of 4,198 daily trip ends during a typical weekday (2,099 inbound trips and 2,099 outbound trips).



A qualitative review was conducted to determine if the Alternative D project would likely result in an increase in project impacts when compared to the proposed project. During the weekday conditions, the Alternative D project is expected to generate 16 fewer vehicle trips than the proposed project during the AM peak hour. During the PM peak hour, the Alternative D project is expected to generate 74 fewer vehicle trips than the proposed project. Over a 24-hour period, the Alternative D project is forecast to generate 766 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined that the Alternative D project would likely result in an overall decrease in traffic impacts during the weekday conditions when compared to the proposed project.

Weekend Conditions

The Saturday trip generation forecast for Alternative D is summarized in *Table C-2*. As presented in *Table C-2*, Alternative D is expected to generate a net increase of 534 vehicle trips (278 inbound trips and 256 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative D is forecast to generate a net increase of 5,288 daily trip ends during a typical Saturday (2,644 inbound trips and 2,644 outbound trips).

A qualitative review was conducted to determine if the Alternative D project would likely result in an increase in project impacts when compared to the proposed project. During the weekend conditions, Alternative D project is expected to generate 98 fewer vehicle trips than the proposed project during the Saturday mid-day peak hour. Over a 24-hour period, the Alternative D project is forecast to generate 964 fewer daily trip ends during a typical weekday. Based on this comparison, it is determined that the Alternative D project would likely result in an overall decrease in traffic impacts during the Saturday mid-day peak hour when compared to the proposed project.

It should be noted that although the Alternative D project is anticipated to result in an overall decrease in traffic impacts when compared to the proposed project, the contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation at seven study intersections as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection will be implemented as part of the Alternative D project.

Alternative E: Alternate Site Plan 1 (Without Tunnel Access and Subterranean Parking)

The Alternative E project will include the construction of 280,000 square feet of commercial retail/restaurant space on two levels over one level of rooftop parking located south of the existing main mall. A new grade plus five parking structure will be constructed extending east past the edge of the Macy's building. In addition, a new east grade plus three-level parking structure along Woodman Avenue would be constructed. It should be noted that this alternative would require the demolition of the existing Macy's parking structure as well as the existing three-level parking



structure south of the main mall to accommodate the new parking structures. As part of the Alternative E project, vehicular access to the site will be provided via four project driveways: two existing driveways on Hazeltine Avenue, one existing driveway on Woodman Avenue, and one new driveway on Riverside Drive at Matilija Avenue (i.e., no tunnel access and subterranean parking). It is anticipated that the Alternative E project would be completed by year 2012.

Weekday Conditions

The weekday trip generation forecast for Alternative E is summarized in *Table D-1*. As presented in *Table D-1*, Alternative E would result in the same number of trips as the proposed project during the weekday conditions. Alternative E is expected to generate a net increase of 95 vehicle trips (58 inbound trips and 37 outbound trips) during the AM peak hour. During the PM peak hour, Alternative E is expected to generate a net increase of 476 vehicle trips (229 inbound trips and 247 outbound trips). Over a 24-hour period, Alternative E project is forecast to generate a net increase of 4,964 daily trip ends during a typical weekday (2,482 inbound trips and 2,482 outbound trips).

In order to determine the operating conditions of the street system in the year 2012 with the Alternative E project, traffic associated with the Alternative E project was assigned to the local roadway system based on an updated trip distribution and assignment characteristics without the proposed tunnel access on Riverside Drive. The updated project traffic volume distribution percentages during AM and PM peak hours at the 17 study intersections are illustrated in *Figure A*. The forecast Alternative E project traffic volumes at the study intersections for the AM and PM peak hours are displayed in *Figures B-1 and B-2*, respectively.

As shown in *Table D-2*, application of the City of Los Angeles' threshold criteria to the "With Alternative E Project" scenario indicates that six of the 17 study intersections are anticipated to be significantly impacted by the Alternative E project during the AM and PM peak hours. Incremental but not significant impacts are noted at the remaining 11 study intersections due to the Alternative E project. The future with Alternative E project (existing, ambient growth, related projects and Alternative E project) traffic volumes at the study intersections during the AM and PM peak hours are illustrated in *Figures C-1 and C-2*, respectively.

The six study intersections forecast to be significantly impacted by the Alternative E project are intersections forecast to be significantly impacted by the proposed project. The traffic mitigation measures recommended for the proposed project (i.e., the contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection) are anticipated to reduce the traffic impacts associated with the Alternative E project to less than significant levels at the six impacted study intersections.



Weekend Conditions

The weekend trip generation forecast for Alternative E is summarized in *Table D-3*. As presented in *Table D-3*, Alternative E would result in the same number of trips as the proposed project during the weekend conditions. As presented in *Table D-3*, Alternative E is expected to generate a net increase of 632 vehicle trips (329 inbound trips and 303 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative E is forecast to generate a net increase of 6,252 daily trip ends during a typical Saturday (3,126 inbound trips and 3,126 outbound trips).

In order to determine the operating conditions of the street system in the year 2012 with the Alternative E project, traffic associated with the Alternative E project was assigned to the local roadway system based on an updated trip distribution and assignment characteristics without the proposed tunnel access on Riverside Drive. The forecast Alternative E project traffic volumes at the six study intersections for the Saturday mid-day peak hour are displayed in *Figure D*.

As shown in *Table D-4*, application of the City of Los Angeles' threshold criteria to the "With Alternative E Project" scenario indicates that four of the six study intersections are anticipated to be significantly impacted by the Alternative E project during the Saturday mid-day peak hour. Incremental but not significant impacts are noted at the remaining two study intersections due to the Alternative E project. The future with Alternative E project (existing, ambient growth, related projects and Alternative E project) traffic volumes at the six study intersections during the Saturday mid-day peak hour are illustrated in *Figure E*.

The four study intersections forecast to be significantly impacted by the Alternative E project are intersections forecast to be significantly impacted by the proposed project during the Saturday mid-day peak hour. The traffic mitigation measures recommended for the proposed project (i.e., the contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection) are anticipated to reduce the traffic impacts associated with the Alternative E project during the Saturday mid-day peak hour to less than significant levels at the four impacted study intersections.

Alternative F: Alternate Site Plan 2 (With Pedestrian Mall Entrance)

Under Alternative F, it is proposed that a public pedestrian mall entrance be provided on Riverside Drive. This new mall entrance on Riverside Drive would be located west of the Macy's building and would provide additional landscape plaza improvements to enhance pedestrian activation at the new entrance. This alternative would consist of the construction of 280,000 square feet of gross leasable floor area on two levels over one level of rooftop parking located south of the existing main mall. A new grade plus five parking structure would be constructed south of the existing Macy's parking structure. In addition, a new east grade plus three-level parking structure along Woodman Avenue would be constructed. It should be noted



that this alternative does not include subterranean parking and would require the demolition of the existing Macy's parking structure as well as the existing three-level parking structure south of the main mall to accommodate the new parking structures. The vehicular access associated with the Alternative F project will be consistent with the proposed site access scheme provided for the site. It is anticipated that the Alternative F project would be completed by year 2012.

Weekday Conditions

The weekday trip generation forecast for Alternative F is summarized in *Table E-1*. As presented in *Table E-1*, Alternative F would result in the same number of trips as the proposed project during the weekday conditions. Alternative F is expected to generate a net increase of 95 vehicle trips (58 inbound trips and 37 outbound trips) during the AM peak hour. During the PM peak hour, Alternative F is expected to generate a net increase of 476 vehicle trips (229 inbound trips and 247 outbound trips). Over a 24-hour period, Alternative F project is forecast to generate a net increase of 4,964 daily trip ends during a typical weekday (2,482 inbound trips and 2,482 outbound trips).

A qualitative review was conducted to determine if the Alternative F project would likely result in an increase in project impacts when compared to the proposed project. The Alternative F project would result in the same number of trips as the proposed project. Based on this comparison, and since the vehicular access associated with the Alternative F project will be consistent with the proposed site access scheme provided for the site, it is determined that the Alternative F project would result in impacts to the same intersections as the proposed project during the AM and PM peak hours. As such, the contribution by the project to the City of Los Angeles' Adaptive Traffic Control System installation at seven study intersections as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection will be implemented as part of the Alternative F project.

Weekend Conditions

The weekend trip generation forecast for Alternative F is summarized in *Table E-2*. As presented in *Table E-2*, Alternative F would result in the same number of trips as the proposed project during the weekend conditions. As presented in *Table E-2*, Alternative F is expected to generate a net increase of 632 vehicle trips (329 inbound trips and 303 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative F is forecast to generate a net increase of 6,252 daily trip ends during a typical Saturday (3,126 inbound trips and 3,126 outbound trips).

A qualitative review was conducted to determine if the Alternative F project would likely result in an increase in project impacts when compared to the proposed project. The Alternative F project would result in the same number of trips as the proposed project. Based on this comparison, it is determined that the Alternative F project would result in impacts to the same intersections as the proposed project during the Saturday mid-day peak hour. As such, the contribution by the project to the City of



Los Angeles' Adaptive Traffic Control System installation at seven study intersections as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection will be implemented as part of the Alternative F project.

Alternative G: Open Air Promenade Alternative (Without Tunnel Access and Subterranean Parking)

The Alternative G project will include the construction of 190,000 square feet of commercial retail/restaurant space in a series of single-story structures to be located along the southern edge of the existing mall. A portion of the new commercial retail space will be constructed as an extension to the existing mall building and the remainder will be constructed on the bottom floor of the existing southern parking structure. A portion of the lower two-levels of the Bloomingdale's parking structure. and the entire footprint of the existing three-level south parking structure would be modified and converted to single-story retail space. A new circulation route to be accessed from the existing northerly driveway on Hazeltine Avenue will be provided between the new retail in the main mall building and the southern parking structure to provide an open air environment/promenade area. In addition, a new grade plus six parking structure would be constructed south of the existing Macy's parking structure. As part of the Alternative G project, vehicular access to the site will be provided via four project driveways: two existing driveways on Hazeltine Avenue, one existing driveway on Woodman Avenue, and one new driveway on Riverside Drive at Matilija Avenue (i.e., no tunnel access and subterranean parking). It is anticipated that the Alternative G project would be completed by year 2011.

Weekday Conditions

The weekday trip generation forecast for Alternative G is summarized in *Table F-1*. As presented in *Table F-1*, Alternative G is expected to generate a net increase of 61 vehicle trips (37 inbound trips and 24 outbound trips) during the AM peak hour. During the PM peak hour, Alternative G is expected to generate a net increase of 311 vehicle trips (149 inbound trips and 162 outbound trips). Over a 24-hour period, Alternative G project is forecast to generate a net increase of 3,246 daily trip ends during a typical weekday (1,623 inbound trips and 1,623 outbound trips).

In order to determine the operating conditions of the street system in the year 2011 with the Alternative G project, traffic associated with the Alternative G project was assigned to the local roadway system based on an updated trip distribution and assignment characteristics without the proposed tunnel access on Riverside Drive. The updated project traffic volume distribution percentages during AM and PM peak hours at the 17 study intersections are illustrated in *Figure F*. The forecast Alternative G project traffic volumes at the study intersections for the AM and PM peak hours are displayed in *Figures G-1 and G-2*, respectively.

As shown in *Table F-2*, application of the City of Los Angeles' threshold criteria to the "With Alternative G Project" scenario indicates that two of the 17 study



intersections are anticipated to be significantly impacted by the Alternative G project during the AM and PM peak hours. Incremental but not significant impacts are noted at the remaining 15 study intersections due to the Alternative G project. The future with Alternative G project (existing, ambient growth, related projects and Alternative G project) traffic volumes at the study intersections during the AM and PM peak hours are illustrated in *Figures H-1 and H-2*, respectively.

The two study intersections forecast to be significantly impacted by the Alternative G project are intersections forecast to be significantly impacted by the proposed project. The traffic mitigation measures recommended for the proposed project at these two impacted study intersections are anticipated to reduce the traffic impacts associated with the Alternative G project to less than significant levels.

Weekend Conditions

The weekend trip generation forecast for Alternative 4A is summarized in *Table F-3*. As presented in *Table F-3*, Alternative G is expected to generate a net increase of 413 vehicle trips (215 inbound trips and 198 outbound trips) during the Saturday mid-day peak hour. Over a 24-hour period, Alternative G is forecast to generate a net increase of 4,092 daily trip ends during a typical Saturday (2,046 inbound trips and 2,046 outbound trips).

In order to determine the operating conditions of the street system in the year 2011 with the Alternative G project, traffic associated with the Alternative G project was assigned to the local roadway system based on an updated trip distribution and assignment characteristics without the proposed tunnel access on Riverside Drive. The forecast Alternative G project traffic volumes at the six study intersections for the Saturday mid-day peak hour are displayed in *Figure I*.

As shown in *Table F-4*, application of the City of Los Angeles' threshold criteria to the "With Alternative G Project" scenario indicates that three of the six study intersections are anticipated to be significantly impacted by the Alternative G project during the Saturday mid-day peak hour. Incremental but not significant impacts are noted at the remaining three study intersections due to the Alternative G project. The future with Alternative G project (existing, ambient growth, related projects and Alternative G project) traffic volumes at the six study intersections during the Saturday mid-day peak hour are illustrated in *Figure J*.

The three study intersections forecast to be significantly impacted by the Alternative G project are intersections forecast to be significantly impacted by the proposed project during the Saturday mid-day peak hour. The traffic mitigation measures recommended for the proposed project at these three impacted study intersections are anticipated to reduce the traffic impacts associated with the Alternative G project during the Saturday mid-day peak hour to less than significant levels.

It should be noted that although the Alternative G project is anticipated to result in an overall decrease in traffic impacts when compared to the proposed project, the contribution by the project to the City of Los Angeles' Adaptive Traffic Control



System installation at seven study intersections as well as the redesignation of the southbound Woodman Avenue right-turn only lane to an optional through/right-turn lane at the Woodman Avenue/Riverside Drive intersection will be implemented as part of the Alternative G project.

Please feel free to contact us with any questions or comments.

Attachments

Jonathan Krausche, Westfield Corporation
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File

APPENDIX A

PROJECT ALTERNATIVES TRIP GENERATION TABLES

ALTERNATIVES E AND G
LEVEL OF SERVICE TABLES
PROJECT TRAFFIC VOLUME FIGURES

Table A-1
ALTERNATIVE B WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		PEAK I OLUME			PEAK F OLUME	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed Shopping Center Less 10% Pass-by [4] Subtotal Proposed	975,000 GLSF	29,840 (2,984) 26,856	375 (38) 337	239 (24) 215	614 (62) 552	1,351 (135) 1,216	1,463 (146) 1,317	2,814 (281) 2,533
Existing Shopping Center Less 10% Pass-by [4] Subtotal Existing	867,000 GLSF	27,647 (2,765) 24,882	349 (35) 314	223 (22) 201	572 (57) 515	1,250 (125) 1,125	1,354 (135) 1,219	2,604 (260) 2,344
NET CHANGE	108,000 GLSF	1,974	23	14	37	91	98	189

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table A-2
ALTERNATIVE B WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed	075 000 CLOF	. 20.700	1.070	1 005	2 002
Shopping Center Less 10% Pass-by [4]	975,000 GLSF	38,790 (3,879)	1,978 (198)	1,825	3,803 (381)
Subtotal Proposed		34,911	1,780	1,642	3,422
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
NET CHANGE	108,000 GLSF	2,488	131	119	250

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table B-1
ALTERNATIVE C WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		PEAK I OLUME		1	PEAK I	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed Shopping Center Less 10% Pass-by [4] Subtotal Proposed	1,102,000 GLSF	32,312 (3,231) 29,081	403 (40) 363	257 (26) 231	660 (66) 594	1,464 (146) 1,318	1,587 (159) 1,428	3,051 (305) 2,746
Existing Shopping Center Less 10% Pass-by [4] Subtotal Existing	867,000 GLSF	27,648 (2,765) 24,883	349 (35) 314	223 (22) 201	572 (57) 515	1,250 (125) 1,125	1,354 (135) 1,219	2,604 (260) 2,344
NET CHANGE	235,000 GLSF	4,198	49	30	79	193	209	402

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table B-2
ALTERNATIVE C WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed Shaming Contar	1 102 000 CLSE	41.001	2 141	1.077	4 1 1 0
Shopping Center Less 10% Pass-by [4]	1,102,000 GLSF	41,901 (4,190)	2,141 (214)	1,977 (198)	4,118 (412)
Subtotal Proposed		37,711	1,927	1,779	3,706
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
NET CHANGE	235,000 GLSF	5,288	278	256	534

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table C-1
ALTERNATIVE D WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		PEAK F			PEAK E	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed Shopping Center Less 10% Pass-by [4] Subtotal Proposed	1,102,000 GLSF	32,312 (3,231) 29,081	403 (40) 363	257 (26) 231	660 (66) 594	1,464 (146) 1,318	1,587 (159) 1,428	3,051 (305) 2,746
Existing Shopping Center Less 10% Pass-by [4] Subtotal Existing	867,000 GLSF	27,648 (2,765) 24,883	349 (35) 314	223 (22) 201	572 (57) 515	1,250 (125) 1,125	1,354 (135) 1,219	2,604 (260) 2,344
NET CHANGE	235,000 GLSF	4,198	49	30	79	193	209	402

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table C-2
ALTERNATIVE D WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]	Ī	AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed	1 102 000 CLSE	41.001	2 141	1.077	4 1 1 0
Shopping Center Less 10% Pass-by [4]	1,102,000 GLSF	41,901 (4,190)	2,141 (214)	1,977 (198)	4,118 (412)
Subtotal Proposed		37,711	1,927	1,779	3,706
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
NET CHANGE	235,000 GLSF	5,288	278	256	534

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table D-1
ALTERNATIVE E WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY	AM	PEAK I	HOUR	PM	PEAK F	IOUR
		TRIP ENDS [2]	V	OLUME	S [2]	V	OLUME	S [2]
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed Shopping Center	1,147,000 GLSF	22 162	412	264	(22	1 504	1.600	
Less 10% Pass-by [4]	1,147,000 GLSF	33,162 (3,316)	413 (41)	264 (26)	677 (67)	1,504 (150)	1,629 (163)	3,133 (313)
Subtotal Proposed		29,846	372	238	610	1,354	1,466	2,820
Existing	0.00.00.00.00							
Shopping Center Less 10% Pass-by [4]	867,000 GLSF	27,647 (2,765)	349 (35)	(22)	572 (57)	1,250 (125)	1,354 (135)	2,604 (260)
Subtotal Existing		24,882	314	201	515	1,125	1,219	2,344
NET CHANGE	280,000 GLSF	4,964	58	37	95	229	247	476

^[1] Source: ITE "Trip Generation", 7th Edition, 2003.

^[2] Trips are one-way traffic movements, entering or leaving.

^[3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.

^[4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table D-2 ALTERNATIVE E PROJECT WEEKDAY SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE AM AND PM PEAK HOURS

L			Ξ	ľ	[2]		[3]	r		ľ	4			153		
		PEAK	STI	007 NG	MBI OW	012 ENT TH	ELA E	TED TS	AL.		CHANGE SIGNIF.	SIGNIF. IMPACT	AE.		CHANGE V/C	MITT- GATED
ġ,	INTERSECTION	HOUR	ı	SOT '		SO G	1	SO ,		<u>.</u>	(4)-(3)	q ;	1	FOS	(S) (S)	
_	Van Nuys Boulevard/ Riverside Drive	PM PM	0.770	a O	0.762 0.854) D	0.802 0.893	<u> </u>	0.808	Оп	0.006	YES	0.890	υD	-0.024 -0.003	YES
2	Van Nuys Boulevard/ US-101 Freeway Westbound Ramps	AM PM	0.655	В	0.698 0.843	В	0.721	C	0.722 0.885	D D	0.001	NO NO	0.722	υQ	0.001	1 1
3	Van Nuys Boulevard/ US-101 Freeway Eastbound Ramps	AM PM	0.793 0.955	EС	0.850 1.027	D	0.877 1.063	D	0.878 1.068	ΟH	0.001	NO NO	0.878	Он	0.001	
4	Tyrone Avenue/ Moorpark Street	AM PM	0.539 0.862	A D	0.600	E	0.622 0.983	Вп	0.622 0.994	αп	0.000	NO YES	0.592	∢ ш	-0.030	YES
5	Tyrone Avenue-Beverly Glen Boulevard/ Ventura Boulevard	AM PM	0.613	വ	0.651 0.789	CB	0.717	C	0.718 0.873	C	0.001	NO NO	0.718	C	0.001	1 1
9	Hazeltine Avenue/ Magnolia Boulevard	AM PM	0.701	C	0.748	C	0.766	υQ	0.770	C	0.004	NO NO	0.900	C	0.004	
7	Hazeltine Avenue/ Riverside Drive	AM PM	0.778	υυ	0.863	ΩO	0.882	D D	0.891	D	0.009	NO YES	0.861	D	-0.021 0.001	YES
∞	Hazeltine Avenue/ Fashion Square Lane	AM PM	0.361	4 4	0.404	4	0.412	4 4	0.414	ВА	0.002	NO NO	0.384	В	-0.028 0.028	1 1
6	Hazeltine Avenue/ Moorpark Street	AM PM	0.709	υυ	0.757 0.790	ပပ	0.779 0.824	C	0.780	C	0.001	NO NO	0.780	C	0.001	
10	Hazeltine Avenue/ Ventura Boulevard	AM PM	0.797	ВС	0.853	D	0.907 0.755	CE	0.908	E C	0.001	NO	0.908	C	0.001	1 1

Table D-2 (Continued) ALTERNATIVE E PROJECT WEEKDAY SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE AM AND PM PEAK HOURS

			Ξ	r	[7]	r	[3]	\vdash		[4]				[5]		
			YEAR 2007		YEAR 2012 W/ AMBIENT	012 ENT	YEAR 2012 W/ RELATED		YEAR 2012 W/ALTE		CHANGE SIGNIF.	SIGNIF.	YEAR 2012 W/ ALT E		CHANGE	-ILIW
NO.	INTERSECTION	PEAK HOUR	EXISTING V/C LC	NG LOS	GROWTH V/C LC	TH LOS	PROJECTS V/C LOS	Š	PROJECT V/C L(SC	V/C]	IMPACT [b]	MITIGATION V/C LOS		V/C [(5)-(3)]	GATED
11	Woodman Avenue/ Magnolia Boulevard	AM PM	0.857	C	0.919	E	0.927 E 0.847 D		0.929	E	0.002	ON ON	0.929	E O	0.002	
12	Woodman Avenue/ Riverside Drive	AM PM	0.959 0.880	ЭE	1.061	гг гл	1.107 F 1.003 F		1.117	<u>ъ</u> ъ	0.010	YES	1.016	ъ п	-0.091	YES
13	Woodman Avenue/ US-101 Freeway Westbound Ramps	AM PM	0.743	υυ	0.824	ДΩ	0.841 D 0.819 D		0.847		0.006	NO YES	0.817	Q	-0.024	YES
14	Woodman Avenue/ US-101 Freeway Eastbound Ramps	AM PM	0.654	ВВ	0.69°0	ВВ	0.720 C 0.700 B		0.725	ပ	0.00 <i>5</i> 0.031	ON ON	0.725 0.731	ပ	0.005	
15	Woodman Avenue/ Moorpark Street	AM PM	0.850	ДΩ	0.942	шш	0.991 E		0.993	<u>п</u> н	0.002 0.012	NO YES	0.963	пп	-0.028 -0.018	 YES
16	Woodman Avenue/ Ventura Boulevard	AM PM	0.717	Da	0.766	C	0.826 D 0.741 C		0.829	O	0.003	NO	0.829	C	0.003	
17	Project Driveway-Matilija Avenue Riverside Drive [a]	AM PM	0.518	4	0.570 0.610	ВВ	0.585 A 0.628 B		0.471	A M	-0.114 0.061	NO NO	0.471	В	-0.114	

Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

According to LADOTs "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection shall be deemed significant in accordance with the following table:

Level of Service Final V/C Poiset-Related Increase in V/C C > 0,700 - 0.800 equal to or greater than 0,040 [a] [b]

Project-Related Increase in V/C equal to or greater than 0.040 equal to or greater than 0.020 equal to or greater than 0.010 > 0.800 - 0.900 > 0.900 C D E/F

Table D-3 ALTERNATIVE E WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed					
Shopping Center	1,147,000 GLSF	42,972	2,198	2,029	4,227
Less 10% Pass-by [4]		(4,297)	(220)	(203)	(423)
Subtotal Proposed	.	38,675	1,978	1,826	3,804
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
NET CHANGE	280,000 GLSF	6,252	329	303	632

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table D-4
ALTERNATIVE E PROJECT SUMMARY OF VOLUME TO CAPACITY RATIOS
AND LEVELS OF SERVICE
WEEKEND PEAK HOUR

			Ξ		[2]	Ī	[3]				[4]			[2]		
					YEAR 2012	3012	YEAR 2012	112	YEAR 2012				YEAR 2012	2012		,
		PEAK	YEAR 2007 EXISTING	NG 7	W/ AMBIENT GROWTH	TH	W/ RELATED PROJECTS	TED TS	W/ ALT E PROJECT		CHANGE SIGNIE. V/C IMPACT	SIGNIF. IMPACT	W/ALTE	LE E	CHANGE V/C	GATED
NO.	INTERSECTION	HOUR	V/C	TOS	V/C	ros	A/C	ros	V/C	TOS	[(4)-(3)]	[P]	V/C	LOS	[(5)-(3)]	
7	Hazeltine Avenue/ Riverside Drive	Saturday Mid-day	0.684	В	0.760	ر ر	0.795	၁	0.851	Q	0.056	YES	0.750	C	-0.045	YES
∞	Hazeltine Avenue/ Fashion Square Lane	Saturday Mid-day	0.636	ш	0.707	S	0.719	၁	0.774	၁	0.055	YES	0.744	C	0.025	YES
12	Woodman Avenue/ Riverside Drive	Saturday Mid-day	0.874	Q	896.0	Ш	1.024	Į.,	1.086	ΙΉ	0.062	YES	0.997	E	-0.027	YES
13	Woodman Avenue/ US-101 Freeway Westbound Ramps	Saturday Mid-day	0.757	C	0.840	D	0.856	D	0.900	D	0.044	YES	0.870	D	0.014	YES
14	Woodman Avenue/ US-101 Freeway Eastbound Ramps	Saturday Mid-day	0.590	A	0.626	В	0.644	В	0.688	В	0.044	ON	0.688	В	0.044	l
17	Project Driveway-Matilija Avenue Riverside Drive [a]	Saturday Mid-day	0.472	А	0.519	A	0.547	Y	0.663	В	0.116	ON	0.663	В	0.116	ı

Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. According to LADOT's "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection shall be deemed significant in accordance with the following table: [a]

 Level of Service
 Final V/C
 Project-Related Increase in V/C

 C
 > 0.700 - 0.800
 equal to or greater than 0.040

 D
 > 0.800 - 0.900
 equal to or greater than 0.020

 E/F
 > 0.900
 equal to or greater than 0.010

Table E-1
ALTERNATIVE F WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		PEAK I DLUME			PEAK F	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
Proposed Shopping Center Less 10% Pass-by [4] Subtotal Proposed	1,147,000 GLSF	33,162 (3,316) 29,846	413 (41) 372	264 (26) 238	677 (67) 610	1,504 (150) 1,354	1,629 (163) 1,466	3,133 (313) 2,820
Existing Shopping Center Less 10% Pass-by [4] Subtotal Existing	867,000 GLSF	27,647 (2,765) 24,882	349 (35) 314	223 (22) 201	572 (57) 515	1,250 (125) 1,125	1,354 (135) 1,219	2,604 (260) 2,344
NET CHANGE	280,000 GLSF	4,964	58	37	95	229	247	476

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

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Table E-2 ALTERNATIVE F WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed					
Shopping Center	1,147,000 GLSF	42,972	2,198	2,029	4,227
Less 10% Pass-by [4]		(4,297)	(220)	(203)	(423)
Subtotal Proposed		38,675	1,978	1,826	3,804
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
				,,,,,	
NET CHANGE	280,000 GLSF	6,252	329	303	632

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table F-1
ALTERNATIVE G WEEKDAY PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		PEAK I			PEAK I	
LAND USE	SIZE	VOLUMES	IN	OLUME OUT	TOTAL	IN	OLUME OUT	TOTAL
			~	001	101112	<u> </u>	001	101785
Proposed								
Shopping Center	1,057,000 GLSF	31,448	393	251	644	1,425	1,543	2,968
Less 10% Pass-by [4]		(3,145)	(39)	(25)	(64)	(143)	(154)	(297)
Subtotal Proposed		28,303	354	226	580	1,282	1,389	2,671
Existing								
Shopping Center	867,000 GLSF	27,648	349	223	572	1,250	1,354	2,604
Less 10% Pass-by [4]		(2,765)	(35)	(22)	(57)	(125)	(135)	(260)
Subtotal Existing		24,883	314	201	515	1,125	1,219	2,344
NET CHANGE	190,000 GLSF	3,420	40	25	65	157	170	327

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table F-2
ALTERNATIVE G PROJECT WEEKDAY SUMMARY OF VOLUME TO CAPACITY RATIOS
AND LEVELS OF SERVICE
AM AND PM PEAK HOURS

L			1		[7]	r	2	r		-	141			2		
NO.	INTERSECTION	PEAK HOUR	YEAR 2007 EXISTING V/C LO	NG LOS		ENT TH LOS		TED TED	YEAR 2011 W/ ALT G PROJECT V/C LC	8	CHANGE SIGNIF. V/C IMPACT [(4)-(3)] [b]	SIGNIF. IMPACT [b]	YEAR 2011 W/ ALT G MITIGATION V/C LOS	r C TON TON LOS	CHANGE V/C [(5)-(3)]	MITI- GATED
	Van Nuys Boulevard/ Riverside Drive	AM PM	0.687	СВ	0.747 0.837	υQ	0.787 0.876	υД	0.791 0.895	υd	0.004	NO NO	0.761	C	-0.026 -0.011	l l
2	Van Nuys Boulevard/ US-101 Freeway Westbound Ramps	AM PM	0.655	CB	0.683 0.826	ВΩ	0.707	υQ	0.707	υQ	0.000	ON ON	0.707	C	0.000	1 1
æ	Van Nuys Boulevard/ US-101 Freeway Eastbound Ramps	AM PM	0.793	Сm	0.832 1.007	Оř	0.860	D F	0.860	Он	0.000	NO NO	0.860	ОH	0.000	1 1
4	Tyrone Avenue/ Moorpark Street	AM PM	0.539	A	0.588	∢ ш	0.610	шп	0.610	Вп	0.000	ON ON	0.580	E A	-0.030	1 1
5	Tyrone Avenue-Beverly Glen Boulevard/ Ventura Boulevard	AM PM	0.613	e D	0.638 0.773	CB	0.703 0.847	C	0.704 0.854	C	0.001	NO NO	0.704	O Q	0.001	1 1
9	Hazeltine Avenue/ Magnolia Boulevard	AM PM	0.701	C	0.733	O D	0.751	C	0.754	C	0.003	NO NO	0.754	D C	0.003	1 1
7	Hazeltine Avenue/ Riverside Drive	AM PM	0.778	ပပ	0.846 0.781	C	0.865	Q	0.872 0.822	Q	0.007	ON ON	0.842	O D	-0.023	1 1
∞	Hazeltine Avenue/ Fashion Square Lane	AM PM	0.361	< <	0.396	< <	0.404	A A	0.405	A B	0.001	NO NO	0.375	< <	-0.029	1 1
6	Hazeltine Avenue/ Moorpark Street	AM PM	0.709	υυ	0.741 0.774	υυ	0.764	C	0.765	C	0.001	ON ON	0.765	C	0.001	11
10	Hazeltine Avenue/ Ventura Boulevard	AM PM	0.797	В	0.836	D B	0.889	C	0.890 0.745	D	0.001	ON ON	0.890	CD	0.001	1 1

Table F-2 (Continued) ALTERNATIVE G PROJECT WEEKDAY SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE AM AND PM PEAK HOURS

			[1]	H	[2]	r	[3]	-			[4]			[2]		
			YEAR 2007		YEAR 2011 W/ AMBIENT	n k	YEAR 2011 W/RELATED	TED T	YEAR 2011 W/ALT G		CHANGE SIGNIF.	SIGNIF.	YEAR 2011 W/ALT G	5.	CHANGE	MITI-
NO.	INTERSECTION	PEAK HOUR	EXISTING V/C LOS	S	GROWTH V/C L(TH TOS	PROJECTS V/C LO	LOS	PROJECT V/C LC	CT LOS	V/C [(4)-(3)]	IMPACT [b]	MITIGATION V/C LOS	NOL	V/C [(5)-(3)]	GATED
=	Woodman Avenue/ Magnolia Boulevard	AM PM	0.857 D 0.780 C		0.901	D	0.908	D E	0,909	ЭС	0.001	ON ON	0.909	E	0.001	1 1
12	Woodman Avenue/ Riverside Drive	AM PM	0.959 E 0.880 D		1.041	<u></u> 보 교	1.086	ь п	1.093	<u> </u>	0.007	NO YES	0.997	ээ	-0.089	 YES
13	Woodman Avenue/ US-101 Freeway Westbound Ramps	AM PM	0.743 C		0.808 0.797	Ωυ	0.825	D	0.829	D	0.004	NO YES	0.799 0.797	သ	-0.026	YES
14	Woodman Avenue/ US-101 Freeway Eastbound Ramps	AM PM	0.654 B		0.681 0.676	вв	0.706	В	0.710	ပ	0.004	NO NO	0.710	ပ	0.004	: 1
15	Woodman Avenue/ Moorpark Street	AM PM	0.850 0.867 D		0.923 0.942	шш	0.972 0.986	шш	0.973	μш	0.001	NO NO	0.943	пп	-0.029	-
16	Woodman Avenue/ Ventura Boulevard	AM PM	0.717 C 0.640 B		0.750	ВС	0.810	CD	0.812 0.737	C	0.002	NO NO	0.812	C	0.002	1 1
17	Project Driveway-Matilija Avenue Riverside Drive [a]	AM PM	0.518 A 0.555 A		0.559	4 4	0.574	B A	0.459	ВВ	-0.115	NO NO	0.459	ВЪ	-0.115 0.041	1 1

Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. According to LADOT's "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection shall be deemed significant in accordance with the following table: [a]

on cook, tage to, a amoportation		Project-Related Increase in V/C	equal to or greater than 0.040	equal to or greater than 0.020	equal to or greater than 0.010
mid i locedules, ivin	he following table:	Final V/C	> 0,700 - 0,800	> 0.800 - 0.900	> 0.900
Letter 1 a manne blady 1 divises min 1 locations, min on 2002, 1 ago 10, a amisportation	ned significant in accordance with the following table:	Level of Service	υ	Q	E/F

Table F-3
ALTERNATIVE G WEEKEND PROJECT TRIP GENERATION [1]

		DAILY TRIP ENDS [2]		AY PEAK OLUMES	
LAND USE	SIZE	VOLUMES	IN	OUT	TOTAL
Proposed Shopping Center	1,057,000 GLSF	40,816	2,084	1,924	4,008
Less 10% Pass-by [4]	1,057,000 GESI	(4,082)	(208)	(192)	(400)
Subtotal Proposed		36,734	1,876	1,732	3,608
Existing Shopping Center Less 10% Pass-by [4]	867,000 GLSF	36,026 (3,603)	1,832 (183)	1,692 (169)	3,524 (352)
Subtotal Existing		32,423	1,649	1,523	3,172
NET CHANGE	190,000 GLSF	4,311	227	209	436

- [1] Source: ITE "Trip Generation", 7th Edition, 2003.
- [2] Trips are one-way traffic movements, entering or leaving.
- [3] ITE Land Use Code 820 (Shopping Center) trip generation equation rates.
- [4] Pass-by trips include traffic passing the site on an adjacent street with direct access to the land use. Pass-by reductions were based on the City of Los Angeles Department of Transportation policy on pass-by trips.

Table F-4
ALTERNATIVE G PROJECT SUMMARY OF VOLUME TO CAPACITY RATIOS
AND LEVELS OF SERVICE
WEEKEND PEAK HOUR

			Ξ		[2]		[3]	<u> </u>			[4]			[5]		
					YEAR 2011	110	YEAR 2011	011	YEAR 2011	011			YEAR 2011	2011		
		21 1 11 11	YEAR 2007	7007	W/ AMBIENT	ENT	W/ RELATED	TED	W/ALT G		E	SIGNIF.	W/ALT G	ГG	CHANGE	MITI-
NO.	INTERSECTION	FEAR	V/C LO	LOS	GKOWIH V/C LC	LOS	V/C LO	LOS	V/C LO	LOS	V/C [(4)-(3)]	IMPACI [b]	MITIGATION V/C LOS	LOS	V/C [(5)-(3)]	GATED
7	Hazeltine Avenue/ Riverside Drive	Saturday Mid-day	0.684	В	0.745	C	0.780	Ú	0.819	Q	0.039	YES	0.789	U	0.009	YES
∞	Hazeltine Avenue/ Fashion Square Lane	Saturday Mid-day	0.636	В	0.693	В	0.704	Ü	0.736	O O	0.032	ON	0.706	C	0.002	l
12	Woodman Avenue/ Riverside Drive	Saturday Mid-day	0.874	Q	0.949	ш	1.005	Ľ.	1.049	LL	0.044	YES	0.959	m	-0.046	YES
13	Woodman Avenue/ US-101 Freeway Westbound Ramps	Saturday Mid-day	0.757	Ü	0.823	Q	0.839	Q	0.869	Ω	0.030	YES	0.839	Q	0.000	YES
41	Woodman Avenue/ US-101 Freeway Eastbound Ramps	Saturday Mid-day	0.590	Ą	0.613	В	0.631	B	0.661	В	0.030	ON	0.661	В	0.030	I
17	Project Driveway-Matilija Avenue Riverside Drive [a]	Saturday Mid-day	0.472	٧	0.509	A	0.537	А	0.622	В	0.085	NO	0.622	В	0.085	I

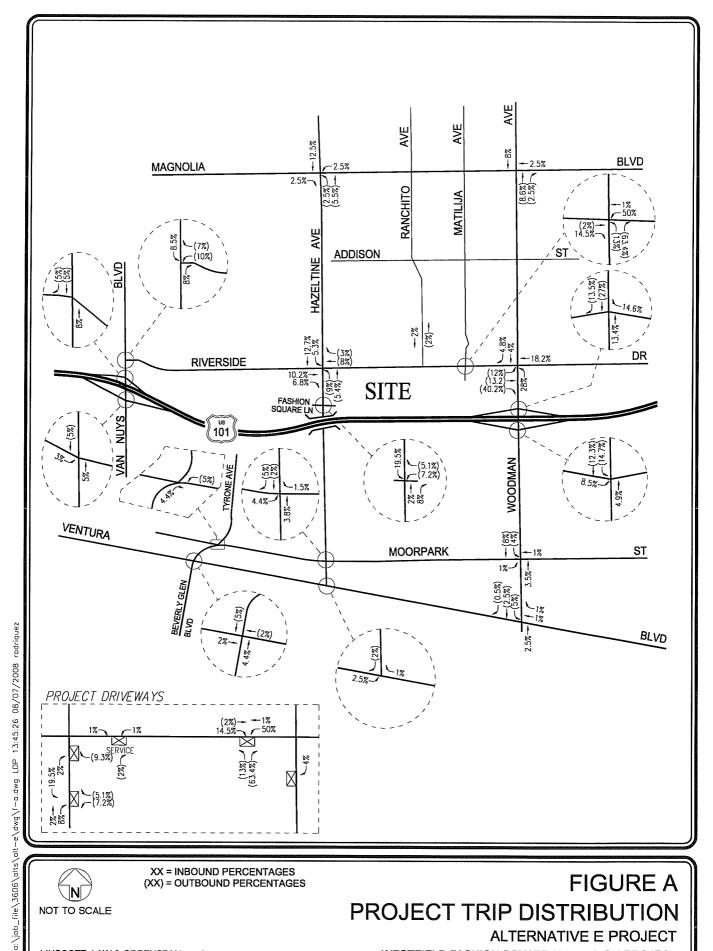
Intersection proposed to be signalized as part of the proposed project. V/C ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6. According to LADOT's "Traffic Study Policies and Procedures," March 2002, Page 10, a transportation impact on an intersection shall be deemed significant in accordance with the following table: [a]

 Level of Service
 Final V/C
 Project-Related Increase in V/C

 C
 > 0.700 - 0.800
 equal to or greater than 0.040

 D
 > 0.800 - 0.900
 equal to or greater than 0.020

 E/F
 > 0.900
 equal to or greater than 0.010





XX = INBOUND PERCENTAGES (XX) = OUTBOUND PERCENTAGES

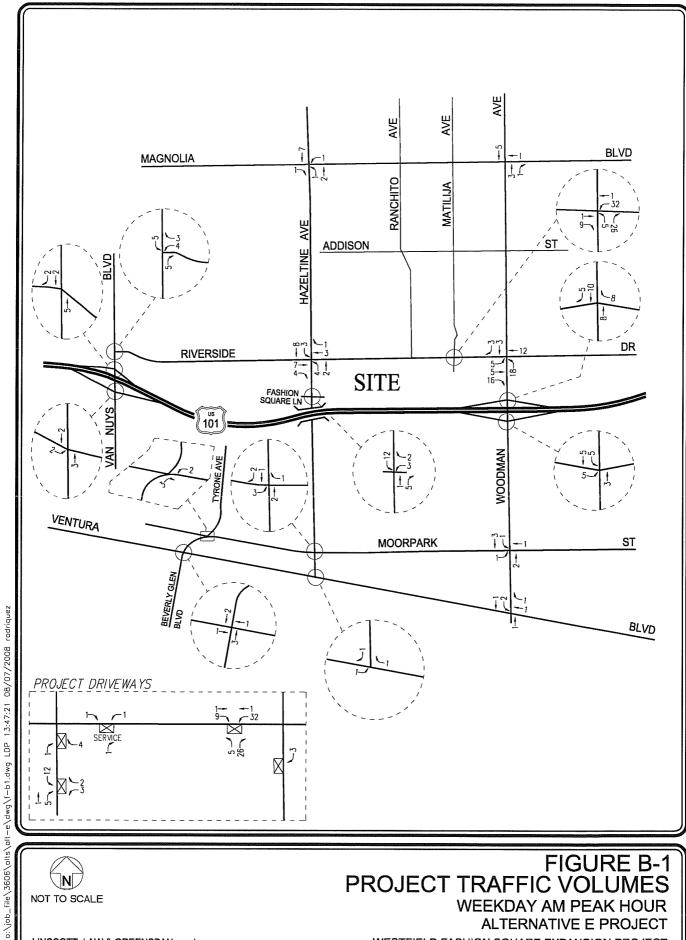
FIGURE A PROJECT TRIP DISTRIBUTION

ALTERNATIVE E PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

LINSCOTT, LAW & GREENSPAN, engineers



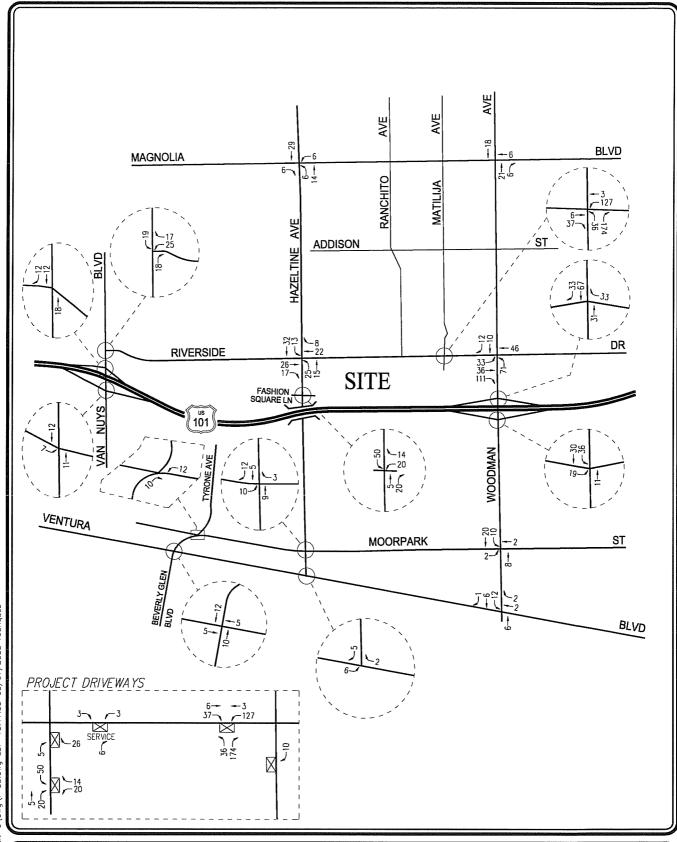


LINSCOTT, LAW & GREENSPAN, engineers

FIGURE B-1 PROJECT TRAFFIC VOLUMES

WEEKDAY AM PEAK HOUR ALTERNATIVE E PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT





LINSCOTT, LAW & GREENSPAN, engineers

FIGURE B-2 PROJECT TRAFFIC VOLUMES WEEKDAY PM PEAK HOUR

ALTERNATIVE E PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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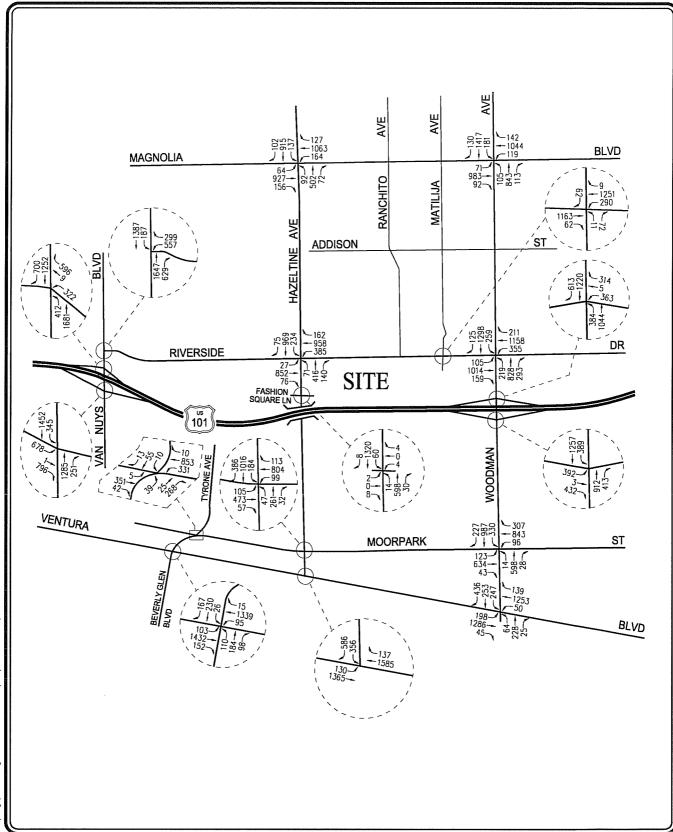




FIGURE C-1 FUTURE WITH PROJECT TRAFFIC VOLUMES

WEEKDAY AM PEAK HOUR

ALTERNATIVE E PROJECT WESTFIELD FASHION SQUARE EXPANSION PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

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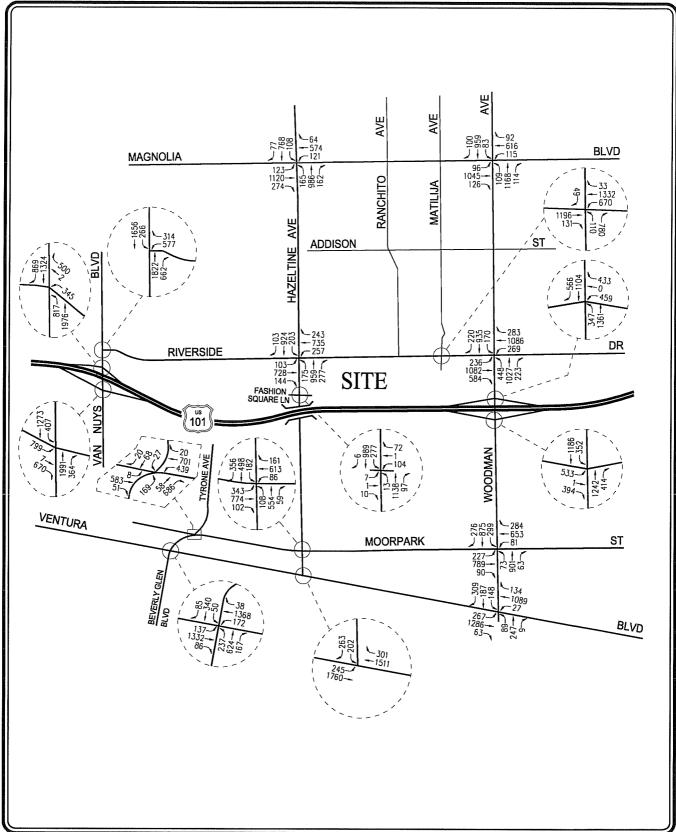




FIGURE C-2 FUTURE WITH PROJECT TRAFFIC VOLUMES

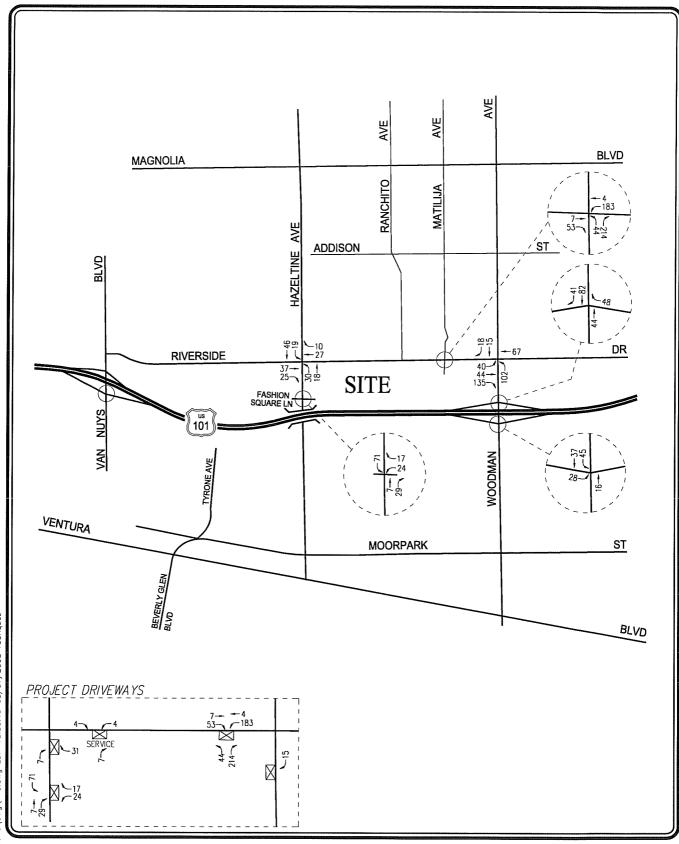
WEEKDAY PM PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

ALTERNATIVE E PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

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LINSCOTT, LAW & GREENSPAN, engineers

FIGURE D PROJECT TRAFFIC VOLUMES

SATURDAY MID-DAY PEAK HOUR ALTERNATIVE E PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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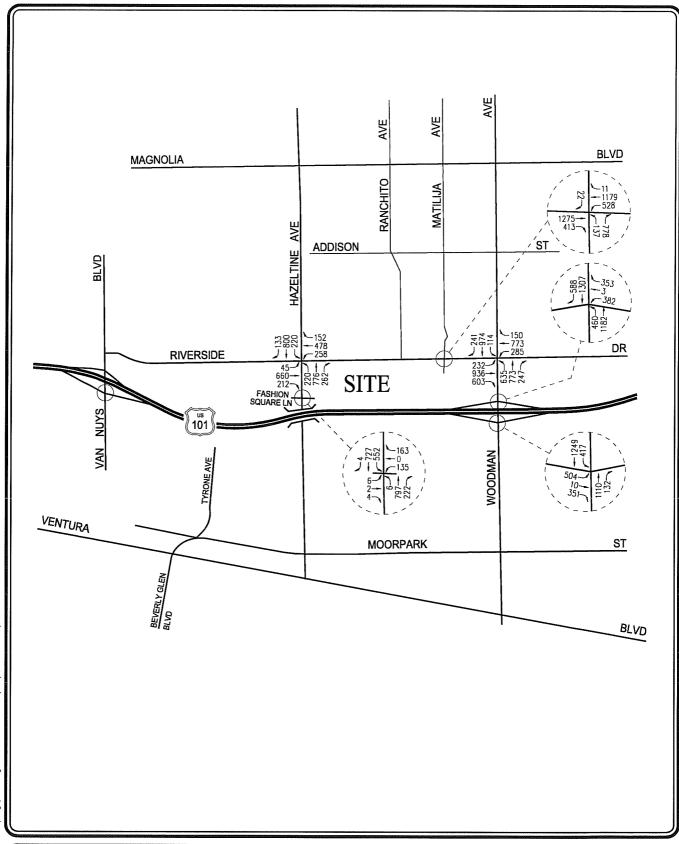




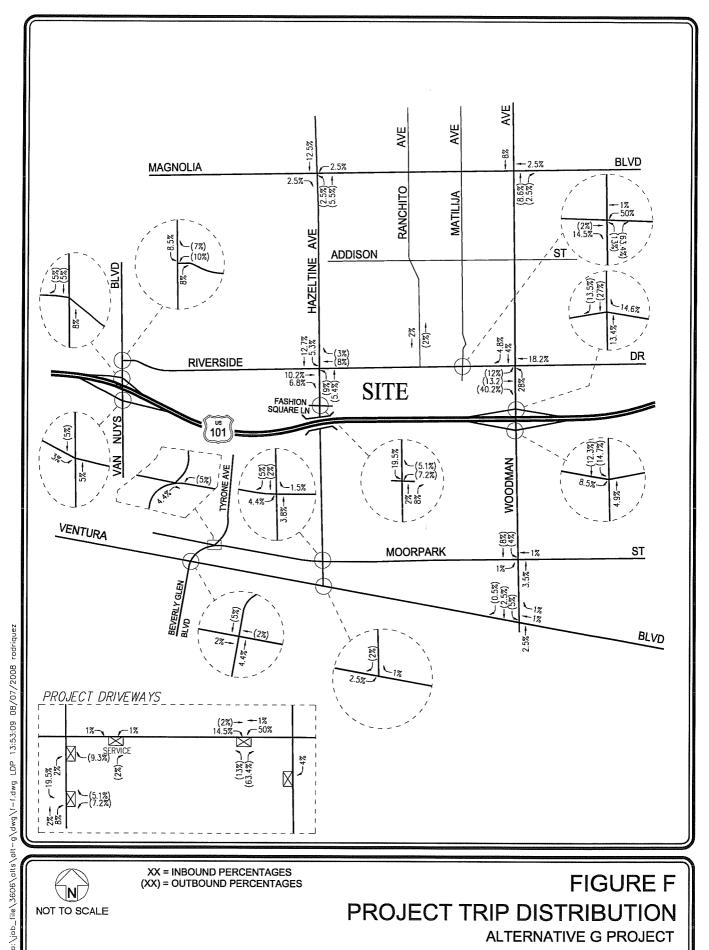
FIGURE E FUTURE WITH PROJECT TRAFFIC VOLUMES

SATURDAY MID-DAY PEAK HOUR

WESTFIELD FASHION SQUARE EXPANSION PROJECT

ALTERNATIVE E PROJECT

LINSCOTT, LAW & GREENSPAN, engineers





XX = INBOUND PERCENTAGES (XX) = OUTBOUND PERCENTAGES

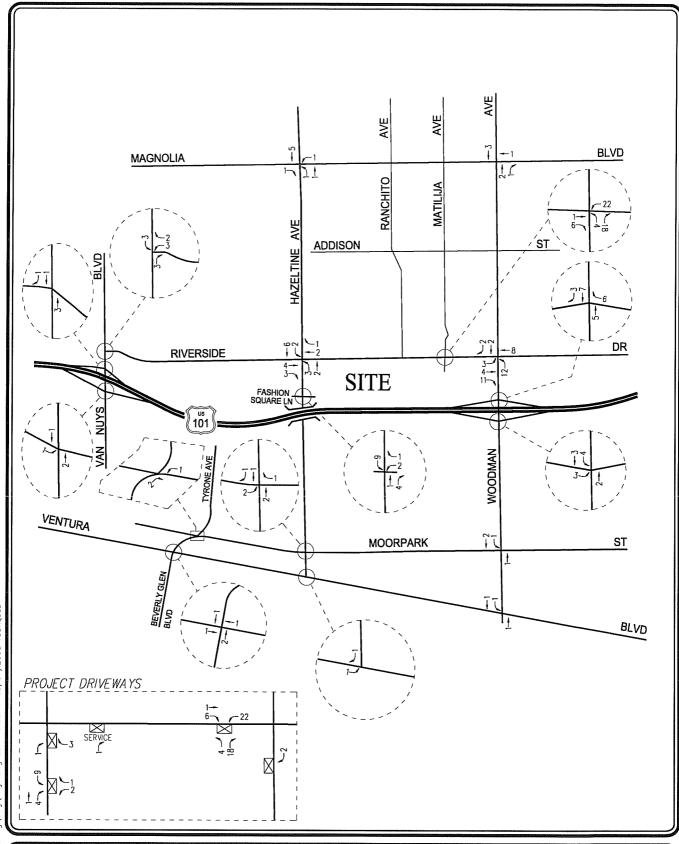
PROJECT TRIP DISTRIBUTION

ALTERNATIVE G PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

FIGURE F

LINSCOTT, LAW & GREENSPAN, engineers





LINSCOTT, LAW & GREENSPAN, engineers

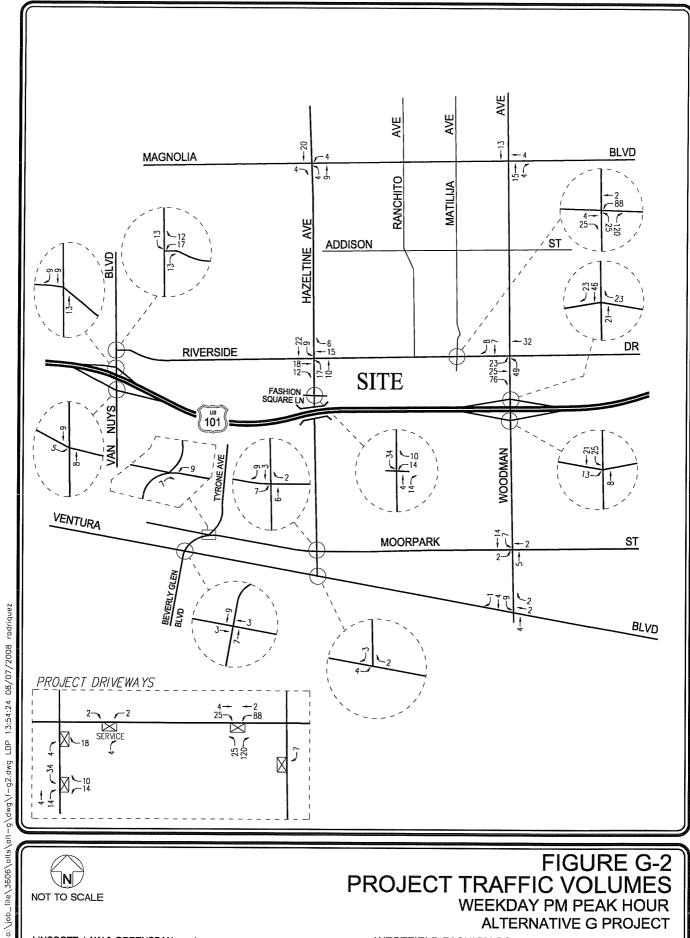
FIGURE G-1 PROJECT TRAFFIC VOLUMES WEEKDAY AM PEAK HOUR

ALTERNATIVE G PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

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LINSCOTT, LAW & GREENSPAN, engineers

FIGURE G-2 PROJECT TRAFFIC VOLUMES WEEKDAY PM PEAK HOUR

ALTERNATIVE G PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

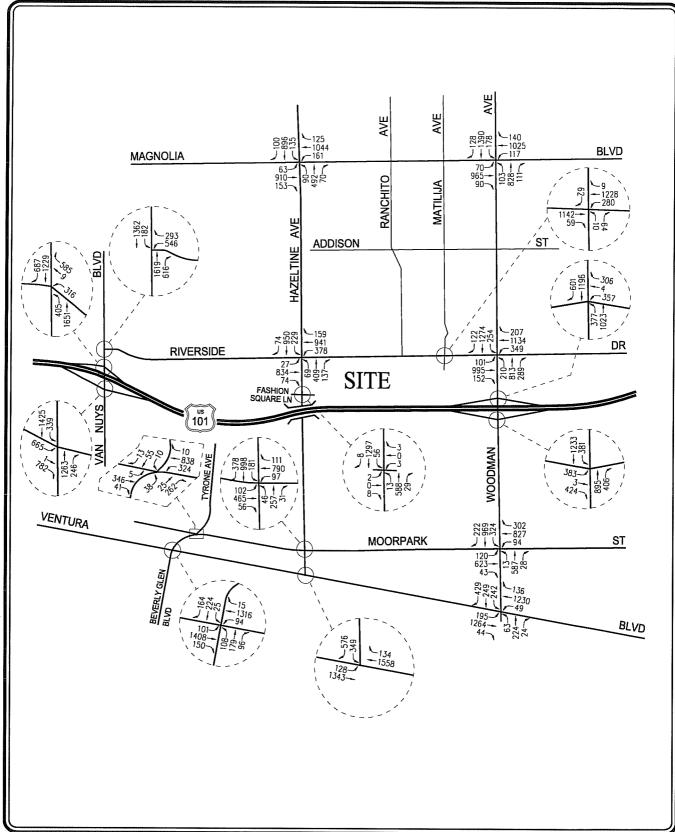




FIGURE H-1 FUTURE WITH PROJECT TRAFFIC VOLUMES

WEEKDAY AM PEAK HOUR

ALTERNATIVE G PROJECT WESTFIELD FASHION SQUARE EXPANSION PROJECT

LINSCOTT, LAW & GREENSPAN, engineers

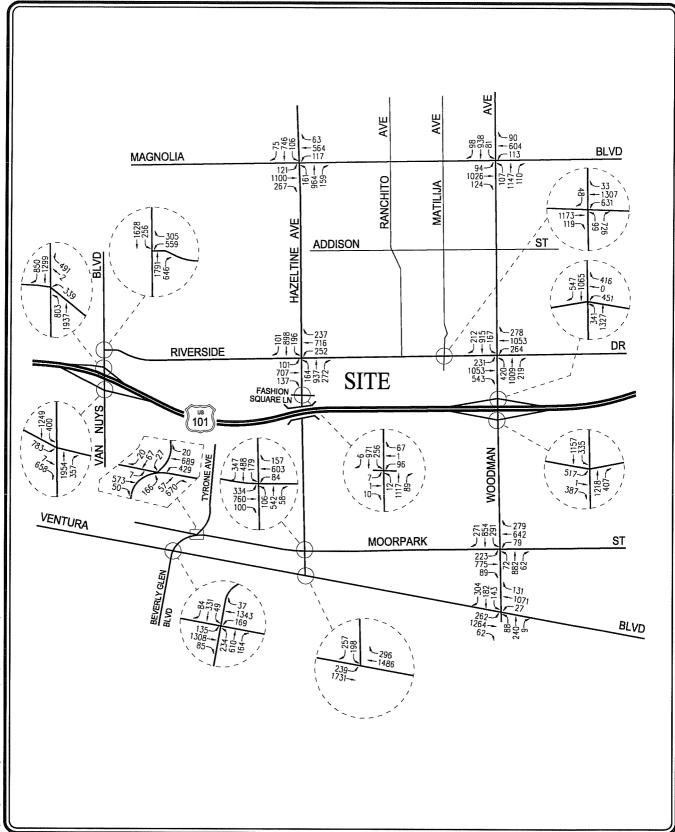


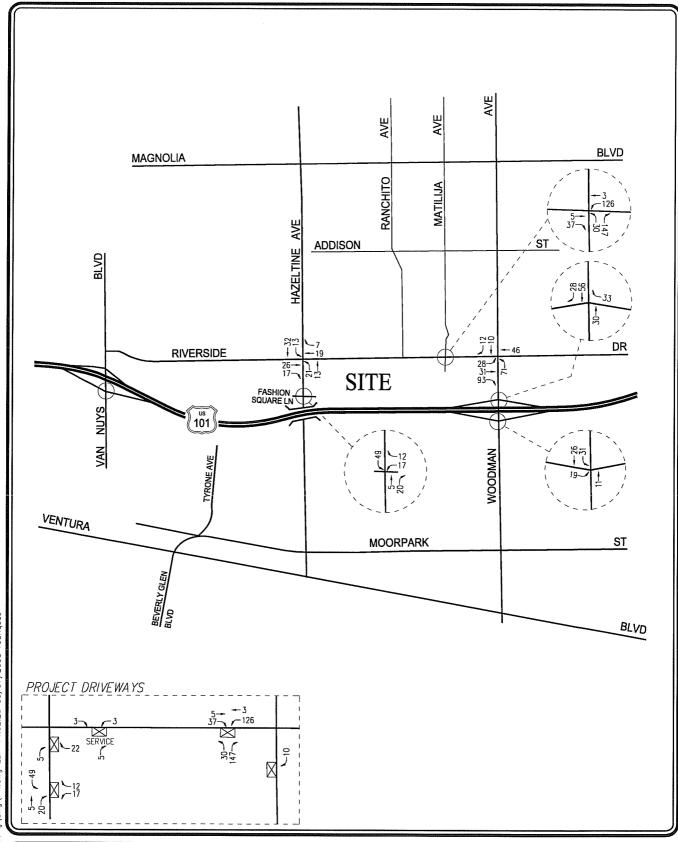


FIGURE H-2 FUTURE WITH PROJECT TRAFFIC VOLUMES

WEEKDAY PM PEAK HOUR

ALTERNATIVE G PROJECT WESTFIELD FASHION SQUARE EXPANSION PROJECT

LINSCOTT, LAW & GREENSPAN, engineers





LINSCOTT, LAW & GREENSPAN, engineers

FIGURE I PROJECT TRAFFIC VOLUMES

SATURDAY MID-DAY PEAK HOUR ALTERNATIVE G PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

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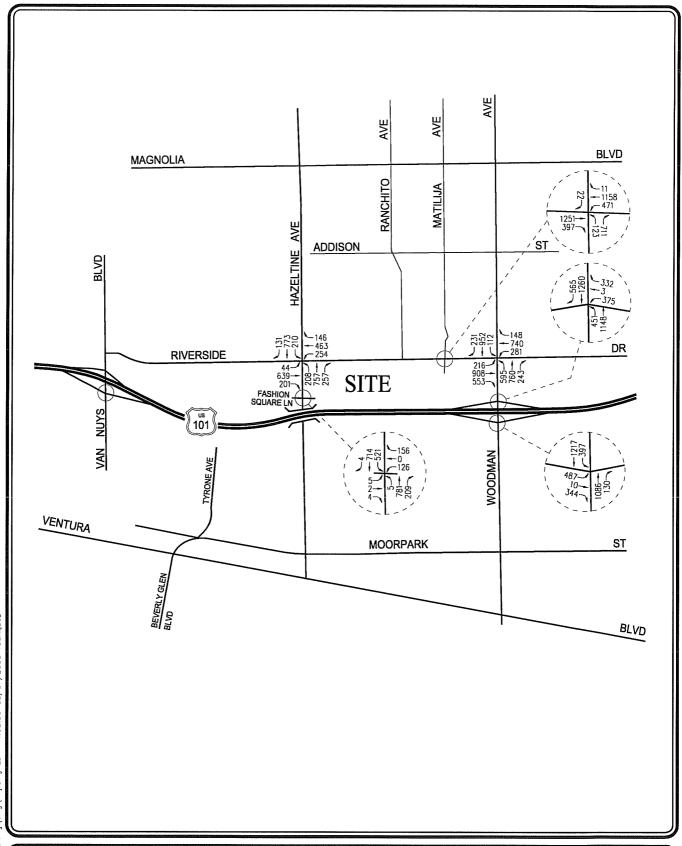




FIGURE J **FUTURE WITH PROJECT TRAFFIC VOLUMES**

SATURDAY MID-DAY PEAK HOUR ALTERNATIVE G PROJECT

WESTFIELD FASHION SQUARE EXPANSION PROJECT

NOT TO SCALE

LINSCOTT, LAW & GREENSPAN, engineers

APPENDIX B

CMA DATA WORKSHEETS

ALTERNATIVES E AND G
WEEKDAY AM & PM PEAK HOURS
WEEKEND MID-DAY PEAK HOUR

APPENDIX B-1

ALTERNATIVE E CMA DATA WORKSHEETS

WEEKDAY AM & PM PEAK HOURS
WEEKEND MID-DAY PEAK HOUR

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA1 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

08/07/2008 2007 2012 Projection Year: Date: Date of Count:

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012 \	N/ AMBIE	2012 W/ AMBIENT GROWTH	 	2012 V	W OTHER	2012 W/ OTHER PROJECTS	STS	2012	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume		Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	0	0	ı	0	0	0	1	0	0	0		0	0	0	•	0	0	0	,
Comb. L-T		0	,			0				0	,			0	,			0	,
NB Thru	1410	7	652	141	1551	2	717	96	1647	7	757	0	1647	7	759	0	1647	7	759
Comb. T-R		-	652			-	717			-	757			-	759			-	75
NB Right	246	0		22	601	0		23	624	0		ည	629	0	ı	0	629	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	158	-	158	16	174	-	174	8	182	-	182	5	187	-	187	0	187	-	187
Comb. L-T		0	•			0	,			0	•			0	į			0	
SB Thru	1227	က	409	123	1350	m	450	37	1387	ო	462	0	1387	ო	462	0	1387	က	462
Comb. T-R		0				0				0				0				0	1
SB Right	0	0		0	0	0	,	0	0	0	ı	0	0	0	1	0	0	0	1
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0	-	0	0	0	-	0	0	0	_	0	0	0		0	0	0	,
Comb. L-T		0	ŧ			0				0	ı			0	ı			0	,
EB Thru	0	0	•	0	0	0	,	0	0	0		0	0	0		0	0	0	
Comb. T-R		0				0				0				0				0	1
EB Right	0	0 (•	0	0	0 (0	0	0 (0	0	0 (ı	0	0	0 (
Comb. L-1-K-		0				0				ɔ				0				0	
WB Left	488	2	268	49	537	2	295	16	553	2	304	4	557	2	306	0	557	2	306
Comb. L-T		0	•			0	,			0	,			0	j			0	
WB Thru	0	0 (0	0	0 0		0	0	0 0		0	0	0 (,	0	0	0 (
Comb. I-K	257	⊃ ~	. 257	90	283	⊃ -	- 283	-	294	o -	- 294	Œ	200	o -	- 200	c	200	o -	,
Comb. L-T-R -		. 0	}	}	9	. 0	}	:	2	. 0	}	•		. 0	}	•		- 0)
Crit. Volumes:		N-S:	810			.S-5	891			N-S:	939			N-S:	946			N-S:	946
		E-W: SUM:	268 1078			E-W: SUM:	295 1186			E-W: SUM:	304 1243			E-W: SUM:	306 1252			E-W: SUM:	306 1252
No. of Phases:			3				3				6				က				e
Volume / Capacity:	acity:	[1]	0.687			[2]	0.762			Ξ	0.802			Ξ	0.808			[2]	0.778
Level of Service:			В			1	S				D			'	٥			,	C
																			ı

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8 fight turns on red from excl. lanes = 50% of overlapping left turn.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA 1 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST, TRAFFIC	4FFIC	2012	2012 W/ AMBIEN	ENT GROWTH	Ŧ	2012 V	V/ OTHEF	2012 W/ OTHER PROJECTS	TS	2012 V	W PROPC	2012 W/ PROPOSED PROJECT	JECT	2012 \	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0		0	0	0	,	0	0	0 (0	0	0 (0	0	0 (
Comb. L-T NB Thru	1592	0 0	719	159	1751	0 7	791	71	1822	9 C	822	0	1822	9 С	828	0	1822	7 C	- 828
Comb. T-R		•	719	1		-	791	i	•		822	:	;		828	•		- 1	828
NB Right Comb. L-T-R -	264	00	1	26	620	00	1	24	644	00	,	6	662	00		0	662	00	•
SB Left	216	-	216	22	238	-	238	6	247	-	247	19	266	-	266	0	266	-	266
Comb. L-T	1431	0 6	- 477	143	1574	0 11	525	82	1656	OM	552	c	1656	0 0	- 552	c	1656	0 6	- 552
Comb. T-R		0	:	-		0	}	1		0		N		0		•		0	,
SB Right	0	0		0	0	0		0	0	0		0	0	0	,	0	0	0	
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0	1	0	0	0		0	0	0		0	0	0	
Comb. L-T		0	,			0	,			0				0	•			0	
EB Thru	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. T-R	Ċ	0 0		C	(0 0		c	c	0 0		c	c	0 0	:	c	c	0 0	
EB Kight	0	э (0	O) (•	5	>	> (>	5	> (1	5	>	o (ı
Comb. L-T-R -		0				0				0				0				0	
WB Left	475	2	261	48	523	2	288	29	552	2	303	25	577	2	317	0	577	7	317
Comb. L-T		0	,			0	1			0				0				0	
WB Thru	0	0 0		0	0	00		0	0	0 0	•	0	0	0 0		0	0	0 0	
WB Right	234	·	234	23	257	· -	257	7	264	-	264	20	314	·	314	0	314	·	314
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	935			N-S:	1028			N-S:	1069			S-S:	1094			N-S:	1094
		E-W.:	261			E-W:	288			E-W:	303			E-W:	317			E-W:	317
		SUM:	1196			SUM:	1316			SUM:	1373			SUM:	1411			SUM:	1411
No. of Phases:		:	က				ю				က				3				9
Volume / Capacity:	icity:	Ξ	0.770			Ξ	0.854			Ε	0.893			Ξ	0.920			[2]	0.890
Level of Service:	, 		O			•	۵			•	D				ш				D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual tum lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA2 Project: File Name: Counts by: N-S St: E-W St:

Accutek

AM 2.0% Peak Hour: Annual Growth:

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps

CRITICAL MOVEMENT ANALYSIS

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 V	W AMBIE	2012 W/ AMBIENT GROWTH	H	2012 V	2012 W/ OTHER PROJECTS	PROJEC	TS	2012	W PROP	2012 W/ PROPOSED PROJECT	JJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	354	2	194	35	389	2	214	23	412	2	227	0	412	2	227	0	412	7	227
Comb. L-T		0	,			0				0				0				0	
NB Thru	1423	ო	474	142	1565	m	522	1	1676	ന	228	വ	1681	ന	260	0	1681	ന	560
Comb. T-R		0	1	,		0		•	,	0 (•	•	0 0		•	(0 (ı
NB Right	0	0	1	0	0	0		0	0	0 (0	0	0 0		0	>	-	,
Comb. L-T-R -		0				0				0				o				>	
SB Left	0	0		0	0	0	,	0	0	0		0	0	0	-	0	0	0	
Comb. L-T		0				0				0				0				0	,
SB Thru	1093	2	459	109	1202	8	505	48	1250	7	522	2	1252	7 -	522	0	1252	7	522
Comb. T-R	Č	, ,	459	8	Ċ	- ,	202	ų	ć	- τ	522	r	9		522	c	202	- +	522 385
SB Right Comb. IT-R	630	- 0	347	63	583	- 0	381	n	989	- 0	384	7	90/	- 0	000	0	9	- 0	000
		•				•													
EB Left	0	0	-	0	0	0	,	0	0	0	,	0	0	0	,	0	0	0	
Comb. L-T		0	,			0	,			0		,	•	0		4	•	0 1	
EB Thru	0	0	,	0	0	0		0	0	0 (0	0	0 1	,	0	0	0 0	
Comb. T-R	,	0 (,	•	(0 0		•	c	0 0		c	c	> 0		c	c	> c	
EB Right	>	> 0	ı	0	>	o (>	>	> 0	,	>	>	> C	,	0	>	o c	
Comb. F-1-X-		>				>				>				o				>	
WB Left	291	1	160	29	320	-	176	2	322	-	177	0	322	-	177	0	322	-	177
Comb. L-T	(0 (1	•	(0 0	,	c	c	0 0	,	c	c	0 0	,	c	c	00	
WB Thru	œ	0 0	380	-	מ	> c	418	0	33	> c	477	0	n	5 C	774	0	מ	> C	422
WB Right	535	→ c	294	53	588	- c	323	ω	596	→	328	0	596	→	328	0	596	-	328
Comb, L-T-R -		-				-				Ψ-				-					
Crit. Volumes:		N-S:	653			N-S:	719			N-S:	748			N-S:	749			N-S:	749
		E-W.	380			E-W.	418			E-W:	422			Ε-V. .:	422			E-W:	422
		SUM:	1033			SUM:	1137			SUM:	1171			SUM:	1171			SUM:	1171
No. of Phases:			3				က				က				က				3
	ith c	5	0.055			5	809 0			152	0.724			[2]	0.722			2	0.722
Volume / Capacity:	olty.	Ξ	C.053			7	о. 0.030			<u>v</u>	2.5			[7]	37.5°			<u>v</u>	27/5
בפגפו פו פפוגופ	از																		

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project:

Accutek CMA2 File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps Peak Hour: PM Annual Growth: 2.00% Peak Hour: Annual Growth:

08/06/2008 2007 2012

Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007	2007 EXIST, TRAFFIC	AFFIC	2012 \	V/ AMBIE	2012 W/ AMBIENT GROWTH	TH HE	2012 \	N/ OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	JJECT	2012 \	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	724	2	398	72	796	5	438	21	817	2	449	0	817	8	449	0	817	7	449
Comb. L-T		0	,			0	,			0	,			0	•			0	
NB Thru	1698	ო	999	170	1868	ო	623	90	1958	ო	653	18	1976	ო	629	0	1976	က	629
Comb. T-R		0	,			0	,			0	,			0				0	1
NB Right	0	0		0	0	0	,	0	0	0	,	0	0	0	,	0	0	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0	-	0	0	0		0	0	0	1	0	0	0	,	0	0	0	
Comb. L-T		0	,			0				0	,		•	0	r			0	,
SB Thru	1101	7	483	110	1211	2	531	101	1312	7	266	12	1324	2	572	0	1324	2	572
Comb. T-R		-	483	1		-	531	!	ļ	-	566	,	i i	•	572	(Ċ	Ψ.	572
SB Right	770	-	423	77	847	-	466	10	857	-	471	12	869	-	478	0	869	-	478
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0		0	0	0	,	0	0	0		0	0	0	
Comb. L-T		0	,			0	•			0	•			0				0	
EB Thru	0	0	,	0	0	0	,	0	0	0	,	0	0	0	,	0	0	0	
Comb. T-R		0	,			0				0	,			0	1			0	
EB Right	0	0		0	0	0	,	0	0	0	:	0	0	0		0	0	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	304	-	167	30	334	-	184	11	345	-	190	0	345		190	0	345	1	190
Comb. L-T		0	,			0				0	•			0	,			0	
WB Thru	7	0	341	0	7	0 1	375	0	2	0 (383	0	7	0 (383	0	2	0 0	383
Comb. T-R		0		!	•	ο.		•		o •	1	•		Э •	;	(i	Э,	1
WB Right	449	-	247	45	494		272	φ	200		2/5	0	200		2/2	0	200	- ,	5/2
Comb. L-T-R -		-				₩				-				-				-	
Crit. Volumes:		N-S:	881			N-S:	696			S-S:	1015			N-S:	1021			Ņ-S:	1021
		У-М	341			Ē-₩.	375			E-W:	383			E-W:	383			E-W:	383
		SUM:	1222			SUM:	1344			SUM:	1398			SUM:	1404			SUM:	1404
No. of Phases:			က				6				m				ო				က
Volume / Capacity:	acity:	Ξ	0.787			[2]	0.843			[2]	0.881			[2]	0.885			[2]	0.885
Level of Service:	.e.		ပ				D				٥				۵				

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
For one excl. and one opt. turn lane, 55% of overlapping left turn.
Right turns on red from excl. lanes = 50% of overlapping left turn.
[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSACATCS system installation.

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

Accutek CMA3

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

No. of Lane Added Movement Volume Lanes Volume Volume Volu	Total Volum 115 24 24 34			Added	Total No		Lane	Added Total		No. of	Lane	Added	Total	No. of	oue !
A Volume Lanes Volume 1050 - 1318 -R- 223 0 - 318 -R- 310 - 318 1276 2 638 R 1276 2 638 R 1607 1 334	105 1155 22 245 31 341 128 1404														2010
-R- 1050 3 318 -R- 223 0 - 318 -R- 310 1 310 - 1276 2 638 -R- 0 - 638 -R- 0 0 - 634 -R- 0 0 - 652	4	0 (Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	ı	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
R 1050 3 318 R-R- 223 0 - 318 310 1 310 1276 2 638 R 0 0 - 638 R 1 0 - 692 1 1 0 592		•	,	0	0	0	,	0	0	0	1	0	0	0	
R 1050 3 318 R-R- 0 - 310 1276 2 638 R 0 0 - 638 R 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 - 638 R 1 0 0 0 - 638 R 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		>	,			0				0	,			0	
FR- 223 0 - 310 310 310 1276 2 638 5 638 5 607 1 334 607 1 334 607 1 0 592	-	en +	350	127	1282	m +	383	ю	1285	ი -	384	0	1285	ო ჯ	384
-R- 0 1 310 1276 2 638 3 0 - 638 -R- 0 0 - 7 -R- 0 0 0 - 7 -R- 0 0 0 - 7 -R- 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		- c	000	g	251	- 0		0	251	- 0	500	0	251	- 0	ģ,
310 1 310 1276 2 638 0 0 - R- 0 - 607 1 334		0				0				0				0	
1276 2 638 0 0 - R- 0 0 - 607 1 334		-	341	4	345	-	345	0	345	-	345	0	345	-	345
1276 2 638 0 0 - R- 0 - 607 1 334 1 0 592		0				0				0				0	
R- 0 0 - 607 1 334 6		7 (702	46	1450	8	725	7	1452	27.0	726	0	1452	21.0	726
R- 0 607 1 334 6	_	o c		c	c	o c		c	c	> C		C	c	-	
607 1 334 6 0 - 1 0 592		0		,	,	0			·	0		•	•	0	
0 - 1 0 592	61 668	1	367	8	9/9	-	372	2	678	-	373	0	678	-	373
1 0 592		0 (,	(,	0 (Ó	,	0 (,	•	•	0 (1
-	0	0 0	651	0	-	0 0	663	0	-	o c	664	0		0 0	664
706 1 388	717 17	→	427	19	962	→	438	0	796	-	438	0	796	c	438
_R.		-				-				-				-	
WB Left 0 0 -	0 0	0		0	0	0		0	0	0		0	0	0	
- 0 L		0	,			0				0				0	
. 0 6	0	0 0		0	0	0 0	1	0	0	0 0		0	0	00	
Comb. I-K 0 -	c	o c		c	c	> c		c	c	> C		c	c	5 C	
-유-		0		,)	0		>	•	0		•	•	00	
		N-S:	702			.S:	728			N-S:	729			N-S:	729
E-W: 592		М.	651		Шζ		663			М М	664			В . В .	664
		oolie.	222		o		1392			OCINI.	282			ocivi.	<u> </u>
No. of Phases:			က			÷	m				က				က
ly: [1]		[2]	0.850			[2]	0.877			[2]	0.878		:	[2]	0.878
Level of Service: C															

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.
55% of volume is assigned to exclusive lane.
56% of volume is assigned to exclusive lane.
67% of volume is assigned to exclusive lane.
67% of voerdapping left tum.
61 work ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
62 who ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA3 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps Peak Hour: PM Annual Growth: 2.00% Peak Hour: Annual Growth:

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007 EXIST. TRAFFIC	TRAFFIC	2012	2012 W/ AMBIEN	ENT GROWTH	TH	2012 V	2012 W/ OTHER PROJECTS	PROJEC	TS	2012 V	W PROP	2012 W/ PROPOSED PROJECT	JUECT	2012	2012 W/ MITIGATION	VOIT	
***********	No. of	f Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	s Volume	Volume Volume	Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	,	0	0	0	1	0	0	0	,	0	0	0	i	0	0	0	ĵ
Comb. L-T	!	1			0				0	•			0				0	
NB Thru	1707	3 507	171	1877	e -	558 558	103	1980	ю г	586 586	=======================================	1991	m ~	589	0	1991	m +	589
NB Right	322	'	32	355	- 0	3	O	364	- 0	3	0	364	- 0	3	0	364	- 0	5
Comb. L-T-R -		0			0				0				0				0	
SB Left	362	1 362	36	398	-	398	6	407	-	407	0	407	-	407	0	407	-	407
Comb. L-T SB Thru	1054	0 - 527	105	1159	٥	- 579	102	1261	٥ ،	- 630	7	1273	0 ^	. 636	c	1273	٥ ۸	- 636
Comb. T-R		•			0		!		0		!		0		1	i	0	
SB Right	0	,	0	0	0 0		0	0	0 (0	0	0 (0	0	0 (,
Comb. L-1-K		5			ɔ				0				0				0	
EB Left	713	1 392	71	785	(432	7	792	- 6	435	7	799	- (439	0	799	- 1	439
Corno. L-1 EB Thru	φ		_	7	0	- 650	0	7	0	- 665	0	7	00	- 668	0	7	0	. 668
Comb. T-R		, 0			0	,			0	1			0	,			0	1
EB Right	587	1 323	65	645		355	22	670	₩ 1	369	0	670	- τ	369	0	670		369
Corno. F-1-A .		_			_				-				-				-	
WB Left	0	- 0	0	0	0		0	0	0		0	0	0	,	0	0	0	,
Comb. L-T					0	,			0				0				0	•
WB Thru	0		0	0	0 0		0	0	0 0	,	0	0	0 0		0	0	0 0	1
W/B Richt	c	, ,	c	c	> c		c	c	> c		c	c	> C		c	c	> C	•
Comb. L-T-R -		. 0))	0)	•	0		•	•	0))	0	
Crit. Volumes:	N-S:				N-S:	926			N-S:	993			N-S:	966			N-S:	986
	<u>Б</u> .	591			 	650		1	E-K	999			E-W:	999			E-W:	899
	SOM				SUM:	1606		-,	SUM:	1658			SUM:	1664			SUM:	1664
No. of Phases:		Ю				က				m				က				3
Volume / Capacity:		[1] 0.955			[2]	1.027			[2]	1.063			[2]	1.068			[2]	1.068
Level of Service:	ĕ.	ш				ட				ш.		:		u_				ı.

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
55% of volume is assigned to exclusive lane.
56% of volume is assigned to exclusive lane.
57% of volume is assigned to exclusive lane.
57% of vordapping left turn.
51 furn.
51 furn.
52 wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
52 wc ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Tyrone Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA4 N-S St. E-W St. Project: File Name: Counts by:

Accutek

Tyrone Avenue @ Moorpark Street Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

AM 2.0%

08/06/2008 2007 2012 Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	W AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	V/ OTHE	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012 \	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	34	o	,	က	38	0	•	-	39	0	1	0	39	0	,	0	39	0	ı
Comb. L-T		τ-	4			-	43	!	ļ	-	63	,		- 1	63	•	;	(63
NB Thru	വ	0 0		τ	9	0 0		19	52	0 0		0	52	0 0	1 1	0	52	00	
NB Right	239	→	239	24	263	> ←	263	2	265	-	265	က	268	· -	268	0	268	~	268
Comb. L-T-R -		0				0				0				0				0	
SB Left	8	0		-	6	0	1	F	10	0	,	0	10	0	-	0	10	0	,
Comb. L-T		0				0	•			0				0				0	1
SB Thru	35	0 0	55	က	35	0 0	26	20	22	0 0	79	0	22	00	79	0	22	00	79
SB Right	10	0		~	=	0		2	13	00		0	13	00	, ,	0	13	0	
Comb. L-T-R -		•				-				-				~				τ-	
EB Left	က	-	3	0	က	- 9	3	2	5	- 4	5	0	S	- 0	5	0	5	- (2
Comb. L-1 EB Thru	284	⊃ 	- 284	28	312	o ←	312	39	351	-	351	0	351	o 	351	0	351	o	351
Comb. T-R		0				0				0				0				0	
EB Right	37	~ C	37	4	4	 ⊂	4	-	45	- c	42	0	42	- c	42	0	45	 c	45
Comb. r-1-K -		>				>				0				>				>	
WB Left	297	- 0	297	30	327	- 0	327	7	329	- 0	329	2	331	- - c	331	0	331	€	331
WB Thru	759	0		76	835	0		18	853	0	. ,	0	853	00		0	853	0	
Comb. T-R		-	766			-	843			-	863			-	863			-	863
WB Right	7	00		~	80	00		7	5	٥٥		0	9	0 0	1	0	9	00	,
Comb. L-1-R -		>				>				>				•				•	
Crit. Volumes:		N-S:	66			N-S:	109			S-S:	118			:S-Z	118			S Z	118
		E-W: SUM:	898			SUM:	84 / 955			SUM:	986			SUM:	986			SUM:	986 986
No. of Phases:			ო				က				3				ო				က
Volume / Capacity:	city:	Ε	0.539			Ξ	0.600			Ξ	0.622			Ξ	0.622			[2]	0.592
Level of Service:	.;		A			-	A				В				В				A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wo ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Tyrone Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA4

Accutek

Tyrone Avenue @ Moorpark Street Peak Hour: 2.00% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	7000	CONT. EXIST TO A EELC	CEELC	2042	A// AMBIE	2012 W/ AMBIENT GBOWTH	1	2042 V	W OTHER	2012 W/ OTHER PROJECTS	<u>ا</u>	2012	W PROP	2012 W/ PROPOSED PRO IECT	J. IFCT	2012	2012 W/ MITIGATION	ATION	
	7007		2 6	7107		10 0 0 0 1 N	-	Addad	10 10 1)	946	Poppor	Total	No of		Added Total	Total	No of	or e
Movement	Volume	No. of	Volume	Volume Volume		No. or	Volume			ru, or Lanes	Volume		Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
	1	,		Ļ				c	1				6				7		
NB Left Comb. I -T	701	- c	186	2	0	-	205	7	<u> </u>	- c	227	0	0	· c	. 227	•	200	·	227
NB Thru	34	0		ო	38	0		2	28	0		0	58	0		0	28	0	
Comb. T-R		0				0	ı		:	0	•	:	;	0				0	,
NB Right	612	- -	612	61	673	- (673	က	9/9	- (929	6	989	~~ (989	0	989	 (989
Comb. L-T-R -		0				0				0				0				0	· · · · · · · · · · · · · · · · · · ·
SB Left	23	0		2	25	0		2	27	0		0	27	0	-	0	27	0	
Comb. L-T		0				0				0				0				0	,
SB Thru	32	0	75	4	39	0	82	53	99	0	115	0	68	0	115	0	89	0	115
Comb. T-R	!	0	,	•	!	0		•	;	0 (•	ć	0 (•		0 (1
SB Right	17	0 7		7	18	o +		7	20	o +		0	25	o +		0	22	⊃ -	1
Comb. L-1-K-		_				-				-				-				-	
EB Left	4	1	4	0	2	-	5	3	80	-	8	0	80	-	80	0	80	1	8
Comb. L-T		0				0				0	•			0	ı			0	,
EB Thru	200	-	200	20	220	- '	220	ဗ္ဗ	583	 (583	0	583	- (283	0	583	 (583
Comb. T-R	ų,	0 +	,	•	2	0 +	,	r	ŭ	o +	, ,	c	ŭ	o +	, ,	c	ŭ	o +	, 7
	?	- c	?	t	7	- c	?	J	5	- c	5	•		- c	5	•	5	- c	5
College First		•				•				o				•				•	
WB Left	386	-	386	38	424	-	424	3	427	-	427	12	439	-	439	0	439	-	439
Comb. L-T		0				0				0				0	1		i	0	1
WB Thru	607	0		61	999	0		33	5	0		0	701	0	,	0	5	0	,
Comb. T-R	!	-	624			-	989			-	721	•	;		721	•	;	, (721
WB Right	17	0		2	9	0	1	2	20	0		0	20	0		0	50	0	,
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	441			N-S:	486			N-S:	489			N-S:	493			N-S:	493
		Ę-∀.	886			E-W:	975			E-W:	1011			E-W:	1023			E-W:	1023
		SUM:	1328			SUM:	1460			SUM:	1500			SUM:	1516			SUM:	1516
No. of Phases:			3				9				က				၈				6

Volume / Capacity:	city:	Ξ	0.862			Ξ	0.955			Ξ	0.983			Ξ	0.994			[2]	0.964
Level of Service:	äi		D				ш				ш				Ш				E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 Project: File Name: N-S St: E-W St:

Accutek CMA5 Counts by:

CRITICAL MOVEMENT ANALYSIS

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour: AM Annual Growth: 2.0%

Date: Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	V/ OTHER	2012 W/ OTHER PROJECTS	TS	2012	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	87	-	87	6	96	-	96	14	110	ν-	110	0	110	-	110	0	110	-	110
Comb. L-T		0				0				0	1			0	•			0	1
NB Thru	163	-	117	16	180	τ	129	-	181	-	139	m	184	Ψ.	14	0	184	-	141
Comb. T-R	ì	. .	117	,	į	- 1	129	Ş	į	~ (139	•	ć	 (141	•	8	- 0	141
NB Right	71	0	ţ	7	78	0		20	88	0		0	800	0	,	0	80	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	11	-	11	-	13	-	13	13	26	-	26	0	26	-	56	0	56	-	26
Comb. L-T		0				0	•			0	,			0	,			0	1
SB Thru	206	-	206	21	227	-	227	-	228		228	2	230	-	230	0	230	-	230
Comb. T-R	;	۰ م	,	;	,	0 •	,	c	7	0 1		c	101	0 +		c	101	0 +	-
SB Right Comb. L-T-R -	1	- 0	<u>‡</u>	<u> </u>	000	- 0	000	n	0	- 0	<u> </u>	5	201	- 0	20	0	ò	- 0	2
						1													
EB Left	79	- (6/	æ	87	- (87	16	103	- (103	0	103	- 0	103	0	103	- 0	103
Comb. L-1	1104	⊃ -	י טעט	0,7	1212	o -	- 723	7	1431	o -	792	•	1432	⊃ ~	797	C	1432	-	. 792
Comb T-R	<u> </u>		929	-	2		722	2	2		792	-	10		792	•	1		792
EB Right	119	0		12	130	0		22	152	0		0	152	0	,	0	152	0	,
Comb. L-T-R -		0				0				0				0				0	
WB Left	75	+	75	7	82	-	82	13	95	-	95	0	95	-	95	0	92	-	95
Comb. L-T		0				0	,			0	1			0				0	
WB Thru	1146	-	929	115	1261	-	635	11	1338	-	929	-	1339	•	27.9	0	1339	-	677
Comb. T-R		τ-	578			_	635			_	929	•	!	•	677	•	!	Ψ.	677
WB Right	တ	0	ı		9	0	,	ιΩ	15	0		0	15	0		0	15	0	
Comb, L-T-R -		0				0				0				0				Ö	
Crit. Volumes:		-S-	293			N-S:	323			N-S:	338			Ŋ-Ś:	340			N-S:	340
		E-W:	731			E-W:	804			.: М	887			.: 	888			 	888
		SUM:	1024			SUM:	1127			SUM:	1225			SUM:	1227			SUM:	1227
No. of Phases:			2				2				2				2				2
Volume / Canacity	scitv:	111	0.613			[2]	0.651			2	0.717			[2]	0.718			[2]	0.718
Volunte / Cap	ery.	Ξ	2			<u>.</u>	5			Ī	;			Ξ	<u>;</u>			Ξ	<u> </u>
Level of Service.			٥								,								

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA5 Accutek N-S St: E-W St: Project: File Name: Counts by:

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour: 2.00% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST, TRAFFIC	4FFIC	2012	N/ AMBIL	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	V/ OTHER	2012 W/ OTHER PROJECTS	TS.	2012 \	W PROP(2012 W/ PROPOSED PROJECT	JUECT	2012	2012 W/ MITIGATION	ATION	
••••		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	190	- 0	190	19	209	← 0	209	28	237	0	237	0	237	₩ (237	0	237	← 0	237
NB Thru	557	o	347	56	613	> ← ₹	382	-	614) -	391	10	624) -	396	0	624	o ← •	396
Comb. L-R Comb. L-T-R -	137	-00	, ,	14	151	-00	305	16	167	- 0 0	6 .	0	167	-00) ()	0	167	-00	-
SB Left	36	- 0	36	4	40	- 0	40	10	20	- 0	50	0	90	- 0	20	0	50	← 0	90
SB Thru	297	o ← 0	- 297	30	327	⊃ - 0	327	τ	328	o ← 0	328	12	340	o c	340	0	340	o	340
Comb. 1-K SB Right Comb. L-T-R -	- 56	0 - 0	. 56	ø	62	0 - 0	- 62	23	82	0 - 0	- 85	0	85	0 + 0	85	0	85	0 - 0	- 85
EB Left	109	- 0	109	11	120	← 0	120	17	137	- 0	137	0	137	- 0	137	0	137	- 0	137
EB Thru	1099	o	579	110	1209) 1	637	118	1327	· - c	707	ß	1332) - -	709	0	1332) v	709
Conno. 1-R EB Right Comb. L-T-R -	. 28	-00	8/0	ဟ	64	-00	è,	22	98	-00	ē,	0	88	-00	6	0	98	-00	60
WB Left	146	- 0	146	15	160	- 0	160	12	172	- 0	172	О	172	← c	172	0	172	- c	172
WB Thru	1124	o — -	576	112	1237	o + +-	- 634 634	126	1363) - -	007 007	ĸ	1368) - -	703	0	1368) 	703
WB Right Comb. L-T-R -	- 28	. 0 0) ;	ო	31	.00	3	7	38	. 0 0	3	0	38	.00	3	0	38	-00	3
Crit. Volumes:		N-S: E-W: SUM:	488 724 1212			N-S: E-W: SUM:	537 797 1333			N-S: E-W: SUM:	566 879 1444			N-S: E-W: SUM:	578 881 1459			N-S: E-W: SUM:	578 881 1459
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	Ξ	0.738 C			[2]	0.789 C			[2]	0.863 D			[2]	0.873 D			[2]	0.873 D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

70% of vortiapping left turn.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. Assumptions:

Hazelitne Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Magnolia Boulevard AM 2.0% Peak Hour: Annual Growth:

08/07/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007 FXIST TRAFFIC	ST TRAF	FIC	2012	W AMBIE	2012 W/ AMBIENT GROWTH	E	2012	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012 V	2012 W/ MITIGATION	NOIL	
****	No.	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total		No. of	Lane
Movement	Volume Lar		Volume	Volume Volume	Volume	Lanes	Volume	Volume	_	Lanes	Volume		Volume	Lanes	Volume	Volume		Lanes	Volume
NB Left	78	- c	78	80	98	← 0	98	5	91	٠- ٥	91		92	~- 0	92	0	92	- 0	92
Comb. L-1 NB Thru Comb. T-R NB Right Comb. L-T-R -	441	000	252 252	44 8	485	000	277 277 277 -	<u>4</u> ε	499	000	285	m 0	502	000	287	0 0	502	000	287
SB Left Comb. L-T SB Thru Comb. T-R SB Right Comb. L-T-R -	126 813 93	-000	126 - 453 - 453	13	138 895 102	-000	. 138 . 498 . 498	t- £1 0	137 908 102	-000	137 505 505	7	137 915 102	-000	137 508 508	0 0 0	137 915 102	-000	137 - 508 508
EB Left	57	-	57	9	63	-	63	-	64	-	64	0	64	-	64	0	64	-	64
Comb. L-T EB Thru Comb. T-R EB Right Comb. L-T-R -	822 136	000	479	87	904	00	527	23	927	000	, 541 541	0 +	927	00 0	541	0 0	927	00	541
WB Left Comb. L-T WB Thru Comb. T-R WB Right Comb L-T-R	147 964 118	-000	147 - 541 541	15 96 12	161 1060 129	-000	. 595 595 595	2 8 2	163 1063 127	-000	. 163 595 595	- 0 0	164 1063 127	-000	164 - 595 595	0 0 0	164 1063 127	-000	. 595 595 . 595
Crit. Volumes:		N-S: E-W: SUM:	531 626 1156			N-S: E-W: SUM:	584 688 1272			N-S: E-W: SUM:	596 704 1300			N-S: F-W: SUM:	600 706 1306			N-S: E-W: SUM:	600 706 1306
No. of Phases:			5				2				2				2				2
Volume / Capacity: Level of Service:	acity:	E o	0.701			[2]	0.748 C			[2]	0.766 C			[2]	0.770 C			[2]	0.770 C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes = 50% of voerdapping left turn.

7if we ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

7if we ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Magnolia Boulevard Peak Hour: 2.00% Peak Hour: Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST. TRAFFIC	VEFIC	2012	2012 W/ AMBIENT	ENT GROWTH	HIM	2012 \	W OTHE	2012 W/ OTHER PROJECTS	TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total		Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume		Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	140	← 0	140	14	154	← (154	ß	159	- 0	159	9	165	₩.	165	0	165	 (165
Comb. L-1 NB Thru Comb. T-D	858	⊃ -	502	86	944) — r	553 553	12	956	⊃ ←	, 559 550	31	986	⊃ -	574	0	986	⊃ -	574
NB Right Comb. L-T-R -	147	-00	7	15	161	-00	,	-	162	- 0 0	,	0	162	-00	,	0	162	-00	,
SB Left	66	- 0	66	10	109	0	109	-	108	← c	108	0	108	- 0	108	0	108	- 0	108
SB Thru	657	o -	363	99	723) -	399	16	739	o +	408	29	768	o •	422	0	768	c	422
SB Right Comb. L-T-R -	69	-00	,	7	76	-00	n n n n	~	77	-00	-	0	77	-00	774	0	11	-00	,
EB Left	111	- 0	111	F	122	- 0	122	-	123	- 0	123	0	123	- 0	123	0	123	- 0	123
EB Thru	1006	· - ·	622	101	1106	o	685	4	1120) - -	694	0	1120) 	697	0	1120) - -	697
EB Right Comb. L-T-R -	239	-00	-	24	263	-00	3	ហ	268	-00	,	9	274	-00	È.	0	274	-00	i i
WB Left Comb 1-T	102	- c	102	10	112	← c	112	3	115	F C	115	9	121	- 0	121	0	121	0	121
WB Thru	512) 	285	51	563		314	=	574		319	0	574	·	319	0	574) + +-	319
WB Right Comb. L-T-R -	29	.00	,	O	65	.00	;	7	64	00		0	64	.00	?	0	9	. 0 0	2
Crit. Volumes:		N-S: E-W: SUM:	601 724 1325			N-S: E-W: SUM:	661 797 1458			N-S: E-W: SUM:	667 809 1476			N-S: SUM:	682 818 1500			N-S: E-W: SUM:	682 818 1500
No. of Phases:			7				2				2				2				2
Volume / Capacity: Level of Service:	scity:	Ξ	0.814 D			[2]	0.872 D			[2]	0.884 D			Z	0.900			[2]	006.0
1			1)				1				1			•	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes = 50% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC, as part of the Victory System No. 6.

[2] wo ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazelithe Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Riverside Drive

AM 2.0% Annual Growth: Peak Hour:

08/07/2008 2007 2012 Projection Year: Date of Count:

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	H	2012 \	√/ OTHE	2012 W/ OTHER PROJECTS	STS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	6	- c	91	ဖ	67	- c	29	0	29	₩ 0	29	4	77	~ 0	71	0	71	← 0	71
NB Thru	363	0 10 0	181	36	399	0 00 0	200	15	414	0 00 0	207	2	416	0 10 0	208	0	416	O 70 C	208
NB Right Comb. L-T-R	121	0 - 0	121	12	133	0-0	133	7	140	0 - 0	140	0	140	0 - 0	140	0	140	0-0	. 140
SB Left	205	← c	205	20	225	- c	225	9	231	← c	231	в	234		234	0	234	0	234
SB Thru	860) -	463	86	946) +	510	15	961	· · ·	518 518	80	696	· - ·	522	0	696	· c	522
SB Right Comb. L-T-R -	67	- 0 0	} ,	7	73	-00	· ,	2	75	-00	2	0	75	-00		0	75	-00	-
EB Left	24	← c	24	2	26	- €	26	-	27	- 0	27	0	27	- 0	27	0	27	- 0	27
EB Thru	740	· - ·	403	74	815	· - c	443	30	845	· v	458	7	852	· •	464	0	852	· - c	464
Comb. L-T-R	99	-00	5	7	72	-00	?	0	72	-00	,	4	76	-00	,	0	76	-00	- 4 0 4
WB Left	344	- c	344	34	379	- 0	379	9	385	- 0	385	0	385	- 0	385	0	385	€	385
WB Thru	844	0 70 0	422	84	929	000	464	24	953	0 00 0	476	9	928	0 10 0	479	0	958	0 00 0	479
WB Right Comb. L-T-R -	138	0 0	138	41	152	0 - 0	152		158	0 0	158	4	162	0 - 0	162	0	162	0 ~ 0	162
Crit. Volumes:		N-S: E-W: SUM:	525 747 1272			N-S: E-W: SUM:	577 822 1399			N-S: E-W: SUM:	586 843 1429			N-S: E-W: SUM:	594 848 1442			N-S: E-W: SUM:	594 848 1442
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	icity:	[5]	0.778 C			Ε	0.863 D			[1]	0.882 D			[1]	0.891 D			[2]	0.861 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 70% or volume is assigned to exclusive lane. 70% or volume is assigned to exclusive lane. 70% or volume is assigned to exclusive lane. 50% or overlapping left turn. 13 fly for ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] wo ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazelithe Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Hazeltine Avenue @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	2012 W/ AMBIEI	ENT GROWTH	F	2012 V	W OTHE	2012 W/ OTHER PROJECTS	TS.	2012 \	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012 W	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume V	Volume	Lanes	Volume
NB Left	136	T	136	4	150	₩.	150	0	150	₩.	150	25	175	₩.	175	0	175	-	175
Comb. L-T NB Thru	844	0 8	422	84	929	5 0	464	15	944	9 0	472	5	959	р 0	479	0	959	5 0	- 479
Comb. T-R NB Right Comb. L-T-R -	249	0 - 0	249	25	273	0 0	273	4	277	0 - 0	277	0	277	0 - 0	277	0	277	0-0	- 277
SB Left	168	(168	11	185	- 0	185	r	190	F- 0	190	13	203	F- 0	203	0	203	- 0	203
SB Thru	795	o ₹	444	79	874	o	488	18	892	o ← ₹	497	32	924) -	513	0	924	o ← +	513
SB Right Comb. L-T-R -	69	-00	-	O	102	-00	1 0	~	103	-00	ĝ '	0	103	- 0 0	;	0	103	-00	2 .
EB Left	92	- 0	92	6	101	- 0	101	2	103	- 0	103	0	103	- 0	103	0	103	- 0	103
EB Thru	610	o	363	61	672	o — •	399	30	702	o •	414	56	728) - 1	436	0	728	o •	436
Comb. L-R-Comb. L-T-R-	115	-00	g, '	12	127	-00	660	0	127	-00	, <u>4</u>	17	144	-00	,	0	144	-00	-
WB Left	229	- 0	229	23	252	- 0	252	5	257	- 0	257	0	257	- c	257	0	257	- 0	257
WB Thru	587	0 00 0	293	29	645	0 10 0	323	35	089	0 70 0	340	25	735	0 70 0	368	0	735	0 00 0	368
WB Right Comb. L-T-R -	179	0 0	179	18	197	0-0	197	ω	202	0 - 0	202	41	243	0 + 0	243	0	243	0 ~ 0	243
Crit. Volumes:		N-S: E-W: SUM:	591 592 1182			N-S: E-W: SUM:	650 651 1301			N-S: E-W: SUM:	662 671 1333			N-S; E-W; SUM;	688 692 1381			N-S: E-W: SUM:	688 692 1381
No. of Phases:			2		The state of the s		2				2				2				2
Volume / Capacity: Level of Service:	city:	Ξ	0.718 C			[1]	0.797 C			[1]	0.819 D			[1]	0.850 D			[2]	0.820 D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

Fight turns on red from excl. lanes = 120% of overlapping left turn.

Fight craits includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Fight craits reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 CMA8 Accutek N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Fashion Square Lane AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

08/07/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	OUNT EVIET TOACEIC	DACEIC	2042	2042 W/ AMERIC	HENT COOMTH	HLM	2012	THE /W	2012 W/ OTHER DRO IFCTS	CTS	2012	W PROP	2012 W/ PROPOSED PROJECT	O.F.CT	2012 V	NOITABITINA W 1906	NOIT	
	No of		Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	ume Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	12 1	12	-	4	← (14	0	4	~ (14	0	14	← 0	14	0	14	- 0	4
Comb. L-T NB Thru	523 1	273	52	575	o - ∙	300	22	597	o v	311	-	598) - -	314	0	598	o - ∙	314
Comb. I-K NB Right Comb. L-T-R -	23 0		2	25	-00	006	0	25	-00	- - -	ហ	30	-00	، د	0	30	-00	1
SB Left	44 1	44	4	48	- 0	48	0	48	- 0	48	12	09	- 0	09	0	09	← c	09
SB Thru	1180 1	594	118	1298) -	653	22	1320	O r	664	0	1320) -	664	0	1320	- -	664
SB Right Comb. L-T-R -	7 0	, 9 4	~	ω	-00	8 .	0	ю	-00	, ,	0	∞	- 0 0	5	0	ω	- 0 0	,
EB Left	2 1	2	0	2	- 0	2	0	2	- 0	2	0	2	- د	5	0	2	← C	2
EB Thru	0	1 1	0	0	0 0 7		0	0	00 +		0	0	0 0 4		0	0	0 0 7	
Comb. I-R EB Right Comb. L-T-R -	7 0	,	τ	ω	-00	•	0	ω	-00	0	0	ω	-00	0	0	ω	-00	0
WB Left	+ 0	-	0	-	F- C	-	0	-	← c	-	3	4	- c	4	0	4	- c	4
WB Thru	0	, ,	0	0	0 0	. ,	0	0	0 +	,	0	0	000	1 1	0	0	000	1 1
WB Right Comb. L-T-R -	- 0 0	,	0	7	- 0 0	1	0	2	- 0 0	1	7	4	0 0	4	0	4	0 - 0	4
Crit. Volumes:	N-S: E-W: SUM:	606 8 615			N-S: E-W: SUM:	667 9 676			N-S: E-W: SUM:	678 9 687			N-S: E-W: SUM:	678 12 690			N-S: E-W: SUM:	678 12 690
No. of Phases:		ъ				3				က				က				3
Volume / Capacity: Level of Service:	[1]	0.361 A			Ξ	0.404 A			Ξ	0.412 A			Ξ	0.414 A			[2]	0.384 A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

I'll wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

I'll x ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA8 Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Fashion Square Lane Peak Hour: PM Annual Growth: 2.00% Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2012

ALTERNATIVE E PROJECT

	1000			1 0700		1000	11211	1 0700	THE CAN	11 000	OTC	2040	0000 118	TOTI OGG CEOCED MY CANCE	TO 10	2040	MOITA SITING 181 CACC	MOIL	
	2007 EXIST. IRAFFIC	SI. IR	AFFIC	2072	W/ AMBI	2012 W/ AMBIENI GROWIN	E .	2012 V	Total	2012 W/ OTHER PROJECTS	<u>ה</u>	Addad	TOTAL	אר לה פון היי לה פון	ביים ביים מפר	Addod	Total	2 2	0
Movement	No. or Movement Volume Lanes		Lane Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	1	-	11	-	13	-	5	0	13	-	13	0	13	-	<u>რ</u>	0	5	~	13
Comb. L-T		0				0				0				0	1			0	1
NB Thru	1013	τ- τ	541	101	1114	- τ	595	19	1133		605	5	1138	. .	617	0	1138	- τ	617
Comb. 1-X	70	- c	541	7	77	- c	CAC -	c	11	- c	600	20	26	- 0	<u> </u>	0	97	- 0	<u>.</u>
Comb. L-T-R	,	0		-	:	00		•	•	0		i	;	0		i		0	
SB Left	206	-	206	21	227	-	227	0	227	-	227	50	277	-	277	0	277	-	277
Comb. L-T		0	,			0				0	,			0				0	•
SB Thru	878		441	88	996	-	486	23	989	-	497	0	989	- -	497	0	989	-	497
Comb. T-R		← c	441	•	ď	₩ 0	486	c	ď	← c	497	c	u	- c	497	c	ď	 c	497
Comb. L-T-R	n ı	00		-	Ď	00		>	o	00	•	•	•	0	ı	•	•	0	ı
EB Left	9	-	9	-	7	-	7	0	7	-		0	7	-	7	0	7	-	7
Comb. L-T		0				0				0				0				0	,
EB Thru	-	0		0	τ-	0		0	~	0		0	_	0		0	_	0	
Comb, T-R		-	0			Ψ-	7			•	=			-	Ξ			•	7
EB Right	6	0	,	-	9	0		0	9	0		0	9	0		0	9	0 (
Comb. L-T-R	•	0				0				0				0				0	
WB Left	76	-	76	8	84	-	84	0	84	-	84	20	104	-	104	0	104	-	104
Comb. L-T		0	,			0				0				0				0	ŧ
WB Thru	_	0	1	0	-	0		0	ν	0	,	0	Ψ-	0		0	•	0	
Comb. T-R		, 	24	1	i	- '	29	•	i	 (29		Î	0 -	, '	C	ŗ	0 1	, ,
WB Right	53	0	,	C)	28	0	ı	0	28	0		14	7.5	, ,	(.5		7./	- (7/
Comb. L-T-R		0				0				0				0				0	
Crit. Volumes:		ίς	747			N-S:	822			:S-N	831			S-S	894			N-S:	894
		E-W:	98			Е-W.	92			E-W:	95			E-W.	115			E-W:	115
	ns	.: M	834			SUM:	917			SUM:	926			SUM:	1009			SUM:	1009
No. of Phases:	S:		3				9				3				ო				6
Volume / Capacity:	pacity:	Ξ	0.515			Ξ	0.573			Ξ	0.580			Ξ	0.638			[2]	0.608
Level of Service:	ice:		A				A			:	A				В			:	В

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes. 55% of volume is assigned to heavier lane. 70% of volume is assigned to exclusive lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Fight turns on red from excl. lanes = 50% of overlapping left turn. Assumptions:

For one excl. and one opt. turn lane, 50% of volume is assigned to exclusive lane.

13 v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

12 v/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA9 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Moorpark Street AM 2.0% Annual Growth: Peak Hour:

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	2007 EXIST. TRAFFIC	TRAFFIC	2012	2012 W/ AMBIEN	ENT GROWTH	H.	2012 V	V/ OTHE	2012 W/ OTHER PROJECTS	TS.	2012 \	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012 V	2012 W/ MITIGATION	VIION	
	No. of	of Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume	Volume Lanes	s Volume	Volume	Volume Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	43	1 43	4	47	~	47	0	47	·	47	0	47	- (47	0	47	- (47
Comb. L-T NB Thru	215	1 121	22	237	0	133	22	259	o -	145	2	261	0	146	0	261	0	146
Comb. T-R	;			;	← (133	(ć	- 0	145	ć	ć	← (146	Ċ	Ċ	~ (146
NB Right Comb. L-T-R -	56		m	29	00		m	32	00	1	0	32	00	,	0	35	00	1
SB Left	167	1 167	17	184	-	184	0	184	-	184	0	184	- 1	184	0	184	- (184
Comb, L-T SB Thru	904	0 - 1 627	06	994	0 -	- 689	21	1015	0	700	-	1016	0	701	0	1016	0	701
Comb. T-R SB Right	349	•	35	384	- 0	689	0	384	- 0	. 100	2	386	r- 0	- 701	0	386	- 0	- 70
Comb. L-T-R -		0			0				0				0				0	
EB Left	93	1 93	ō	102	- c	102	О	102	- c	102	င	105	- c	105	0	105	- c	105
EB Thru	392	1 392	39	431	o (431	42	473	o ← 0	473	0	473	· c	473	0	473) (473
Comb. 1-R EB Right	52	1 52	ហ	57	o —	. 57	0	22	o €	. 57	0	25	o ← (- 57	0	22	o ← (57
Comb. L-T-R -		0			0				0				0				0	
WB Left	86	1 86	6	95	- c	96	4	66	- c	66	0	66	 C	66	0	66	۰ ۵	66
WB Thru	711	1 407	71	782	-	447	22	804	·	458	0	804	, —	459	0	804) (-	459
Comb. T-R	102	1 407	Ç	112	- c	447	c	112	c	458	-	113	- c	459	c	113	- c	459
Comb. L-T-R -	2	0 0	2	!	0		,	!	0		•		0		•	2	0	
Crit. Volumes:	N-S:				N-S:	736			N-S:	747			N-S:	748				748
	E-W: SUM:	: 499 1: 1168			SUM:	549 1285			E-W: SUM:	572 1319			E-W: SUM:	572 1320			E-W: SUM:	572 1320
No. of Phases:	į	2				2				2				2				2
Volume / Capacity:		[1] 0.709			[2]	0.757			[2]	0.779	The state of the s		[2]	0.780			[2]	0.780
Level of Service:	.;	ပ				O				ပ				U				C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA9 Accutek N-S St; E-W St: Project: File Name; Counts by;

Hazeltine Avenue @ Moorpark Street
Peak Hour: PM
Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

Annual Growth:

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

No. of Movement Volume Lanes NB Left 98 Comb. L-T C	of Lane						i)	!				NOTING AN WILLIAM STOR		:	
<u>+</u>		Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
	nes Volume		Volume Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
	1 98	9 10	108	← (108	0	108	← (108	0	108	- (108	0	108	~- ¢	108
Comb T-B	1 266 1 266	5 48	527) - -	292	18	545) - -	302	σ	554) - -	307	0	554	> ← ←	307
Comb. L-T-R -	- 0 0	ທ	57	-00	767	73	59	-00	700	0	29	-00) -	0	29	-00	 205 -
SB Left 165 Comb I-T	1 165	5 17	182	c	182	0	182	- c	182	0	182	- 0	182	О	182	- c	182
SB Thru 430	371	43	472	· ·	408	21	493) +	419	ιΩ	498	·	427	0	498	· ··· ·	427
SB Right 313 Comb. L-T-R -		31	344	-00	9	0	344	-00	, <u>,</u>	12	356	-00	77.	0	356	-00	
EB Left 303	1 303	3 30	333	- 0	333	0	333	- c	333	10	343	- 0	343	0	343	- 0	343
EB Thru 669	. 669	. e7	736	o — 0	736	38	774	o ← 0	774	0	774	o c	774	0	774) - (774
Comb. 1-K EB Right 93 Comb. L-T-R -	. 93	о	102	0 - 0	102	0	102	0 - 0	102	0	102	o + c	102	0	102	0 - 0	102
WB Left 76	1 76	3	84	- 0	84	2	98	- 0	86	0	86	← c	86	0	86	- 0	86
WB Thru 523 Comb. T-R WB Right 144 Comb. L-T-R -	1 333 1 333 0 -	3 52	575	00	367	38	613	000	386	о м	613	000	387	0 0	613	000	387
Crit. Volumes: N-S: E-W: SUM:	S: 469 W: 745 M: 1214	m 10 →		N-S: E-W: SUM:	516 819 1335			N-S: E-W: SUM:	526 859 1386	E		N-S: E-W: SUM:	535 859 1394			N-S: E-W: SUM:	535 859 1394
No. of Phases:		2			2				2				2				2
Volume / Capacity: Level of Service:	[1] 0.739 C	6		[2]	0.790 C			[2]	0.824 D			[2]	0.829 D			[2]	0.829 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.
Right turns on red from excl. lanes = 50% of overlapping left turn.
[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Hazeltine Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA10 Accutek N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Ventura Boulevard AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST, TRAFFIC	VFFIC	2012	W AMBIE	2012 W/ AMBIENT GROWTH	F	2012	W OTHER	2012 W/ OTHER PROJECTS	TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Гапе	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	ane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume		_	Lanes	Volume		Volume	Lanes	Volume	Volume		Lanes	Volume
NB Left	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. L-T		0				0				0				0	,			0	1
NB Thru	0	0		0	0	0		0	0	0	,	0	0	0	,	0	0	0	ı
Comb. T-R		0				0	,			0	,			0				0	ı
NB Right	0	0		0	0	0	•	0	0	0		0	0	0		0	0	0	•
Comb. L-T-R -		0				0				0				0				0	
SB Left	322	2	177	32	355	2	195	-	356	2	196	0	356	2	196	0	356	2	196
Comb. L-T		0	,			0				0				0				0	
SB Thru	0	0	,	0	0	0		0	0	0		0	0	0	,	0	0	0	
Comb. T-R		0	,			0				0				0	,			0	1
SB Right	513	-	513	51	564	-	564	21	585	-	585	~	586	-	586	0	586	-	586
Comb. L-T-R -		0				0				0				0				0	
EB Left	96	1	96	10	105	-	105	24	129	-	129	-	130	F	130	0	130	-	130
Comb. L-T		0				0				0	1			0	•			0	
EB Thru	1101	7	551	110	1211	2	909	154	1365	2	683	0	1365	2	683	0	1365	2	683
Comb. T-R		0				0				0				0				0	1
EB Right	0	0		0	0	0	ı	0	0	0	ı	0	0	0		0	0	0	1
Camb. L-T-R -		0				0				0				0				0	
WB Left	0	0		0	0	0	,	0	0	0		0	0	0		0	0	0	-
Comb. L-T		0	,			0	1			0				0	ı			0	
WB Thru	1356	-	739	136	1492	-	813	93	1585	-	860	0	1585	•	861	0	1585	-	861
Comb. T-R	į	Ψ.	739		,	- '	813		1	- -	860			 1	861			τ	861
WB Right	123	o (12	135	0 (-	136	0 0	,	_	137	0 0	,	0	137	0 (
Comb. L-1-K -		>				o)				0				0	
Crit. Volumes:		N-S:	465			N-S:	511			N-S:	520			N-S:	521			N-S:	521
		E-W:	835			E-W:	919			E-W:	066			Ë-W:	991			E-W:	991
		SUM:	1300			SUM:	1430			SUM:	1510			SUM:	1512			SUM:	1512
No. of Phases:			2				2				2				2			ì	2
Wolume / Canacity:	ity	Ξ	0.797			[6]	0.853			[2]	700.0			2	8000			2	0000
volunte / capar			<u>.</u>											7]				7	
Level of Service.			اد				اد				וע								п

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Hazeltine Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA10 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Ventura Boulevard Peak Hour: PM 2.00% Annual Growth: Peak Hour:

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	7006	SONT EXIST TRAFFIC	VEELC	2042	W AMBIE	MAMBIENT GROWTH	H	2012	W OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
	7007	No of	2 6	Added	Total	No of		Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement Volume			Volume	Volume Volume	Volume	Lanes	Volume		-	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB I off			,	c	C	o	-	0	.0	0	,	0	0	0	,	0	0	0	,
Comb. L-T	•	0		•	o	0	,	,		0				0				0	,
NB Thru	0	0		0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. T-R		0				0				0		•	•	0 (•	•	0 0	1
NB Right	0	0		0	0	0		0	0	0	,	0	0	0 0		0	0	0 0	
Comb. L-T-R -		0				0				0				0				o	
SB Left	181	2	100	18	199	2	109	3	202	2	111	0	202	2	111	0	202	2	111
Comb. L-T		0				0	,			0			,	0	1	•	,	0 (•
SB Thru	0	0	,	0	0	0 (0	0	0 0		0	0	0 0		0	>	o c	1
Comb. T-R	216	o -	216	22	238	- c	238	20	258	o ←	258	Ŋ	263	-	263	0	263	-	263
Comb. L-T-R -		0	i i			0				0				0				0	
EB Left	203	٢	203	50	223	-	223	16	239	-	239	9	245	-	245	0	245	- (245
Comb. L-T	77	0 (. 797	147	1631	۰ ٥	, 84	130	1760	۰ م	088	c	1760	o 6	- 880	С	1760	o 0	- 880
Comb T-R	4/4	v C	<u>.</u>	<u> </u>	707	4 0	- - '	3	3	10	3	•		0	})		0	1
EB Right	0	0	1	0	0	0		0	0	0	,	0	0	0		0	0	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0	,	0	0	0 (0	0	0 0		0	0	0 0	1	0	0	0 0	
Comb. L-T	!	0		•		.	'	,	1	> 4	,	•	404	7	י ט	c	1511	· •	900
WB Thru	1237		753	124	1360		828	<u>.</u>	<u> </u>		905	0	2		906	•	2		906
WB Right	269	. 0	3	27	296	0		က	299	0	,	2	301	0	1	0	301	0	ı
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	115			N-S:	126			N-S:	138			S-N	140			N-S:	140
		:: ≧ ⊽	956			 ∑ 	1051 1178				1144			SUM:	1151 1292			SUM:	1292
			<u>-</u>			<u>.</u>				1									
No. of Phases:			2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.644			[2]	0.685			[2]	0.755			[2]	0.761			[2]	0.761
Level of Service:	,	:	œ	i			В				ပ				O				ပ

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Woodman Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA11 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Magnolia Boulevard Peak Hour: AM 2.0% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/06/2008 Date of Count: Projection Year:

2007

ALTERNATIVE E PROJECT

	7002	COULT EXIST TRAFFIC	AFFIC	2012	W/ AMRII	2012 W/ AMBIENT GROWTH	E	2012	W OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	NOIT	
	7007	ביים אלים	2 1	7177		, o old	900	Added	Total	Jo CN	ane I	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	No. or Lanes	Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
NB Left	96	-	96	10	105	-	105	0	105	Ψ.	105	0	105	- 0	105	0	105	← 0	105
Comb. L-T NB Thru	740	0 1	421	74	815	0 - 1	463	28	843	0 - 1	477	0	843	o	478	0	843	O *~ *-	478
Comb. T-R NB Right Comb. L-T-R -	102	-00	- 421	0	112	-00	, 463	0	112	-00	,	***	113	-00) }	0	113	-00	,
SB Left	165	-	165	17	182	- 0	182	77	181	- 0	181	0	181	- 0	181	0	181	- 0	181
Comb. L-T SB Thru	1265	0	692	126	1391	o •	761	21	1412	o +	- 177 177	ស	1417) - -	774 774	0	1417) - -	774
Comb. I-K SB Right Comb. L-T-R -	119	-00	,	12	130	-00	2	0	130	-00		0	130	-00		0	130	00	1
EB Left	64	-	64	9	71	- (71	0	71	- 0	71	0	71	- 0	71	0	71	- c	71
Comb. L-1 EB Thru	872	o − .	477	87	959	o — ,	525	24	983	o	537	0	983) +	537	0	983) - -	537
Comb. T-R EB Right Comb. L-T-R -	83	-00		α	92	-00	676 ,	0	92	-00	ec -	0	92	-00	3	0	95	-00	3
WB Left	107	- (107	17	118	- 0	118	-	119	- 0	119	0	119	- 0	119	0	119	- c	119
Comb. L-1 WB Thru	945	→ ~ C	538	95	1040	>	592	ю	1043) - -	593 593	•	1044	o	593 593	0	1044) 	593 593
WB Right Comb. L-T-R -	131	-00	,	13	144	-00	1	?	142	.00		0	142	00		0	142	00	ı
Crit. Volumes:		N-S: BUM:	787 603 1390			N-S: E-W: SUM:	866 663 1529			N-S: E-W: SUM:	877 663 1540			N-S: E-W: SUM:	879 664 1543			N-S: E-W: SUM:	879 664 1543
No. of Phases:	1.5		2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	Ξ	0.857 D			[2]	0.919 E			[2]	0.927 E			[2]	0.929 E			[2]	0.929 E

Assumptions:

Maximum Sum of Cnitical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA11 Woodman Avenue Project: File Name: N-S St: E-W St:

Accutek Counts by:

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

Woodman Avenue @ Magnolia Boulevard Peak Hour:

2.00%

Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

	7000	OUT 1 4 CL	CILL	1		CO HIGH	į	0700	1	11.000		1			1				
	7007	17 K) L	ZU1Z	W/ AMBI	2012 W AMBIENI GROWIN		7 7 1 7 1		2012 W/ OTHER PROJECTS	2	2012	Z Z	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	66	Ψ-	66	10	109	-	109	0	109	-	109	0	109	-	109	0	109	-	109
Comb. L-T		0				0	•			0				0	,			0	,
NB Thru	1042	-	570	104	1146		627	22	1168	-	638	0	1168	-	641	0	1168	۲	641
Comb. T-R		-	570			•	627			Ψ-	638			-	641			۲	641
NB Right	86	0		9	108	0	,	0	108	0		ဖ	114	0	,	0	114	0	
Comb. L-T-R -		0				0				0				0				0	
SB Left	92	-	76	8	84	-	84	-	83	-	83	0	83	-	83	0	83	-	83
Comb. L-T		0	•			0	1			0	,			0		•	:	. 0	}
SB Thru	834	-	462	83	917	-	209	24	941	•	521	18	929	_	530	0	929	,	530
Comb. T-R		-	462			-	509			-	521			-	530			-	530
SB Right	8	0		თ	100	0		0	100	0		0	100	0		0	100	0	,
Comb. L-T-R -		0				0				0				0				0	
EB Left	87	-	87	6	96	-	96	0	96	-	96	0	96	۳	96	0	96		96
Comb. L-T		0				0	1			0	ı			0	1			0	
EB Thru	937		526	94	1031	-	578	14	1045	-	585	0	1045	-	585	0	1045	-	585
Comb. T-R		-	526			-	578			_	585			-	585			-	585
EB Kight	114	0 (Ξ	126	0		0	126	0		0	126	0	,	0	126	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	104	-	104	10	114	-	114	-	115	-	115	0	115	-	115	0	115	1	115
Comb. L-T		0				0	,			0	•			0				0	
WB Thru	545	-	315	54	299	-	346	Ξ	610	-	351	9	616	-	354	0	616	-	354
Comb. T-R	i	(315	1	;	- -	346				351			-	354				354
WB Right	84	0		œ	93	0		Τ	95	0	1	0	95	0	,	0	95	0	
Comb. r-1-x -		0				0				0				0				0	
Crit. Volumes:		N-S:	646			N-S:	710			N-S:	720			N-S:	723			N-S:	723
-		E-W.	630			E-W:	693			E-W:	701			Ē-Ķ	701			E-W:	701
		SUM:	1276			SUM:	1403			SUM:	1421			SUM:	1424			SUM:	1424
No. of Phases:			2				2				2				2				2
]																	
Volume / Capacity:	cıty:	Ξ	0./80			[2]	0.835			[2]	0.847			[2]	0.849			[2]	0.849
Level of Service:	e:		ပ				٥				٥				D				Q

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

80% of volume is assigned to exclusive lane.

71 w craito includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w craitos reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Westfield Fashion Square /1-05-3606-1 Woodman Avenue Riverside Drive Project: File Name: N-S St: E-W St:

Accutek CMA12

Counts by:

Woodman Avenue @ Riverside Drive AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012 \	W AMBIE	2012 W/ AMBIENT GROWTH	 	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	JECT	2012 V	2012 W/ MITIGATION	NOIT	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	183	0.0	101	18	201	25.0	11	0	201	20.00	111	18	219	0.0	121	0	219	2.0	121
NB Thru	734	O 00 C	367	73	808	0 70 0	404	20	828	0 10 0	414	0	828	0 00 0	414	0	828) N C	414
Comb. I-R NB Right [2] Comb. L-T-R -	219	0 0	219	22	241	0 - 0	241	52	293	0 - 0	293	0	293	0 ~ 0	. 293	0	293	0 ~ 0	293
SB Left	229	- 0	229	23	252	- 0	252	7	259	- 0	259	0	259	c	259	0	259	0	259
SB Thru	1165	O 70 C	582	116	1281	0 00 0	641	4	1295	0 70 0	648	က	1298	0 10 0	649	0	1298	7 N C	475
SB Right [2] Comb. L-T-R -	111	0 0	111	=	122	0 0	122	0	122	0 0	122	ო	125	0 - 0	125	0	125	-00	, 6,
EB Left	92	- 0	95	5	104	- 0	104	-	105	- 0	105	0	105	 د	105	0	105	- 0	105
EB Thru	880	0 77 0	440	88	968	0 70 0	484	4	1009	0 00 0	504	ស	1014	0 00 0	507	0	1014	0 70 0	507
Comb. L-T-R -	116	0 - 0	116	12	128	0 - 0	128	~	129	0 - 0	129	30	159	0 - 0	159	0	159	0 - 0	159
WB Left	291	- 0	291	58	320	- 0	320	35	355	- 0	355	0	355	- 0	355	0	355	- 0	355
WB Thru	1010	o 00 0	505	101	1111	0 70 0	555	35	1146	9 79 0	573	12	1158	0 70 1	579	0	1158	9 70 0	579
WB Right	185	o	185	19	204	o c	204	7	211	⊃ - 0	211	0	211	o	211	0	211	o - c	211
כסוום. ב-ז-א-		>				0				5				5				5	
Crit. Volumes:		N-S: F-W:	683 731			Ŋ-S: F-W:	751 804			N-S: E-W:	758 860			N-S: E-W:	770 862	: :		N-S: E-W:	673 862
		SUM:	1414			SUM:	1556			SUM:	1618			SUM:	1632			SUM:	1535
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	city:	[5]	0.959 E			Ξ	1.061 F			Ξ	1.107 F			Ξ	1.117 F			[3]	1.016 F

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

For one excl. and form excl. lanes = 50% of overlapping leff turn.

Solved to excl. and the explication of ATSAC as part of the Victory System No. 6.

[2] Northbound right turn has an overlapping phase with the wastbound left-turn movement. and southbound right turn has an overlapping phase with the eastbound left-turn movement. [3] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Westfield Fashion Square /1-05-3606-1 Woodman Avenue Riverside Drive Project: File Name: Counts by: N-S St: E-W St:

Accutek CMA12

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Riverside Drive Peak Hour: Annual Growth:

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST TRAFFIC	AFFIC	2012	2012 W/ AMRIE	HTWORU THE	E	2012	V/ OTHE	2012 W/ OTHER PROJECTS	TS	2012	W PROP	2012 W/ PROPOSED PROJECT	D.IECT	2012	2012 W/ MITIGATION	NOITA	
		No of		Valdod	- F	, o	906	Added	Total	No.	9 -	Pappy	Total	No.	000	Added	Total	N I	900
Movement	Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		_	Lanes	Volume		Volume	Lanes	Volume
1		2	188	34	376	2	207		377	2	208		448	2	247		448	7	247
Comb. L-T		0	•			0				0				0	•			0	
NB Thru Comb T-R	917	0	459	92	1009	N C	505	<u>~</u>	1027	00	514	0	1027	2 0	514	0	1027	0 0	514
NB Right [2]	197	, 4	197	20	216	-	216	^	223	· —	223	0	223) 	223	0	223	· 	223
Comb. L-T-R -		0				0				0				0				0	
SB Left	150	-	150	15	165	+	165	5	170	-	170	0	170	-	170	0	170	-	170
Comb. L-T SB Thru	823	0 7	411	82	905	0 %	452	20	925	0 0	462	0	935	0 0	- 467	0	935	0 0	385
Comb. T-R		0				0				0	,			0					385
SB Right [2] Comb. L-T-R -	188	- 0	188	19	207	-0	207	Ψ-	208	- 0	208	12	220	-0	220	0	220	00	
EB Left	213	٠ (213	21	235	- (235	-	236	- 0	236	0	236	- (236	0	236	- (236
Comb. L-1 EB Thru	916) N C	458	95	1008) N C	504	38	1046) N C	- 523	36	1082	0 77	541	0	1082	9 6	541
Comb. T-R EB Right	257	0 -	- 257	26	283	0	283	-	284	o - -	284	300	584	0 -	584	0	584	0 -	584
Comb. L-T-R -		0				0				0				0				0	
WB Left	239		239	24	263	0	263	9	269	~~ C	269	0	269	← c	269	0	269	← ¢	269
WB Thru	206	0 70 0	453	91	866	0 01 0	499	42	1040	0 70	520	46	1086	0 77 0	543	0	1086	0 70 0	543
Comb. 1-R WB Right	254	o – c	254	25	279	o – c	279	4	283	o ~ c	283	0	283	⊃ ~ c	283	0	283	o – c	- 283
Coinb. F-1-R-		>				5				5				>				>	
Crit. Volumes:		P-S: F-W:	608 697			Ŋ-S: E-W:	669 767			N-S: F-W:	683 792			N-S: F-W:	714 810			N-S: E-W:	683 810
	0,	SUM:	1306			SUM:	1436			SUM:	1475			SUM:	1524			SUM:	1493
No. of Phases:			4				4				4				4				4
Volume / Capacity:	oity:	[1]	0.880			Ξ	0.975			[1]	1.003			Ξ	1.038			[3]	0.986
Level of Service:	.;						ш				4				ı				Е

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200,

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

Fight turns on red from excl. lanes = 50% of overlapping left turn.

If If yor exclusion due to installation of ATSAC and to the Victory System No. 6.

[2] Northbound right turn has an overlapping phase with the westbound left-turn movement and southbound right turn has an overlapping phase with the westbound left-turn movement. 3] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps AM 2.0% Annual Growth: Peak Hour:

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST, TRAFFIC	AFFIC	2012 \	W AMBIE	2012 W/ AMBIENT GROWTH	ЛH	2012 \	V/ OTHE	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
	_	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement Volume Lanes	Volume 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	348	-	348	35	383	-	383	-	384	-	384	0	384	-	384	0	384	-	384
Comb. L-T		0				0	1			0				0			:	0	
NB Thru	890	ო	297	88	979	ო	326	27	1036	က	345	ω	1044	ന	348	0	1044	ო (348
Comb. T-R		0				0	•		•	0 1	ı	•	•	0 (•	•	0 (
NB Right	0	0		0	0	0		0	0	0	ı	0	0	0 (0	0	0 (,
Comb. L-T-R -		0				0				0				0				0	
SBLeft	0	0		0	0	0	-	0	0	0	,	0	0	0	-	0	0	0	-
Comb. L-T		0				0	•			0	,			0	,			0	•
SB Thru	1062	4	265	106	1168	4	292	45	1210	4	303	9	1220	4	305	0	1220	4	305
Comb. T-R		0				0	ı			0	,			0				0	,
SB Right	545	Ψ-	545	54	299	-	299	6	909	-	809	2	613	-	613	0	613	-	613
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0		0	0	0	,	0	0	0		0	0	0	-	0	0	0	
Comb. L-T		0	•			0				0				0				0	
EB Thru	0	0	ı	0	0	0		0	0	0		0	0	0		0	0	0 (,
Comb. T-R		0				0			•	0	:	4	•	0 (•	•	0 (
EB Right	0	0	,	0	0	0		0	0	0 1		0	0	0 0		0	0	0 (1
Comb. L-T-R -		0				0				0				5				Э	
WB Left	314	1	173	31	345	- (190	18	363	- (200	0	363	- (200	0	363	- c	200
Comb. L-T	•	0 ((5 (,	•	ı	> c	,	c	L	> C	,	c	ч	0	-
WB Ihru	4	-	762	0	n	o c	87 '	0	n	o c	900	>	O	0 0	500	>	0	0 0	600
WB Right	265	·	146	27	292	·	160	14	306	· 	168	80	314	-	173	0	314	-	173
Comb. L-T-R -										-				-					
Crit. Volumes:		N-S:	893			.; N-0;	983			.S-N -N-∴	993			N-S:	998			N-S:	998 309
		SUM:	1158			SUM:	1274			SUM:	1298			SUM:	1307			SUM:	1307
No. of Phases:			3				ო				က				3				ဂ
Volume / Capacity:	oity:	Ξ	0.743			Ξ	0.824			Ξ	0.841			Ξ	0.847			[2]	0.817
Level of Service:	'n		O				۵				۵				٥				

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 Accutek N-S St: E-W St: Project: File Name: Counts by:

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

Woodman Avenue @ Ventura Freeway Westbound Ramps Peak Hour: PM

2.00%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	2000	CHEACT TOINE TOOK	CIEDA	2040	DOLD IN ABOUT	UTAICOC TIME	1	1 6406	A OTUE	2012 W/ OTHER BBO IECTS	,TC	2042	0000 //V	TOST COO 0300000 /W 0100	TOE C	2012	NOITS IN MITIGATION	ACITA		
	7007) L £	7107	EGMIN /A	ייין פאס	<u> </u>	7107		1	2	707		מייים איי	2 -	4104			-	
		No. of	Lane	Added	lotal	No. of	Lane	Added		No. of	rane		otal	NO. OI	Lane	Added		NO. 01	Lane	
Movement \	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	
NB Left	314	τ-	314	31	345	-	345	2	347	-	347	0	347	•	347	0	347	~	ď	347
Comb. L-T		0	,			0	,			0	,			0	•			0		
NB Thru	1186	က	395	119	1304	ო	435	56	1330	ო	443	34	1361	ო	454	0	1361	က	4	454
Comb. T-R		0	ı			0	,			0	,			0	,			0		
NB Right	0	0	i	0	0	0	,	0	0	0	,	0	0	0		0	0	0		
Comb. L-T-R -		0				0				0				0				0		
SBLeft	0	0		0	0	0		0	0	0		0	0	0		0	0	0		
Comb. L-T		0	,			0	,			0	ı			0	,			0		
SB Thru	917	4	229	92	1009	4	252	28	1037	4	259	29	1104	4	276	0	1104	4	2	276
Comb. T-R		0				0	ı			0				0	,			0	,	
SB Right	486	-	486	49	534	-	534	7	533	-	533	33	999	-	266	0	266	-	ιΩ	566
Comb. L-T-R -		0				0				0				0				0		
EB Left	0	0		0	0	0	,	0	0	0		0	0	0		0	0	o		
Comb. L-T		0	1			0				0	•			0	1			0		
EB Thru	0	0		0	0	0	,	0	0	0	•	0	0	0		0	0	0		
Comb. T-R		0				0	,			0	ì			0				0		
EB Right	0	0	,	0	0	0	ı	0	0	0	,	0	0	0 (0	0	0 (
Comb. L-T-R -		0				0				0				0				0		
WB Left	402	-	221	40	443	-	244	16	459	1	252	0	459	-	252	0	459	1	2	252
Comb. L-T	1	0		•	,	0 1	. !	•	•	0 ('	•	•	0 (,	•	•	0 0		3
WB Thru	0	0 0	344	0	0	0 0	379	0	0	0 0	387	0	0	o c	401	0	0	o c	4	
W/R Picht	363	- c	,	98	300	- c	220	*	400	o	220	33	433	-	238	0	433	· -		238
Comb. L-T-R -			}	3		-				•				-				-		
Crit. Volumes:		: N-S:	800			S-N	880			N-S:	881			N-S:	914			N-S:	6	914
		SUM:	1144 144			SUM:	3/9 1259			SUM:	387 1267			SUM:	1315			SUM:	4 ξ	1315
No. of Phases:			က				3				က				က					ю
Volume / Capacity:	ity:	Ξ	0.733			Ξ	0.813			[1]	0.819			Ξ	0.853			[2]	0.8	0.823
Level of Service:			C				D			,	D								D	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Woodman Avenue @ Ventura Freeway Eastbound Ramps AM 2.0% Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/06/2008 Date: Date of Count: Projection Year;

2007

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	V/ OTHEF	2012 W/ OTHER PROJECTS	TS	2012 \	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0	ı	0	0	0	ı	0	0	0	•	0	0	0	
Comb. L-T		0	ı			0	,			0	ı			0	,			0	•
NB Thru	786	თ -	262	79	865	ო -	288	4	606	თ •	303	ო	912	ი -	304	0	912	m ·	304
Comb. T-R	i	 (329	Ċ	C	- (395	ç	,	- c	413	c		- 0	413	c	6	- c	4 د ر
NB Kight	326	5 (ı	8	385	o 0	1	20	513	> 0	1	>	4 13	o c	,	0	2	o c	1
Comb. r-1-k		>				>				>				>				•	
SB Left	340	-	340	34	374	-	374	10	384	-	384	5	389	٢	389	0	389	-	389
Comb. L-T		0	į			0				0				0	,			0	
SB Thru	1093	2	547	109	1202	2	601	20	1252	2	929	ഗ	1257	7	629	0	1257	7	629
Comb. T-R		0	ı			o				0	ı		,	0				0	
SB Right	0	0	1	0	0	0		0	0	0	ı	0	0	0	,	0	0	0	•
Comb. L-T-R -		0				0				0				0				0	
EB Left	339	-	186	34	373	-	205	14	387	-	213	5	392	1	216	0	392	-	216
Comb. L-T		0	1			0	;	,	•	0	,	•	•	0 (•	•	0 (
EB Thru	က	0 0	332	0	ო	0 0	365	0	ო	0 0	372	0	ო	0 0	374	0	m	0 0	374
Comb K	ć	э,	,	ć	Š	5	1	•	ţ	> •	,	c	7	> •	,	c	,	> v	,
Comb 1-T-R	385		716	e.	4 2	- •	/87	_	432		220	>	432		007	0	432		730
		•				-				-				•					
WB Left	0	0 (,	0	0	0 0		0	o	0 0	1	0	0	0 0		0	o	0 0	
Comb. L-1		Э.				Э.	•	,		יכ		4	•	o (,	יכ	,
WB Thru	0	0 (0	0	0 (·	0	0	0 0		0	0	0 (0	0	0 0	ŧ
Comb.	•	э (ı	•	•	o ((c	.		c	c	5 6	,	c	c	0 0	
Comb. L-T-R -		00	1	0	>	0	1	0	>	00		0	•	0		•	0	0	
																			1
Crit. Volumes:		7- М-М:	332			.Υ. .Υ. .Υ.	365			М.: М.:	372			ii ii Na Na Na Na Na Na Na Na Na Na Na Na Na	374 374			:;;; - - - - - - - - - - - - - - - - - -	374 374
		SOM:	1031			SOIM:	45.			OO	6011			o O IVI	0/-			OOM:	0/
No. of Phases:			m				ю				က				m	:			က
Volume / Capacity:	acity:	Ξ	0.654			[2]	0.696			[2]	0.720			[2]	0.725			[2]	0.725
Level of Service:	, ji		æ				В				O				O				ပ

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.
For one excl. and one opt. tum lane, 55% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wc ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Freeway Eastbound Ramps

CRITICAL MOVEMENT ANALYSIS

ã 2.00% Annual Growth: Peak Hour:

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	/TH	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0		0	0	0	ı	0	0	0	,	0	0	0	,
Comb. L-T		0	ļ			0				0	,			0	,			0	•
NB Thru	1094	ന	365	109	1203	හ ·	401	28	1231	ო -	410	=	1242	ტ -	414	0	1242	ლ •	414
Comb. T-R	į	-	365	1	;		401	•	;	- (414	•	;	- (414	•	3	 (414
NB Right	364	0 0	ı	36	400	0 0		4	414	00		0	414	o c		0	414	5 C	
Comb. r		>				>				>				>				>	
SB Left	287	-	287	29	316	-	316	0	316	1	316	36	352	-	352	0	352	-	352
Comb. L-T		0	1			0				0	1			0	•			0	
SB Thru	1010	7	505	101	1111	2	222	45	1156	2	278	8	1186	2	593	0	1186	2	593
Comb. T-R		0				0				0	ı			0			•	0	•
SB Right	0	0	,	0	0	0	,	0	0	0	,	0	0	0 (,	0	0	0 (
Comb. L-T-R -		0				0				0				0				0	
EB Left	467	-	257	47	514	-	283	0	514	-	283	19	533	-	293	0	533	-	293
Comb. L-T		0	į			0				0				0	,			0	
EB Thru	-	0	372	0	~	0	409	0	-	0	410	0	-	0	418	0	-	0	418
Comb. T-R		0				0				0	•			0	•	•	;	0	
EB Right	357	-	196	36	392		216	2	394		217	0	394	,	217	0	394		217
Comb. L-T-R -		-				-				-								-	
WB Left	0	0		0	0	0	-	0	0	0		0	0	0		0	0	0	1
Comb. L-T		0				0	,			0				0				0	1
WB Thru	0	0	,	0	0	0	1	0	0	0		0	0	o	,	0	0	0	•
Comb. T-R		0	ţ			0				0	•			0	,			0	
WB Right	0	0		0	0	0		0	0	0		0	0	0 (0	0	0 (
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	652			N-S:	717			N-S:	730			N-S:	766			N-S:	992
		Σ Μ.:	372 1023			.; ₩. ₩.	409			E-W:	410			SUM:	418			E-W:	418
		:	2				1			,	• • •								
No. of Phases:			က				က				ဗ				က				n
Volume / Capacity:	city:	Ξ	0.648			[2]	0.690			[2]	0.700			[2]	0.731			[2]	0.731
Level of Service:	 O		В				В				В				S				c

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 55% of volume is assigned to exclusive lane. Fight turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15 N-S St: E-W St:

Accutek Project: File Name: Counts by:

Woodman Avenue @ Moorpark Street

AM 2.0%

Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012 V	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	DJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume \	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	12	1	12	-	14	-	14	0	4	₩.	4	0	4	₩ (4	0	4	~ (4
Comb. L-T NB Thru	490	7 0	- 245	49	539	7 0	- 269	22	596	5 0	298	2	598	9 0	- 299	0	598	9 7 C	- 299
Comb. T-R NB Right Comb. L-T-R -	. 24	0 - 0	- 24	2	26	0 - 0	. 26	7	28	0 - 0	- 28	0	28	0-0	. 28	0	28	0 - 0	- 28
SB Left	297	- 1	297	30	327	- (327	2	329	- 0	329	-	330	- 0	330	0	330	- 0	330
Comb. L-T SB Thru	851	o - ·	528	85	936	o	581	48	984	o ₁	605	ო	987	O +	607	0	987) - -	607
Comb. T-R SB Right Comb. L-T-R -	206	-00	- 528	21	227	-00	1 281	0	227	-00	coe -	0	227	-00	200	0	227	-00	3
EB Left	111	-	111	11	122	-	122	0	122	-	122		123	- 0	123	0	123	- 0	123
Comb. L-T EB Thru	551	0 -	551	55	909	o ← (909	28	634	⊃ - (634	0	634	o	634	0	634) ~ (634
Comb. T-R EB Right Comb. L-T-R -	. 40	0 - 0	- 40	4	43	0-0	43	0	43	o c	43	0	43	o - c	43	0	43	0 - 0	43
WB Left	85	-	85	6	94	- 0	94	2	96	- 0	96	0	96	- 0	96	0	96	F 0	96
Comb. L-T WB Thru	726	0 - 0	726	73	799	o ← 0	799	43	842	o	842	-	843	o ← c	843	0	843	o — c	843
Comb. I-K WB Right Comb. L-T-R -	276	0 - 0	276	28	303	0 0	303	4	307	0 - 0	307	0	307	0-0	307	0	307	0-0	307
Crit. Volumes:		N-S: E-W: SUM:	542 837 1380			N-S: E-W: SUM:	597 921 1518			N-S: E-W: SUM:	627 964 1591			N-S: E-W: SUM:	629 966 1595			N-S: E-W: SUM:	629 966 1595
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	Ξ	0.850 D			[7]	0.942 E			[1]	0.991 E			[1]	0.993 E			[2]	0.963 E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Moorpark Street Peak Hour: 2.00% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	-0000	CONT. EVICT TOAREIL	2042	2042 MA AMBIEN	TWI COOMTI	F	V C F U C	THEE	2012 W/ OTHER BOOLECTS	ST.	2042	9099 //	TOEL MI PROPOSED PRO IECT	D IFCT	2042	2012 W/ MITIGATION	NOITY	
	2007 EAIS	יויוארויי	7107		ACCES TALL	_	7107		ייייייייייייייייייייייייייייייייייייייי	2	7 .		22.0		7			
	No. of	of Lane	Added	Added Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement V	Volume Lanes	ss Volume	Volume	Volume Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	67	1 67	7	73	-	73	0	73	-	73	0	73	~	73	0	73	τ-	73
Comb. L-T	277	. 0 . 386	77	849	0 0	- 424	44	893	0 8	- 446	ω	901	0 8	450	0	901	0 7	450
Comb. T-R	1	1			0	į ,		}	0		ļ.		0		1		0	1
NB Right	22	1 55	9	61	-	61	2	63	-	63	0	63	•	63	0	63	•	63
Comb, L-T-R -		0			0				0				0				0	
SB Left	263	1 263	26	289	1	289	0	289	-	289	10	299	-	299	0	299	-	299
Comb. L-T		- 0			0				0	•			0	1			0	1
SB Thru	734	1 492	73	808	-	542	47	855	-	565	70	875	•	575	0	875	,	575
Comb. T-R		1 492			.	542	•	į	τ- (265	•	į	· (575	•	į	 1	575
SB Right	251		25	276	0		0	276	0	,	0	276	0		0	276	0 (•
Comb. L-T-R -		0			0				0				0				0	
EB Left	205	1 205	20	225	-	225	0	225	-	225	2	227	-	227	0	227	-	227
Camb. L-T		. 0			0			1	0		,	i	0	,	1	į	ο ·	,
EB Thru	685	1 685	69	754	- 0	754	32	789	- c	789	0	789	~ ς	789	0	789	- c	789
FR Right	83	- 1	60	06	-	6	0	6	- c	- -	0	6	·	6	0	06	→	 06
Comb. L-T-R -	!	. 0		:	0				0				0				0	
WB Left	71	1 71	7	78	- c	78	ო	2	 ⊂	8	0	81	~ C	20	0	8	- c	. 84
WB Thru	551	1 551	55	909	-	909	45	651	·	651	2	653	,	653	0	653	-	653
Comb. T-R		. 0			0				0				0				0	
WB Right	258	1 258	26	284	-	284	0	284	-	284	0	284	τ-	284	0	284	-	284
Comb. L-T-R -		0			0				0				0				0	
Crit. Valumes:	0-X				N-S:	714			N-S:	736			N-S:	750			N-S:	750
	E-W:	756			Ē-₩.	832			Ë-₩.	877			Ë-W:	881			E-W:	881
	SUN				SUM:	1546			SUM:	1613			SUM:	1631			SUM:	1631
No. of Phases:		2				2				2				2				2
Volume / Capacity:	itv:	[1] 0.867			E	0.960			E	1.005			Ε	1.017			[2]	0.987
Level of Service:	, hi	Ω				Ш				Ŀ			,	F				П

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wic ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wic ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 N-S St: E-W St: Project:

File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Boulevard AM 2.0% Annual Growth: Peak Hour:

08/06/2008 2007 2012

Projection Year: Date of Count:

ALTERNATIVE E PROJECT

	2007	2007 EXIST TRAFFIC	AFFIC	2012	W AMBI	2012 W/ AMBIENT GROWTH	HL.	2012 V	W OTHER	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SUECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Totai	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume			Lanes	Volume	Volume	Volume	Lanes	Volume		Volume	Lanes	Volume
NB Left	51	-	51	ĸ	99	-	56	8	64	-	64	0	64	-	64	0	64	+	64
Comb, L-T		0 (,	;	i	0 (•	(1	0 (t	•	Ċ	0 (ı	C	Ċ	0 0	1
NB Thru	201	0 +	. 210	20	221	o -	- 241	9	757	o -	251	-	228	o -	- 252	0	528	⊃ -	252
NB Right	19	- 0	2 ,	2	21	- 0		4	25	- 0	3 ,	0	25	- 0	,	0	25	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	216	1	216	22	238	-	238	7	245	-	245	2	247	+	247	0	247	-	247
Comb. L-T SB Thru	225	o ~	. 225	22	247	0 -	247	5	252	o -	252	_	253	o –	253	0	253	0 -	253
Comb. T-R	362	0 +	362	36	398	0 -	398	38	436	o -	436	0	436	0	436	0	436	o -	- 436
Comb. L-T-R -		0				0				0				0				0	
EB Left	142	-	142	14	157	-	157	41	198	-	198	0	198	+	198	0	198	- 1	198
Comb. L-T FB Thru	1082	0 -	554	108	1190	o -	- 609	96	1286	0	- 665	0	1286	o +-	- 665	0	1286	o -	- 665
Comb. T-R		-	554			-	609			-	665	,	!		999	,	!	-	999
EB Right	26	00	,	ო	29	00		16	45	0 0		0	45	0 0		0	45	00	
Comb. L-1-7		>				>				>				>				>	
WB Left	45	- 0	45	4	49	- 0	49	+	20	- 0	20	0	20	۰ ۵	20	0	20	- 0	20
WB Thru	1091	- c	- 603	109	1200	- c		52	1252	· -	695	•	1253	·	969	0	1253	- c	969
Comb. T-R	-		603	3		-	963	i			695	•		· -	969	1		-	969
WB Right	114	0	,	=	126	0		12	138	0		-	139	0		0	139	0	•
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	436			N-S:	479			N-S:	496			N-S:	499			N-S:	499
		E-W.	745			E-W:	820			E-W:	893			E-W.	894			Щ-	894
		SUM:	1181			SUM:	1299			SUM:	1389			SUM:	1393			SUM:	1393
No. of Phases:			2				2				2				7				2
Volume / Capacity:	city:	Ξ	0.717			[2]	0.766			[2]	0.826			[2]	0.829			[2]	0.829
Level of Service:	es.		O				ပ				۵				D				D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] wc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Boulevard 2.00% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

Date of Count: Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

	2007	2007 EXIST, TRAFFIC	4FFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	Ŧ	2012	N/ OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	59	-	59	9	65	+	65	24	89	-	88	0	68	-	89	0	89	-	89
Comb. L-T NB Thru	213	00	, ,	21	235	00	, ,	9	241	00		9	247	00		0	247	00.	1 1
Comb. T-R NB Right	φ	~ 0	219	*	7	- 0	241	2	თ	- 0	- 249	0	6	- 0 (255	0	σ	- 0 0	. 255
Comb. L-T-R -		0				0				0				0				o	
SB Left	125	- (125	12	137	- 0	137	-	136	- 0	136	12	148	~ с	148	0	148	← C	148
Comb. L-T SB Thru	161	0 -	161	16	177	o ← (177	4	181	o ← c	181	ဖ	187	o ← c	187	0	187	o ← c	187
Comb. T-R SB Right Comb. L-T-R -	237	0 0	237	24	261	o + 0	261	47	308	0 - 0	308	-	309	0 - 0	309	0	309	0 - 0	309
EB Left	206	-	206	21	227	-	227	40	267	-	267	0	267	- (267	0	267	- 0	267
Comb. L-T EB Thru	1093	0 -	567	109	1202	o	. 623	84	1286	o ← ·	674	0	1286	o ·	674	0	1286	· - c	674
Comb. T-R EB Right Comb -T-R -	14	-00	567	4	45	0 0	623	18	63	-00	- 674	0	63	-00	6/4	0	63	-00	, ,
WB Left	24	-	24	2	26	-	26	-	27	-	27	0	27	-	27	0	27	-	27
Comb. L-T	; ;	· O +		· 6	ç	0 +	1	a	1087	o -	י	0	1089	0 +		C	1089	0 -	- 611
WB Infu Comb. T-R	0		515	n n	3		200 200	3	3		609	1	2		611	,		 .	611
WB Right Comb. L-T-R -	120	00	1	12	132	00	1	0	132	00	t	23	134	00	,	0	134	00	ı
Crit. Volumes:		N-S: E-W: SUM:	344 721 1065			N-S: E-W: SUM:	379 793 1171			N-S: E-W: SUM:	386 876 1261			N-S: E-W: SUM:	404 878 1281			N-S: E-W: SUM:	404 878 1281
No. of Phases:			2				2	-			2				2		:		2
Volume / Capacity:	acity:	[5]	0.640			[2]	0.681			[2]	0.741 C			[2]	0.754 C	ī		[2]	0.754 C
Level of Service.	ij		0												,				

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

Accutek CMA17

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT

	2007	2007 EXIST, TRAFFIC	MFFIC	2012	2012 W/ AMBIE!	ENT GROWTH	HE.	2012	N/ OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume
NB Left	0	0	t	0	0	0	,	0	0	0	,	7	Ξ	4	#	0	1	-	1
Comb. L-T		0	ı			0				0				0	1			0	,
NB Thru	0	0	ì	0	0	0	•	0	0	0		0	0	0	•	0	0	0	
Comb. T-R		0				0				0	,			0				0	,
NB Right [3]	0	0	,	0	0	0	•	0	0	0	ı	72	72	1/3	40	0	72	7	40
Comb. L-T-R -		0				0				0				0				0	
SB Left	35	0	-	4	39	0		0	39	0		-39	ę	0		0	P	0	,
Comb. L-T		0				0	,			0				0				0	
SB Thru	0	0	59	0	0	0	65	0	0	0	65	0	0	0	,	0	0	0	,
Comb. T-R		0	,			0				0	,			0	,			0	,
SB Right	24	0		7	56	0		0	56	0	1	36	62	_	62	0	62	Ψ-	62
Comb. L-T-R -		τ				-				-				0				0	
EB Left	9	-	9	-	7	1	7	0	7	1	7	-7-	P	0	-	0	o-	0	
Comb. L-T		0				0				0	,			0	•			0	1
EB Thru	1017	2	509	102	1119	2	559	43	1162	2	581	-	1163	7	281	0	1163	7	581
Comb. T-R		0				0	,			0				0	ı			0	
EB Right	0	0		0	0	0	,	0	0	0		62	62	_	62	0	62	-	62
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0	,	0	0	0		0	0	0		290	290	2	160	0	290	2	160
Comb. L-T		0	,			0				0	1			0	ı			0	,
WB Thru	1103	-	556	19	1214		611	36	1250	Ψ-	629	Ψ-	1251	-	630	0	1251		630
Comb. T-R		_	556			-	611				629			_	630			-	630
WB Right	ω	0		-	თ	0		0	თ	0		0	თ	0		0	ത	0	•
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:	;	N-S:	59			N-S:	65			N-S:	65			N-S:	73			:S-Z	73
		E-W.	562			E-W:	618			E-W.	636			E-W.	741			Ē-₩.	741
		SUM:	621			SUM:	684			SUM:	702			SUM:	814			SUM:	814
No. of Phases:))				n				D				က				3
Volume / Capacity:	city:		0.518				0.570				0.585			141 191	0.471			141 [2]	0.471
Voidine / Capi			2								3			[4] ([4]				[7] [7]	
Level of Service.	Ď.		۲				۲				۲				τ.				ť

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Wor ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1

Accutek CMA17 N-S St: E-W St: Project: File Name: Counts by:

Date: Date of Count: Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour:

Projection Year:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT

Annual Growth:

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	2012 W/ AMBIE	ENT GROWTH	Ŧ	2012 \	W OTHE	2012 W/ OTHER PROJECTS	STS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0		0	0	0		110	110	-	110	0	110	•	110
Comb. L-T		0	,			0				0	,			0	,			0	
NB Thru	0	0	,	0	0	0		0	0	0	,	0	0	0		0	0	0	ı
Comb. T-R		0	1			0				0	,			0	,			0	į
NB Right [3]	0	0		0	0	0		0	0	0		780	780	7	429	0	780	7	429
Comb. L-T-R -		0				0				0				0				0	
SB Left	27	0	,	က	30	0		0	30	0	,	-30	q	0		0	o o	0	
Comb. L-T		0				0				0	,			0	•		,	0	,
SB Thru	0	0	46	0	0	0	20	0	0	0	20	0	0	0	ı	0	0	0	
Comb. T-R		0	1			0				0	•			0				0	1
SB Right	19	0	1	2	21	0		0	21	0		28	49	-	49	0	49	-	49
Comb. L-T-R -		-				₩.				-				0				0	
EB Left	21	-	21	2	23	-	23	0	23	-	23	-23	Ŷ	0		P	P	С	
Comb. L-T		0				0				0				0	,			0	,
EB Thru	1046	7	523	105	1151	7	575	33	1190	7	295	9	1196	2	298	0	1196	2	598
Comb. T-R		0				0				0	1			0				0	
EB Right	0	0		0	0	0		0	0	0		131	3	-	131	0	131	**	131
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0	ļ.	0	0	0	-	0	0	0		670	670	2	369	0	670	2	369
Comb. L-T		0	,			0				0				0	1			0	1
WB Thru	1168	-	299	117	1285	-	629	44	1329	-	681	ო	1332	•	682	0	1332	-	682
Comb. T-R		-	599			-	629			-	681			-	682			-	682
WB Right	9	0		ო	33	0		0	33	0	ı	0	33	0		0	33	0	
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	46			N-S:	50			N-S:	50			Ŋ-S:	158			N-S:	158
		Ę-Ķ	620			Ę-Ķ	682			E-W:	704			E-W.	996			Ę-W.	996
		SUM:	999			SUM:	732			SUM:	754			SUM:	1125			SUM:	1125
No. of Phases:			D				Э				ח			,	က				က
Volume / Caracity	ihr		0 555				0,00				000			5	000			12	
volume, capa											0.628			[1], [2]	0.689			[7]. [2]	0.689
Level of Service:	iii		Α				В				В				В				œ

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of vowerapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Wo ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name:

CMA7

City Traffic Counters Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/07/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST TRAFFIC	AFFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	Ę	2012 \	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	1	_	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	173	- 0	173	17	190	- 0	190	0	190	0	190	30	220	- 0	220	o	220	~ 0	220
NB Thru	899	O 70 C	334	67	735	O 70 C	367	23	758	0 10 0	379	18	776	O 00 C	388	0	776	0 10 0	388
Colinb. 1-R NB Right Comb. L-T-R -	229	0 - 0	229	23	252	0 - 0	252	5	262	0-0	262	0	262	0 - 0	262	0	262	0-0	262
SB Left	178	- 0	178	18	196	- 0	196	5	201	- 0	201	19	220	- 0	220	0	220	← C	220
SB Thru	662) - -	391	99	728	o	430	26	754	· c	444	46	800) 	467	0	800) - -	467
SB Right Comb. L-T-R -	120	-00		12	132	-00	2	₩-	133	-00		0	133	-00	ĵ,	0	133	-00)
EB Left	39	← c	39	4	43	- 0	43	7	45	- 0	45	0	45	⊂	45	0	45	r- c	45
EB Thru	519) 	345	52	571	· - ·	379	52	623	· •	405	37	099	· - ·	436	0	099	0 (4) (330
Comb. L-R-Comb. L-T-R-	170	-00	,	17	187	-00	n o	0	187	-00	9	25	212	-00	,	0	212	0 - 0	212
WB Left	223	- c	223	22	245	F C	245	13	258	- 0	258	0	258	⊢ c	258	0	258	← C	258
WB Thru	358	0 00 0	179	36	394	0 7 0	197	43	437	. 4 0	218	42	478	0 KV C	239	0	478	0 00 0	239
WB Right Comb. L-T-R	17	0 - 0	Ξ	#	122	0-0	122	ເດ	127	0 0	127	25	152	0 - 0	152	0	152	0-0	152
Crit. Volumes:		N-S: E-W: SUM:	564 568 1132			N-S: E-W: SUM:	620 624 1245			N-S: E-W: SUM:	634 663 1297			N-S: E-W: SUM:	687 694 1381			N-S: E-W: SUM:	687 588 1275
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	[1]	0.684 B			[1]	0.760 C			[1]	0.795 C			[1]	0.851 D			[2]	0.750 C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] vc ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] vc ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 N-S St: E-W St:

CMAB Project: File Name: Counts by:

City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Fashion Square Lane Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/07/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	2007 EXIST. TRAFFIC	TRAFFIC	2012 V	V/ AMBIL	2012 W/ AMBIENT GROWTH	WTH	2012	V/ OTHE	2012 W/ OTHER PROJECTS	CTS	2012	W PROP	2012 W/ PROPOSED PROJECT	OJECT	2012	2012 W/ MITIGATION	VLION	
Movement	No. of Volume Lanes	Lane Volume	Added Total Volume Volume	Total Volume	No. of Lanes	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	Added	Total Volume	No. of Lanes	Lane Volume	Added	Total Volume	No. of Lanes	Lane
											1							
NB Left	æ ← c	2	*	9	~ €	9	0	φ	- c	9	0	9	- - c	9	0	9	- c	ဖ
NB Thru	688	432	69	757	c	475	33	790	- c	491	7	797	o +-	509	c	797	→ c	509
Comb. T-R	}	432	3	5	Ψ-	475	3		Ψ-	491	•		-	509)	5		203
NB Right	175	•	18	193	0	•	0	193	0		58	222	0		0	222	0	,
Comb. L-T-R -	0				0				0				0				0	***************************************
SB Left	437 1	437	44	481	-	481	0	481	-	481	71	552	-	552	0	552	-	552
Comb. L-T	0	1			0	,			0	,			0	,			0	1
SB Thru	625 1	315	63	688	τ-	346	39	727	-	365	0	727	-	365	0	727	-	365
Comb. T-R	•	315			τ-	346			•	365			-	365			Ψ-	365
SB Right	4		0	4	0 0		0	4	0 0		0	4	0 0		0	4	0 (,
Comb. L-1-K-					>				>				5				0	
EB Left	5	5	-	9	-	9	0	9	-	9	0	9	-	9	0	9	-	9
Comb. L-T	0	•			0	,			0				0				0	,
EB Thru	2 0	ı	0	2	0		0	2	0		0	2	0		0	2	0	,
Comb. T-R	-	9			τ-	7			Ψ-	7			•	7			τ-	7
EB Right	4	•	0	4	0		0	4	0	,	0	4	0		0	4	0	ı
Comb. L-T-R	0				0				0				0				0	
WB Left	101 1	101	10	111	-	111	0	111	-	111	24	135	-	135	0	135	-	135
Comb. L-T	0	,			0	,			0				0				0	
WB Thru	0	1	0	0	0		0	0	0		0	0	0	•	0	0	0	•
Comb. T-R		133			Ψ-	146			•	146			0	•			0	
WB Right	133 0	ı	13	146	0	į	o	146	0	Í	17	163	,	163	0	163	•	163
Comb. L-T-R	0 -				0				0				0				0	
Crit. Volumes:		869			N-S:	955			-S-K	972			S-S:	1061			io-Z	1061
	E-W:	138			E-W.	152			E-W.	152			Ē-¥.	142			E-W:	142
	SUM:	1007			SUM:	1107			SUM:	1124			SUM:	1203			SUM:	1203
No. of Phases:		3				3				п				က				3
					:										į			
Volume / Capacity:	acity: [1]	0.636 B			Ξ	0.707			Ξ	0.719			Ξ	0.774			[2]	0.744
Level of Get vi						,				,				>				ر

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 50% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Woodman Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA12 City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST. TRAFFIC	AFFIC	2012	W/ AMBII	2012 W/ AMBIENT GROWTH	Ŧ	2012 \	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012 \	W PROP	2012 W/ PROPOSED PROJECT	SUECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	484	~ ~	266	48	532	~ <	293	~	533	2.0	293	102	635	7 0	349	0	635	77	349
NB Thru	674	0 10 0	337	29	741	0 00 0	371	32	773	9 79 6	387	0	773	0 10 0	387	0	773	0 00 0	387
Comb. L-T-R -	. 209	0 ~ 0	509	21	230	0 + 0	230	17	247	0 - 0	247	0	247	0 0	247	0	247	0 ~ 0	247
SB Left	66	- 0	66	10	109	- 0	109	5	114	- 0	114	0	114	F 6	114	0	114	- 0	114
SB Thru	842	0 10 5	421	84	926	0 10 0	463	33	959	0 10 0	480	15	974	0 00 0	487	0	974	· 10 C	405
SB Right [2] Comb. L-T-R -	500	0 + 0	200	20	220	0-0	220	ო	223	0 0	. 223	18	241	0 - 0	241	0	241	-00	- 403
EB Left	197	- 0	197	20	217	← c	217	4	221		221		232	- 0	232	0	232	- 0	232
EB Thru	753	0 00 0	377	75	828	0 00 0	414	64	892	0 10	446	44	936	0 101 0	468	0	936	0 00 0	468
Comb. L-T-R -	386	0 0	389	38	428	0-0	428	-	429	0 - 0	429	174	603	0 0	. 603	0	603	0 - 0	603
WB Left	234	0	234	23	257	+- €	257	28	285	~ c	285	0	285	- 0	285	0	285	0	285
WB Thru	591	N C	296	99	650	0 N C	325	56	706	0 N C	353	29	773	0 70 0	387	0	773	2 12 0	387
WB Right Comb. L-T-R -	134	0 - 0	134	13	147	0 - 0	147	ო	150	0 - 0	150	0	150	0 0	150	0	150	0 - 0	150
Crit. Volumes:		N-S: E-W: SUM:	687 611 1298			N-S: E-W: SUM:	756 672 1427			N-S: E-W: SUM:	773 732 1505			N-S: E-W: SUM:	837 754 1590			N-S: E-W: SUM:	755 754 1508
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	acity:	Ξ	0.874 D			[1]	0.968 E			[1]	1.024 F			[1]	1.086 F			[6]	0.997 E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volumis assigned to exclusive lane.

80% of volumis assigned to exclusive lane.

17 of reduction axed. lanes = 50% of overlapping left turn.

18 of ratio from excl. lanes = 70% of volumis in the Victory System No. 6.

19 Northbound right turn has an overlapping phase with the wastbound left-turn movement and southbound right turn has an overlapping phase with the wastbound left-turn movement [3] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 Woodman Avenue N-S St: E-W St: Project: File Name:

City Traffic Counters CMA13 Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Annual Growth:

Projection Year: Date: Date of Count:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST. TRAFFIC	4FFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	νтн	2012	N/ OTHER	2012 W/ OTHER PROJECTS	TS	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SUECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Totai	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume
NB Left	416	- (416	42	458	- (458	7	460	₩ (460	0	460	- (460	0	460	← (460
Comb. L-T NB Thru	991	ဝက	330	66	1090	0 ო	. 363	48	1138	oπ	379	4	1182	ဝက	394	0	1182	O M	394
Comb. T-R NB Right Comb. L-T-R -	0	000	1 1	0	0	000	1 1	0	0	000	1 1	0	0	000	1 1	0	0	000	1 1
SB Left	0	0	,	0	0	0	,	0	0	0	,	0	0	0		0	0	0	1
Comb. L-T SB Thru	1062	0 4	266	106	1168	04	292	57	1225	04	306	82	1307	04	. 327	0	1307	04	. 327
Comb. T-R SB Right Comb. L-T-R -	493	0 - 0	493	49	542	0 0	542	ស	547	0 - 0	547	4	588	0-0	. 588	0	588	0 - 0	- 588
EB Left	0	0		0	0	0		0	0	0	-	О	0	0	,	0	0	0	
Comb. L-T EB Thru	0	001		0	0	000	1 1	0	0	000		0	0	000	1 1	0	0	00	
Comb. T-R EB Right Comb. L-T-R -		000		0	0	000		0	0	000		0	0	000		0	0	000	1 1
WB Left	318	- 0	175	32	350	0	192	32	382	- 0	210	0	382	- 0	210	0	382	- 0	210
WB Thru	ო	000	270	0	ო	000	297	0	ო	000	312	0	ო	000	334	0	ო	000	334
Comb. I-K WB Right Comb. L-T-R -	275	C	151	28	303	C	166	2	305	> ~ ~	167	48	353	c	194	0	353	>	194
Crit. Volumes:		N-S: E-W: SUM:	909 270 1179			N-S: E-W: SUM:	1000 297 1297			N-S: E-W: SUM:	1007 312 1319			N-S: E-W: SUM:	1048 334 1382			N-S: E-W: SUM:	1048 334 1382
No. of Phases:			m				က				က				က				ဗ
Volume / Capacity: Level of Service:	acity:	Ξ	0.757 C			[1]	0.840 D			E	0.856 D			Ε	0.900 D			[2]	0.870 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes = 50% of volume spart of turn.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] V/C ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14

City Traffic Counters

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

CRITICAL MOVEMENT ANALYSIS

08/06/2008 2007 2012 Date: Date of Count: Projection Year:

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	10000			1				1	1		9								
	2007 EAIS	2		7107		ZUIZ WI AMBIENI GROWIN	<u> </u>	7107	ביים ביים	ZUIZ WOUNER PROSECIS	0	7 7 107	בור אינו ביינו	ZUIZ WI PROPOSED PROJECT	200	7107	5	2	
	No. of			Added Total		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume	Volume Lanes		Volume Vol	Volume Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume
NB Left	0	00		0	0	00	ŧ i	0	0	00		0	0	00		0	0	00	,
NB Thru	951	o ო -	262	92	1046	o m +	288	48	1094	o e> ←	306	16	1110	o m ←	310	0	1110	. თ -	310
NB Right Comb. L-T-R -	96	.00		9	106	-00	}	26	132	- 0 0	3	0	132	-00) ,	0	132	- 0 0	2
SB Left	333	- 0	333	33	366	- 0	366	9	372	- 0	372	45	417	- 0	417	0	417	- 0	417
SB Thru	1027	0 00 0	514	103	1130) N C	595	82	1212	0 10 0	909	37	1249	0 70 0	624	0	1249) N C	- 624
SB Right Comb. L-T-R -	0	, ,		0	0	000		0	0	000	1 1	0	0	000	1 1	0	0	000	
EB Left	432	- 0	238	43	475	← c	261	-	476	- 0	262	28	504	- 6	277	0	504	- 0	277
EB Thru	თ		346	~	10	000	381	0	10	000	382	0	10	000	395	0	10	000	395
Comb. L-T-R -	317	o 	174	32	349	O	192	7	351	o	193	0	351	O	193	0	351	o	193
WB Left Comb I-T	0	0 0		0	0	0 0		0	0	0 0	, ,	0	0	0 0		0	0	0 0	
WB Thru	0	00		0	0	0		0	0	0	: 1	0	0	0		0	0	0 0	
Comb. T-R WB Right Comb. L-T-R -	0	000		0	0	000		0	0	000	1 1	0	0	000	1 1	0	0	000	1 1
Crit. Volumes:	N-S: E-W: SUM:	;;; >	595 346 941			N-S: E-W: SUM:	654 381 1035			N-S: E-W: SUM:	679 382 1061			N-S: E-W: SUM:	728 395 1122			N-S: E-W: SUM:	728 395 1122
No. of Phases:			ю				က				8				9				က
Volume / Capacity: Level of Service:	oity:	[1] A	0.590			[2] B	0.626			[2]	0.644 B			[2]	0.688 B			[2]	0.688 B

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name:

CMA17

City Traffic Counters Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

Projection Year: Date: Date of Count:

08/06/2008 2007 2012

ALTERNATIVE E PROJECT - WEEKEND ANALYSIS

	2007 E	2007 EXIST, TRAFFIC	AFFIC	2012	W/ AMBIE	2012 W/ AMBIENT GROWTH	 	2012 \	W OTHER	2012 W/ OTHER PROJECTS	TS.	2012	W/ PROP	2012 W/ PROPOSED PROJECT	SJECT	2012	2012 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0	,	0	0	0	,	137	137	,	137	0	137	+-	137
Comb. L-T		0				0	,			0				0				0	,
NB Thru	0	0		0	0	0		0	0	0		0	0	0	•	0	0	0	
Comb. T-R		0	,			0	,			0	,			0	ì			0	1
NB Right [3]	0	0		0	0	0		0	0	0		778	778	C 3	428	0	778	2	428
Comb. L-T-R -		0				0				0				0				0	
SB Left	Ю	0		0	က	0		0	က	0	\righta\right\right\right\right\right\right\right\right\right\right\rig	ကု	0	0		0	0	0	
Comb. L-T		0				0	,			0				0	1			0	,
SB Thru	0	0	20	0	0	0	22	0	0	0	22	0	0	0		0	0	0	,
Comb. T-R		0				0				0	,			0				0	•
SB Right	17	0		2	9	0		0	6	0	•	က	22	-	22	0	22	-	22
Comb. L-T-R -		-				-				τ-				0				0	
EB Left	24	1	24	2	26	1	26	0	26	1	26	-26	0	0	:	0	0	0	,
Comb. L-T	000	0 ('	ç	,	0 0	,	3	000	0 0	,	1	1	0 1	,	•	100	0 0	,
Comb T.P	7601	N C	040	80	1021	N C	3	ρ	9971	N C	924	•	12/3	V C	938	0	12/5	7 0	638
EB Right	0	0		0	0	0		0	0	0		413	413	· -	413	0	413	· *-	413
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0 0	,	0	0	00	,	0	0	00		528	528		290	0	528	2 0	290
M/B Thu	1013	-	. 512	101	1111	> -	- 563	č	1175	> -	. 503	_	1170	o 4	, 505	c	1170	o +	-
Comb. T-R	2		512	2	<u>:</u>		263	5	-		263	t	2		595	•	2		595
WB Right	9	0		τ-	Ξ	0		0	Ξ	0		0	=	0		0	Ξ	0	
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	20			N-S:	22			N-S:	22			N-S:	159			N-S:	159
		E-W: SUM:	546 566			E-W: SUM:	601 623			E-W: SUM:	634 656			E-W: SUM:	928 1087			E-W: SUM:	928 1087
No. of Phases:			n				D.)				ღ				е
Volume / Capacity:	ıcity:		0.472				0.519				0.547			[1]. [2]	0.663			[1], [2]	0.663
Level of Service:	ë.		A			-	A				A			,	В				В

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. Assumptions:

70% of volume is assigned to exclusive lane.

For one excl. and one opt. turn lane,

Right turns on red from excl. lanes = 50% of overlapping left turn.
[13] Intersection will be signalized as part of the proposed project.
[14] Intersection will be signalized as part of the proposed project.
[15] Wor ratio includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.
[16] Wote: Pass-by reductions not applied to this intersection per LADOT standards.

APPENDIX B-2

ALTERNATIVE G CMA DATA WORKSHEETS

WEEKDAY AM & PM PEAK HOURS
WEEKEND MID-DAY PEAK HOUR

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA1 N-S St: E-W St: Project: File Name: Counts by:

Accutek

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

Van Nuys Boulevard @ Riverside Drive Peak Hour: AM Annual Growth: 2.0%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	7000	CIDEACT FOINT FOOD	Cinn	7500	HONN A WATER	DAY AMBIENT OBOARD		2044	orrow.	STORY OF CHIEF CAR	T.	25.00	0000 1141	TOTI ORG GEOGRAPH PROFILE	1021	* 700	MOLTA OFFICE WAY A PAGE	i de	
	200	2 .	2 .	3		ייין פעס	= -	107		יייייייייייייייייייייייייייייייייייייי	2	107		י בי	0750	107			
		No. of	Lane	Added Total	Total	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	i	0	0	0	ı	0	0	0	i	0	0	O	ı	0	0	0	ı
Comb. L-T		0	,			0	1			0	,			0	,			0	•
NB Thru	1410	77	652	113	1523	77	704	96	1619	C1 +	744	0	1619	N 7	745	0	1619	0.4	745
NB Richt	546	- c	700 -	44	590	- c	40,	23	613	- c	44/	m	616	- c	, 45	c	616	- c	. (45
Comb. L-T-R -) }	0		-		0		ł	:	0)		0) :	0	
SB Left	158	-	158	13	171	-	171	80	179	-	179	3	182	-	182	0	182	-	182
Comb. L-T		0				0	1			0	ı			0	1			0	
SB Thru	1227	m	409	86	1325	m	442	37	1362	က	454	0	1362	က	454	0	1362	က	454
Comb. T-R	c	0 0	ı	c	c	0 0		c	c	00		c	c	0 0	•	c	c	0 0	1
SB Right Comb. L-T-R -	0	00		>	>	0	ı	>	>	0		0	0	00		>	0	00	•
		,																	
EB Left	0	0 0	,	0	0	0		0	0	0 (0	0	0 0		0	0	0 0	
Comb. L-1	c	5 C	1	c	c	o c		c	c	o c		c	c	> c		c	c	o c	•
Comb. T-R	•	0		•	0	0		•	•	0		•	0	0		•)	0	
EB Right	0	0		0	0	0		0	0	0	•	0	0	0	1	0	0	0	1
Comb. L-T-R -		0				0				0				0				0	
WB Left	488	2 2	268	39	527	2 5	290	16	543	2 5	299	က	546	2	300	0	546	2	300
Comb. L-1	c	0 0		c	c	o c		c	c	> c		c	c	o c		c	c	o c	ı
Comb. T-R	>	00		5	0	00	, ,	0	>	00	, ,	5)	0		>	>	00	
WB Right Comb. L-T-R -	257	- 0	257	21	277	- 0	277	=======================================	288	۰ 0	288	ស	293	-0	293	0	293	- 0	293
Crit. Volumes:		N-S: SUM:	810 268 1078			SUM:	875 290 1165			N-S: E-W: SUM:	923 299 1221			R-S: SUM:	927 300 1227			N-S: E-W: SUM:	927 300 1227
No. of Phases:			ო				က				ო				က				8
your / ownho/	-ite.	Ξ	7830			5	777 0			15	787.0			Ξ	704			2	192.0
volune / Capacity.		Ξ	, a				, t			Ξ	رة رة			Ξ				[7]	- - - - -
רבאבו חו ספואור	,										,								

Assumptions:

Maximum Sum of Ortical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Van Nuys Boulevard Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA1 Accutek N-S St: E-W St: Project: File Name: Counts by:

Van Nuys Boulevard @ Riverside Drive Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

										1		1700			1011	7700	MOLE A CHEST WAY A PAGE	14012	
	2007 E.	2007 EXIST. INAFFIC	ייייי	LL07	W/ AMBIE	2011 W/ AMBIENI GROWIN	<u> </u>	1107	1 1 1 1 1 1 1	2011 W/ OTHER PROJECTS	2	1.07	ב ב ב	ZUIT WI PROPOSED PROJECT			VV 1911 1154	2	
	_	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume
NB Left	0	0	,	0	0	0		0	0	0	•	0	0	0	1	0	0	0	
Comb. L-T		0				0	1			0	,			0	,			0	1
NB Thru	1592	7	719	127	1720	7	776	71	1791	2	808	0	1791	2	812	0	1791	2	812
Comb. T-R		-	719			-	776			τ-	808			-	812			-	812
NB Right	564	0		45	609	0		24	633	0		13	646	0		0	646	0	•
Comb. L-T-R -		0				0				0				0				0	
SB Left	216	-	216	17	234	-	234	6	243	-	243	13	256	-	256	0	256	-	256
Comb, L-T		0	,			0	,			0				0				0	•
SB Thru	1431	ო	477	114	1546	က	515	82	1628	ო	543	0	1628	က	543	0	1628	က	543
Comb. T-R		0				0	,			0	1			0	•			0	,
SB Right	0	0		0	0	0	•	0	0	0		0	0	0		0	0	0	
Comb. L-T-R -		0				0				Ö				0				0	
EB Left	0	0		0	0	0	1	0	0	0	1	0	0	0		0	0	0	
Comb. L-T		0	1			0	1			0				0	,			0	•
EB Thru	0	0	,	0	0	0		0	0	0		0	0	0	,	0	0	0	•
Comb. T-R		0				0				0				0				0	,
EB Right	0	0		0	0	0	1	0	0	0		0	0	0		0	0	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	475	5	261	38	513	2	282	29	542	2	298	17	559	2	308	0	559	2	308
Comb. L-T		0	,			0	ı			0				0		,	,	Э,	,
WB Thru	0	0 0		0	0	0 0		0	0	0 0		0	0	0 0		0	0	00	ı
2019 7-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	ć	۰ د		ç	i C	> 1	,	1	ć	> 4	,	Ļ	Ċ	7	,	c	C	· c	,
VVB Kignt Comb. L-T-R -	734	- 0	452	<u> </u>	Sc.7	- 0	292	•	797	- 0	790		9	- 0	202	5	202	- 0	606
Crit. Volumes:		N-S: E-W:	935			N-S: E-W:	1010			Ŗ-Ś: Ę-W:	1050 298			N-S: E-W:	1068 308			N-S: E-W:	1068
	•,	SUM:	1196			SUM:	1292			SUM:	1349			SUM:	1375			SUM:	1375
No. of Phases:		- Annana	m				ю				က				ю				3
Volume / Capacity:	ity:	Ξ	0.770			Ξ	0.837			Ξ	0.876			Ξ	0.895			[2]	0.865
Level of Service:	ä		C				D				D				D				D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA2 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps AM 2.0% Annual Growth: Peak Hour:

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007 E	2007 EXIST. TRAFFIC	4FFIC	2011	2011 W/ AMBIEN	ENT GROWTH	F	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W PROP	2011 W/ PROPOSED PROJECT	SJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume		Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
	354		194	28	382	2	210	23	405	2	223	0	405	2	223	0	405	2	223
Comb. L-T NB Thru	1423	0 %	474	114	1537	0 %	512	11	1648	O 10	549	m	1651	O 10	. 550	0	1651	O m	- 550
Comb. T-R NB Right	0	00		0	0	00		0	0	00	1 1	0	0	00		0	0	00	1 1
Comb. L-T-R -		0				0				0				0				0	
SB Left	0	0		0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T	1003	0 0	- 459	78	1180	٥،	, 496	48	1228	0 0	512	*	1229	0 0	513	c	1229	٥ ۸	513
Comb. T-R	2	ı 	459	i	2	ı 	496	!		ı -	512			· - ·	513			-	513
SB Right Comb. L-T-R -	630	- 0	347	20	681	- 0	374	S	989	- 0	3//	-	/89	- 0	3/8	0	/89	- 0	3/6
FRIAR	c	c	,	c	c	c		c	c	c	,	c	c	0		O	0	0	
Comb. L-T	•	0		•	,	0)	•	0	,		•	0				0	,
EB Thru	0	0		0	0	0		0	0	0	,	0	0	0		0	0	0	,
Comb. T-R	c	0 0		c	C	0 0		c	c	00		C	c	0 0	, ,	c	c	0 0	
Comb. L-T-R -		0		•	•	0		1	1	0		•)	0		1	1	0	
WB Left	291	-	160	23	314	- (173	2	316	- (174	o	316	- (174	0	316	- 0	174
Comb. L-T WB Thru	ω	00	380	-	σ	00	410	0	თ	00	415	0	თ	00	415	0	თ	00	415
Comb. T-R WB Right Comb. L-T-R -	535	0	294	43	577	0	318	ω	585	0	322	0	585	0	322	0	585	0	322
1		i z	c u			i	302			Ö	725			Ö	736			i d	736
Orit. Volumes.		E-W: SUM:	380 1033			SUM:	410 1116			E-W: SUM:	415 1150			SUM:	415 1150			E-W: SUM:	415 415 1150
No. of Phases:			6				8				3				m				၉
Volume / Capacity:	ıcity:	Ξ	0.655			[2]	0.683			[2]	0.707			[2]	0.707			[2]	0.707
Level of Service:	.e.		В				В				O				S				O

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Van Nuys Boulevard Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA2 Accutek N-S St: E-W St: Project: File Name: Counts by:

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

Van Nuys Boulevard @ Ventura Freeway Westbound Ramps Peak Hour:

2.00%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	2007	CIEDACT TOYER	V E E I O	2044	AND AND	PROGRAMA WAY FROM	1	2011	V/ OTHE	2011 W/ OTHER PROJECTS	T.	2011	W PROP	2011 W/ PROPOSED PROJECT	FCT	2011	2011 W/ MITIGATION	NOIL	
	200		2 .	: :						1	, <u>;</u>		ļ	30 414		777	1	, ,	-
		No. of	Lane	Added	Total	No. of	Lane	Added Total		NO. O	Lane	Added lotal	otal	No. of	Lane	Added	otai	NO, OT	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	724	8	398	58	782	5	430	21	803	7	442	0	803	81	442	0	803	2 0	442
Comb. L-T	1698	0 11	- 566	136	1834	0 0	611	06	1924	၁ က	. 641	5	1937	၁ ო	- 646	0	1937	၁ ო	. 646
Comb. T-R	2	00	3	2	2	0	;	3		0	,	!		0				0	
NB Right Comb. L-T-R -	0	00		0	0	00		0	0	00		0	0	00		0	0	00	•
SB Left	0	0 (0	0	0 0	,	0	0	0 0	1	0	0	0 0	,	0	0	0 0	,
Comb. L-T SB Thru	1101	9 Z	483	88	1189	o 6	. 521	101	1290	O 04	556	6	1299	о N	561	0	1299	9 0	561
Comb. T-R	770		483	62	831		521 457	Ç	841	⊷ ⊷	556 463	on	850		561 468	0	850		561 468
Comb. L-T-R -	-	- 0	ļ	}	3	0	}	!		0				0				0	
EB Left	0	0		0	0	0	,	0	0	0 0		0	0	0 0	,	0	0	0	1
Comb. L-1 EB Thru	0	00		0	0	0		0	0	00		0	0	0 0		0	0	0	1 1
Comb. T-R	C	0 0	,	c	c	0 0		c	c	00		c	c	00	,	c	c	00	
Comb. L-T-R -		00		>	>	00		0	0	0 0	1	•	•	00		•	o	00	
WB Left	304	- 0	167	24	328	- 0	180	1	339	- 0	186	0	339	- 0	186	0	339	- 0	186
WB Thru	2	00	341	0	2	001	368	0	2	000	376	0	7	000	376	0	2	000	376
Comb. T-R WB Right Comb. L-T-R -	449	0	247	36	485	0	267	ø	491	C	270	0	491	7 - C	270	0	491	O	270
Crit. Volumes:		N-S: E-W: SUM:	881 341 1222			N-S: E-W: SUM:	951 368 1319			N-S: E-W: SUM:	998 376 1374			N-S: E-W: SUM:	1002 376 1378	:		N-S: E-W: SUM:	1002 376 1378
No. of Phases:			ю				က				က				က				6
Volume / Capacity:	acity:	Ξ	0.787			[2]	0.826			[2]	0.864			[2]	0.867			[2]	0.867
Level of Service:	.ec		٥																٥

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
55% of volume is assigned to exclusive lane.
55% of volume is assigned to exclusive lane.
56% of volume is assigned to exclusive lane.
57% of voridaping left turn.
51 furn.
51 wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
52 wor ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 N-S St: E-W St:

Accutek Counts by:

CMA3 Project: File Name:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps AM 2.0% Peak Hour: Annual Growth:

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	7000	SOUT EXIST TRAFFIC	AFFIC	2011	W/ AMRI	2011 W/ AMRIENT GROWTH	E	2011	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	DJECT	2011	2011 W/ MITIGATION	ATION	
		N C	912	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume			Lanes	Volume	Volume	Volume	Lanes	Volume		Volume	Lanes	Volume
1		0	,	0	0	0	,	0	0	0	-	0	0	0	ı	0	0	0	1
Comb. L-T	•	0				0				0	,			0	,			0	,
NB Thru	1050	ო	318	84	1134	ო	344	127	1261	ო	377	2	1263	თ -	377	0	1263	ო -	377
Comb. T-R		τ-	318			-	344	•	;	- 1	377	•	,	(377	•	9	- α	377
NB Right	223	0	•	9	240	0	,	ဖ	246	0 (,	0	246	0 0	1	0	246	> c	
Comb. L-T-R -		0				0				0				0				5	
SB Left	310	-	310	25	335	-	335	4	339	-	339	0	339	-	339	0	339	-	339
Comb. L-T		0				0	ì			0				0				0	
SB Thru	1276	2	638	102	1378	7	689	46	1424	2	712		1425	2 0	713	0	1425	0 0	713
Comb. T-R	c	0 0		c	c	0 0		c	c	0 0		c	c	o c	, ,	c	С	00	, ,
Comb. L-T-R -		0	ı	•	•	00))	0))	0		•	ı	0	
#0 00	708	-	PEE	φV	858	1	361	α	664	-	365	-	665	-	366	0	999	-	366
Comb 1-T	ŝ	- c	† ?	ř	3	- 0	3	•		- 0		•	2	0	;	1		0	
EB Thru	-	0	592	0	-	0	639	0	Ψ-	0	652	0	~	0	652	0	-	0	652
Comb. T-R		0	ı			0				0	•			0				0	
EB Right	200	-	388	26	763	-	419	19	782	-	430	0	782	<u>. </u>	430	0	782		430
Comb. L-T-R -		-				-				τ-				-				_	
WB Left	0	0		0	0	0	,	0	0	0		0	0	0		0	0	0	,
Comb. L-T		0	,			0	1			0	,			0				0	•
WB Thru	0	0	,	0	0	0		0	0	0	1	0	0	0	1	0	0	0	
Comb. T-R		0	•			0				0				0	,	•	•	0 (,
WB Right	0	0		0	0	0	•	0	0	0	1	0	0	0		0	0	0 (•
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	638			N-S:	689			S-S:	716			N-S:	716			is-N	716
		E-W:	592			E-W.	639			Ę-W:	652			E-W.	652			E-W:	652
		SUM:	1230			SUM:	1329			SUM:	1367			SUM:	1368			SUM:	1368
No. of Phases:			3				3				က				3				3
Volume / Capacity:	icity:	[1]	0.793			[2]	0.832			[2]	0.860			[2]	0.860			[2]	0.860
Level of Service:	Ď.		O				٥				۵				D				٥

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 55% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Van Nuys Boulevard Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA3 Acoutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Van Nuys Boulevard @ Ventura Freeway Eastbound Ramps Peak Hour: PM Annual Growth: 2.00%

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007 EXIST. TRAFFIC	IST. TRA	FFIC	2011	2011 W/ AMBIENT	ENT GROWTH	VTH	2011 V	V/ OTHER	2011 W/ OTHER PROJECTS	:TS	2011 \	W PROPC	2011 W/ PROPOSED PROJECT	SJECT	2011	2011 W/ MITIGATION	VION	
	Ž	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume La	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0	1	0	0	0	,	0	0	0		0	0	0	ı
Comb. L-T		0	i			0				0	,			0				0	•
NB Thru	1707	თ +	507	137	1843	eo ≁	548	103	1946	თ -	576	ω	1954	m +	578	0	1954	m +	578
NB Right	322	- 0) 	56	348	- 0	, 6	თ	357	- 0	p /c -	0	357	- 0	0/0	0	357	- 0	9/0
Comb. L-T-R -		0				0				0				0				0	
SB Left	362	-	362	29	391	-	391	თ	400	-	400	0	400	-	400	0	400	-	400
Comb. L-T SB Thru	1054	0 0	527	84	1138	0 0	- 569	102	1240	0 0	- 620	o	1249	0 0	624	0	1249	0 0	- 624
Comb. T-R		0				0		!	! '	0				0				0	
SB Right Comb. L-T-R -	0 .	00		0	0	00	1	0	0	00		0	0	00		0	0	00	
EB Left	713	-	392	57	777	-	424	7	778	-	428	c,	783	-	430	0	783	-	430
Comb. L-1 EB Thru	9	00	591	0	7	00	639	0	7	00	- 653	0	7	00	. 655	0	7	00	- 655
Comb. T-R	587	0 -	323	47	633	0 +	348	25	858	0 -	362	c	858	۰ ۳	- 362	c	a a	0+	- 362
꾸	3	- 4	3	Ŧ	3		5	3	3		700	•	3	·	3	•	3		200
WB Left	0	0	,	0	0	0	-	0	0	0		0	0	0	,	0	0	0	
Comb. L-T	ć	0 ((0 ((,	0 (•	•	0 (•		0	ı
WB Inru	5	o c	1 1	0)	0 0	, ,	0	0	o c		0	0	0 0		0	0	0 0	
WB Right	0	0		0	0	0		0	0	0		0	0	0		0	0	0	
Comb. L-T-R -	,	0				0				0				0				0	
Crit. Volumes:		:S-	869			N-S:	939			N-S:	976			N-S:	978			N-S:	978
	Ш	E-W:	591			E-W:	639			E-W:	653			E-W:	655			E-W:	655
	w	Ë.	1460			SUM:	1577			SUM:	1629			SUM:	1633			SUM:	1633
No. of Phases:			3				6				က				3				8
Volume / Capacity:	acity:	Ξ	0.955			[2]	1.007			[2]	1.043			[2]	1.046			[2]	1.046
Level of Service:	.;;		ш				ட				ш				ட			•	ш

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
55% of volume is assigned to exclusive lane.
55% of volume is assigned to exclusive lane.
56% of volume is assigned to exclusive lane.
57% of volume is assigned to exclusive lane.
57% of voerlapping left turn.
[1] we ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
[2] we ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Tyrone Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA4 Accutek N-S St: E-W St: Project: File Name: Counts by:

Tyrone Avenue @ Moorpark Street Annual Growth: Peak Hour:

AM 2.0%

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

2007 08/07/2008

ALTERNATIVE G PROJECT

	2007	COUT EXIST TRAFFIC	AFFIC	2011	W/ AMR	2011 W/ AMBIENT GROWTH	HE	2011	W OTHE	2011 W/ OTHER PROJECTS	STS	2011	W PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	VOIL	
	2	1 20 014	2 :	Addod	1404) o o o o		· Poppo	Total	No.	oue -	Added	Total	Jo ON	oue	Added	Total	No of	ane
Movement	Volume	No. or Lanes	Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
	34	0	,	ю	37	0	ì	l .	38	0	,	0	38	0	1	0	38	0	•
Comb. L-T NB Thru	ιņ	- 0	- 40	0	9	- 0	. 43	19	25	- 0	83	0	25	0	. 63	0	25	~ O	. 63
Comb. T-R NB Right Comb. L-T-R -	239	0 - 0	239	19	258	0 - 0	258	7	260	0-0	260	2	262	0 0	. 262	0	262	0 - 0	- 262
SB Left Comb. L-T	8	00	, ,	-	6	0 0		-	9	00	, ,	0	10	00	1 1	0	10	0 0	
SB Thru	32	00	51	ო	35	0 0	. 55	20	22	00	78	0	22	00	78	0	55	00	. 78
ıκ	,		· •	-	-	00-		~	13	0	1	0	13	0 -	•	0	6	0 -	ı
EB Left	3	- 0	က	0	3	- 0	က	2	5	- 0	5	0	ഹ	 c	5	0	5	- 0	5
EB Thru	284	o ← (284	23	307	o — (307	39	346	o (346	0	346	o ← c	346	0	346	o — c	346
Comb. 1-R EB Right Comb. L-T-R -	37	0 + 0	37	ო	40	0 0	40	-	4	0 - 0	- 41	0	41	0 - 0	14	0	4	0 - 0	- 44
WB Left	297	- 0	297	24	321	- c	321	2	323	۰ ۵	323	-	324	- 0	324	0	324	- c	324
WB Thru Comb. T-R WB Right Comb. L-T-R -	759	00-00	766	19	820	00-00	828	18	10	00-00	. 848	0 0	10	00-00	. 848	0 0	10	0-00	848
Crit. Volumes:	.,	N-S: E-W: SUM:	99 770 868			N-S: E-W: SUM:	107 831 938			N-S: E-W: SUM:	116 853 969			N-S: E-W: SUM:	116 853 969			N-S: E-W: SUM:	116 853 969
No. of Phases:	,,,		м				က				က				3				3
Volume / Capacity: Level of Service:	acity: ce:	[1]	0.539 A			[1]	0.588 A			Ξ	0.610 B			[2]	0.610 B			[2]	0.580 A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

CRITICAL MOVEMENT ANALYSIS

Tyrone Avenue @ Moorpark Street Peak Hour: PM

Tyrone Avenue Moorpark Street

N-S St: E-W St: Project: File Name: Counts by:	Tyrone Avenue Moorpark Street Westfield Fashio CMA4 Accutek	venue c Street l Fashion S	Tyrone Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA4 Accutek	-3606-1				Peak Hour: Annual Growth: AL TERNATIVE	PM PM PM PM PM PM PM PM PM PM PM PM PM P	Peak Hour: PM Annual Growth: 2.00% ALTERNATIVE G PROJECT						Date: Date of Count: Projection Year:	ount: Year:		08/07/2008 2007 2011	008 7.1
	2007	2007 EXIST. TRAFFIC	AFFIC	2011	2011 W/ AMBIE	ENT GROWTH	WTH	2011 \	W/ OTHE	2011 W/ OTHER PROJECTS	CTS	2011 \	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT Lane	2011 Added	2011 W/ MITIGATION	ATION No. of	Lane	
Movement	Volume		Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	
NB Left	152	0		12	164	0	,	2	166	0		0	166	0	,	0	166	0	•	
Comb. L-T	!	-	186	!		_	201		ļ	-	223	,	}	Ψ.	223	,		-	22	223
NB Thru	34	0 0		က	37	0 0	, ,	20	2/	00		0	2/	0 0		0	2/5	00		
NB Right	612) (612	49	099	· - 0	099	ო	663	· c	663	7	670	· - c	670	0	670	· — (29	029
Comb. L-1-K-		0				0				o				0)		
SB Left	23	0		2	25			2	27	0	,	0	27	0		0	27	0		Γ
Comb. L-T SB Thru	35	00	75	ო	38	00		29	67	00	114	0	67	00	114	0	9	00		114
Comb. T-R		0	•		!	0		•	;	0	ì	•	;	0	r	•	;	0		
SB Right	17	0 +	,	Ψ-	9	0 +		7	50	0 +	,	0	20	0 -		0	20	0 +	1	
		-				-				-				-				-		
EB Left	4	- (4	0	4	- (4	3	7	- (7	0	7	- (7	0	7	- (:	7
Comb. L-T EB Thru	200	o - -	500	40	540	o - -	540	33	573	→	573	0	573	o –	573	0	573	→ c	- 57	573
Comb. T-R		0								0	,			0	,			0		
EB Right	45	 C	45	4	48	- C	48	7	20	- 0	22	0	20	- 0	20	0	20	- 0	ιΩ	20
(01110)		>				•				•				•)		
WB Left	386	⊷ c	386	31	417	c	417	3	420	 c	420	6	429	- c	429	0	429	 C	42	429
WB Thru	607	0	, ,	49	656	00		33	689	0		0	689	0	,	0	689	0		
Comb. T-R	!	-	624	•		 (674	,	;	- 1	709	•	;	- 1	709	•	;	-	02	209
WB Right	17	0 (-	9	0 (2	2	0 (,	0	20	0 (0	20	0 (
Comb. L-T-R		0				0				0				0				0		
Crit. Volumes:	122	S-S	441			S-N	477			.;;-N	480			:0-Z	483			.sZ	48	<u>ε</u>
		 ≱ 	1328				1434			 ∑ Ω	1473			. M.	1485				1002	7
		500	270			5	<u> </u>				Ē				2				2	
No. of Phases	.;		င				m				ო				ო					က
Volume / Capacity:	acity:	Ε	0.862			Ε	0.936			Ξ	0.964			Ξ	0.972			[2]	0.942	22
Level of Service:		:	۵			:	ш			:	ш			:	Ш			:	ш	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes = 50% of overlapping left turn.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA5 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour: AM Annual Growth: 2.0%

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

Movement Mounte Lane Modele Teal No. of Lane Modele Teal No. of Lane Modele Teal No. of Lane Modele Teal Movement Modele Teal Movement Modele Teal Teal Te		2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	Ŧ	2011 \	N/ OTHER	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	NOIT	
			No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
	Movement	Volume		Volume	Volume	Volume		Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
	NB Left	87	- 0	87	7	94	← 0	94	4	108	- 0	108	0	108	← (108	0	108	- 0	108
1	NB Thru Comb. T-R	163	c	117	13	176) - -	126 126	-	177) - -	137	2	179) +-	138 138	0	179	o	- 138 138
-T. 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	NB Right Comb. L-T-R -		00	1	Θ	76	00	ı	20	96	00		0	96	00		0	96	00	
1.	SB Left	11	F C	=	-	12	- 0	12	13	25	- c	25	0	25	- c	25	0	25	- c	25
TTR-	SB Thru	206	o ← c	206	16	222	o ← c	222	~	223	o c	223	~	224	o ← c	224	0	224	o ← c	224
1	SB Right Comb. L-T-R -		0-0	144	=======================================	155	0 - 0	155	O	164	0 - 0	164	0	164	0-0	164	0	164	0 - 0	164
H194 F 656 96 1289 F 128 F 709 F 18 1407 F 779 F 1 1408 F 779 F 1 408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F 1 1408 F 779 F	EB Left	6/	- 0	79	9	85	- 0	85	16	101	- 0	101	0	101	- 0	101	0	101	- c	101
119	EB Thru	1194	· ·· ·	929	96	1289) -	709	118	1407	· - •	779	-	1408	· · ·	779	0	1408	· «	779
75 1 75 6 81 1 81 13 94 1 94 0 94 1 94 0 94 1 94 0 94 1 94 0 94 1 94 0 94 0 94 1 94 0	Comb. L-T-R Comb. L-T-R -		-00	900	σ	128	-00	60	22	150	-00		0	150	-00	2	0	150	-00	
H146 1 578 92 1238 1 624 77 1315 1 665 1 1316 1 665 0 1316 R-A.	WB Left	75	← c	75	9	81	- 0	81	13	94	- 0	94	0	94	0	94	0	94	- 0	94
R- 9 0 - 1 10 0 - 6 15 0 - 0 15 0 - 0 15 0 - 0 15 0 - 0 15 1	WB Thru	1146) 	578 578	95	1238) - -	624	11	1315) - -	665 665	-	1316) 	665 665	0	1316		665
N-S: 293 N-S: 317 N-S: 332 N-S: 333	WB Right Comb. L-T-R -		. 0 0	;	-	5	.00		တ	15	00		0	15	00		0	15	00	•
y: [1] 0.613 [2] 0.638 [2] 0.703 [2] 0.700 C	Crit. Volumes:		N-S: E-W: SUM:	293 731 1024			N-S: E-W: SUM:	317 790 1106			N-S: E-W: SUM:	332 873 1204			N-S: E-W: SUM:	333 873 1206			N-S: E-W: SUM:	333 873 1206
y: [1] 0.613 [2] 0.638 [2] 0.703 [2] C C	No. of Phases:			2				2				2				2				2
	Volume / Capa Level of Servic	acity:	Ξ					1 1			[2]				[2]					0.704 C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

70% of volume is assigned to exclusive lane.

8ight turns on red from excl. lanes = 50% of voerdapping left turn.

[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wor ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Tyrone Avenue/Beverly Glen Boulevard Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA5 Accutek

N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Tyrone Avenue/Beverly Glen Boulevard @ Ventura Boulevard Peak Hour:
PM
Annual Growth: 2.00%

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007	2007 EXIST TRAFFIC	AFFIC	2011	W/ AMBII	2011 W/ AMBIENT GROWTH	E	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS.	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	VIION	
	:	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume			Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	190	-	190	15	206		206	28	234	← (234	0	234	← (234	0	234	← (234
Comb. L-T NB Thru	557	0	347	45	602	o	375		603) - -	384	7	610)	387	0	610) - -	387
NB Right Comb. L-T-R -	137	-00	.	=	148	-00	5	16	164	- 0 0	3	0	164	-00	3	0	164	.00	,
SB Left	36	- 0	36	3	39	- (39	19	49	- 0	49	0	49	- 0	49	0	49	- 0	49
SB Thru	297	o	297	24	321	o ~ c	321	-	322	o ← c	322	6	331	o — c	331	0	331	o c	331
Comb. 1-R SB Right Comb. L-T-R -	26	0 ~ 0		4	61	0 - 0	61	23	84	0 - 0	84	0	84	0 - 0	84	0	84	0-0	84
EB Left	109	- 0	109	6	118	- 0	118	17	135	- c	135	0	135	- 0	135	0	135	- 0	135
EB Thru	1099	o ← •	579	88	1187	· - c	625	118	1305) +	695	3	1308) 	697	0	1308) 	697
Comb. I-R EB Right Comb. L-T-R -	28	-00	ñ ,	ហ	63	-00		22	85	- 0 0	3	0	82	-00	3	0	82	-00	
WB Left	146	- 0	146	12	157	۰- ۵	157	12	169	- c	169	0	169	- c	169	0	169	- c	169
WB Thru Comb T-R	1124) 	576	06	1214)	622	126	1340)	689	ო	1343) 	069	0	1343) + +	069
WB Right Comb. L-T-R -	28	.00	;	7	30	00		7	37	00	1	0	37	00	,	0	37	00	ı
Crit. Volumes:		N-S: E-W: SUM:	488 724 1212			N-S: E-W: SUM:	527 782 1309			N-S: E-W: SUM:	556 864 1420			N-S: E-W: SUM:	565 866 1431			N-S: BUM: SUM:	565 866 1431
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	city:	Ξ	0.738 C			[2]	0.773 C			[2]	0.847 D			[2]	0.854 D				0.854 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 70% of volume is assigned to exclusive lane. Fight turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 N-S St: E-W St: Project: File Name: Counts by:

Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Magnolia Boulevard AM 2.0% Annual Growth: Peak Hour:

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	CHEAT TRAFFIC	ST TDA	0133	2011	2011 W/ AMRIE	HTWORE THE	E	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	DJECT	2011	2011 W/ MITIGATION	ATION	
		No of	9 00	Added	Total	Jo of	908	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume La		Volume	Volume Volume	Volume	Lanes	Volume	Volume	-	Lanes	Volume		Volume	Lanes	Volume			Lanes	Volume
		١.	5	ď	2	,	ō	ч	1	-	Q		8	-	G		6	-	CB
NB Left Comb 1-T	8/	- 0	φ '	O	9	- 0		n	0	- 0	B '	-	8	- 0	2	•	3	- 0	}
NB Thru	441	-	252	35	476	-	272	14	490	-	280	2	492	-	281	0	492	-	281
Comb. T-R		-	252			-	272			-	280			-	281			-	281
NB Right	62	0		വ	29	0	,	ო	2	0		0	20	0		0	2	0 (,
Comb. L-T-R -		0				0				0				0				0	
SB Left	126	-	126	10	136	٦	136	۲	135	+	135	О	135	-	135	0	135	-	135
Comb. L-T		0	,			0	,			0				0	ı			0	•
SB Thru	813	-	453	65	878	•	489	13	891	-	496	ις	896	-	498	0	896	-	498
Comb. T-R		-	453			-	489			-	496			-	498	•		₹ (498
SB Right	93	0		7	9	0		0	9	0		0	6	0 1		0	9	0 (
Comb. L-T-R -		0				0				0				0				0	
EB Left	57	1	57	ည	62	-	62	-	63	-	63	0	63	-	63	0	63	- (83
Comb. L-T		0				0	•			0		,		o ·		•		ο.	1
EB Thru	822	-	479	99	887	-	517	23	910	ψ	531	0	910		532	0	910	- ,	532
Comb. T-R	;	-	479	-	!	. .	517	١		- 0	531	•	5	- c	232	c	47.0	- c	255
EB Right	136	0		-	147	0 '		v	152	o (,	_	55	> 0		>	3	> 0	•
Comb. L-T-R -		0				0				o				o				0	
WB Left	147	- 6	147	12	158	0	158	2	160	€	160	-	161	← C	161	0	161	- 0	161
COMD. L-1	;	.		I	;	۰ د		•		۰ د		Ċ	,,,,,	,	u c	c	*****	7	101
WB Ihru	964		541	>	1041		584 584	ກ	440		585	>	104		285	>	1		585
WB Right	118	- 0	<u>;</u>	თ	127	- 0	5	-5	125	- 0	}	0	125	0		0	125	0	,
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		-S:	531			N-S:	573			S-S:	585			N-S:	588				588
	Ш	E-W:	979			 	929			. К	692			М	693			М.	693
	S	.: O <u>W</u>	1156			SUM:	1249			SOM:	12/6			SUM:	1281			SOM:	1871
No. of Phases:			2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.701			[2]	0.733			[2]	0.751			[2]	0.754			[2]	0.754
Level of Service:	.e.		U				ပ				၁				v				C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Hazeltine Avenue Magnolia Boulevard Westfield Fashion Square /1-05-3606-1 CMA6 N-S St. E-W St: Project: File Name:

Accutek Counts by:

Hazeltine Avenue @ Magnolia Boulevard CRITICAL MOVEMENT ANALYSIS

PM 2.00%

Peak Hour: Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007	2007 EXIST. TRAFFIC	4FFIC	2011	2011 W/ AMBIER	ENT GROWTH	E	2011	W OTHER	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Гале	Added	Total	No. of	Гапе	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	140	~	140	#	152	-	152	z,	157	-	157	4	161	- (161	0	161	← (161
Comb. L-T NB Thru	858	o –	505	69	927	0 -	543	12	939	0	549	56	964	0	562	0	964	0 —	562
Comb. T-R NB Right Comb. L-T-R -	147	-00	- 502	12	158	-00	- 543	-	159	-00	- 549	0	159	-00	- 562 -	0	159	-00	7995
SB Left	66	- - (66	ω	107	- (107	1-	106	- (106	0	106	- 0	106	0	106	- 0	106
SB Thru	657	o - 7	363	53	710	o - •	392	16	726	o	400	20	746	o r	410	0	746	o -	410
Comb. I-K SB Right Comb. L-T-R -	69	-00	595	ιΩ	74	-00	786	~	75	-00		0	75	-00	,	0	75	-00	2
EB Left	111	- (111	6	120	- 0	120	-	121	- 0	121	0	121	- 0	121	0	121	-	121
Comb. L-1 EB Thru	1006	o	622	80	1086	o ∙	672	4	1100	o	682	0	1100) •	684	0	1100	o — •	684
Comb. I-K EB Right Comb. L-T-R	239	-00	. p877	19	258	-00	7/9	ß	263	-00	786	4	267	-00	, 00 4	0	267	-00	-
WB Left	102	- 0	102	ω	110	- 0	110	ო	113	- 0	113	4	117	- 0	117	0	117	- 0	117
WB Thru	512) - -	285	41	553	o	308	Ξ	564) - +	313	0	564) - -	313	0	564) 	313
WB Right Comb. L-T-R	69	-00	-	52	64	-00	3	7	63	-00	2	0	63	-00	<u>.</u>	0	63	-00	2
Crit. Volumes:	12	N-S: E-W: SUM:	601 724 1325			N-S: E-W: SUM:	649 782 1432			N-S: E-W: SUM:	655 795 1450			N-S: E-W: SUM:	667 801 1468			N-S: E-W: SUM:	667 801 1468
No. of Phases:	iń		2				2				2				2				2
Volume / Capacity: Level of Service:	acity:	[1]	0.814 D			[2]	0.854 D			[2]	0.866 D			[2]	0.879 D			[2]	0.879 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
70% of volume is assigned to exclusive lane.
70% of volume is assigned to exclusive lane.
8ight turns on red from excl. lanes = 50% of volume sparing furn.
[1] wor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.
[2] wor ratios reflect additional 0.03 reduction due to the tuture citywide ATSAC/ATCS system installation.

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 Accutek N-S St. E-W St. Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Riverside Drive Peak Hour: AM 2.0% Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007	2007 EXIST TRAFFIC	AFFIC	2011	W/ AMBI	2011 W/ AMBIENT GROWTH	HL	2011	V/ OTHE	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	DJECT	2011	2011 W/ MITIGATION	VOIL	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume		- 1	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	61	~ (61	ស	99	← (99	0	99	← (99	ო	69	← 0	69	0	69	~ 0	69
NB Thru	363) N C	181	29	392	0 70 0	196	15	407	0 77 0	203	2	409	0 00 0	204	0	409	O 00 C	204
Comb. L-R NB Right Comb. L-T-R -	121	o ~ c	121	01	130	0 - 0	130	7	137	0 - 0	137	0	137	0 - 0	137	0	137	0 - 0	- 137
SB Left	205	← c	205	16	221	- 0	221	9	227	- 0	227	2	229	- 0	229	0	229	- 0	229
SB Thru	860) 	463	69	929	o	500	15	944	o — -	509	9	950) - -	512 512	0	950) - -	512 513
SB Right Comb. L-T-R -	29	-00	}	ស	72	-00		2	74	-00	3	0	74	-00	,	0	74	-00	1
EB Left	24	- 0	24	2	26	- 0	26	-	27	- 0	27	0	27	- c	27	o	27	- 0	27
EB Thru	740) + +	403	29	800) +	435	30	830) -	450	4	834) -	454	0	834) -	454
Comb. L-T-R -	99	- 0 0	5	ហ	71	-00	?	0	71	-00	,	ო	74	-00	;	0	74	- 0 0	ţ,
WB Left	344	- c	344	28	372	← c	372	9	378	- c	378	0	378	- 0	378	0	378	- 0	378
WB Thru	844	0 70 0	422	99	912	0 N C	456	24	936	0 N C	468	5	941	. ~ c	470	0	941	0 C	470
WB Right Comb. L-T-R -	138	0 0	138	Έ	149	0 - 0	149	9	155	0-0	155	4	159	0 - 0	159	0	159	o 0	159
Crit. Volumes:		N-S: E-W: SUM:	525 747 1272			N-S: E-W: SUM:	567 807 1374			N-S: E-W: SUM:	575 828 1403			N-S: E-W: SUM:	581 832 1413			N-S: E-W: SUM:	581 832 1413
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	city:	[1]	0.778 C			[1]	0.846 D			[1]	0.865 D			[1]	0.872 D			[2]	0.842 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 60% of overlapping left turn.

[1] wor ratio includes a 0.07 reduction and the to installation of ATSAC as part of the Victory System No. 6.

[2] wor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 Accutek CMA7 N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue @ Riverside Drive Peak Hour: 2.00% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007 E	2007 EXIST. TRAFFIC	4FFIC	2011	W/ AMBIL	2011 W/ AMBIENT GROWTH	E	2011 V	V/ OTHE	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	136	← (136	-	147	← (147	0	147	 (147	17	164	- 0	164	0	164	(164
Comb. L-1 NB Thru	844	o 10 c	422	68	912	5 N C	456	15	927	O 10 C	464	10	937	5 N C	469	0	937	0 70 0	469
NB Right Comb. L-T-R -	249	0-0	249	20	268	0 0	268	4	272	0 - 0	272	0	272	0 0	272	0	272	0 - 0	272
SB Left	168	- c	168	13	182	c	182	ro	187	c	187	6	196	← c	196	0	196	- 0	196
SB Thru	795	- ·	, 444 444	64	828	c	479	18	876	o	489	22	898	O	500	O	898) - -	500
SB Right Comb. L-T-R -	93	-00	;	7	100	-00	, ,	-	101	-00	,	0	101	-00	000	0	101	-00	000
EB Left	92	← c	92	_	66	- 0	66	2	101	- 0	101	0	101	- 0	101	0	101	- 0	101
EB Thru	610	· c	363	49	629		392	93	689) +	407	18	707	· - ·	422	0	707	> ⊷ •	422
Comb. L-T-R -	115	-00	,	σ	125	-00	,	0	125	-00	j '	12	137	-00	774	0	137	-00	-
WB Left	229	- c	229	18	247	- c	247	ιΩ	252	- c	252	О	252	- 0	252	0	252	- 0	252
WB Thru	587	0 C	293	47	633	0 00 0	317	35	899	N C	334	48	716	0 N C	358	0	716	000	358
WB Right Comb. L-T-R -	179	0 - 0	179	4	193	0 - 0	193	ശ	198	0 0	198	38	237	0 0	237	0	237	0 - 0	237
Crit. Volumes:		N-S: E-W: SUM:	591 592 1182			N-S: E-W: SUM:	638 639 1277			N-S: E-W: SUM:	650 659 1310			N-S: E-W: SUM:	664 674 1339			N-S: E-W: SUM:	664 674 1339
No. of Phases:			2				2				2				2				2
Volume / Capacity: Level of Service:	city:	[1]	0.718 C			[1]	0.781 C			[1]	0.803 D			[1]	0.822 D			[2]	0.792 C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 56% of overlapping left turn.

[1] wo ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] wo ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 N-S St: E-W St:

Project: File Name: Counts by:

CMA8

Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Fashion Square Lane Peak Hour: AM Annual Growth: 2.0%

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

2	2007 EXIST. TRAFFIC	TRAFFIC	2011	2011 W/ AMBIE	ENT GROWTH	VTH	2011 V	V/ OTHER	2011 W/ OTHER PROJECTS	STS	2011	V/ PROP(2011 W/ PROPOSED PROJECT	OJECT	2011 V	2011 W/ MITIGATION	NOIT	
····	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume Lanes	ume Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	12 1	12	-	13	-	13	0	13	-	13	0	13	Ψ-	13	0	13	τ-	13
Comb. L-T	0	٠			0				0				0				0	,
	523 1	273	42	565	-	295	22	287	-	306	-	588	Ψ-	308	0	588	-	308
Comb. T-R	-	273			-	295			Ψ-	306				308			•-	308
NB Right	23 0	,	2	22	0		0	25	0	•	4	29	0		0	58	0	
Comb. L-T-R -	0				0				0				0				0	
SB Left	44	44	3	47	-	47	0	47	-	47	6	56		56	0	56	-	99
Comb. L-T	0				0				0				0				0	
	1180 1	594	94	1275	-	641	22	1297	-	652	0	1297	-	652	0	1297	-	652
Comb. T-R	-	594			-	641			-	652			-	652		,	-	652
SB Right	7 0		•	ω	0		0	ω	0	•	0	80	0		0	∞	0	,
Comb. L-T-R -	0				0				0				0				0	
EB Left	2 1	2	0	2	-	2	0	2	1	2	0	2	-	2	0	2	-	2
Comb. L-T	0	,			0				0	•			0				0	
EB Thru	0	,	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. T-R	_	7			-	œ			-	œ			τ-	œ			₹	80
EB Right	7 0	ı	_	∞	0		0	ω	0	,	0	∞	0		0	œ	0	,
Comb. L-T-R -	0				0				0				0				0	
WB Left	1	-	0	-	1	1	0	-	1	-	2	8	-	3	0	3	-	3
Comb. L-T	0				0	,			0	•			0				0	
WB Thru	0 0		0	0	0		0	0	0		0	0	0	,	0	0	0	
Comb. T-R		2			-	7			.	7			0		,	•	0	•
WB Right	2 0	,	0	7	0		0	7	0		•	m	-	က	0	m	-	ო
Comb. L-T-R -	0				0				0				0				0	
Crit. Volumes:	N-S:	909			N-S:	655			S-S	999			is-S	999			N-S:	999
	E-W:	80			E-W:	თ			E-W:	თ			Ę-W	Ξ			E-W.∵	=
	SUM:	615			SUM:	664			SUM:	675			SUM:	229			SUM:	677
No. of Phases:		б				က				8				က				က
Volume / Capacity:	[1]	0.361			Ξ	0.396			Ξ	0.404			Ξ	0.405			[2]	0.375
Level of Service:		A				A				A				A				A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] vor actio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] vor actio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1 N-S St: E-W St:

CMA8 Accutek Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazelline Avenue @ Fashion Square Lane Peak Hour: PM Annual Growth: 2.00%

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	CHEAT TRYET	TDACEL	2044	MANA WA	HTWOOD THEIGHT WAY 1800	WTE	2011 V	V OTHE	2011 W/ OTHER PROJECTS	STE	2011	W/ PROP	2011 W/ PROPOSED PROJECT	O.IFCT	2011	2011 W/ MITIGATION	NOIT	
	No of	Jane I	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Movement Volume Lanes			Volume	Lanes	Volume		اہ	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
NB Left	-	=	-	12	τ-	12	0	12	-	12	0	12	-	12	0	12	-	12
Comb. L-T	:				0				0	,			0	,			0	
NB Thru	1013	541	81	1094	τ.	585	19	1113	τ- τ	594	4	1117		603	0	1117	***	603
Comb. 1-K		54.1	ď	75	- c		c	7.5	- c	† n	14	68	- c	3 ,	0	89	- 0	8
Comb. L-T-R	2		o	2	0)	2	0		:	}	0				0	
SB Left	206 1	1 206	16	222	-	222	0	222	-	222	34	256	-	256	0	256	-	256
Comb. L-T	J				0				0				0				0	
SB Thru	878	1 441	70	948		477	23	971		488	0	971		488	0	971		488
Comb. I-K	L.	1 441	C	œ	- 0	74 -	o	တ	- 0	400	0	9	- 0	,	0	9	- 0	2
Comb. L-T-R -)		1	,	0		,		0				0				0	
EB Left	6	9	0	7	-	7	0	7	-	7	0	7	-	7	0	7	-	7
Comb. L-T)				0				0	,			0		•		0	
EB Thru	-	,	0	-	0		0	-	o ·		0	-	۰ م	,	0	-	0 1	,
Comb. T-R		-	•	,	← 0	-	c	7	- c	=	c	•	~ ⊂	=	c	ç	- c	-
EB Right	on .	,	-	2	5 C		0	2	> c		>	2	o c		>	2	o c	
Comb. L-1-K-					o				•				•					
WB Left	76 1	1 76	9	82	- 1	82	0	82	- (82	14	96	← (96	0	96	- 0	96
Comb. L-T			•	,	0 (c	•	> 0	,	c	۳	> 0		c	•	> C	
WB Thru	-		0	-	⊃ -	, g	>	-	o -	, g	>	-	o c		>	-	o c	
W/B Right	53		4	57	- 0	3	0	57	- 0	3	9	67		29	0	29		29
Comb. L-T-R		0			0				0				0				0	
Crit. Volumes:		747			N-S:	807			N-S:	817			-is-Z	860			N-S:	860
	E-W:				E-W:	93			E-W	93			E-W.	107			E-W:	107
	SUM:	834			SUM:	006			SUM:	910			SUM:	967			SUM:	2967
No. of Phases:	S.	3				က				3				6				3
Volume / Capacity:	pacity: [1]				[1]	0.562			Ξ	0.568			[2]	0.608			[2]	0.578
Level of Service:	ice:	∢				V				A				a				A

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] v/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA9 Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Moorpark Street
Peak Hour: AM
Annual Growth: 2.0% Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007 EXIST. TRAFFIC	TRAFFIC	2011	W/ AMBI	2011 W/ AMBIENT GROWTH	H	2011 V	// OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
	No. of	of Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes		Volume Volume	Volume	Lanes	Volume		_	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB I eff	43	1 43	m	46		46	0	46	-	46	0	46	-	46	0	46	1	46
Comb. L-T		. 0			0				0				0				0	,
NB Thru	215	1 121	17	233	-	130	22	255		143	2	257	-	144	0	257	-	144
Comb. T-R					_	130			-	143			-	144			-	144
NB Right	56		2	28	0	ı	6	31	0	,	0	3	0	,	0	34	0	
Comb. L-T-R -		0			0				0				0				0	
SB Left	167	1 167	13	181	-	181	0	181	-	181	0	181	-	181	0	181	-	181
Comb. L-T		. 0			0	•			0				0				0	
SB Thru	904	1 627	72	976	-	677	21	266	-	687	-	998	-	688	0	998	~	688
Comb. T-R		1 627				677			-	687			•	688		į	-	688
SB Right	349	0	28	377	0 1		0	377	0 (-	378	0 0		0	378	0 0	•
Comb. L-T-R -		0			0				0				0				o	
EB Left	93	1 93	7	100	-	100	0	100	-	100	2	102	-	102	0	102	- (102
Comb. L-T	ļ	0	i		ο,		9	ţ	۰ ۰	,	•		۰ ۰	,	c	107	0 4	,
EB Thru	392	1 392	31	423	~- c	423	45	465	- c	465	0	465	- c	465	5	465	- c	465
בים מו	53	. 52	7	7,	- c		c	r.	- c	, 55	c	55	· -	, 95	c	55	· -	56
Comb 1-T-R-	70	- 0	٢	3	- 0	3	,	3	- 0	8	•	3	- 0	3	•	}	0	3
		1)				'									
WB Left	98	1 86	7	93	-	93	4	26	-	97	0	97	1	97	0	26	Ψ.	26
Comb. L-T			ļ	i	ο.		;		o ·	,	•	Î	ο,		•	í	۰ د	,
WB Thru	711	1 407	25	768		439	22	06/		0.4	0	06/	- τ	154	Э	35		157
Comb. T-R	,		c	7	- c	439	c	÷	c	450	•	7	c	104	c	÷	- c	45
Comb. L-T-R -	20	' o o	0	2	00	ı	•	2	00		-	=	0 0	ı	•	=	00	
Crit. Volumes:	S-N				N-S:	723			N-S:	733			N-S:	734			N-S:	734
	E-W: SUM:	f: 499 1: 1168			E-W: SUM:	539 1262			E-W: SUM:	563 1296			E-W: SUM:	563 1297			E-W: SUM:	563 1297
No. of Phases:		2				2				2				2				2
Volume / Capacity:		[1] 0.709			[2]	0.741			[2]	0.764			[2]	0.765			[2]	0.765
Level of Service:		ပ				S				S				S				ပ

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Hazeltine Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA9 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Moorpark Street Peak Hour: 2.00% Peak Hour: Annual Growth:

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	1	CIBBAGT TOINE VOOL	AEELO	2044	HOMA WAL	DEALOGO FINEIGNA 1/6/ 1/20	1	2044	DITUE.	STUBIO DE DECTO	OTC.	2044	acad //w	2044 W/ PROPOSED PRO IECT	D. IECT	2011	NOTA WITIGATION	NOIF	
	1007	- AIO - I	2 6	1000	Total		946	, Poppe	i letot	No of) -	Added	Total	No.	ana	Added	Total	No.	9
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume		Volume	Lanes	Volume
	•	-	86	80	106	-	106	0	106		106	0	106	(106	0	106	+- (106
Comb. L-T NB Thru	479	0 - 1	266	38	518	0	287	18	536	0	297	φ	542) -	300	0	542	O + +-	300
Comb. I-R NB Right Comb. L-T-R -	52	-00	,	4	99	-00	,	N	28	-00	Ĉ,	0	28	-00	3	0	28	-00	,
SB Left	165	← c	165	13	179	⊢ c	179	0	179	- c	179	0	179	- c	179	0	179	- 0	179
SB Thru	430	o +	371	34	464) -	401	21	485	·	411	ო	488	,	417	0	488	·	417
SB Right Comb. L-T-R -	313	-00	ē '	25	338	-00	,	0	338	- 0 0	,	თ	347	- 0 0	,	0	347	.00	
EB Left	303	- (303	24	327	- 0	327	0	327	- 0	327	7	334	- c	334	0	334	- 0	334
Comb. L-1 EB Thru	699	o (699	53	722	o ← 0	722	38	760	o c	760	0	760	o (760	0	760	o ← 0	760
Comb. T-R EB Right Comb. L-T-R -	93	0 0	. 83	7	100	o - c	100	0	100	0 0	100	0	100	0 + 0	100	0	100	0 - 0	100
WB Left	76	- 0	76	9	82	← c	82	2	84	- 0	84	0	84	c	84	0	84	- 0	84
WB Thru	523) 	333	42	565	o	360	38	603	o	379	0	603) - -	380	0	603) 	380
WB Right Comb. L-T-R -	144	.00	}	1	155	00	,	0	155	00	ŧ	7	157	00	1	0	157	00	
Crit. Volumes:		N-S: E-W: SUM:	469 745 1214			N-S: E-W: SUM:	507 804 1311			N-S: E-W: SUM:	517 844 1361			N-S: E-W: SUM:	523 844 1367			N-S: E-W: SUM:	523 844 1367
No. of Phases:			2				2				2				2				2
Volume / Capacity:	oity: e:	Ξ	0.739 C			[2]	0.774 C			[2]	0.808 D			[2]	0.812 D			[2]	0.812 D

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual tum lanes, 55% of volume is assigned to heavier lane. 70% of volume is assigned to exclusive lane. For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane. 8ight tums on red from excl. lanes = 50% of overlapping left tum. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Hazeltine Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 Project: File Name: Counts by: N-S St: E-W St:

CMA10 Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Ventura Boulevard Peak Hour: AM Annual Growth: 2.0%

08/07/2008 2007 2011 Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	CIDACT TOYER	ACEIO	2544	DO11 W/ AMBIE	HTWOGS THE	E	2011	V/ OTHER	2011 W/ OTHER PROJECTS	TS.	2011	W PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	NOIL	
	7007	באוטו. וה	2 6	7 70	1040) o	900	Added	Total	No.	ane I	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	No. or Lanes	Volume	Volume		Lanes	Volume			Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
		0		0	٥	0		0	0	0		0	0	0	1	0	0	0	
Comb. L-T	•	0				0				0				0				0	ı
NB Thru	0	0	1	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. T-R		0				0	,			0				0		•	•	0	1
NB Right	0	0		0	0	0	,	0	0	0 (0	0	0 0		0	0	0 0	1
Comb. L-T-R -		0				0				0				0)	
SB Left	322	2	177	26	348	2	192	-	349	2	192	0	349	2	192	0	349	2	192
Comb. L-T		0	,			0				0		•	•	0 1		,	•	0 (
SB Thru	0	0 (,	0	0	0 0		0	0	0 0		0	0	0 0		0	0	o c	
SB Right	513	o	513	41	554	→	554	21	575	→	575	-	576	→	576	0	576	-	576
Comb. L-T-R -		0				0				0				0				0	
EB Left	96	-	96	8	103	-	103	24	127	-	127	1	128	- 0	128	0	128	- (128
Comb. L-T	2	0 0	1	0	4	0 1	. 505	7	13/3	0 0	- 673	c	1343	o 6	- 672	c	1343	o د	- 672
Comb T-P	1011	V C	<u>,</u>	8		V C	ce ce ce ce ce ce ce ce ce ce ce ce ce c	5	5	N 0	י ל	•	2	4 0	1	•	2	. 0	i
EB Right	0	0	,	0	0	0		0	0	0		0	0	0	•	0	0	0	
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0 (0	0	0 0		0	0	0 0		0	0	00		0	0	0 0	
Comb. L-1	1		730	400	1465	-	700	Ö	1558	> -	- 846	C	1558	- c	R46	C	1558	· -	846
Comb. T-R	1220		739	2			799	3	2		846	•	2		846))	-	846
WB Right	123	0		10	133	0		~	134	0	1	0	134	0		0	134	0 (ı
Comb. L-T-R	,	0				0				0				0				5	
Crit. Volumes:		N-S:	465			N-S:	505			.S-Z-	511			S-Z	512				512
		 М.	835			 ₹	902			 Σ Σ Σ	973			:` À 	9/4 1486			 ₩	9/4 1486
		SOM.	000				† •				-				3				
No. of Phases:	23:		2				2				2		7747		7				2
Volume / Capacity:	acity:	Ξ	0.797			[2]	0.836			[2]	0.889			[2]	0.890			[2]	0.890
Level of Service:	ce:		ပ				D				۵				О				٥

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

N-S St: E-W St: Project: File Name: Counts by:

Hazeltine Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA10 Accutek

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Ventura Boulevard Peak Hour: PM 2.00% Annual Growth:

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

		1000	City	7700	110000	S C C	ŀ	1,700	TITO SA	STOTI OGG GTITO WILLIAM	7.	2000	000	TOTI OGG G19000000 (18) 2200	1021	2044	MOLTA CITIES WAY A PACE	I CIF	
	7007	ZUU/ EXISI. IRAFFIC)	1107	W/ AMBIC	ZULI W/ AMBIENI GROWIN	<u> </u>	107		יייייייייייייייייייייייייייייייייייייי	2 .	7	DE		5 .	107			
		No. of	Lane	Added Total	Total	No. of	Lane	Added		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volите	Volume	Lanes	Volume
NB Left	0	0		0	0	0	,	0	0	0	,	0	0	0	1	0	0	0	
Comb. L-T		0	•			0	,			0	•			0	;			0	
NB Thru	0	0	,	0	0	0	,	0	0	0	,	0	0	0	1	0	0	0	•
Comb. T-R		0				0				0				0				0	
NB Right	0	0	•	0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	181	2	100	14	195	2	107	3	198	2	109	0	198	2	109	0	198	2	109
Comb. L-T		0				0	,			0				0	•			0	
SB Thru	0	0 (0	0	0 (•	0	0	0 0		0	0	0 0		0	0	0 0	ı
Comb. I-K	216	o +	م	7	737	o +	- 237	00	25.4	o -	. 254	"	257	⊃ -	- 257	c	257	> -	- 257
Comb. L-T-R -		- 0	2	-	†) 4	- 0	3	3	5	- 0	})	ì	0	ì	•	į	0)
EB Left	203	- 0	203	16	219	- 0	219	16	235	- 0	235	4	239	- 0	239	0	239	0	239
Comb. L-1 EB Thru	1474	» с	737	118	1592	о 0	- 796	139	1731	о О	- 865	0	1731	э с	. 865	0	1731	ν с	. 865
Comb. T-R		0	•			0	,			0	,			0				0	,
EB Right	0	0	•	0	0	0		0	0	0 (,	0	0	0 (0	0	0 (1
Comb. L-T-R -		0				0				0				0				9	
WB Left	О	0 0		0	0	0 0	,	0	0	0 0		0	0	00	, ,	0	0	0 0	
M/B Thu:	1227	-	753	g	1335	> ~	, , ,	1,7	1486	· -	280	c	1486	- c	891	C	1486	· -	891
Comb. T-R	2		753	3	2		813	2	2		890	•	2		891	•	2	- +-	891
WB Right	269	00		22	291	0 0		ო	294	00	,	7	296	00	,	0	296	00	1
בין-א		>				>				0				>				0	- 11 - 11
Crit. Volumes:		N-S: E-W: SLM:	115 956 1071			N-S: E-W: SUM:	124 1032 1156			N-S: E-W: SUM:	136 1125 1261			N-S: SUM:	137 1130 1267			N-S: E-W: SUM:	137 1130 1267
			5))													
No. of Phases:			2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.644			[2]	0.671			[2]	0.741			[2]	0.745		Ē	[2]	0.745
Level of Service:	3e:		В				В				O				S				U

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Assumptions:

N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue Magnoila Boulevard Westfield Fashion Square /1-05-3606-1 CMA11 Accutek

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Magnolia Boulevard Annual Growth: Peak Hour:

08/07/2008 2007 2011 Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	E	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS.	2011	W PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	96	~	96	ω	103	-	103	0	103	-	103	0	103	-	103	0	103	-	103
Comb. L-T		0	,			0			;	0		•	į	o ·		•	;	ο·	
NB Thru	740	-	421	23	800	-	455	78	828		469	0	828		469	0	828		469
Comb. T-R		-	421			τ	455		:	- '	469	•			469	•	•	- 1	469
NB Right	102	0		80	110	0	,	0	110	0		τ-		0		0	=	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	165	-	165	13	179	-	179	-	178	-	178	0	178	-	178	0	178	-	178
Comb. L-T		0				0				0	1			0	,			0	
SB Thru	1265	-	692	101	1366	 .	747	21	1387	-	757	ო	1390	 .	759		1390	Ψ,	759
Comb. T-R	,	 (692	Ċ	6	- c	/4/	c	6	 c	/ç/	c	60,7	- c	62/	c	000	- 0	86/
SB Kignt Comb. L-T-R -	<u> </u>	00		n	071	0		>	971	0		>	07	0	:	•	07	0	ı
!																			
EB Left	64	Ţ	64	22	2	-	70	o	70	-	70	0	70	-	70	0	70	-	70
Comb. L-T		0				0	1			0	1			0	•			0	
EB Thru	872		477	2	941		516	24	962		528	0	965	-	528	0	965		528
Comb. T-R	;	- 1	477	•		- (516	•	;	- (228	•	8	- 0	228	c	ć	- (228
EB Right	833	0		7	8	0 +		0	3	0 1		0	3	o (0	25	0 (
Comb. L-T-R -		0				0				0				0				0	
WB Left	107	-	107	6	116	-	116	-	117	1	117	0	117	-	117	0	117	-	117
Comb. L-T		0	•			0	,			0	,			0				0	
WB Thru	945		538	9/	1021		581	က	1024	τ,	582	_	1025	- 1	582	0	1025		582
Comb. T-R	į	- (ç	,	- (581	•	,	- (282	c	,	, c	285	c	•	- c	285
We Right	13.	o c		2	142	o c		7-	041	o c		>	5	o c		0	04-	o c	
		•				•				•				•				•	
Crit. Volumes:			787				850			:S-Z:	861			:S-Z	862			S-N	862
		SUM:	603 1390			E-W: SUM:	1501			SUM:	651 1512			SUM:	652 1514			SUM:	1514
No. of Phases:	12		2				2				2				7				2
Volume / Capacity:	acity:	Ξ	0.857			[2]	0.901			[2]	0.908			[2]	606.0			[2]	0.909
Level of Service:	ce:		۵				E				ш				ш				ш

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Magnoila Boulevard Westfield Fashion Square /1-05-3606-1 CMA11 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Magnolia Boulevard Peak Hour: PM Annual Growth: 2.00%

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	SOUT EXIST TRAFFIC	ART TRI	JEE IC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	Ŧ	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS.	2011	W/ PROP	2011 W/ PROPOSED PROJECT	JJECT	2011 \	2011 W/ MITIGATION	VOIT	
		No of	9 20	Added Total	Total	No. of	lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume La	Lanes	Volume	Volume Volume	Volume	Lanes	Volume			Lanes	Volume		Volume	Lanes	Volume			Lanes	Volume
	i	-	66	ω	107	← ·	107	0	107		107	0	107	← (107	0	107	- 0	107
Comb. L-T NB Thru	1042	0 +-	570	83	1125	0 -	616	22	1147	o •	627	0	1147	o ∙	629	0	1147) -	629
Comb. T-R NB Right Comb. L-T-R -	86	-00	. 570	κο	106	-00	. 616	0	106	-00	- 62/	4	110	-00	679	0	110	-00	670
SB Left	76	- 0	9/	9	82	- 0	82	٢	81	- 0	81	0	81	- 0	8	0	81	r- c	81
SB Thru	834	o − ₁	462	29	901) - - c	499	24	925	o	511	€	938) 	518 518	0	938) - -	518
SB Right Comb. L-T-R -	06	-00	407	7	86	-00	1	0	86	- 0 0		0	86	-00	;	0	86	-00	,
EB Left	87	- (87	7	94	- 0	94	0	94	- 0	94	0	94	← c	94	0	94	- 0	94
Comb. L-1	937	o 1	526	75	1012) - - •	568	14	1026	o •	575	0	1026) -	575	0	1026) , . .	575
Comb. 1-K EB Right Comb. L-T-R -	114	-00	970	O	124	-00	9	0	124	-00	'	0	124	-00	5	0	124	-00	,
WB Left	104	- (104	80	112	-	112	-	113	- 0	113	0	113	- 0	113	0	113	← C	113
WB Thru	545) - -	315 315	44	589) -	340 340	7	009) 	345	4	604) 	347	0	604	o	347
WB Right Comb. L-T-R	- 84	-00	5	7	91	- 0 0	}	7	06	-00))	0	06	.00		0	06	00	1
Crit. Volumes:		N-S: E-W: SUM:	646 630 1276			N-S: E-W: SUM:	698 680 1378			R-V: SUM:	708 688 1396			N-S: E-W: SUM:	710 688 1398			N-S: E-W: SUM:	710 688 1398
No. of Phases:			2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.780			[2]	0.818			[2]	0.830			[2]	0.832			[2]	0.832
Level of Service:	ce:		c																

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Woodman Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA12 N-S St: E-W St: Project:

Accutek

File Name: Counts by:

AM 2.0% Peak Hour: Annual Growth:

Woodman Avenue @ Riverside Drive CRITICAL MOVEMENT ANALYSIS

07/22/2008 2007 2011 Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007 E.	2007 EXIST, TRAFFIC	FFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	E	2011 \	W OTHER	2011 W/ OTHER PROJECTS	:TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	DJECT	2011	2011 W/ MITIGATION	NOIT	
	_	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume		Lanes	Volume		Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	183	2 5	101	15	198	2	109	0	198	2.0	109	12	210	87.0	115	0	210	0.0	115
Comb. L-1 NB Thru	734	9 K C	367	29	793	0 0 0	396	20	813	0 77 0	406	0	813	0 00 0	406	0	813	0 00 0	406
Comb. T-R NB Right [2] Comb. L-T-R -	219	0 - 0	219	18	237	0 - 0	237	52	289	0 0	- 289	0	289	0 - 0	- 289	0	289	0 - 0	289
SB Left	229	-	229	18	247	- 0	247	7	254	- 0	254	0	254	- 0	254	0	254	- 0	254
SB Thru	1165	o 70 C	582	93	1258	0 00 0	629	4	1272	O 70 C	636	2	1274	O 101 C	637	0	1274	- 13 C	465
Comb. 1-K SB Right [2] Comb. L-T-R -	11	0 0	E	თ	120	0 0	120	0	120	0 + 0	120	7	122	0 - 0	122	0	122	-00	9
EB Left	95	-	98	80	102	- (102	-	103	- (103	-2	101	- 0	101	0	101	0	101
Comb. L-T EB Thru	880	0 N C	440	70	950) N C	475	4	991) N C	496	4	995	0 10 0	498	0	966	o 10 C	498
Comb. T-R EB Right Comb. L-T-R -	116	0 - 0	116	σ	126	o c	126	-	127	0 0	127	25	152	0 - 0	152	0	152	0 0	. 152
WB Left	291	- 0	291	23	314	r- 0	314	35	349	- 0	349	0	349	- c	349	0	349	← C	349
WB Thru	1010	9 0	505	81	1091	9 67	545	35	1126	0 70 0	563	ω	1134	0 00	567	O	1134	0 00 0	267
Comb, T-R WB Right Comb, L-T-R -	185	0 0	185	15	200	0 - 0	200	7	207	0 - 0	207	0	207	0 0	207	0	207	0 - 0	207
Crit. Volumes:		N-S: E-W: SUM:	683 731 1414			N-S: E-W: SUM:	738 790 1527			N-S: E-W: SUM:	745 845 1590			N-S: E-W: SUM:	752 847 1599			N-3: SUM: SUM:	661 847 1508
No. of Phases:			4				4				4				4				4
Volume / Capacity: Level of Service:	city:	[1]	0.959 E			[1]	1.041 F			[1]	1.086 F			Ξ	1.093 F			[6]	0.997 E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

If I yor capacity as a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[3] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA12 Accutek N-S St: E-W St: Project: File Name: Counts by:

Date: Date of Count: Projection Year:

07/22/2008 2007 2011

ALTERNATIVE G PROJECT

Woodman Avenue @ Riverside Drive CRITICAL MOVEMENT ANALYSIS

Z d 2.00%

Annual Growth: Peak Hour:

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	HT.	2011	W OTHE	2011 W/ OTHER PROJECTS	STS	2011	W PROPC	2011 W/ PROPOSED PROJECT	JECT	2011 W/ MITIGATION	MITIGATI	No.	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added To	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Vol	Volume L	Lanes	Volume
NB Left	342	0.0	188	27	370	0.0	203	-	371	0.0	204	49	420	7 0	231	0	420	27.0	231
NB Thru	917	0 10 0	459	73	991	0 00 0	495	18	1009	0 10 0	504	0	1009	0 70 0	504	0	1009	0 77 0	504
Comb. I-K NB Right [2] Comb. L-T-R -	197	0 - 0	197	16	212	0 - 0	212	7	219	0 ~ 0	219	0	219	0 - 0	219	0	219	0 - 0	219
SB Left	150	- 0	150	12	162	- 0	162	5	167	- 0	167	0	167	- 0	167	0	167	- 0	167
SB Thru	823	O 70 C	411	99	888	O 70 C	444	20	908	0 10 0	454	7	915	0 00 0	458	0	915	o 01 +	376
Comb. I-R SB Right [2] Comb. L-T-R -	188	0 - 0	188	5	203	0 0	. 203		204	0 - 0	204	ю	212	0 - 0	212	0	212	-00	9/0
EB Left	213	- 0	213	17	230	- 0	230	-	231	- 0	231	0	231	- 0	231	0	231	← ¢	231
EB Thru	916	0 00 0	458	73	066	0 00 0	495	38	1028	0 10 0	514	25	1053	0 00 0	526	0	1053	0 77 0	526
Comb. I-R EB Right Comb. L-T-R -	257	0-0	257	21	277	0 - 0	277	~	278	0 - 0	278	265	543	0 - 0	543	0	543	o - c	543
WB Left	239	- 0	239	19	258	- 0	258	9	264	- 0	264	0	264	- 0	264	0	264	- 0	264
WB Thru	206	0 10 0	453	73	979) N C	490	42	1021	0 10 0	511	32	1053	0 10 0	527	0	1053	0 10 0	527
Comb. L-T-R Comb. L-T-R-	254	0 0	254	20	274	0-0	274	4	278	0 - 0	278	0	278	0 - 0	278	0	278	0 0	278
Crit. Volumes:		N-S: E-W: SUM:	608 697 1306			N-S: E-W: SUM:	657 753 1410			N-S: E-W: SUM:	671 778 1449			N-S: E-W: SUM:	688 791 1479		2 11 8	N-S: E-W: SUM:	671 791 1462
No. of Phases:			4		<u> </u> 		4				4				4				4
Volume / Capacity: Level of Service:	e:	[1]	0.880 D			[1]	0.956 E			[1]	0.984 E			Ξ	1.006 F			[3] E	0.963

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane.

For one excl. and one opt. tum lane, 50% of volume is assigned to exclusive lane.

For one excl. and one opt. tum lane, 50% of volume is assigned to exclusive lane.

For one excl. and one opt. tum lane, 50% of volume is assigned to exclusive lane.

Fight tums on red from excl. lanes = 50% of volume land in left. tum novement and southbound right tum has an overlapping phase with the westbound left-tum movement and southbound right tum has an overlapping phase with the westbound left-tum movement [3] wc ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 Accutek N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps Annual Growth: Peak Hour:

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	H	2011 \	W OTHE	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	SUECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	348	€	348	28	376	€	376	-	377	← 0	377	0	377	← 0	377	0	377	← (377
NB Thru	890	0 m m	297	71	961) က (320	27	1018	- m (339	co.	1023	⊃ m (341	0	1023	0 m	341
Comb. 1-R NB Right Comb. L-T-R -	0	000	1 1	0	0	000		0	0	000		0	0	000	1 1	0	0	000	
SB Left	0	.		0	0	0		0	0		1	0	0) 0		0	0		
Comb. L-T SB Thru	1062	040	. 265	85	1147	040	- 287	42	1189	040	297	7	1196	041	- 299	0	1196	04	- 299
Comb. I-R SB Right Comb. L-T-R -	545	0 - 0	. 545	4	589	0 + 0	589	თ	598	0 - 0	598	ო	601	0 - 0	601	0	601	0 - 0	- 601
EB Left	0	0 0		0	0	0 0		0	0	0 0		0	0	0 0		0	0	0 0	
EB Thru	0	000		0	0	000		0	0	000		0	0	000	1 1	0	0	00	
Comb. I-R Comb. L-T-R -	0	000		0	0	000	: :	0	0	000	: 1	0	0	000		0	0	000	1 1
WB Left	314	-	173	25	339	-	187	18	357	_	196	0	357	-	196	0	357	-	196
Comb. L-T WB Thru	4	000	- 265	0	4	000	- 286	0	4	000	300	0	4	000	303	0	4	001	- 303
Comb. 1-K WB Right Comb. L-T-R -	265	O	146	21	286	o	158	4	300) - -	165	9	306	o	169	0	306	0	169
Crit. Volumes:		N-S: E-W: SUM:	893 265 1158			N-S: E-W: SUM:	965 286 1251			N-S: E-W: SUM:	975 300 1275			N-S: E-W: SUM:	978 303 1281			N-S: E-W: SUM:	978 303 1281
No. of Phases:			8				3				က				က				8
Volume / Capacity: Level of Service:	city:	Ξ	0.743 C			Ε	0.808			Ξ	0.825 D			[1]	0.829 D			[2]	0.799 C

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 70% of volume is assigned to exclusive lane. 70% of volume is assigned to exclusive lane. 70% of volume is assigned to exclusive lane. 80% of volume is assigned from excl. lanes = 50% of volume is assigned to exclusive lane. 10% of volume is assigned to exclusive lane. 11% or eation includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] who ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA13 Accutek

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

Woodman Avenue @ Ventura Freeway Westbound Ramps

Ā 2.00%

Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

	2007 EXIST. TRAFFIC	TRAFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	H	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	SUECT	2011	2011 W/ MITIGATION	ATION	
	No. of	f Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume Lanes		Volume	Volume Volume	Lanes	Volume		_	Lanes	Volume		Volume	Lanes	Volume	Volume		Lanes	Volume
NB Left	314	1 314	25	339	← (339	2	341	- 4	341	0	341	₩ (341	0	341	~ ·	341
Comb. L-1 NB Thru	1186	3 395	95	1280	၁၈၊	427	56	1306	၁၈၊	435	21	1327	၁ ၈ (442	0	1327	၁က	- 442
Comb. T-R NB Right Comb. L-T-R -	0		0	0	000	1 1	0	0	000		0	0	000	. ,	0	0	000	
SB Left	0	,	0	0	0		0	0	0	1	0	0	0	,	0	0	0	
Comb. L-T SB Thru	917	0 - 4 229	73	991	04	248	28	1019	041	. 255	46	1065	040	. 266	0	1065	04	266
Comb. I-R SB Right Comb. L-T-R -	486	0 . 1 486 0	39	525	0 - 0	525	7	524	0 - 0	524	23	547	0 - 0	547	0	547	o ~ c	547
EB Left	0		0	0	0	,	0	0	0	ı	0	0	0		0	0	0	1
Comb. L-T EB Thru	0	, ,	0	0	00		0	0	00	1 1	0	0	00	, ,	0	0	00	
Comb. T-R EB Right	0		0	0	000		0	0	000	t 1	0	0	000		0	0	000	
Comb. L-1-R					0				>				0				o	
WB Left Comb 1-T	402	1 221	32	435	- c	239	16	451	- 0	248	O	451	- 0	248	0	451	- 0	248
WB Thru	0	344	0	0	00	372	0	0	00	380	0	0	00	390	٥	0	00	390
WB Right Comb. L-T-R -	363	1 200	59	392) 	216	-	393) 	216	23	416	o	229	0	416	o 	229
Crit. Volumes:	N-S: E-W: SUM:	800 344 1144			N-S: E-W: SUM:	864 372 1236			N-S: E-W: SUM:	865 380 1244			N-S: E-W: SUM:	888 390 1278			N-S: E-W: SUM:	888 390 1278
No. of Phases:		ღ				3				3				6				ю
Volume / Capacity:	ty:	[1] 0.733			Ξ	0.797			Ξ	0.803			Ξ	0.827			[2]	0.797
רבאבו חו סבו אותר	;	اد												ا د				اد

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 70% of volume is assigned to exclusive lane. For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane. 8ight turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Freeway Eastbound Ramps AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007 E	2007 EXIST, TRAFFIC	YFFIC	2011	2011 W/ AMBIENT	ENT GROWTH	TH.	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W PROP	2011 W/ PROPOSED PROJECT	SJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total		Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume		-	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
	0	0	,	0	0	0	•	0	0	0	,	0	0	0		0	0	0	•
Comb. L-T		0				0	,			0	,		;	0 1	'	•	i	0 (,
NB Thru	786	o +	262	63	849	m ≁	283 388	44	893	ო -	298 406	7	892	w ~	298 406	0	895	ი ←	406
NB Right	359	- 0	3	59	388	- 0	3	18	406	0		0	406	0		0	406	0	1
Comb. L-T-R -		0				0				0				0				0	
SB Left	340	-	340	27	367	-	367	10	377	-	377	4	381	- (381	0	381	- (381
Comb. L-T		0 (,	1		0 (,	S	1220	0 (, 4	٣	1233	o د	- 617	c	1233	o د	- 617
SB Thru	1093	N C	54/	/8	0811	N C	O&C ,	2	062	7 0	<u>.</u>	י	555	40	5 '	•	2	40	;
SB Right	0	0	. ,	0	0	0	1	0	0	0		0	0	0		0	0	0	
Comb. L-T-R -		0				0				0				0				0	
EB Left	339	-	186	27	366	-	201	14	380	- (509	9	383	- 0	211	0	383	- 0	211
Comb. L-T	"	00	333	C	C.	o c	359	c	m	0	365	0	ო	0	367	0	ო	0	367
Comb. T-R	,	0	7	•)	0	}	Ì	•	0				0				0	1
EB Right	392	-	216	31	423		233	Ψ-	424	- τ	233	0	424		233	0	424		233
Comb. L-T-R -		-				-				-				-				-	
WB Left	0	0		0	0	0 (0	0	0 0	-	0	0	0 0	1	О	0	0 0	
Comb. L-T	•	0 (•	c	o (c	c	o c	ı	c	c	0 0		c	c	oc	
WB Ihru	0	> C		5	>	o c	, ,	>	0	0		0	•	0		•	•	0	1
WB Right	0	0	•	0	0	0		0	0	0		0	0	0	•	0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	669			N-S:	755			N-S:	783			N-S:	787			N-S:	787
		E-W: SUM:	332 1031			E-W: SUM:	359 1113			E-W: SUM:	365 1148			E-W: SUM:	367 1154			E-W: SUM:	367 1154
No. of Phases:			ო				m				က				က				ო
Volume / Capacity:	icity:	Ε	0.654			[2]	0.681			[2]	0.706			[2]	0.710			[2]	0.710
Level of Service:	.; ;		В				8				O				C				0

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 55% of volume is assigned to exclusive lane. For one excl. and one opt. turn lane, 56% of volume is assigned to exclusive lane. Right turns on red from excl. lanes = 50% of overlapping left turn. [1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] v/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 Acoutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour:

2.00%

Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	2011 W/ AMBIER	ENT GROWTH	HI	2011	W OTHE	2011 W/ OTHER PROJECTS	TS	2011	W PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	Na. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0	•	0	0	0	ı	0	0	0	•	0	0	0	1
Comb. L-T		0	,	;	•	0 (0 (ď	3	0 (,	•	3	0 0	,
NB Thru	1094	m +	365	88	1182	m +	394	28	1210	יי ניו	403	20	1218	.n +	406	0	1218	ω ←	406
NB Right	364	- 0	g '	29	393	- 0	† 60 .	4	407	- 0	ř,	0	407	- 0	ř,	0	407	- 0	ì
Comb. L-T-R -		0				0				0				0				0	
SB Left	287	ŀ	287	23	310	۴	310	0	310	-	310	25	335	-	335	0	335	-	335
Comb. L-T	•	0 (1	7	,	0 0		ţ	,	0 (,	č		0 (,	c	1	0 (1
SB Thru	1010	0 0	202	8	1091	N C	545	4	1136	N C	208	17	115/	N C	, 5/8	>	113/	N C	9/g
SB Right	0	0		0	0	0		0	0	0		0	0	0		0	0	0	,
Comb. L-T-R -		0				0				0				0				0	
EB Left	467	-	257	37	504	-	277	0	504	-	277	13	517	-	285	0	517	- 1	285
Comb. L-T	•	0 0	,	c	•	0 0	, 5	c	•	0 0	,	c	•	0 0	-	c	•	00	ď
Comb. T-R	-	0	3/2	>	-	00	, 5	>	-	0	407	0	-	0 0	,	ס	-	0	5
EB Right	357	-	196	59	385	-	212	7	387	-	213	0	387		213	0	387	-	213
Comb. L-T-R -		-				₩				-				-				-	
WB Left	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	1
Comb. L-T		0	ı			0	ı	•		0	,	•	4	0		•	•	0 (,
WB Thru	0	0 0		0	0	0 0		0	0	0 0		0	0	0 0		0	O	00	1
WR Right	С	0 0		0	0	0 0		0	0	00		0	0	0	. ,	0	0	00	
Comb. L-T-R -	,	0				0				0				0				0	
Crit. Volumes:		S-S	652			N-S:	704			S-S	717			S-S	742			N-S:	742
		E-W: SUM:	372 1023			E-W: SUM:	1105			SUM:	402 1119			E-W: SUM:	408 1150			E-W: SUM:	408 1150
No. of Phases:			က		<u>.</u> <u>.</u>	-	က				က				ო				က
Volume / Capacity:	acity:	Ξ	0.648			[2]	0.676			[2]	0.686			[2]	0.707			[2]	0.707
Level of Service:	.e:		В				В				В				O				S

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.
55% of volume is assigned to exclusive lane.
56% of volume is assigned to exclusive lane.
67% of vordapping left turn.
68/20 to red from excl. lanes = 50% of vordapping left turn.
69/20 vor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

61/20 vor ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15 N-S St: E-W St: Project: File Name:

Accutek

Counts by:

Woodman Avenue @ Moorpark Street AM 2.0% Annual Growth: Peak Hour:

CRITICAL MOVEMENT ANALYSIS

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	2007 EXIST, TRAFFIC	AFFIC	2011	W AMBI	2011 W/ AMBIENT GROWTH	E	2011	W OTHE	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Totai	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume		Volume	Volume Volume	Volume	Lanes	Volume		Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	12	-	12	-	13	-	13	0	13	-	13	0	13	-	13	0	13	₩ (13
Comb. L-T NB Thru	490	0 0 0	245	39	529	0 0 0	. 265	57	586	0 77 0	293	-	587	0 77 0	294	0	587	5 N C	294
Comb. 1-R NB Right Comb. L-T-R -	24	0 - 0	24	7	26	0 + 0	. 26	7	28	0 - 0	- 28	0	28	0 0	- 28	0	28	0-0	- 28
SB Left	297	- 0	297	24	321	- 0	321	2	323	- 0	323	-	324	- 0	324	0	324	- 0	324
SB Thru	851	o •	528	99	919	o - -	57.1	48	296	o r	595	7	898) -	596	0	696	o	596
Comb. 1-K SB Right Comb. L-T-R -	206	-00	976	16	222	-00	- 70	0	222	-00	,	0	222	-00	, '	0	222	-00	· ·
EB Left	111	-	111	თ	120	- (120	0	120	- 0	120	0	120	- 0	120	0	120	- 0	120
Comb. L-1 EB Thru	551	o ← (551	44	595	o ← (595	28	623	o	623	0	623	o	623	0	623	o c	623
Comb. 1-K EB Right Comb. L-T-R -	40	0 - 0	40	ო	43	0 0	43	0	43	0 - 0	,	0	43	0 - 0	, &	0	43	0 0	43
WB Left	85	- 0	85	7	92	- 0	92	2	94	⊢ c	94	0	94	0	94	0	94	- 0	94
WB Thru	726	> ← 0	726	28	784	o ← c	784	43	827	o ← c	827	0	827	o ← c	827	0	827	o c	827
Comb. L-T-R -	276	0 - 0	276	22	298	0 0	298	4	302	0 - 0	302	0	302	o 0	302	0	302	0 0	302
Crit. Volumes:		N-S: E-W: SUM:	542 837 1380			N-S: E-W: SUM:	586 904 1490			N-S: E-W: SUM:	616 947 1563			N-S: E-W: SUM:	618 947 1565			N-S: E-W: SUM:	618 947 1565
No. of Phases:	1:6		2				2				2				2				2
Volume / Capacity: Level of Service:	acity: ce:	[1]	0.850 D			[1]	0.923 E			[1]	0.972 E			[1]	0.973 E			[2]	0.943 E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Project: File Name: Counts by: N-S St: E-W St:

Accutek

Woodman Avenue Moorpark Street Westfield Fashion Square /1-05-3606-1 CMA15

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Moorpark Street 2.00% Peak Hour: Annual Growth:

Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

National National		2007	2007 FXIST TRAFFIC	AFFIC	2011	W AMBIE	2011 W/ AMBIENT GROWTH	Ŧ	2011 V	V/ OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	JUECT	2011	2011 W/ MITIGATION	VOIT	
Mathematic Mat			No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
The control of the	Movement	Volume	Lanes	Volume	Volume			Volume	Volume		Lanes	Volume			Lanes	Volume		Volume	Lanes	Volume
1	NB Left	67		29	5	72	- 0	72	0	72	← 0	72	0	72	~ 0	72	0	72	 c	72
The color of the	Comb. L-T NB Thru	772	0 77 0	386	62	833) N C	417	44	877) N C	439	5	882) N C	, 4 14	0	882) N C	441
The color of the	Comb. I-R NB Right Comb. L-T-R		0 - 0	22	4	90	0 - 0	09	2	62	0 - 0	62	0	62	0 - 0	62	0	62	0 - 0	62
1	SB Left	263	- (263	21	284	- 0	284	0	284	- 0	284	7	291	- 0		0	291	- c	291
T-R- 251 0 - 10 205 1 0 20 271 0 2 271 0 221 0 221 1 221 2 223 1 223 0 223 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Comb. L-T SB Thru	734	0 7	492	59	793	O	532	47	840) - -	555 555	4	854	>		0	854) - -	562
-FR 685 1 685 55 740 1 740 35 775 1 775 0 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	SB Right Comb. L-T-R		-00	1	20	271	-00	7	0	271	-00	8	0	271	00		0	271	. 0 0	1
He He He He He He He He	EB Left	205	- (205	16	221	- 0	221	0	221	- 0	221	2	223	- 0	223	0	223	- 0	223
T-R-	Comb. L-1 EB Thru	685	o (- 685	22	740	o	740	35	775	o c	775	0	775	o ← c	775	0	775	o (775
71 1 71 6 76 1 76 3 79 1 79 1 79 0 79 1 1 79 0 79 1 0 1 0 <td< td=""><td>Comb. I-K EB Right Comb. L-T-R</td><td></td><td>0 - 0</td><td>. 82</td><td>7</td><td>89</td><td>0 - 0</td><td>- 89</td><td>0</td><td>89</td><td>0-0</td><td>68</td><td>0</td><td>88</td><td>0 - 0</td><td>68</td><td>0</td><td>88</td><td>0 - 0</td><td>68</td></td<>	Comb. I-K EB Right Comb. L-T-R		0 - 0	. 82	7	89	0 - 0	- 89	0	89	0-0	68	0	88	0 - 0	68	0	88	0 - 0	68
551 1 551 44 595 1 595 45 640 1 640 2 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 642 1 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 7 7 7 7 7 7 7 7 7 7 8 7 8 7 8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 9 8 9 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	WB Left	71	- 0	71	9	76	- 0	76	3	79	- 0	79	0	79	← c	79	0	79	← c	6/
258 0 - 26	WB Thru	551	- c	551	44	595	o — (595	45	640) - 1	640	2	642	o (642	0	642	o ← 0	642
N-S: 649 N-S: 701 N-S: 723 N-S: N-S: E-W: 756 E-W: 817 E-W: 862 E-W: 866 E-W: SUM: 1517 SUM: 1584 SUM: 1598 SUM: 2 2 2 2 2 A y; [1] 0.942 [1] 0.986 [1] 0.995 [2] b E E E E E E E	Comb. T-R WB Right Comb. L-T-R		0-0	258	21	279	0-0	279	0	279	0 0	279	0	279	o ~ c	279	0	279	0 - 0	279
2 2 2 2 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Crit. Volumes		N-S: E-W: SUM:	649 756 1405			N-S: E-W: SUM:	701 817 1517			N-S: E-W: SUM:	723 862 1584			N-S: E-W: SUM:	732 866 1598			N-S: E-W: SUM:	732 866 1598
iy: [1] 0.867 [1] 0.942 [1] 0.986 [1] 0.995 [2] E E	No. of Phases			2				2				2				2				2
	Volume / Cap Level of Servi	acity: ce:	Ξ				1 1				Ξ				Ξ					0.965 E

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 N-S St; E-W St; Project; File Name; Counts by;

Accutek

Date: Date of Count; Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

Woodman Avenue @ Ventura Boulevard

AM 2.0%

Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2011 \	N/ AMBIE	2011 W/ AMBIENT GROWTH	Ŧ	2011 V	V/ OTHER	2011 W/ OTHER PROJECTS	TS	2011	W PROP	2011 W/ PROPOSED PROJECT	SUECT	2011	2011 W/ MITIGATION	ATION	
	-	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane
Movement \	Volume L	Lanes	Volume	Vоlите	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	51	-	51	4	55	-	55		63	-	63	0	63	-	63	0	63	₩	63
Comb. L-T		0				0				0				0	,			0	,
NB Thru	201	0	•	16	217	0		9	223	0	•	-	224	0		0	224	0	
Comb. T-R	;	Ψ.	219		į	← (237	•	į	 (247	•	č	← (248	•	į	τ (248
NB Right	<u>6</u>	0 (τ-	20	0 0		4	24	0 0		0	54	0 0		0	24	0 (•
Comb. L-T-R -		0				0				0				5				ɔ	
SB Left	216	-	216	17	234	-	234	7	241	-	241	1	242	-	242	0	242	-	242
Comb. L-T		0				0	1			0	1			0	•			0	1
SB Thru	225	-	225	18	243	-	243	5	248	-	248	-	249	Ψ-	249	0	249	₩.	249
Comb. T-R	į	o ·	,	ć	ì	۰ ۰		ć	ģ	۰ ،	,	•	ç	0 ,	,	,	9	o ·	,
SB Right	362	- 1	362	53	391	- (391	28	429	- (429	5	429	- (429	0	429	, (429
Comb. L-T-R -		0				0				0				0				0	
EB Left	142	-	142	11	154	1	154	41	195	1	195	0	195	-	195	0	195	-	195
Comb. L-T		0				0	,			0				0				0	1
EB Thru	1082	-	554	87	1168	•	598	96	1264	-	654	0	1264	-	654	0	1264	₩.	654
Comb. T-R			554	•	;	- ι	298		;		654	•		- (654	•		₹ (654
EB Right	56	0		2	28	0 +	ı	16	44	0		0	4	o (0	44	0 (
Comb. L-T-R -		0				0				0				0				0	
WB Left	45	-	45	4	48	1	48	-	49	-	49	0	49	-	49	0	49	_	49
Comb. L-T		0	,			0	į			0				0				0	1
WB Thru	1091	Ψ.	603	87	1178	-	651	25	1230		683	0	1230	Ψ.	683	0	1230	Ψ.	683
Comb. T-R		, ,	603	•	•	- 1	651	;	•	- 1	683	•	•		683	•	,	- 1	683
WB Right	114	0 (,	ത	124	0 (12	136	o (0	136	0 0		0	136	0 (•
Comb. L-1-K-		0				5				0				0				0	
Crit. Volumes:		N-S:	436			N-S:	471			N-S:	488			N-S:	490			N-S:	490
	=	E-W.	745			E-W:	805			E-W:	878			E-W:	878			E-W:	878
	-	SUM:	1181			SUM:	1275			SUM:	1365			SUM:	1367			SUM:	1367
No. of Phases:			2				2				2				2				2
Volume / Capacity:	ify:	Ξ	0.717			[2]	0.750	•		[2]	0.810			[2]	0.812			[2]	0.812
Level of Service:	2.5		ပ				C				٥				٥				D 0

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane,

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

Woodman Avenue Ventura Boulevard Westfield Fashion Square /1-05-3606-1 CMA16 Accutek N-S St: E-W St: Project: File Name: Counts by:

Woodman Avenue @ Ventura Boulevard Peak Hour: PM Peak Hour: Annual Growth:

CRITICAL MOVEMENT ANALYSIS

Date: Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT

	2007 E	2007 EXIST, TRAFFIC	FFIC	2011	V/ AMBIE	2011 W/ AMBIENT GROWTH	Ŧ	2011 V	V/ OTHER	2011 W/ OTHER PROJECTS	STC	2011	W/ PROP	2011 W/ PROPOSED PROJECT	SJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	59	-	69	5	64	-	64	24	88	-	88	0	88	-	88	0	88	+	88
Comb. L-T		0				0				0	,			0				0	,
NB Thru	213	0	,	17	230	0	ļ	9	236	0	•	4	240	0	1	0	240	0	1
Comb. T-R			219			-	237			-	245			-	249			τ-	249
NB Right	9	0		0	7	0		7	ത	0		0	თ	0		0	თ	0	,
Comb. L-T-R -		0				0				0				0				0	
SB Left	125	-	125	10	135	-	135	-	134	-	134	6	143	-	143	0	143	-	143
Comb. L-T		0	•			0				0				0	į			0	,
SB Thru	161	Ψ-	161	13	174	-	174	4	178	-	178	4	182	-	182	0	182		182
Comb. T-R		0				0		!	;	0			į	0		•		ο.	
SB Right	237	-	237	19	256	-	256	47	303	-	303		304	-	304	0	304	-	304
Comb. L-T-R -		0				0				0				0				0	
EB Left	206	-	206	16	222	-	222	40	262	1	262	0	262	-	262	0	262	1	262
Comb. L-T		0	,			0				0	1			0	,			0	•
EB Thru	1093	-	267	87	1180		612	84	1264	-	663	0	1264	-	663	0	1264	-	663
Comb. T-R		-	267				612			-	663			-	663			-	99
EB Right	41	0		ო	44	0		8	62	0	,	0	62	0	ı	0	62	0	,
Comb. L-T-R -		0				0				0				0				0	
WB Left	24	-	24	2	26	1	26	-	27	-	27	0	27	-	27	0	27	-	27
Comb. L-T		0				0	1			0	1			0	1			0	,
WB Thru	910	-	515	73	983	-	556	86	1069	-	299	2	1071	-	901	0	1071		601
Comb. T-R		-	515			-	556			-	599			-	601			-	109
WB Right	120	0		10	129	0	•	0	129	0	,	2	131	0		0	131	0	
Comb, L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	344			N-S:	372			N-S:	379			S-N	392			N-S:	392
		E-W:	721			E-W:	778			Ē-Ķ	861			E-W:	863			E-W:	863
		SUM:	1065			SUM:	1150			SUM:	1240			SUM:	1255			SUM:	1255
No. of Phases:			2				2				2				2	***************************************			2

Volume / Capacity:	icity:	Ξ	0.640			[2]	0.667			[2]	0.727			[2]	0.737			[2]	0.737
Level of Service:	é)		æ			_	80				O				ပ				

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation. Assumptions:

Fashion Square Project Driveway-Matilija Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 N-S St: E-W St: Project: File Name: Counts by:

CMA17 Accutek

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: AM Annual Growth: 2.0% Annual Growth:

08/07/2008 2007 2011

Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBII	2011 W/ AMBIENT GROWTH	/TH	2011	W OTHER	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	SUECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement Volume	Volume	Lanes	Volume	Volume	Volume Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	0	0	,	0	0	0		0	0	0	ı	10	9	-	6	0	5	-	9
Comb. L-T		0	1			0	•			0				0				0	
NB Thru	0	0		0	0	0	1	0	0	0		0	0	0		0	0	0	•
Comb. T-R		0	,			0	,			0				0	,			0	
NB Right [3]	0	0	,	0	O	0	,	0	0	0		64	9	2	32	0	64	2	35
Comb. L-T-R -		0				0				0				0				0	
SB Left	35	0	1	3	38	0		0	38	0		-38	0	0	,	0	0	0	
Comb. L-T		0				0				0	,			0				0	
SB Thru	0	0	29	0	0	0	64	0	0	0	94	0	0	0		0	0	0	,
Comb. T-R		0				0	ı			0				0	1			0	
SB Right	24	0		7	26	0	,	0	56	0	,	39	62	-	62	0	62	-	62
Comb. L-T-R -		•				-				-				0				0	
EB Left	9	1	9	0	7	F	7	6	7	-	7	<u> </u>	o	0		0	o o	0	
Comb. L-T		0				0	,			0	•			0	1			0	,
EB Thru	1017	7	509	81	1098	2	549	43	1141	2	571	-	1142	7	571	0	1142	7	571
Comb. T-R		0	1			0				0	,	1	i	0		,	i	0	
EB Right	0	0		0	0	0		0	0	0	1	65	29	- 1	29	0	29	, (29
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0		0	0	0	,	0	0	0		280	280	2	154	0	280	2	154
Comb. L-T		0				0	,			0				0	•			0	,
WB Thru	1103	-	556	88	1192	-	900	36	1228	-	618	0	1228	-	618	0	1228	•	618
Comb. T-R		•	556			-	900				618	•	,	-	618	,	•	•	618
WB Right	ထ	0		-	ന	0	,	0	ത	0	1	0	თ	0	į	0	o	0	•
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S:	59			N-S:	64			N-S:	64			N-S:	72			S-S:	72
		E-W:	562			E-W:	607			E-W:	625			E-W.	725			E-W:	725
		SUM:	621			SUM:	671			SUM:	689			SUM:	797			SUM:	797
No. of Phases:			D				D				ח				m				8
Volume / Capacity:	city:		0.518				0.559				0.574			[1]. [2]	0.459			[1], [2]	0.459
Level of Service:	jo;		∢				∢				⋖				⋖				∢

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1426, 4+ Phase=1475, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Wc raifo includes a 0.10 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Fashion Square Project Driveway-Matilija Avenue Riverside Drive N-S St: E-W St:

Westfield Fashion Square /1-05-3606-1 Accutek CMA17 Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour. Annual Growth: 2.00% Annual Growth:

08/07/2008 2007 2011

Date: Date of Count: Projection Year;

ALTERNATIVE G PROJECT

	2007 E	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBI	2011 W/ AMBIENT GROWTH	VTH	2011	N/ OTHER	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	VOIT	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	- 1	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
# alv	c	c	,	c	c	c	•	c	c	c		đ	Ġ.	-	66	c	66	-	66
Comb 1-T	•	0	,	•	•	0		•	1	0				0		•		0	
NB Thru	0	0	,	0	0	0		0	0	0		0	0	0		0	0	0	
Comb. T-R		0	,			0	1			0	,			0	,			0	,
NB Right [3]	0	0		0	0	0		0	0	0		726	726	2	388	0	726	7	389
Comb. L-T-R -		0				0				0				0				0	
SBleff	27	0	,	2	29	0		0	29	0	,	-29	0	0		0	0	0	,
Comb. L-T	i	0					,			0	1			0	1			0	,
SB Thru	0	0	46	0	0	0	49	0	0	0	49	0	0	0		0	0	0	1
Comb. T-R		0				0				0	,		!	0		•	!	0	,
SB Right	19	0		-	20	0	1	0	20	0	,	28	48	-	48	0	48	τ-	48
Comb. L-T-R -		-				-				-				0				0	
EB Left	21	1	21	2	22	-	22	0	22	-	22	-22	0	0		0	0	0	-
Comb. L-T		0	,			0	1			0	1			o				0	,
EB Thru	1046	2	523	84	1130	7	565	39	1169	7	584	4	1173	2	586	0	1173	2	586
Comb. T-R		0				0	•			0	•			0				0	
EB Right	0	0	•	0	0	0		0	0	0		119	119	-	119	0	119	-	119
Comb. L-T-R -		0				0				0				0				0	
WB Left	0	0		0	0	0		0	0	0		631	631	2	347	0	631	2	347
Comb. L-T		0	,			0	,			0	1			0	1			0	•
WB Thru	1168	-	599	93	1261	-	647	44	1305	•	699	2	1307	-	670	0	1307	-	670
Comb. T-R		-	599			-	647			•	699			-	670			-	670
WB Right	8	0	1	2	33	0		0	33	0	•	0	33	0		0	33	0	1
Comb. L-T-R -		0				0				0				0				o	
Crit. Volumes:		N-S:	46			N-S:	49			N-S:	49			N-S:	147			N-S:	147
		E-W	620			E-W:	699			E-W:	691			Ë-¥	934			E-W:	934
		SUM:	999			SUM:	719			SUM:	741			SUM:	1080			SUM:	1080
No of Phases:															8				8
)																
Volume / Capacity:	city:		0.555				0.599				0.617			[1], [2]	0.658			[1]. [2]	0.658
Level of Service:	į.		V				∢				ω				ш				B

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1425, 4+ Phase=1475, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of vowedapping left turn.

[3] Northbound right turn has an overlapping phase with the westbound left-turn movement.

[2] Wor ratio includes a so 1.0 reduction due to installation of ATSAC/ATCS as part of the Victory System No. 6.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Hazeltine Avenue Riverside Drive Westfield Fashion Square /1-05-3606-1 CMA7 N-S St: E-W St: Project: File Name: Counts by:

City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

Date of Count: Projection Year:

08/07/2008 2007 2011

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

	2007 E	2007 EXIST. TRAFFIC	\FFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	TH.	2011 V	V/ OTHEF	2011 W/ OTHER PROJECTS	TS.	2011	2011 W/ PROPOSED PROJECT	SED PRO	DIECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added Total	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	173	-	173	14	187	₩.	187	0	187	-	187	21	208	- 1	208	O	208	 (208
Comb. L-T NB Thru	899	0 0	. 334	53	721	0 0	361	23	744	0 7	372	13	757	р И	379	0	757	9 C	379
Comb. T-R NB Right	229	0 -	229	18	247	0 -	- 247	9	257	o -	. 257	0	257	0 -	- 257	0	257	0 ~	257
Comb. L-T-R -		0				O				0				0				0	
SB Left	178	- 0	178	14	192	- 0	192	2	197	- 0	197	13	210	- 0	210	0	210	- 0	210
SB Thru	662	o ·	391	53	715) - ,	422	56	741	o — 1	436	32	773	· - c	452	0	773	· - c	452
SB Right	120	-01	Eg	10	130	-01	774		131	- 0 (. 456	0	131	- 0 0	764	0	131	- 0 0	764
Comb. L-T-R -		0				0				0				>				5	
EB Left	39	- 0	39	က	42	- 0	42	2	44	- c	44	О	44	c	44	0	44	- c	44
EB Thru	519	o ·	345	42	561	· c	372	52	613	· (398	26	639	· - ·	420	0	639	· - ·	420
Comb. T-R EB Right	170	- 0	345	14	184	- 0	3/2	0	184	- 0	398	17	201	- 0	420	0	201	- 0	- 420
Comb. L-T-R -		0				0				0				0				0	
WB Left	223	- 0	223	18	241	- 0	241	13	254	- 0	254	0	254	- c	254	0	254	- 0	254
WB Thru	358	o 0	179	59	387	9 0	193	43	430	7 0	215	34	463	N 0	232	0	463	7 0	232
Comb. T-R WB Right	111	0 -	111	σ	120	0	- 120	ĸ	125	o -	125	22	146	0	146	0	146	0 -	146
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		N-S: E-W: SUM:	564 568 1132			N-S: E-W: SUM:	609 613 1222			N-S: E-W: SUM:	623 652 1275			N-S: E-W: SUM:	660 673 1333			N-S: E-W: SUM:	660 673 1333
No. of Phases:	ļ		2				2				2				2				2
Volume / Capacity:	acity:	Ξ	0.684			Ξ	0.745			Ξ	0.780			Ξ	0.819			[2]	0.789
Level of Service:	.e.		B				0				اد								اد

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane. 55% of volume is assigned to exclusive lane. 70% of volume is assigned to exclusive lane. 70% of vorerlapping left turn. 8 Fight turns on red from excl. lanes = 50% of overlapping left turn. 11 yor ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6. [2] wor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Note: Pass-by reductions not applied to this intersection per LADOT standards.

N-S St: E-W St: Project: File Name:

Hazeltine Avenue Fashion Square Lane Westfield Fashion Square /1-05-3606-1

CMA8 City Traffic Counters Counts by:

CRITICAL MOVEMENT ANALYSIS

Hazeltine Avenue @ Fashion Square Lane Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/07/2008 2007 2011

Projection Year: Date of Count:

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

	2007 EXIST TRAFFIC	TRAFFIC	2011 W/ AMB	/ AMBIE	IENT GROWTH	WTH	2011	W/ OTHE	2011 W/ OTHER PROJECTS	CTS	2011	W PROP	2011 W/ PROPOSED PROJECT	OJECT	2011 V	2011 W/ MITIGATION	NOLLY	
;	No. of		Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume Lanes	Nolume	Volume V	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	ro.	5	0	2	_	ιΩ	0	2	-	ß	0	ις	-	S)	0	ß	-	ഗ
Comb. L-T	-				0				0	,			0	,			0	1
NB Thru	. 889	1 432	55	743	-	466	33	176	.	483	ιΩ	781	Ψ.	495	0	781	Ψ.	495
Comb. T-R		1 432			-	466			•	483	1		Υ	495			-	495
NB Right	175	,	4	189	0		0	189	0		50	508	0		0	209	0	
Comb. L-T-R -		0			0				0				0				0	
SB Left	437	1 437	35	472	-	472	0	472	-	472	49	521	-	521	0	521	-	521
Comb. L-T	_	٠ (0				0				0	,			0	,
SB Thru	. 625	315	90	675	-	340	39	714	-	359	0	714	Ψ-	359	0	714	Ψ-	359
Comb. T-R		315			_	340			τ	328	•			329	•		-	359
SB Right	4		0	4	0	ı	0	4	0		0	4	0	ı	0	4	0	
Comb. L-T-R -		0			0				0				0				0	
EB Left	5	1 5	0	5	-	5	0	5	1	5	0	5	-	5	0	5	-	5
Comb. L-T	-				0				0				0				0	,
EB Thru	7		0	7	0	•	0	7	0		0	2	ο.		0	2	0	,
Comb. T-R		·	(٠	(9	•	•	- σ	φ	•	•	- (ဖ	•	•	~ (9
Comb 1.T.R.	4	,	0	4	o c		-	4	o c		0	4	o c		>	4	o c	
					•				•				•				•	:
WB Left	101	101	8	109	-	109	0	109	-	109	17	126	-	126	0	126	1	126
Comb. L-T	-				0				0				0				0	
WB Thru	0		0	0	0 1		0	0	O 7	. **	0	0	0 0		0	0	0 0	
WB Right	133	3 '	7	144	- 0		C	144	- 0	<u> </u>	12	156	o +-	156		156	· · ·	156
Comb. L-T-R		. 0	:		0				0		ļ		0				0	
Crit. Volumes:		869			N-S:	938			N-S:	954			N-S:	1016			N-S:	1016
	Щ.W.				Ē-₩:	149			Ę.W.	149			Ē-W:	133			Ē-₩.	133
	SUM:	1			SUM:	1087			SUM:	1104			SUM:	1149			SUM:	1149
No. of Phases:	::	3				က				8				3				8
		ı			3	600			3	707.0			5	27.00			15	0000
Volume / Capacity:	acity: [1]	J 0.636			Ξ	U.693			Ξ	0.70 40.70			Ξ	ر. ر. در در			7]	0.70e

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200. Assumptions:

For dual tum lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. tum lane, 50% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

[1] who ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] who ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Note: Pass-by reductions not applied to this intersection per LADOT standards.

Westfield Fashion Square /1-05-3606-1 Woodman Avenue Riverside Drive Project: File Name: N-S St: E-W St:

CMA12 City Traffic Counters Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Annual Growth:

Date of Count: Projection Year:

2007 07/22/2008

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST. TRAFFIC	(FFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	TH.	2011 V	V/ OTHEF	2011 W/ OTHER PROJECTS	TS	2011	W PROP	2011 W/ PROPOSED PROJECT	DJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume
NB Left	484	2	266	39	523	7	287	-	524	7 0	288	71	595	21.0	327	0	595	8 6	327
Comb. L-T NB Thru	674	0 0	337	54	728	o 0	364	32	760	9 C	380	0	760	D 0	380	0	760	N C	380
Comb. T-R NB Right [2]	209	0 -	209	17	226	0 +	226	17	243	0 -	243	0	243	0	243	0	243	o ← i	- 243
Comb. L-T-R -		0				0				0				0				0	
SB Left	66	- 0	66	8	107	- 0	107	c)	112	- 0	112	0	112	- c	112	0	112	- c	112
SB Thru	842) N C	421	29	606) N C	455	33	942	0 70 0	471	9	952	0 01 0	476	0	952	· N ·	394
Comb. T-R SB Right [2]	200	0 - 0	200	16	216	o c	216	ო	219	⊃ c	219	12	231	o	231	0	231	- o c	100
Comb. L-1-K-	l		107	-	24.0	-	240		747	-	747	-	210	-	218	c	216	, -	216
Comb 1-T	18/	- 0	<u> </u>	<u>o</u>	512	- o	2 -	1	/17	- 0	2	ī	2	- 0	2 '	•	2	- 0	2
EB Thru	753	0 70 1	377	90	813	000	407	64	877	2.0	439	31	908	0.0	454	0	806	8 6	454
Comb. 1-R EB Right	389	o -	389	31	420	o –	420	+	421	-	421	132	553	o ←	553	0	553	-	553
Comb. L-T-R -		0				0				0				0				0	
WB Left	234	- 0	234	19	253	- 0	253	28	281	- 0	281	0	281	- c	281	0	281	0	281
WB Thru	591	7 0	296	47	638	9 6	319	56	694	0 77 0	347	46	740	0 10	370	0	740	0 70 0	370
Comb. T-R WB Right	134	0 -	134	-	145	0 -	145	ო	148	- c	148	0	148	o –	148	0	148	⊃ ←	148
Comb. L-T-R -		0				0				0				0				0	
Crit. Volumes:		Ŋ-Ś:	687			N-S:	742			N-S:	759			:0 : :0 :	803			N-S:	722
		E-W: SUM:	611 1298			SUM:	659 1402			SUM:	1479			SUM:	735 1538			SUM:	735 1456
No. of Phases:]		4				4				4				4				4
Volume / Capacity:	acity:	Ξ	0.874			Ξ	0.949			Ξ	1.005			[2]	1.049			[6]	0.959
Level of Service:	œ.	•	۵				Ш				ш.				ш				m

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual tum lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. tum lane, 70% of volume is assigned to exclusive lane.

Right tums on red from excl. lanes = 50% of overlapping left tum.

Fight tum son red from excl. lanes = 50% of overlapping left tum.

If I y/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] Northbound right tum has an overlapping phase with the westbound left-tum movement and southbound right tum has an overlapping phase with the eastbound left-tum movement [3] Nor ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS.

Woodman Avenue Ventura Freeway Westbound Ramps Westfield Fashion Square /1-05-3606-1 CMA13 City Traffic Counters N-S St: E-W St: Project: File Name: Counts by:

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Westbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Annual Growth:

07/22/2008 2007 2011

Projection Year: Date: Date of Count:

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST. TRAFFIC	VFFIC	2011	2011 W/ AMBIENT	ENT GROWTH	E	2011 V	W OTHER	2011 W/ OTHER PROJECTS	TS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	SJECT	2011	2011 W/ MITIGATION	ATION	
		No. of	Lane	Added	Total	No. of	Lane	Added Total		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume		Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume
NB Left	416	₹~	416	33	449	-	449	2	451	-	451	0	451	-	451	0	451	•	451
Comb. L-T NB Thru	991	0 M	330	79	1070	ဝက	357	48	1118	0 m	373	30	1148	0 10	383	0	1148	oю	383
Comb. T-R	c	00		C	c	00		c	c	0 0	, ,	c	C	00		C	c	00	1 1
Comb. L-T-R -		0	ı	•	•	00		•	o	0		•)	00		•	•	00	
SB Left	0	0	-	0	0	0		0	0	0		0	0	ŀ	,	О	0	0	1
Comb. L-T SB Thru	1062	0 4	- 266	85	1147	0 4	287	57	1204	0 4	301	56	1260	0 4	315	0	1260	0 4	315
Comb. T-R SB Right	493	0 -	- 493	99	532	0 -	- 532	Ŋ	537	0 -	- 537	28	565	0 -	565	0	565	0 -	- 565
Comb. L-T-R -		0				0				0				0				0	
EB Left	0	0	1	0	0	0	,	0	0	0		0	0	0		0	0	0	-
Comb. L-T EB Thru	0	00		0	0	00		0	0	00		0	0	0	, ,	0	0	00	1 1
Comb. T-R	Ċ	0 0	1	Ċ	C	0 0		c	c	0 0		c	c	00		c	c	00	1
EB Kignt Comb. L-T-R -	>	00	•	>	>	00	ı	0	>	00	1	0	0	00		5	0	00	1
WB Left	318	_	175	25	343	-	189	32	375	-	206	0	375	-	206	0	375	-	206
Comb. L-T		0	,			0	ı	,	•	0			į	0		•	•	0 (
WB Thru	က	0 0	270	0	ო	0 0	291	0	ო	0 0	307	0	m	0 0	322	0	m	0 0	322
WB Right	275	·	151	22	297	-	163	2	299	· •	164	33	332	~	183	0	332	· -	183
Comb, L-T-R -		-				-				-				-				-	
Crit. Volumes:		Ņ-Ś:	606			N-S:	982			N-S:	989			N-S:	1017			N-S:	1017
		E-W: SUM:	270 1179			E-W: SUM:	291 1273			E-W: SUM:	307 1295			E-W: SUM:	322 1338			E-W: SUM:	322 1338
			6				c				c				ſ				c
No. of Phases:			า				2				n				ာ				ე
Volume / Capacity:	city:	[1]	0.757			Ξ	0.823			Ξ	0.839			Ξ	0.869			[2]	0.839
Level of Service:	 		ပ				D				D				D				D

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

For one excl. and one opt. turn lane, 70% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] W/C ratio reflects reduction of additional 0.03 due to the mitigation measure consisting of the upgrade to ATCS. Assumptions:

Woodman Avenue Ventura Freeway Eastbound Ramps Westfield Fashion Square /1-05-3606-1 CMA14 N-S St: E-W St: Project: File Name: Counts by:

City Traffic Counters

CRITICAL MOVEMENT ANALYSIS

Woodman Avenue @ Ventura Freeway Eastbound Ramps Peak Hour: Saturday Mid-Day Annual Growth: 2.0%

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

	2007	2007 EXIST. TRAFFIC	AFFIC	2011	W/ AMBIE	2011 W/ AMBIENT GROWTH	HL	2011 V	2011 W/ OTHER PROJECTS	PROJEC	STS	2011	W PROP	2011 W/ PROPOSED PROJECT	DJECT	2011 W/ MITIGATION	MITIGAT	NOI	
		No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added	Total	No. of	Lane	Added To	Total	No. of	Lane
Movement	Volume	Lanes	Volume	Volume Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume	Volume	Lanes	Volume	Volume Volt	Volume L	Lanes	Volume
NB Left	0	0	,	0	0	0	ı	0	0	0	1	0	0	0	,	0	0	0	ı
Comb. L-T		0				0	,			0	1			0				0	1
NB Thru	951	က	262	76	1027	ო	283	48	1075	ო	301	=	1086	ო	304	0	1086	က	304
Comb. T-R		-	262			₹~	283			-	301			τ-	304			-	304
NB Right	96	0		œ	104	0	,	26	130	0	•	0	130	0		0	130	0	1
Comb. L-T-R -		0				0				0				0				0	
SB Left	333	-	333	27	360	1	360	9	366	1	366	31	397	-	397	0	397	-	397
Comb. L-T		0	•			0				0	,			0	,			0	
SB Thru	1027	7	514	82	1109	2	555	82	1191	7	596	26	1217	2	609	0	1217	7	609
Comb. T-R		0				0				0				0	•	,	1	0	,
SB Right	0	0	,	0	0	0	1	0	0	0		0	0	0	1	0	0	0	
Comb. L-T-R -		0				0				0				0				0	
EB Left	432	-	238	35	467	-	257	-	468	+	257	19	487	-	268	0	487	1	268
Comb. L-T		0	,			0	,			0	,			0	,			0	
EB Thru	თ	0	346	_	9	0	374	0	9	0	375	0	9	0	384	0	9	0	384
Comb. T-R		0	•			0				0				0	,			0	1
EB Right	317	-	174	52	342	-	188	7	344	-	189	0	344		189	0	344	Ψ,	189
Comb. L-T-R -		-				-				-				-				-	
WB Left	0	0		0	0	0		0	0	0	,	0	0	0	,	0	0	0	1
Comb. L-T		0	,			0				0	1			0	,			0	
WB Thru	0	0	,	0	0	0	,	0	0	0	,	0	0	0		0	0	0	•
Comb. T-R		0	,			0	•			0				0		1		0	,
WB Right	0	0 (0	0	0 (0	0	0 0	•	0	0	0 0	,	0	0	0 0	
Comb. L-1-R-		0				.				>				>				>	
Crit. Volumes:		.SN⊓	595 346			Ŗ. S. Ņ.	642			N-S: E-W:	975			Ŗ-Ś: E-W:	701			N-S: E-W:	701
		SUM:	941			SUM:	1016			SUM:	1042			SUM:	1084		0,	SUM:	1084
No. of Phases:			က				က				က		***************************************		m				6
Volume / Capacity:	acity:	Ξ	0.590			[2]	0.613			[2]	0.631			[2]	0.661			[2]	0.661
Level of Service:	, ,		⋖			•	8				6				ш			EC.	

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phasa=1375, Unsignalized=1200. For dual turn lanes, 55% of volume is assigned to heavier lane.

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For one excl. and one opt. turn lane,

Right turns on red from excl. lanes = 50% of volume is assigned to exclusive lane.

Right turns on red from excl. lanes = 50% of overlapping left turn.

[1] w/c ratio includes a 0.07 reduction due to installation of ATSAC as part of the Victory System No. 6.

[2] w/c ratios reflect additional 0.03 reduction due to the future citywide ATSAC/ATCS system installation.

Fashion Square Project Driveway-Matilija Avenue N-S St: E-W St:

Riverside Drive
Westfield Fashion Square /1-05-3606-1
CMA17 Project: File Name:

City Traffic Counters

Counts by:

ALTERNATIVE G PROJECT - WEEKEND ANALYSIS

Fashion Square Project Driveway-Matilija Avenue @ Riverside Drive Peak Hour: Saturday Mid-Day Annual Growth: 2.0% Annual Growth:

CRITICAL MOVEMENT ANALYSIS

08/07/2008 2007 2011 Date: Date of Count: Projection Year:

	2007	2007 EXIST. TRAFFIC	4FFIC	2011	2011 W/AMBIEI	ENT GROWTH	YTH.	2011	N/ OTHE	2011 W/ OTHER PROJECTS	STS	2011	W/ PROP	2011 W/ PROPOSED PROJECT	OJECT	2011	2011 W/ MITIGATION	ATION	
Movement	Volume	No. of Lanes	Lane	Added Total Volume Volume	Total Volume	No. of Lanes	Lane	Added	Total Volume	No. of Lanes	Lane	Added	Total Volume	No. of Lanes	Lane	Added Total Volume Volume	Total Volume	No. of Lanes	Lane
NR I aff		_		_ c	_ c	c		c	-	-	1	123	123	+	123	c	123	,	123
Comb. L-T	•	9 0		•	•	0		•	•	0		3	2	- 0	3 ,	•	2	- 0	
NB Thru	0	0		0	0	0	,	0	0	0		0	0	0		0	0	0	•
Comb. T-R		0	,			0				0				0	ı			0	,
NB Right [3]	0	0	•	0	0	0		0	0	0	1	711	711	7	391	0	711	2	391
Comb. L-T-R	,	0				0				0				0				0	
SB Left	က	0	,	0	3	0		0	8	0	-	6-	0	0	,	0	0	0	-
Comb. L-T		0	•			0	,			0	,			0	,			0	
SB Thru	0	0	20	0	0	0	22	0	0	0	22	0	0	0	,	0	0	0	
Comb. T-R	ţ	0 1		,	,	0 0	,	•	,	0 (ć	۰ ۰		•	ć	ο.	,
SB Kight	-	5 •	•	_	20	۰ د	•	0	28	5 •	r	9	77	(7.7	5	7.7	- (77
Comb. L-T-R -		-				-				-				0				0	
EB Left	24	1	24	2	26	-	26	0	56	-	26	-26	0	0		0	0	0	1
Comb. L-T	:	0		ļ	!	0		!		0		•		0				0	•
EB Thru	1092	~ 0	246	87	1179	~ 0	290	29	1246	~ ~	623	ഗ	1251	7	929	0	1251	0 0	626
EB Right	0	0		0	0	0		0	0	0		397	397	o ⊷	397	0	397	-	397
Comb. L-T-R -		0				0				0				0				0	
WBleff	c	С		c	c	c		c	c	c	,	471	471	2	259	c	471	6	259
Comb. L-T	1	0		ı	1	0		ı	1	0	1	:	:	0)		0	1
WB Thru	1013	-	512	81	1094	•	552	61	1155	-	583	ო	1158		584	0	1158	-	584
Comb. T-R	3	- 0	512	•	7	0	552	c	7	- •	583	c	;	· (284	c	;	- -	584
αĻ	2	00		-	=	00	1	0	=	00		>	=	00		>	=	00	
Crit. Volumes:		N-S:	20			N-S:	22			N-S:	22				144		į	N-S:	144
-		E-W:	546			E-W:	290			E-W:	623			Ē-W:	885			E-W.	885
		SUM:	566			SUM:	611			SUM:	645			SUM:	1029			SUM:	1029
No. of Phases:			D				D) D				ო				8
Volume / Capacity:	scity:		0.472				0.509				0.537			[1], [2]	0.622		:	[1]. [2]	0.622
Level of Service:	ě		∢				<				⋖				ω				α

Assumptions:

Maximum Sum of Critical Volumes (Intersection Capacity): 2 Phase=1500, 3 Phase=1425, 4+ Phase=1375, Unsignalized=1200.

For dual turn lanes, 55% of volume is assigned to heavier lane.

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Right turns on red from excl. lanes = 50% of overlapping left turn.

[3] Northbound right turn has an overlapping placed proposed pro

APPENDIX L

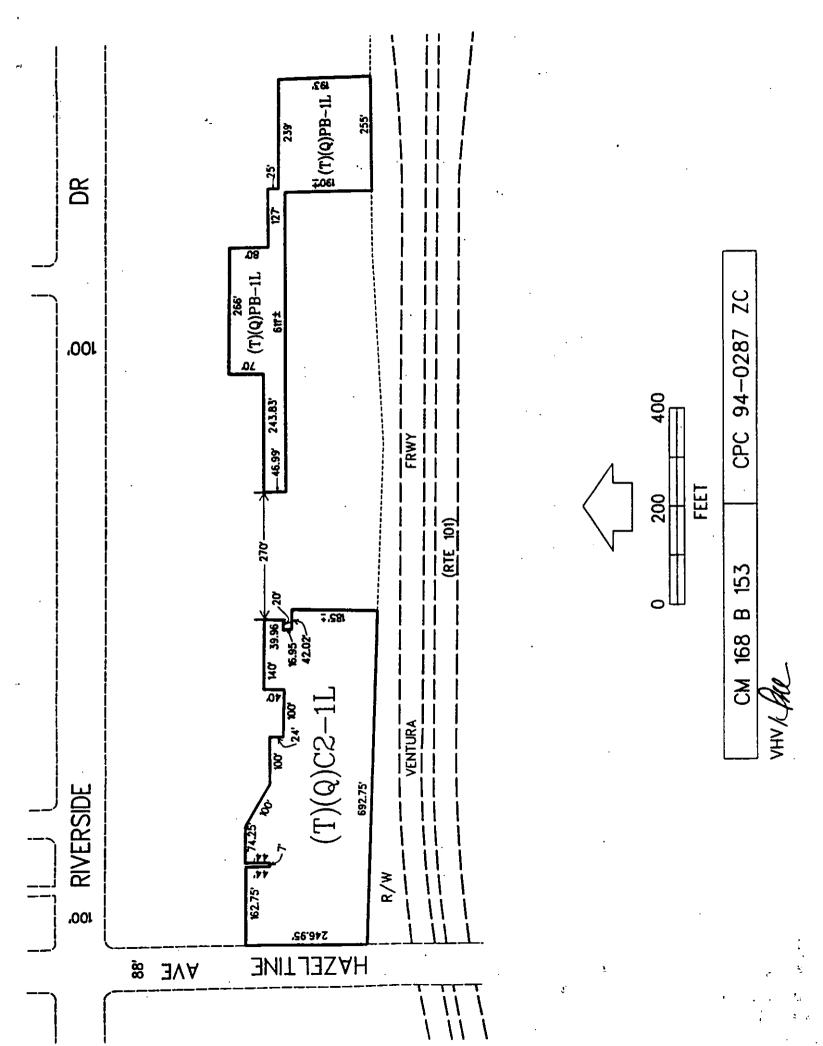
ZONING (Q) CONDITIONS

ORDINANCE NO. 170151

An ordinance amending Section 12.04 of the Los Angeles Municipal Code by amending the zoning map.

THE PEOPLE OF THE CITY OF LOS ANGELES DO ORDAIN AS FOLLOWS:

Section 1. Section 12.04 of the Los Angeles Municipal Code is hereby amended by changing the zones and zone boundaries shown upon a portion of the zone map attached thereto and made a part of Article 2, Chapter 1, of the Los Angeles Municipal Code, so that such portion of the zoning map shall be as follows:



Sec. 2 Pursuant to Section 12.32-J of the Los Angeles Municipal Code, the following limitations are hereby imposed upon the use of that property shown in Section 1 hereof which is subject to the "Q" Qualified classification.

(Q) QUALIFIED CONDITIONS OF APPROVAL

Sec. 2. Pursuant to Section 12.32-J of the Los Angeles Municipal Code the following limitations are hereby imposed upon the use of that property shown in Section 1 hereof which is subject to the "Q" Qualified classification.

1. Administration.

- a. Covenant. Prior to the issuance of any permits relative to this matter, an agreement concerning all the information contained in these conditions shall be recorded in the County Recorder's Office. The agreement shall run with the land and shall be binding on any subsequent property owners, heirs or assigns. The agreement must be submitted to the Planning Department for approval before being recorded. After recordation, a copy bearing the Recorder's number and date shall be provided to the Planning Department.
- b. Approval verification and submittals. Copies of any approvals, guarantees or verification of consultations, review or approval, plans, etc., as may be required by the subject conditions, shall be provided to the Planning Department for placement in the subject file.
- c. Enforcement. Compliance with these conditions and the intent of these conditions shall be to the satisfaction of the Planning Department and any other designated agency, or the agency's successor, and in accordance with any stated laws or regulations, or any amendments thereto.
- 2. Environmental. The following conditions are required by proposed Mitigated Negative Declaration, MND 94-0240 ZC, for this project.

a. Seismic Safety

- 1. Recommendations set forth in the geotechnical engineering report shall be incorporated into the structural engineering design of the project.
- 2. The project shall conform to the City's adopted Seismic Safety Plan.
- 3. Structures shall be designed in accordance with Division 23 of the Building Code and the recommended lateral force requirements and commentary of the Structural Engineers Association of California.
- b. Air quality. The project shall be provided with an air filtration system to improve the air quality for the project's tenants/residents. However, this requirement shall not preclude the installation of operable windows which permit passive heating and cooling.

- c. Air quality (dust). A dust control plan, including but not limited to sprinkler systems, to reduce dust impacts of the subject use on the surrounding neighborhood, shall be submitted and implemented.
- d. Air Quality.
 - 1. Materials such as oil-based architectural coatings, paints and asphalt used in construction shall be controlled according to SCAQMD regulations.
 - 2. Streets adjacent to the site must be swept as needed to remove silt which may have accumulated from construction activities.
 - 3. Equipment. Engines must be maintained in good condition according to manufacturer's specification. Construction activities shall be scheduled to even out emission peaks.
 - 4. The site shall be fenced to reduce wind-blown dust. Construction materials not stored behind temporary fences shall be covered. All debris shall be cleaned up daily and put into a dumpster, which shall have a lid and the lid shall be secured at the end of the day.
- e. Drainage. Construction of drainage facilities to the satisfaction of the City Engineer.
- f. Noise (Parking Structure Ramps) Concrete, not metal, shall be used for construction of parking ramps.
- g. Noise (interior office). Evidence shall be submitted, along with the application for the building permit, by a qualified acoustical engineer specifying the CNEL contour within which the office building will be located and, based on the CNEL contours, the construction necessary to achieve an interior noise (Ldn) of 60, in the interior office areas of the building.
- h. Lighting. Preparation of an outdoor lighting plan according to Department of Building and Safety standards to insure that the light does not adversely affect neighboring residences.
- i. Traffic. Traffic impact mitigation (DOT). The traffic impact mitigation measures set forth in the Transportation Department communication of August 8, 1994, which is attached as Exhibit No. E-5 of the subject report shall be implemented to the satisfaction of the Transportation Department.

- j. A parking plan shall be prepared for approval by the Planning Department which will provide for new or additional off-street parking.
- k. Parking/driveway plan and loading dock/trash collection plan. A parking area and driveway plan, and loading dock/trash collection plan shall be prepared for approval by the appropriate District Offices of the Bureau of Engineering and the Department of Transportation.
- 1. Access. Truck access to the loading dock area (for deliveries) and trash collection enclosures for the new department store (commercial use) shall not be taken from Riverside Drive.
- m. Fire Department plan approval. Recommendations of the Fire Department relative to fire safety, which may include but are not limited to access and interior heat sensitive sprinkler systems, shall be incorporated into the building plans for the subject project, to the satisfaction of the Fire Department.
- n. Police Department plan approval. Recommendations of the Police Department relative to mitigating of impacts on police services shall be approved and implemented to the satisfaction of Police Department. Police recommendations may include but are not limited to secured parking, security fencing, security lighting, bonded security guards, information signs, elevator illumination and design, building design and landscaping to reduce potential hiding places.
- o. Energy conservation. The Department of Water and Power and the Southern California Gas Company shall be consulted regarding feasible energy conservation features which shall be incorporated into the design of the project. If the demand on the utilities exceeds the available resources, said agencies may postpone new connections for the subject project until the power and/or gas supply is deemed adequate.
- p. Water conservation. The Department of Water and Power shall be consulted regarding feasible water conservation features which can be incorporated into the design of the project. If the demand on the water system exceeds the available resources, said agency may postpone new connections for the subject project until the water capacity is deemed adequate.
- q. Public facilities/utilities (project postponement authority). Impacts of the subject project on sewers, storm drain capacity, solid waste disposal and other utilities and facilities may contribute to a demand on the system(s) which exceeds the available resources. In such event, the agency responsible for the impacted facility/utility may cause project to be postponed until the system(s) capacity is deemed adequate.

- r. Landscaping. All open areas not used for buildings, driveways, parking areas, recreational facilities or walks shall be attractively landscaped, including an automatic irrigation system, and maintained in accordance with a landscape plan prepared by a licensed landscape architect or licensed architect.
- s. Archaeological. Prior to issuance of grading or building permits for the subject project, a qualified archaeologist shall be retained to monitor any subsurface operations, including but not limited to grading, excavation, trenching or removal of existing features of the subject property. The archaeologist shall be authorized to and must halt the project and order reasonable protective measures if any significant archaeological resources are uncovered. Such authorization shall be in writing, shall be recorded by covenant and agreement with the county recorder and shall be provided to the Planning Department for placement in the subject file prior to issuance of grading or building permits for subject project.
 - i. A "qualified archaeologist" means securing the services of the Center for Public Archaeology of California State University at Northridge, the Archaeological Survey of the University of California at Los Angeles or a member of the Society of Professional Archaeologists.
 - ii. In addition to a qualified archaeologist, a representative of the local Native Americans shall be retained to assist the archaeologist in monitoring all subsurface operations. Evidence shall be provided for placement in the subject file, prior to issuance of grading or building permits, that the representative has been retained.
 - iii. Copies of any archaeological survey, study, report of findings shall be provided to the Environmental Review Section of the Department of City Planning (Room 655, City Hall, Los Angeles, California 90012) for incorporation into the City archaeological site identification program.
 - iv. The archaeologist may not halt a project for more than 90 days without the approval of the Planning Department. Such approval must be requested by letter from the archaeologist. Said letter must be submitted not more than 30 days after the project is halted. The letter must be accompanied by documentation to support the request and must contain proposed measures to protect the site and to allow all or part of the project to proceed.
- 3. The total floor area (as defined in Section 12.21.1A of the Los Angeles Municipal Code) of all the main buildings on the applicant's C-zoned mall area ownership as described in

Exhibit "E-6" for the City Plan Case No. 94-0287 ZC on file in the City Planning Department shall not exceed 1.5 times the buildable area of such ownership. No more than 148,933 square feet of gross leasable area shall be added to such ownership. The floor area of all structures on the total ownership, exclusive parking, shall not exceed a total gross leasable area of 975,000 square feet. No additional development shall be permitted on the total ownership.

- 4. At least 60,000 square feet of the gross square footage of the Mall area shall be utilized as non-leasable public area.
- 5. Except for the four-level parking structure, the pedestrian bridges to the parking structure, the three-story space formerly occupied by I. Magnin and the enclosure of the mall, a two-level height limitation shall be maintained in the C2, PB, and P Zones within the applicant's mall area ownership. In no event shall any structure exceed 62 feet in height within the mall area.
- 6. With the exception of the expanded department store to replace I. Magnin, no single tenancy of the applicant's mall area ownership shall exceed 20,000 square feet.
- 7. Parking: The applicant shall provide a parking ratio of 4.5 parking spaces per 1,000 square feet of gross leasable area for the subject property, and that the total number of parking spaces shall not be less than 4,388 parking spaces for the total ownership.
- 8. The applicant shall provide direct vehicle ramp access to the third level of the parking structure.
- 9. Parking shall be at no cost to customers and employees provided, however, that a validation system may be utilized. A parking management plan for employee parking, including an identification system for employees' vehicles by decal or similar means should be submitted to the Planning Department prior to the issuance of the Certificate of Occupancy.
- 10. Employee Parking.
 - i. Shall be provided on-site.
 - ii. Employees shall not park on adjacent residential streets during the hours they are working at the subject facility.
- 11. Parking signage interior to the subject property shall be increased. Parking signage fronting the exterior of the property along Woodman and Hazeltine Avenues and Riverside Drive shall also be increased. A parking signage plan shall be prepared to the satisfaction of the Department of Transportation and the Planning Department.

- No more than three full, sit-down table service restaurants shall be located within the mall area. Only these three full-service restaurants shall be permitted to apply for an alcoholic beverage license. separate bar shall be permitted in the restaurants. One full-service restaurant is currently existing. The size of each additional permitted 5,000 full-service restaurant shall not exceed square Entertainment within the restaurants shall be of a low-key character, including strolling, violin, mariachis, piano, guitar, harp, combo (the foregoing being illustrative and not exhaustive). This condition shall not be interpreted as prohibiting other entertainment in any central interior court as part of a special event, festive or holiday show.
- 13. In addition to the restaurants permitted by condition No. 11 above, a food court area shall be permitted within the mall area. The food court shall not exceed 5,000 square feet for the food service area and an additional maximum of 5,000 square feet for the dining area for such food service area (not including public amenity areas such as water fountains, pools, or walkways). No liquor, beer or wine shall be served within such food area.
- 14. No separate cocktail bars shall be permitted.
- 15. All mall shops shall close no later than 10:00 p.m., except for special sales events. The closing hour of the food court shall be identical to the closing hour of the shops.
- 16. Security officers shall be kept on the premises until all restaurants and/or the food service area are closed.
- 17. No movie theater shall be permitted.
- 18. No amusement arcade, dance club or disco shall be permitted.
- 19. All lights emanating from second- and third-story windows and the parking structure shall be shielded from the view of residential properties. Noise from air conditioning units shall be buffered from residential properties to the north.
- 20. A community room of approximately 900 square feet, including a kitchenette, shall be provided for use by community groups. Provision for partition of the room into two separate meeting areas shall be made. Notice of the availability of this room shall he posted at the mall management offices, and annually sent to all recognized community groups in the Sherman Oaks area.

21. Neighborhood Protection Plan.

Prior to the issuance of any building permit the property owner shall guarantee the necessary funding of a Neighborhood Protection Plan through cash or irrevocable letter of credit, payable to the Department of Transportation (DOT). The property owner shall covenant and agree to submit a preliminary Neighborhood Protection Plan to DOT prior to the issuance of any building permit.

The Plan shall be prepared to investigate and implement options to address concerns in reference to the intrusion of "cut-through" traffic into the local residential neighborhoods and the protection of pedestrians in the project vicinity.

No later than the issuance of the first Certificate of Occupancy an Implementation Plan for neighborhood protection shall be determined by DOT in consultation with the Council office, the applicant and representatives of the neighborhood in the project vicinity bounded by Magnolia Boulevard on the north, Moorpark Street on the south, Woodman Avenue on the east, and Hazeltine Avenue on the west. Measures may be implemented prior to the issuance of any certificate of occupancy. Other implementation measures shall be based on monitoring of traffic conditions after occupancy of the project. The Plan may include, but shall not be limited to, traffic control such as turn prohibitions, installation of traffic diverters and other measures designed to discourage "cut-through" traffic in residential neighborhoods and to provide pedestrian related improvements.

The maximum amount required of the property owner to fund possible transportation improvements described in the proposed Neighborhood Protection Plan and any additional improvements deemed necessary by LADOT is \$175,000. Up to \$75,000 of such funds may be used by the City for the street improvements of Valley Heart Drive. In the event the Plan is guaranteed by cash, LADOT shall deposit the monies in an interest-bearing account and shall provide the property owner with an annual statement reflecting interest earned and payments made. expiration of 5 years after the issuance of the final Certificate of Occupancy, the property owner may request a refund from LADOT for any However, the refund shall only be unused portion of these funds. granted if LADOT and the Fifth District Council Office determine that all of the provisions of the Plan have been fully complied with and no additional future improvements are necessary.

- 22. The applicant shall assist the Department of Transportation to increase utilization and/or expand service of the Sherman Oaks "DASH" or other para-transit services, evidence of efforts shall be provided to the Department of City Planning within one year of project approval.
- 23. Landscape plans by a licensed landscape architect depicting existing and proposed landscaping along all frontages of the site should be submitted to the District Council Office for review and shall be submitted to the Planning Department for approval.
- 24. Prior to the issuance of any building permit the applicant shall provide evidence to the Planning Department that all mitigation measures identified in MND 94-0240-ZC have been appropriately incorporated into project designs, or included construction contracts.
- 25. Deliveries. All deliveries and loading/unloading and trash collection shall be limited to the hours 8 a.m. to 8 p.m., Monday through Friday, and 10 a.m. to 4 p.m., Saturday and Sunday.

- 26. Signage. No illuminated flashing, or animated signs shall be visible from adjacent residential neighborhoods. No off-site commercial signs shall be permitted at the subject shopping mall.
- 27. The haul route(s) utilized for the exporting of all materials shall be approved by the Department of Transportation.
- 28. Roof Structures. All structures, such as air conditioning and heating units, elevator and stairwell shafts shall be fully screened from residential areas. The equipment and screening shall include noise attenuation features and shall be located away from residential neighborhoods, as feasible.

29. Fire Department.

- a. Applicant shall submit plot plans for Fire Department review and approval prior to issuance of any building permit.
- b. Adequate off-site public and on-site private fire hydrants shall be required. Their number and location to be determined after the Fire Department's review of the plot plan.
- c. During demolition the Fire Department access will remain clear and unobstructed.
- d. Fire lane width shall not be less than 20 feet. When a fire lane must accommodate the operation of Fire Department aerial ladder apparatus or where fire hydrants are installed, those portions shall not be less than 28 feet in width.
- e. Private roadways for general access use shall have a minimum width of 20 feet.
- f. No building or portion of a building shall be constructed more than 150 feet from the edge of a roadway of an improved street, access road or designated fire lane.

30. Maintenance, trash and storage.

- a. The subject property, including the Riverside Drive frontage, and any associated parking facilities, shall be maintained in an attractive condition and shall be kept free of trash and debris.
- b. Open areas devoted to trash storage or other storage shall not be located adjacent to a residential use or shall be buffered so as not to result in noise, odor or debris impacts on any adjacent residential uses.
- c. All outside trash containers on the subject property shall be enclosed and shall be located so as not to result in noise or smell impacts on any adjacent residential use.

- d. Trash shall not be taken outside for deposit in trash bins or storage between the hours of 8:00 p.m. and 8:00 a.m. Further, the bins shall be locked during those hours.
- e. Trash pick up shall take place only between 8:00 a.m. and 8:00 p.m. Monday through Friday, and 10:00 a.m. to 4:00 p.m. Saturday and Sunday.
- f. Screening walls shall conceal trash bins; wrought iron gates shall be used to gate enclosures.
- 31. Plans. The property shall be developed substantially in accordance with the plot plan and plans identified as Exhibit E-6, attached to City Plan Case No. 94-0287 on file in the City Planning Department, except as the City Council may subsequently approve a modification of the plan(s), or as development of the subject property may require modification to meet other requirements or provisions of the Municipal Code.
- 32. Plans. Prior to the issuance of building permits, a detailed set of complete plot plans showing existing as well as proposed structures, including elevations, landscaping plans, plans for the loading and trash areas, and driveway plans shall be submitted for approval by the City Planning Department.
- 33. Graffiti removal and deterrence. The property owners and all successors shall acknowledge the applicability of the graffiti removal and deterrence requirements pursuant to Municipal Code Sections 91.8101-F, 91.8904.1 and 91.1707-E relative to the subject project, particularly with regard to the following:
 - The first nine feet of exterior walls and doors, measured from grade, and all of any walls enclosing the property shall be built and maintained with a graffiti resistant finish consisting of either a hard, smooth, impermeable surface such as ceramic tile, baked enamel or a renewable coating of an approved, anti-graffiti material or a combination of both pursuant to Section 91.1707-E; and
 - the period for compliance with a graffiti removal order issued by the Building and Safety Department is 15 days following which period with failure to perform, the City or its contractor is empowered to enter the property to remove such graffiti with costs accruing to the property owner (91.8904.1); and
 - the period for compliance with a subsequent order for a subsequent occurrence is three days (91.8904.1.).
 - In addition to a, b and c above, exterior walls of new commercial and residential buildings of other than glass may be covered with clinging vines, screened by oleander trees or similar vegetation capable of covering or screening entire walls up to the height of at least 9 feet, excluding windows and signs.
- 34. Parking-Building-Phasing. Code-required parking shall be provided for existing mall retail commercial uses during the construction of the mall expansion (new department store).

- 35. No commercial activities shall be permitted on the pedestrian bridge(s).
- 36. Noise.
 - . Structural construction work shall be limited to the hours of 7 a.m. to 6 p.m., Monday Friday and 8 a.m to 6 p.m. Saturday.
 - . Construction contracts shall require project contractors to use power construction equipment with noise shielding and muffing devices.
 - During construction, the project shall comply with applicable Section 112.03 of the City Noise Ordinance No. 144,331 and its subsequent ordinances.
 - . Noise barriers such as temporary wooden barrier walls, mufflers and noise entrenching devices shall be employed to reduce the intrusive construction noise to the fullest extent possible.
 - The placement, screening and maintenance of all external mechanical equipment shall be designed to ensure that noise levels do not exceed standards outlined in Article 2 of the City's Noise Ordinance 144,331, 161,574 and their subsequent ordinances at adjacent uses.
- The applicant shall improve the Riverside Riverside Drive Frontage. mall in conformance with frontage οf the landscaping/renovation plan on the file, Exhibit E-6. The plan shall include, but not be limited to: the planting of 37 new trees and 125 shrubs along the south side of Riverside Drive; screen walls shall be raised so that trash bins are concealed; chain link fences and gates shall be replaced with wrought iron fencing and gates to screen loading docks and trash areas; transformer and other equipment will be screened by wrought iron fencing and landscaped; and damaged stucco will be repaired and the frontage will be repainted.
- Prior to the issuance of any building permits, the applicant shall submit detailed plans in substantial conformance with Exhibit E-6 attached to the file, for the loading dock and trash collection areas for the new department store, with truck access to the loading dock area and trash collection enclosures for the new department store not be from Riverside Drive, and plans for the inclusion implementation of the applicant's landscaping/renovation on Riverside Drive, including a landscape plan with a timetable for achieving screening of the Riverside Drive frontage with mature plants in conformance with Exhibit E-6, attached to the file, to the Planning Department for review and approval consistent with Section 2.5 a-d of the Planning Commission's adopted Site Plan Review Guidelines, addressing on-site circulation and access including driveway location, loading docks, trash collection areas, and sanitation and delivery vehicles.

The applicant shall also record a mitigation monitoring agreement on the appropriate form and in a manner satisfactory to the Planning Department

providing for an annual review for a period of three years to verify compliance with the required landscaping, screening and maintenance of landscape on the Riverside Drive frontage.

39. The Department of Transportation may identify and require future traffic-related improvements in the area generally bounded by Magnolia Boulevard, Woodman Avenue, Moorpark Street and Hazeltine Avenue. A bond, or other instrument, shall be posted in the amount of \$50,000 prior to the issuance of a certificate of occupancy. (A B permit or other permit may be required by the Bureau of Engineering and/or the Department of Transportation). The Department of Transportation may impose such conditions during and up to a period of not to exceed three years.

Sec. The City Clerk ordinance and cause the same to be published published in the City of Los Angeles.	shall certify to the passag I in some daily newspaper pr	e of this
I hereby certify that the foregoing ordinal City of Los Angeles, at its meeting of NOV	nce was passed by the Cour 291994	icil of the
Ву	ENAS MARTINEZ, City	y Clerk,
Approved DEC 06 1994	M.	ayor.
Approved as to Form and Legality	1416	iyor.
JAMES K. HAHN, City Attorney,	CNS 1229627	12/9
Deputy.		
File No. 94-2018		
City Clerk Form 23		(ie.

Pursuant to Sec. 97.8 of the City Charter, approval of this ordinance recommended for the City Planning Commission.

NOV 2 3 1994

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Director of Planning

APPENDIX M

MITIGATION MONITORING PROGRAM

AESTHETICS AND VISUAL RESOURCES

MM AES-1: As required by LAMC Section 12.40, the site will be required to prepare a

Landscape Plan which will address replacement of removed trees.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-2: The owners shall maintain the subject property clean and free of debris and

rubbish and to promptly remove any graffiti from the walls, pursuant to

LAMC Sections 91.8101-F, 91.8904-1, and 91.1707-E.

Monitoring Phase: Occupancy

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-3: A minimum of one 24-inch box tree (minimum diameter of two inches and a

height of eight feet at the time of planting) shall be planted for every four new

surface parking spaces.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-4: The Final Expansion Project Landscape Plan, which will be reviewed and

approved by the City of Los Angeles, shall incorporate clinging vines and bamboo screening, which provide a variety of textures and colors, along exterior walls visible along the Riverside Drive and Hazeltine Avenue

frontages.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: Department of City Planning

MM AES-5: The Final Expansion Project Landscape Plan shall include the installation of

healthy mature trees for all replacement trees and new landscaping along

Riverside Drive.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: Department of City Planning

MM AES-6:

New project landscaping along Riverside Drive would provide an opportunity to visually activate this frontage and minimize building massing. A combination of landscape, hardscape, and building finish elements would create a vibrant urban atmosphere that offers more pedestrian-friendly linear banding and gives a fresh, updated look to the shopping center. The landscape plan would incorporate specimen accent plantings, including distinctive palms, large canopy trees, evergreens, seasonal color trees and bold median plantings. The landscape concept also incorporates various hardscape features, including the integration of street furnishings along the Riverside Drive frontage. Street furnishings, including treated wood benches and castin-place concrete seating with integral lighting and water features, would add to the visual interest and appeal of this frontage.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-7:

Directional and security lighting will be required for safety purposes. Through a new plan, lighting can enhance safety along the Riverside Drive and Hazeltine Avenue frontages and add to the perceived security of the neighborhood in general. Lighting would be incorporated into the streetscape environment at several levels, including the use of bollards, wall reveals, seating areas, and crosswalks. The use of plaza strip lighting will afford additional security lighting but with a park-like feel and without significant light intrusion to the surrounding neighborhood. As consistent with safety concerns, the Proposed Project will incorporate low-level lighting that is directed downward and shielded to prevent spillover of light toward sensitive uses.

Monitoring Phase: Occupancy

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-8:

The Riverside Drive building surfaces would be refreshed with a new graphic design treatment that would consist of small visual mosaics of color and pattern that effectively serve to visually minimize the massing of the long linear wall along the frontage. It is intended that a combination of landscaping, hardscaping and building finish elements would create a vibrant urban atmosphere that offers more pedestrian-friendly linear banding and gives a fresh, updated look to the shopping center.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-9:

All open areas not used for buildings, driveways, parking areas, recreational facilities or walks shall be attractively landscaped and maintained in accordance with a landscape plan, including an automatic irrigation plan, prepared by a licensed landscape architect to the satisfaction of the Planning Department.

Monitoring Phase: Occupancy

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-10:

The trees shall be dispersed within the parking area so as to shade the surface parking area and shall be protected by a minimum 6-inch high curb and landscaping.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

MM AES-11:

Outdoor lighting shall be designed and installed with shielding, so that the light sources for the Proposed Project are shielded from spillover to adjacent residential properties.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of City Planning

AIR QUALITY

MM AQ-1: The Proposed Project will comply with applicable CARB regulations and standards. CARB is responsible for setting emission standards for vehicles sold in California and for other emission sources, such as consumer products and certain off-road equipment. CARB oversees the functions of local air pollution control districts and air quality management districts, which in turn administer air quality activities at the regional and county levels.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-2: The Proposed Project will comply with applicable SCAQMD regulations and standards. The SCAQMD is responsible for monitoring air quality, as well as planning, implementing, and enforcing programs designed to attain and maintain State and federal ambient air quality standards in the district. Programs that were developed include air quality rules and regulations that regulate stationary sources, area sources, point sources, and certain mobile source emissions. SCAQMD is also responsible for establishing stationary source permitting requirements and for ensuring that new, modified, or relocated stationary sources do not create net emission increases.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-3: The Proposed Project will be designed to reduce exposure of sensitive receptors to excessive levels of air quality. The Proposed Project is designed and will be built and operated in a manner consistent with the requirements to achieve Leadership in Energy and Environmental Design (LEED) certification from the United States Green Building Council. LEED is a green building rating system that was designed to guide and distinguish high-performance commercial projects. LEED promotes a whole-building approach to sustainability by recognizing performance in five key areas of human and environmental health: sustainable site development, water savings, energy efficiency, materials selection, and indoor environmental quality. The Proposed Project will implement a variety of design and operational features to achieve LEED certification. As a result, the Proposed Project would be proactive in reducing GHG emissions. Examples of design

¹ U.S. Green Building Council (USGBC). 2008. *LEED for New Construction v2.2 Registered Project Checklist*. 19 May 2008 http://www.usgbc.org/DisplayPage.aspx?CMSPageID = 220#v2.2>.

features to be implemented for the Proposed Project in order to achieve LEED certification include, but are not limited to, the following or their equivalent:

- A construction activity pollution prevention program.
- Encouraging the use of mass transit.
- Providing transportation amenities, such as alternative fueling stations, carpool/vanpool programs, bicycle racks, and showering/changing facilities.
- Implementing a stormwater management plan that reduces impervious cover, promotes infiltration, and captures and treats the stormwater runoff from 90 percent of the average annual rainfall using acceptable best management practices.
- Adopting site lighting criteria to maintain safe light levels while avoiding offsite lighting and night sky pollution, minimizing site lighting where possible, and reducing light pollution.
- Providing tenants with a description of the sustainable design and construction features incorporated in the core and shell project.
- Using high-efficiency irrigation technology or reducing potable water consumption for irrigation by 50 percent by using a combination of plant species factor, irrigation efficiency, use of captured rainwater, use of recycled wastewater, and use of water treated and conveyed by public agency specifically for non-potable uses.
- Employing strategies that, in aggregate, use 20 percent less water than the water use baseline calculated for the building (not including irrigation) after meeting the Energy Policy Act of 1992 fixture performance requirements.
- Designing the building envelope and building system to maximize energy performance.
- Selecting refrigerants that reduce ozone depletion while minimizing direct contributions to global warming.
- Implementing a construction waste management plan that identifies the
 materials to be diverted from disposal and whether the materials will be sorted
 on-site or commingled. The waste management plan would include recycling
 and/or salvaging at least 50 percent of non-hazardous construction and
 demolition debris.
- Using materials with recycled content such that the sum of post-consumer recycled content plus one-half of the pre-consumer content constitutes at least ten percent of the total value of the materials in the project.
- Using a minimum of ten percent of the total materials value on building materials or products extracted, harvested, or recovered and manufactured within 500 miles of the project site.

- Adopting an indoor air quality management plan to protect the HVAC system during construction, control pollutant sources, and interrupt contamination pathways.
- Specifying low-volatile organic compounds paints and coatings in construction documents.
- Designing the building with the capability for occupant controls for airflow, temperature and ventilation. Strategies will include underfloor HVAC systems with individual diffusers, displacement ventilation systems with control devices, and ventilation walls and mullions.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-4: The Proposed Project would install carbon monoxide and airflow measurement equipment that would transfer the information to the HVAC system and/or Building Automation System to trigger corrective action, if applicable, and/or use the measurement equipment to trigger alarms that inform building operators or occupants of a possible deficiency in outdoor air delivery. Installation of such a system in areas where carbon monoxide concentrations may escalate (such as in the vicinity of loading docks or valet parking drop-offs) would improve both indoor and localized "hotspot" air quality.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-5: The Proposed Project would provide bicycle racks at a ratio of 2% of the total number of parking spaces on-site, as well as lockers, changing rooms and showers inside the shopping center. A minimum of 20 additional bicycle spaces (in racks) would be provided at multiple locations through out the site. Four showers (two per each gender) would be provided in a dedicated shower facility area. Lockers would be provided in conjunction with the shower facilities.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-6: The Proposed Project would provide a shuttle service connecting the site to a nearby Orange Line station (e.g., Van Nuys Boulevard). This service could be provided by either the provision of a private shuttle or the funding of extended hours for the existing Los Angeles Department of Transportation (LADOT) DASH line. The Orange Line shuttle would complement existing transit services (i.e., the LADOT DASH service) such that the shuttle would operate during hours when other public transit services connecting the site to the Orange Line are not available (e.g., during weekdays evenings and general weekend hours). The

shuttle would operate during regular shopping center hours corresponding with periods of peak parking demand at the site and peak holiday season demand (i.e., everyday during the holiday shopping period between November 15 and January 1, and every Saturday/Sunday throughout the year).

Monitoring Phase: Occupancy Monitoring Agency: LADOT

Enforcement Agency: South Coast Air Quality Management District

MM AQ-7: During construction activity, water or a stabilizing agent shall be applied to exposed surfaces in sufficient quantity to prevent generation of dust plumes.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-8: During construction activity, track-out shall not extend 25 feet or more from any active construction operations, and track-out shall be removed at the conclusion of each workday.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-9: During construction activity, a wheel washing system shall be installed and used to remove bulk material from tires and vehicle undercarriages before vehicles exit the project site.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-10: All haul trucks hauling soil, sand, and other loose materials shall maintain at least six inches of freeboard in accordance with California Vehicle Code Section 23114, and such trucks shall be covered (e.g., with tarps or other enclosures that would reduce fugitive dust emissions).

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety, Department of

Public Works-Bureau of Street Services

Enforcement Agency: Department of Building and Safety, Department of

Public Works-Bureau of Street Services.

MM AQ-11: During construction activity, traffic speeds on unpaved roads shall be limited to 15 miles per hour.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District.

MM AQ-12: During construction activity, operations on unpaved surfaces shall be suspended when winds exceed 25 miles per hour.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-13: Heavy equipment operations shall be suspended during first and second stage smog alerts.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-14: On-site stock piles of debris, dirt, or rusty materials shall be covered or watered at least twice per day.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-15 Heavy-duty equipment shall be equipped with a diesel oxidation catalyst capable of reducing NO_X emissions by 40 percent.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-16 Contractors shall maintain equipment and vehicle engines in good condition and in proper tune per manufacturers' specifications.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-17 Contractors shall utilize electricity from power poles rather than temporary diesel or gasoline generators, as feasible.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-18 Heavy-duty construction shall be prohibited from idling in excess of five minutes, both on- and off-site, to be consistent with State law.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-19 Construction parking shall be configured to minimize traffic interference.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

MM AQ-20 Construction activity that affects traffic flow on the arterial system shall be limited to off-peak hours, as feasible.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: South Coast Air Quality Management District

GEOLOGY AND SOILS

MM GEO-1: Design and construction of the project shall conform to the Uniform Building

Code seismic standards as approved by the Department of Building and

Safety.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-2: All grading and earthwork shall be performed in accordance with the Grading

Ordinances of the City of Los Angeles and the applicable portions of the

General Earthwork Specifications in an approved Geotechnical Report.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-3: All earthwork and construction shall be completed in accordance with

mitigation as defined in Public Resources Code Section 2693(c) to ensure that

issues of potential liquefaction are addressed.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM GEO-4: To address potential soil settlement, all new building construction shall be

supported on deep foundations. Design values for drilled piles shall be

consistent with the recommendations of the approved Geotechnical Report.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM GEO-5: To address potential stability concerns due to buried structures, such as

footings, septic systems, backfilled excavations, and utility lines. Any buried structures should be properly removed and the resulting excavations backfilled with engineered fill. Any other buried structures encountered during construction should be removed and backfilled in accordance with the recommendations of the Soils Engineer. The site should be inspected for possible buried fill material, using heavy excavating equipment. If loose fill material is encountered, excavations should extend to native ground. The

exposed native subgrade should be scarified to a minimum of 6 inches, moisture-conditioned as necessary, and recompacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557. Limits of recompaction should extend 5 feet beyond structural elements. Prior to fill placement, a qualified geotechnical engineer shall inspect the bottom of the excavation to verify no additional excavation will be required.

Any buried structures or loosely backfilled excavations encountered during construction should be properly removed and the resulting excavations backfilled with engineered fill. Excavations, depressions, or soft and pliant areas extending below planned finished subgrade levels should be cleaned to firm, undisturbed soil and backfilled with engineered fill. In general, any septic tanks, debris pits, cesspools, or similar structures should be entirely removed. Concrete footings should be removed to an equivalent depth of at least 3 feet below proposed footing elevations or as recommended by the Soils Engineer. Any other buried structures should be removed in accordance with the recommendations of the Soils Engineer. The resulting excavations should be backfilled with engineered fill.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM GEO-6:

Any fill material encountered within proposed pavement areas shall be removed and/or recompacted. The fill material shall be moisture-conditioned to near optimum moisture and compacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557. At a minimum it is recommended that the upper 12 inches of subgrade soil be moisture-conditioned to at or above optimum moisture and recompacted to a minimum of 90 percent of maximum density based on ASTM Test Method D1557.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM GEO-7:

To minimize the potential soil movement, the upper 24 inches of soil within the building slab and exterior flatwork areas shall be replaced with "non-expansive" soils (with El<20).

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-8:

To minimize seismic-induced settlements, foundations shallower than 30 feet shall be designed to tolerate seismic settlements of one-half inch total and one-quarter inch differential over a distance of 50 feet.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-9:

To address cohesionless sandy soil conditions, shoring or sloping back trench sidewalls shall be required within these loose cohesionless soils.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-10:

If groundwater is encountered during the course of earthwork at the project site and subgrade soils appear to become saturated, "pump," or not respond to densification techniques, remedial measures as prescribed by a qualified geotechnical engineer shall be employed. Groundwater remedial measures include: discing and aerating the soil during dry weather; mixing the soil with dryer materials; removing and replacing the soil with an approved fill material; or mixing the soil with an approved lime or cement product.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-11:

General site clearing shall include removal of vegetation and existing utilities; structures; including foundations basement walls and floors; existing stockpiled soil; trees and associated root systems; rubble; rubbish; and any loose and/or saturated materials. Site stripping shall extend to a minimum depth of 2 to 4 inches, or until all organics in excess of 3 percent by volume are removed. Deeper stripping may be required in localized areas. These materials will not be suitable for reuse as engineered fill, however, stripped topsoil may be stockpiled and reused in landscape or non-structural areas.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-12:

The upper 24 inches of soil within proposed building and exterior flatwork areas shall consist of non-expansive engineered fill. The intent is to support the proposed slab-on-grade and exterior flatwork areas with 24 inches of non-expansive fill. The non-expansive fill material should be a well-graded silty sand or sandy silt soil. A clean sand or very sandy soil is not acceptable for this purpose. A sandy soil will allow the surface water to drain into the

expansive clayey soils below, which may result in soil swelling. Imported fill should be approved by the Soils Engineer prior to placement. The fill shall be placed as specified as engineered fill.

The organic-free, on-site, upper soils are predominately silty sand and sandy silt with various amount of clay. Some of these soils may be suitable for reuse as non-expansive engineered fill, provided they are cleansed of excessive organics and debris. The soils with Expansion Index greater than 20 shall not be used within the upper 24 inches of the building pad and exterior flatwork areas.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-13:

Within the proposed pavement areas, the upper 12 inches of subgrade soil shall be moisture-conditioned to near optimum moisture and recompacted to a minimum of 90 percent of maximum density based on ASTM D1557 Test Method.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-14:

The upper soils, during wet winter months, may become very moist due to the absorptive characteristics of the soil. Earthwork operations performed during winter months may encounter very moist unstable soils, which may require removal to grade a stable building foundation. Project site winterization consisting of placement of aggregate base and protecting exposed soils during the construction phase shall be performed.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-15:

A qualified geotechnical engineer shall be present during all site clearing and grading operations to test and observe earthwork construction, as acceptance of earthwork construction is dependent upon compaction and stability of the material. The Soils Engineer shall reject any material that does not meet compaction and stability requirements.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-16:

The preferred materials specified for engineered fill are suitable for most applications with the exception of exposure to erosion. Project site winterization and protection of exposed soils during the construction phase shall be the sole responsibility of the contractor, since he has complete control of the project site at that time. Imported non-expansive fill shall consist of a well-graded, slightly cohesive, fine silty sand or sandy silt soil, with relatively impervious characteristics when compacted. This material shall be approved by the Soils Engineer prior to use and shall typically possess the following characteristics:

Fill soils shall be placed in lifts approximately 6 inches thick, moisture-conditioned as necessary, and compacted to achieve at least 90 percent of maximum density as determined by ASTM D1577 Test Method. Additional lifts shall not be placed if the previous lift did not meet the required dry density or if soil conditions are not stable.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-17:

All excavations shall comply with the current OSHA requirements. All cuts greater than 3 feet in depth should be sloped or shored. Temporary excavations should be sloped at 1:1 (horizontal to vertical) or flatter, up to a maximum depth of 10 feet. Heavy construction equipment, building materials, excavated soil, and vehicular traffic should not be allowed within five feet of the top (edge) of the excavation.

Where sloped excavations are not feasible due to site constraints, excavations shall require shoring. The design of the temporary shoring shall take into account lateral pressures exerted by the adjacent soil, and, where anticipated, surcharge loads due to adjacent buildings and any construction equipment or traffic expected to operate alongside the excavation.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-18:

To maintain the desired support for existing or new foundations, new utility trenches shall be located such that the base of the trench excavation is located above an imaginary plane having an inclination of 1.0 horizontal to 1.0 vertical, extending downward from the bottom edge of the adjacent footing. Utility trenches shall be excavated according to accepted engineering practices following OSHA standards by a contractor experienced in such work. The responsibility for the safety of open trenches should be borne by the contractor. Traffic and vibration adjacent to trench walls should be kept to a minimum; cyclic wetting and drying of excavation side slopes should be

avoided. Depending upon the location and depth of some utility trenches, groundwater flow into open excavations could be experienced, especially during or shortly following periods of precipitation.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-19:

With the exception of specific requirements of the local utility companies or building department, pipe bedding and shading shall consist of clean medium-grained sand. The sand shall be placed in a damp state and should be compacted by mechanical means prior to the placement of backfill soils. Above the pipe zone, underground utility trenches shall be backfilled with either free-draining sand, on-site soil or approved imported soil. The trench backfill shall be compacted to at least 90 percent relative compaction.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-20:

Concrete slab-on-grade floors shall be underlain by a water vapor retarder. The water vapor retarder shall be installed in accordance with ASTM Specification E 1643-98. In addition, utility trenches within the structure shall be compacted to minimize the transmission of moisture through the utility trench backfill.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-21:

Positive drainage shall be established away from the structure and shall be maintained throughout the life of the structure. Ponding of water shall not be allowed adjacent to the structure. Over-irrigation within landscaped areas adjacent to the structure shall not be performed.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM GEO-22:

Retaining walls shall be constructed according to the recommendations of the approved Geotechnical Report.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

HAZARDOUS MATERIALS AND MAN-MADE HAZARDS

Mitigation Measures identified in Section IV: Environmental Impact Analysis: E-Water Resources, will serve to reduce or eliminate potential environmental concerns related to hazardous materials and man-made hazards. In addition, the following Mitigation Measures are recommended:

MM HAZ-1: The Proposed Project shall comply with SCAQMD Rule 1403 regulating the

removal of ACMs from on-site buildings.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM HAZ-2: The Proposed Project shall comply with Construction Safety Orders 1532.1

(pertaining to lead) from Title 8 of the California Code of Regulations as well

as other applicable federal, state and local rules and regulations.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM HAZ-3: Prior to the issuance of the demolition permit, the applicant shall provide a

letter to the Department of Building and Safety from a qualified asbestos abatement consultant that no ACMs are present in the portion of the building to be demolished. If ACMs are found to be present, the applicant shall abate such ACMs in compliance with the South Coast Air Quality Management District's Rule 1403 as well as other applicable federal, state and local rules

and regulations.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and safety

MM HAZ-4: Prior to the issuance of the demolition permit, the applicant shall provide a

letter to the Department of Building and Safety from a qualified lead-paint abatement consultant that no lead-based paint is present in the portion of the building to be demolished. If lead-based paint is found to be present, it shall be abated in compliance with Construction Safety Orders 1532.1(pertaining to lead) from Title 8 of the California Code of Regulations as well as other

applicable federal, state and local rules and regulations.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM HAZ-5: Prior to issuance of the Certificate of Occupancy the applicant shall provide a

letter from the Fire Department stating that the LAFD has permitted the

facility's use, storage and creation of hazardous wastes.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM HAZ-6: All 55-gallon drums on site shall be stored in secondary containment to

prevent any accidental spills or leaks.

Monitoring Phase: Construction, Occupancy

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM HAZ-7: Hazardous materials generated, as a result of routine maintenance of

equipment shall be disposed of in accordance with legal disposal procedures.

Monitoring Phase: Construction, Occupancy

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of building and Safety

WATER RESOURCES: HYDROLOGY/WATER QUALITY

MM WR-1: The Proposed Project will comply with provisions of the City of Los Angeles Development Best Management Practices Handbook, Part A Construction Activities (3rd Edition), adopted by the Los Angeles Board of Public Works on September 29, 2004, and associated ordinances, which have specific minimum BMP requirements for all construction activities and require that construction projects with one acre or greater of disturbed soil prepare a SWPPP and file a NOI to comply with the State NPDES General Construction Permit with the SWRCB.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-2: The Proposed Project will comply with City of Los Angeles Ordinance No. 172,176 and Ordinance No. 173,494, which specify Stormwater and Urban Runoff Pollution Control requiring the application of Best Management Practices (BMPs), and the LAMC, Chapter IX, Division 70, which addresses grading, excavations, and fills. The Proposed Project will meet the applicable requirements of the Standard Urban Stormwater Mitigation Plan (SUSMP) approved by Los Angeles Regional Water Quality Control Board (LARWQCB), including the sections related to commercial development and the restaurant industry. The following LARWQCB list of stormwater pollution control measures for commercial and restaurant development is required:

For Commercial development (Lot size 100,000 square feet)

- Project applicants are required to implement stormwater BMPs to retain or treat the
 runoff from a storm event producing 3/4 inch of rainfall in a 24 hour period. The
 design of structural BMPs shall be in accordance with the Development Best
 Management Practices Handbook Part B Planning Activities. A signed certificate
 from a California licensed civil engineer or licensed architect that the proposed BMPs
 meet this numerical threshold standard is required.
- Post development peak stormwater runoff discharge rates shall not exceed the estimated pre-development rates for developments where the increased peak stormwater discharge rate will result in increased potential for downstream erosion.
- Concentrate or cluster development on portions of a site while leaving the remaining land in a natural undisturbed condition.
- Limit clearing and grading of native vegetation at the project site to the minimum needed to build lots, allow access, and provide fire protection.

- Maximize trees and other vegetation at each site by planting additional vegetation, clustering tree areas, and promoting the use of native and/or drought tolerant plants.
- Reduce impervious surface area by using permeable pavement materials where appropriate, including: pervious concrete/asphalt; unit pavers, i.e. turf block; and granular materials, i.e. crushed aggregates, cobbles.
- Promote natural vegetation by using parking lot islands and other landscaped areas.
- Preserve riparian areas and wetlands.
- Cover loading dock areas or design drainage to minimize run-on and run-off of stormwater.
- Direct connections to storm drains from depressed loading docks (truck wells) are prohibited.
- Repair/maintenance bays must be indoors or designed in such a way that doesn't allow stormwater run-on or contact with storm water run-off.
- Vehicle/equipment wash areas must be self-contained and/or covered, equipped with a clarifier, or other pretreatment facility, and properly connected to the sanitary sewer
- Any connection to the sanitary sewer must have authorization from the Bureau of Sanitation.
- The following activities are to be conducted under proper cover with drain routed to the sanitary sewer:
 - Storage of industrial wastes
 - Handling or storage of hazardous wastes
 - Metal fabrication or pre-cast concrete fabrication
 - Welding, cutting or assembly
 - Painting, coating or finishing
- Reduce impervious surface area by using permeable pavement materials where appropriate including pervious concrete, unit pavers, and granular materials.
- Store above ground liquid storage tanks (drums and dumpsters) in areas with impervious surfaces in order to contain leaks and spills. Install a secondary containment system such as berms, dikes, liners, vaults, and double-wall tanks. Where used oil or dangerous waste is stored, a dead-end sump should be installed in the drain.
- Toxic wastes must be discarded at a licensed regulated disposal site. Store trash dumpsters either under cover and with drains routed to the sanitary sewer or use nonleaking and water-tight dumpsters with lids. Use drip pans or absorbent materials whenever grease containers are emptied. Wash containers in an area with properly connected sanitary sewer.
- Reduce and recycle wastes, including paper, glass, aluminum, oil and grease.
- Reduce the use of hazardous materials and waste by using detergent-based or water-based cleaning systems, and avoid chlorinated compounds, petroleum distillates, phenols, and formaldehyde.
- Convey runoff safely from the tops of slopes and stabilize disturbed slopes.
- Utilize natural drainage systems to the maximum extent practicable.
- Control or reduce or eliminate flow to natural drainage systems to the maximum extent practicable.
- Stabilize permanent channel crossings.

- Protect slopes and channels and reduce run-off velocities by complying with Chapter IX, Division 70 of the Los Angeles Municipal Code and utilizing vegetation (grass, shrubs, vines, ground covers, and trees) to provide long-term stabilization of soil.
- Cleaning of vehicles and equipment to be performed within designated covered or bermed wash area paved with Portland concrete, sloped for wash water collection, and with a pretreatment facility for wash water before discharging to properly connect sanitary sewer with a CPI type oil/water separator. The separator unit must be designed to handle the quantity of flows, removed for cleaning on a regular basis (at least twice a year) to remove any solids, and the oil absorbent pads must be replaced regularly, once in fall just before the wet season, and in accordance with manufacturer specifications.
- All storm drain inlets and catch basins within the project area must be stenciled with prohibitive language (such as "NO DUMPING - DRAINS TO THE OCEAN") and/or graphical icons to discourage illegal dumping.
- Signs and prohibitive language and/or graphical icons, which prohibit illegal dumping, must be posted at public access points along channels and creeks within the project area.
- Legibility of stencils and signs must be maintained.
- Materials with the potential to contaminate stormwater must be:
 - Placed in an enclosure such as, but not limited to, a cabinet, shed or similar stormwater conveyance system; or
 - o Protected by secondary containment structures such as berms, dikes or curbs.
- The storage area must be paved and sufficiently impervious to contain leaks and spills.
- The storage area must have a roof or awning to minimize collection of stormwater within the secondary containment area.
- The owner(s) of the property will prepare and execute a covenant and agreement (Planning Department General Form CP-6770) satisfactory to the Planning Department binding the owners to post construction maintenance on the structural BMPs in accordance with the Standard Urban Stormwater Mitigation Plan and or per manufacturers instructions.
- Cut and fill slopes in designated hillside areas shall be planted and irrigated to prevent erosion, reduce run-off velocities and to provide long-term stabilization of soil. Plant materials include grass, shrubs, vines, ground covers and trees.
- Incorporate appropriate erosion control and drainage devices such as interceptor terraces, berms, vee-channels, and inlet and outlet structures, as specified by LAMC Section 91.7013. Protect outlets of culverts, conduits or channels from erosion by discharge velocities by installing rock outlet protection. Rock outlet protection is a physical device composed of rock, grouted riprap, or concrete rubble placed at the outlet of a pipe. Install sediment traps below the pipe outlet. Inspect, repair, and maintain the outlet protection after each significant rain.
- Trash container areas must have drainage from adjoining roofs and pavement diverted around the area(s).
- Trash container areas must be screened or walled to prevent off-site transport of trash.
- Reduce impervious land coverage of parking lot areas.
- Infiltrate runoff before it reaches the storm drain system.

- Runoff must be treated prior to release into the storm drain. Three types of treatments are available: (1) dynamic flow separator; (2) filtration; or (3) infiltration. Dynamic flow separators uses hydrodynamic force to remove debris, and oil and grease, and are located underground. Filtration involves catch basins with filter inserts. Filter inserts must be inspected every six months and after major storms, cleaned at least twice a year. Infiltration methods are typically constructed on-site and are determined by various factors such as soil types and groundwater table.
- Prescriptive methods detailing BMPs specific to this project category are available. Applicants are encouraged to incorporate the prescriptive methods into the design plans. These prescriptive methods can be obtained at the Public Counter or downloaded from the City's website at: http://www.lastormwater.org.

For Food Service Industry (Restaurants, Bakeries, Food Processors)

- Project applicants are required to implement stormwater BMPs to retain or treat the
 runoff from a storm event producing 3/4 inch of rainfall in a 24 hour period. The
 design of structural BMPs shall be in accordance with the Development Best
 Management Practices Handbook Part B Planning Activities. A signed certificate
 from a California licensed civil engineer or licensed architect that the proposed BMPs
 meet this numerical threshold standard is required.
- Post development peak stormwater runoff discharge rates shall not exceed the estimated pre-development rates for developments where the increased peak stormwater discharge rate will result in increased potential for downstream erosion.
- Concentrate or cluster development on portions of a site while leaving the remaining land in a natural undisturbed condition.
- Limit clearing and grading of native vegetation at the project site to the minimum needed to build lots, allow access, and provide fire protection.
- Maximize trees and other vegetation at each site by planting additional vegetation, clustering tree areas, and promoting the use of native and/or drought tolerant plants.
- Promote natural vegetation by using parking lot islands and other landscaped areas.
- Preserve riparian areas and wetlands.
- Incorporate appropriate erosion control and drainage devices such as interceptor terraces, berms, vee-channels, and inlet and outlet structures, as specified by LAMC Section 91.7013. Protect outlets of culverts, conduits or channels from erosion by discharge velocities by installing rock outlet protection. Rock outlet protection is a physical device composed of rock, grouted riprap, or concrete rubble placed at the outlet of a pipe. Install sediment traps below the pipe outlet. Inspect, repair, and maintain the outlet protection after each significant rain.
- Any connection to the sanitary sewer must have authorization from the Bureau of Sanitation.
- Cleaning of oily vents and equipment to be performed within designated covered
 area, sloped for wash water collection, and with a pretreatment facility for wash water
 before discharging to properly connected sanitary sewer with a CPI type oil/water
 separator. The separator unit must be: designed to handle the quantity of flows;
 removed for cleaning on a regular basis to remove any solids; and the oil absorbent
 pads must be replaced regularly according to manufacturer's specifications.

- Store trash dumpsters either under cover and with drains routed to the sanitary sewer or use non-leaking and water tight dumpsters with lids. Wash containers in an area with properly connected sanitary sewer.
- Reduce and recycle wastes, including paper, glass, aluminum, oil and grease.
- Store liquid storage tanks (drums and dumpsters) in designated paved areas with impervious surfaces in order to contain leaks and spills. Install a secondary containment system such as berms, curbs, or dikes. Use drip pans or absorbent materials whenever grease containers are emptied.
- All storm drain inlets and catch basins within the project area must be stenciled with prohibitive language (such as "NO DUMPING - DRAINS TO THE OCEAN") and/or graphical icons to discourage illegal dumping.
- Signs and prohibitive language and/or graphical icons, which prohibit illegal dumping, must be posted at public access points along channels and creeks within the project area.
- Legibility of stencils and signs must be maintained.
- Materials with the potential to contaminate stormwater must be:
 - Placed in an enclosure such as, but not limited to, a cabinet, shed or similar stormwater conveyance system; or
 - Protected by secondary containment structures such as berms, dikes or curbs.
- The storage area must be paved and sufficiently impervious to contain leaks and spills.
- The storage area must have a roof or awning to minimize collection of stormwater within the secondary containment area.
- The owner(s) of the property will prepare and execute a covenant and agreement (Planning Department General Form CP-6770) satisfactory to the Planning Department binding the owners to post construction maintenance on the structural BMPs in accordance with the Standard Urban Stormwater Mitigation Plan and or per manufacturers instructions.
- Prescriptive methods detailing BMPs specific to this project category are available. Applicants are encouraged to incorporate the prescriptive methods into the design plans. These prescriptive methods can be obtained at the Public Counter or downloaded from the City's website at: www.lastormwater.org.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-3: The Proposed Project will adopt an erosion and sediment control plan for the project site during the construction phase that would employ strategies such as temporary and permanent seeding, mulching, earth dikes, silt fencing, sediment traps and sediment basins. The erosion and sediment control plan will be reviewed and approved by Department of Building & Safety to insure it complies with U.S. Environmental Protection Agency (EPA) Document No. EPA 832/R-92-005 (September 1992), Storm Water Management for Construction Activities, Chapter 3 (or the local agency equivalent erosion and sedimentation control standards and codes) and would address soil loss, stormwater runoff, wind

erosion, sedimentation, and fugitive dust at a minimum. The erosion and sediment control plan would contribute to minimizing water quality impacts and may indirectly minimize aesthetic effects during the construction phase.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-4: In accordance with the SUSMP requirements, the Proposed Project shall meet (or exceed) all minimum site design and source control BMPs.

Monitoring Phase: Pre-Construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-5: The Proposed Project shall incorporate treatment control BMPs that will minimize urban runoff and associated impacts to receiving water quality and specifically address the identified pollutants of concern. Acceptable BMP alternatives that may be implemented with the Proposed Project include: (1) vegetated treatment BMPs, (2) onsite storage and reuse, (3) permeable paving, (4) roof top BMPs, and (5) media filters.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-6: The Proposed Project shall incorporate vegetated treatment BMPs, including swales, filter strips, bioretention and planter boxes and appropriate and approved by the City.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-7: The Proposed Project shall incorporate permeable (porous) pavement material in pavement areas (such as roadways, driveways, parking areas, and walkways), such that the pavement materials will allow water to drain down to the underlying soil and reduce the volume of wet weather urban runoff. The Proposed Project shall incorporate a mix of porous concrete, pervious asphalt, pervious pavers, grass/gravel pavers, and crushed stone, into the landscape plan and design of surface parking areas as functionally appropriate.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-8:

The Proposed Project shall employ rooftop BMPs for filtering and/or capturing stormwater in order to contribute toward the reduction of small storm events peaks and the overall runoff volume via inter-event evaporation and transpiration. Acceptable rooftop BMPs incorporated into the project design include planters and landscaping on the rooftop portion of the new parking structures, and hanging planters along the parking buildings and along the Riverside Drive mall elevation.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

MM WR-9:

The Proposed Project shall employ media filtration to separate and filter fine particulates and associated pollutants from captured stormwater to the extent feasible and as approved by the City.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works Enforcement Agency: Department of Public Works

WATER RESOURCES: WATER SUPPLY

No mitigation measures are required. Compliance with Title 20 (Public Utilities and Energy) and Title 24 (Building Standards Code) of the California Code of Regulations is already a required standard condition under applicable regulations and will ensure that the Proposed Project incorporates standard water conservation practices.

LAND USE, PLANNING AND URBAN DECAY

With the incorporation of the assumed Project Design Features and Standard Conditions of Approval, the Proposed Project would not result in any significant land use compatibility or land use plan consistency impacts. To ensure the Proposed Project is consistent with these assumptions, the follow mitigation measures are recommended:

MM LU-1: The Proposed Project must obtain the appropriate approvals, including zone change, variances and conditional use permits, prior to commencing project development. Attainment of such approvals shall in turn ensure that the Proposed Project is in full compliance with local codes, procedures and regulations.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-2: The Proposed Project shall comply with the draft RIO and/or adopted RIO in effect at the time of project approval.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-3: In accordance with the SUSMP requirements, the Proposed Project shall meet (or exceed) all minimum site design and source control BMPs.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-4: The Proposed Project shall adopt an erosion and sediment control plan for the project site during the construction phase that would employ strategies such as temporary and permanent seeding, mulching, earth dikes, silt fencing, sediment traps and sediment basins. The erosion and sediment control plan shall comply with U.S. Environmental Protection Agency (EPA) Document No. EPA 832/R-92-005 (September 1992), Storm Water Management for Construction Activities, Chapter 3 (or the local agency equivalent erosion and sedimentation control standards and codes) and shall address soil loss, stormwater runoff, wind erosion, sedimentation, and fugitive dust at a minimum. The erosion and sediment control plan shall contribute to minimizing water quality impacts and may indirectly minimize aesthetic effects during the construction phase.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-5: Consistent with California laws, the Proposed Project shall prohibit smoking in the shopping center buildings, public areas, or exterior areas within 25 feet from entries, outdoor air intakes and operable windows, unless such areas are specifically designated and properly ventilated as a dedicated "smoking area".

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-6: The Proposed Project shall include the provision of a new community room to be made available to the surrounding Sherman Oaks community and to offset a potential increase demand on recreational facilities for community meeting space needs.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-7: The Proposed Project shall provide new landscaping treatment along the Hazeltine Avenue, Riverside Drive and Woodman Avenue frontages that would enhance the visual interest along these road way corridors and the shopping center perimeter through the addition of a sophisticated landscape treatment that includes color, depth, volume and variety.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-8: The Proposed Project shall provide funds for the implementation of a Neighborhood Protection Program (NPP) that focuses on the prevention of "cut through" traffic in the residential neighborhoods north of the project site (across Riverside Drive). The NPP would seek to maintain the quality of the residential area through traffic control and traffic calming measures.

Monitoring Phase: Occupancy

Monitoring Agency: Neighborhood Protection Program Enforcement Agency: Neighborhood Protection Program

MM LU-9: The Proposed Project shall provide an improved pedestrian crossing at the proposed Riverside Drive/Matilija Avenue intersection, a landscape-enhanced pedestrian corridor along Riverside Drive, and more efficient and safer site driveway entrances that will serve to strengthen community linkages to surrounding uses and support non-motorized vehicle travel options.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-10: The Proposed Project Landscape Plan shall incorporate wall-hugging vines and bamboo screening as CPTED strategies which function as graffiti deterrents, minimization of hidden spaces, and creation of more open area for natural surveillance.

Monitoring Phase: Pre-Construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-11 The Proposed Project shall incorporate building access points that would improve public access and circulation throughout the mall and minimize walking distances from remote parking areas, thereby improving public safety (through natural access control, natural surveillance and territorial reinforcement features) and pedestrian activity (through improved convenience and accessibility).

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-12: The Proposed Project shall incorporate treatment control BMPs that will minimize urban runoff and associated impacts to receiving water quality and specifically address the identified pollutants of concern. Many BMP alternatives can be easily integrated into planned landscaping, right-of-ways, and planned infrastructure. BMP alternatives that would be implemented with the Proposed Project include: (1) vegetated treatment BMPs, (2) onsite storage and reuse, (3) permeable paving, (4) roof top BMPs, and (5) media filters.

Monitoring Phase: Pre-construction, Construction, Occupancy

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-13: The Proposed Project shall incorporate a number of vegetated treatment BMPs, including swales, filter strips, bioretention and planter boxes. When properly designed and maintained, vegetated BMPs are among the most effective, cost efficient treatment approaches for dry and wet-weather runoff. Treatment occurs through sedimentation, filtration, adsorption to organic matter, and vegetative

uptake. Additionally, vegetated treatment systems would reduce runoff volumes through soil soaking, infiltration, and evapotranspiration. On-site implementation of these systems would be integrated into surface conveyances and on-site landscaping in innovative ways that provide dual-functional site amenities.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-14: The Proposed Project shall incorporate permeable (porous) pavement material in pavement areas (such as roadways, driveways, parking areas, and walkways). The permeable (porous) pavement materials would allow water to drain down to the underlying soil and reduce the volume of wet weather urban runoff. The Proposed Project would incorporate a mix of porous concrete, pervious asphalt, pervious pavers, grass/gravel pavers, and crushed stone, into the landscape plan and design of surface parking areas as functionally appropriate.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-15: The Proposed Project shall employ rooftop BMPs for filtering and/or capturing stormwater in order to contribute toward the reduction of small storm events peaks and the overall runoff volume via inter-event evaporation and transpiration. Rooftop BMPs incorporated into the project design include planters and landscaping on the rooftop portion of the new parking structures, and hanging planters along the parking building tiers and along the Riverside Drive mall elevation.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-16: The Proposed Project shall employ media filtration to separate and filter fine particulates and associated pollutants from captured stormwater.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-17: The Proposed Project shall provide bicycle racks at a ratio of 2% of the total number of parking spaces on-site, as well as lockers, changing rooms and showers inside the shopping center. A minimum of 20 additional bicycle spaces (in racks) would be provided at multiple locations through out the site. Four showers (two per each gender) would be provided in a dedicated shower facility area. Lockers would be provided in conjunction with the shower facilities.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning

Enforcement Agency: Building and Safety

MM LU-18: The Proposed Project shall designate an area for recyclable collection and storage that is appropriately sized and located in a convenient area to serve mall tenants. As appropriate, the Fashion Square Mall Association shall implement the use of cardboard balers, aluminum can crushers, recycling chutes and other waste management technologies to further enhance and manage a recycling program at the shopping center.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning
Enforcement Agency: Department of Building and Safety

MM LU-19: The Proposed Project shall install carbon monoxide and airflow measurement equipment that would transfer the information to the HVAC system and/or Building Automation System to trigger corrective action, if applicable, and/or use the measurement equipment to trigger alarms that inform building operators or occupants of a possible deficiency in outdoor air delivery. Installation of such a system in areas where carbon monoxide concentrations may escalate (such as in the vicinity of loading docks or valet parking drop-offs) would improve both indoor and localized "hotspot" air quality.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of City Planning

Enforcement Agency: Department of Building and Safety

NOISE

MM N-1:

The City of Los Angeles Noise Ordinance has established policies and regulations concerning the generation and control of noise that could adversely affect its citizens and noise sensitive land uses. Regarding construction, the LAMC indicates that no construction or repair work shall be performed between the hours of 9:00 P.M. and 7:00 A.M. the following day, since such activities would generate loud noises and disturb persons occupying sleeping quarters in any adjacent dwelling, hotel, apartment or other place of residence.² No person, other than an individual home owner engaged in the repair or construction of his/her single-family dwelling, shall perform any construction or repair work of any kind or perform such work within 500 feet of land so occupied before 8:00 A.M. or after 6:00 P.M. on any Saturday or on a federal holiday, or at any time on any Sunday.

The LAMC also specifies the maximum noise level of powered equipment or powered hand tools.³ Any powered equipment or hand tool that produces a maximum noise level exceeding 75 dBA at a distance of 50 feet is prohibited. However, this noise limitation does not apply where compliance is technically infeasible. Technically infeasible means the above noise limitation cannot be met despite the use of mufflers, shields, sound barriers and/or any other noise reduction device or techniques during the operation of equipment.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

² Chapter IV, Article 1, Section 41.40, January 29, 1984 and Chapter XI, Article 2, Section 112.04, August 8, 1996. Los Angeles, City of. 2007 (as amended). *Official City of Los Angeles Municipal Code, Sixth Edition* (LAMC). Cincinnati, OH: American Legal Publishing Corp. 6 June 2008 http://www.amlegal.com/nxt/gateway.dll?f=templates&fn=default.htm&vid=amlegal:lamc_ca.

³ Chapter XI, Article 2, Section 112.05, August 8, 1996. Los Angeles, City of. 2007 (as amended). *Official City of Los Angeles Municipal Code, Sixth Edition* (LAMC). Cincinnati, OH: American Legal Publishing Corp. 6 June 2008

http://www.amlegal.com/nxt/gateway.dll?f=templates&fn=default.htm&vid=amlegal:lamc_ca>.

MM N-2:

The Proposed Project will include certain features to reduce exposure of sensitive receptors to operational noise. For example, mechanical equipment would be enclosed or located on roofs, and mechanical equipment noise would not increase ambient noise levels by more than 5 dBA at off-site locations. In addition, the new loading docks would be located behind mall structures and away from sensitive receptors. As a result, activity associated with the new loading docks would not increase ambient noise levels by 5 dBA or more at the nearest sensitive receptors (e.g. residences on Riverside Drive).

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-3·

All construction equipment shall be equipped with mufflers and other suitable noise attenuation devices.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-4:

Grading and construction contractors shall use quieter equipment as opposed to noisier equipment (such as rubber-tired equipment rather than track equipment).

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-5:

Equipment staging areas shall be located on the southern portion of the project site, as far as possible from multi-family residences on.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-6:

During phase 2 parking structure construction and phase 3 demolition and excavation of the tunnel area, temporary sound barriers (not to exceed a maximum height of ten feet) capable of achieving sound attenuation of at least 10 dBA (e.g., sound attenuation blanket) shall be constructed, such that the line-of-sight is blocked from active construction areas to residential land uses on Riverside Drive.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-7: Construction workers shall be required to park at designated locations and shall be prohibited from parking on nearby residential streets.

Monitoring Phase: Construction

Monitoring Agency: Department of Building and Safety Enforcement Agency: Department of Building and Safety

MM N-8: Pile drivers shall be shrouded with acoustically absorptive shields capable of reducing noise by at least 9 dBA at all times during pile driving operations.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-9: Pile driving activity shall be scheduled for times that have the least impact on adjacent sensitive receptors.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-10: Consistent with previous Conditions of Approval, all residential units located within 2,000 feet of the construction site shall be sent a notice regarding the construction schedule of the Proposed Project. A sign, legible at a minimum distance of 50 feet, shall also be posted at the construction site. All notices and signs shall indicate the dates and duration of construction activities, as well as provide a telephone number where residents can inquire about the construction process and register complaints.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

MM N-11: A "noise disturbance coordinator" shall be established. The disturbance coordinator shall be responsible for responding to any local complaints about construction noise. The disturbance coordinator shall determine the cause of the noise complaint (e.g., starting too early, bad muffler, etc.) and shall be required to implement reasonable measures such that the complaint is resolved. All notices that are sent to residential units within 500 feet of the construction site and all signs, legible at a distance of 50 feet, posted at the construction site shall list the telephone number for the disturbance coordinator.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety
Enforcement Agency: Department of Building and Safety

PUBLIC SERVICES: FIRE

Compliance with the LAMC will be required. Many of the LAMC requirements serve to reduce fire safety concerns to less than significant levels.

MM PSF-1:

The Proposed Project shall comply with all applicable State and local codes and ordinances, and the guidelines found in the Fire Protection and Fire Prevention Plan, which is an element of the General Plan of the City of Los Angeles (CPC 19708).

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: LAFD, Department of Public Works-Bureau of

Engineering, Department of Building and Safety

MM PSF-2:

In accordance with the City of Los Angeles building permit review process, definitive plans and specifications shall be submitted to the Fire Department and any requirements for necessary permits shall be satisfied prior to commencement and/or occupation of any portion of the Proposed Project. Typical site plan and building permit requirements would include, but not be limited to, the following:

- All first story portions of any habitable building shall be within 300 feet of an approved fire hydrant.
- A building smoke alarm system designed to detect any smoke in the building's air-handling systems shall be installed. The system shall cause an alarm to be announced at the central fire control station.
- A fire alarm system shall be installed which uses a dependable method of sounding a fire alarm throughout the building.
- All decorative landscaping surrounding project structures shall use fireresistant plants and materials.
- Brush in the area adjacent to proposed development shall be cleared or thinned periodically by the applicant under supervision of the LAFD.
- New fire hydrants and/or top upgrades to existing fire hydrants shall be installed in accordance with the Los Angeles Fire Code.
- Adequate public and private fire hydrants will be required. The number and location of these hydrants will be determined by the Fire Department after review of the Plot Plan.
- Access for Fire Department apparatus and personnel to and into all structures shall be required.
- At least two different ingress/egress roads for each area, that will accommodate major fire apparatus and provide for major evacuation during emergency situations shall be required.

- Fire lanes, where required, and dead-ending streets should terminate in a cul-de-sac or other approved turning area. No dead-ending street or fire lane should be greater than 700 feet in length or secondary access shall be required.
- Construction of public or private roadways in the proposed development shall not exceed 15 percent in grade, unless otherwise approved.
- No building or portion of a building shall be constructed more than 150 feet from the edge of a roadway of an improved street, access road, or designated fire lane, unless otherwise approved.
- Fire lane width shall not be less than 20 feet. When a fire lane must accommodate the operation of Fire Department aerial ladder apparatus or where fire hydrants are installed, those portions shall not be less than 28 feet in width.
- Additional vehicular access may be required by the Fire Department where buildings exceed 35 feet in height.
- Private streets and entry gates will be built to City standards to the satisfaction of the City Engineer and the Fire Department.
- The Project shall utilize standard cut-corners on all turns, if applicable.
- Fire Department access shall remain clear and unobstructed during demolition.
- If applicable, fire lanes and dead ending streets shall terminate in a cul-desac or other approved turning area. No dead ending street or fire lane shall be greater than 700 feet in length or secondary access shall be required.
- If applicable, where access for a given development requires accommodation of Fire Department apparatus, minimum outside radius of the paved surface shall be 35 feet. An additional six feet of clear space must be maintained beyond the outside radius to a vertical point 13 feet 6 inches above the paved surface on the roadway. Where access for a given development requires accommodation of Fire Department apparatus, overhead clearance shall not be less than 14 feet.
- Where fire apparatus will be driven onto the road level surface of the subterranean parking structure, that structure shall be engineered to withstand a bearing pressure of 8,600 pounds per square foot, unless otherwise approved.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety

Enforcement Agency: LAFD, Department of Public Works-Bureau of Engineering, Department of Building and Safety

MM PSF-3:

Fashion Square Lane will be reconfigured and improved to provide a minimum of two unobstructed vehicle travel lanes (one per each direction) for its entire length along the south edge of the shopping center from Hazeltine Avenue to Riverside Drive. This fire lane shall be unobstructed except for the connection from the existing west parking structure to the new mall. However, this limited area shall have a minimum vertical clearance of 17 feet.

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Building and Safety

Enforcement Agency: LAFD, Department of Public Works-Bureau of

Engineering, Department of Building and Safety

MM PSF-4: New Proposed Project buildings, including parking structures, shall be fully sprinklered.

Monitoring Phase: Pre-construction, Construction
Monitoring Agency: Department of Building and Safety

Enforcement Agency: LAFD, Department of Public Works-Bureau of

Engineering, Department of Building and Safety

PUBLIC SERVICES: POLICE

MM PSP-1: All businesses within the development desiring to sell or allow consumption of alcoholic beverages will require licensing through Alcohol and Beverage Control and approval by the LAPD.

Monitoring Phase: Pre-construction

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-2: The Proposed Project Landscape Plan will incorporate wall-hugging vines and bamboo screening as CPTED strategies which function as graffiti deterrents, minimization of hidden spaces, and creation of more open area for natural surveillance.

Monitoring Phase: Pre-construction, Occupancy

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-3: The Proposed Project shall be maintained as a closed mall campus with controlled access points and operational hours.

Monitoring Phase: Occupancy
Monitoring Agency: LAPD
Enforcement Agency: LAPD

MM PSP-4: The Proposed Project shall result in the addition of more building access points that will improve public access and circulation throughout the mall and minimize walking distances from remote parking areas, thereby improving opportunities for CPTED principals that employee natural access control, natural surveillance and territorial reinforcement features.

Monitoring Phase: Pre-construction

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-5: The Proposed Project shall provide organized roving security patrol, video surveillance, and security lighting to ensure the safety and security of patrons, tenants and employees.

Monitoring Phase: Pre-construction, Occupancy

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-6: The Proposed Project includes reconfiguration of Fashion Square Lane to provide a minimum of two unobstructed vehicle travel lanes (one per each direction) through its entire length of along the south edge of the project site adjacent to proposed structures affording maximum accessibility for emergency service personnel and vehicles.

Monitoring Phase: Pre-construction

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-7: The Proposed Project shall provide sufficient off-street parking for all building employees and anticipated patrons and visitors, thereby minimizing the potential for parking conflicts on off-site locations and providing parking within a controlled environment that can be monitored by on-site patrol and surveillance operations.

Monitoring Phase: Pre-construction

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-8: Directional and security lighting will be required for safety purposes. Through a new plan, lighting can enhance safety along the Riverside Drive and Hazeltine Avenue frontages and add to the perceived security of the neighborhood in general. Lighting would be incorporated into the streetscape environment at several levels, including the use of bollards, wall reveals, seating areas, and crosswalks. The use of plaza strip lighting will afford additional security lighting but with a park-like feel and without significant light intrusion to the surrounding neighborhood.

Monitoring Phase: Pre-construction,

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-9: Incorporate into the plans the design guidelines relative to security, semi-public and private spaces, which may include but not be limited to access control to building, secured parking facilities, walls/fences with key systems, well-illuminated public and semi-public space designed with a minimum of dead space to eliminate areas of concealment, location of toilet facilities or building entrances

in high-foot traffic areas, and provision of security guard patrol throughout the project site if needed. Please refer to <u>Design Out Crime Guidelines: Crime Prevention Through Environmental Design</u> published by the Los Angeles Police Department's Crime Prevention Section (located at Parker Center, 150 N. Los Angeles Street, Room 818, Los Angeles, (213) 485-3134. These measures shall be approved by the Police Department prior to the issuance of building permits.

Monitoring Phase: Pre-Construction, Occupancy

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-10: Elevators, lobbies, and parking areas shall be well illuminated and designed with minimum dead space to eliminate areas of concealment.

Monitoring Phase: Pre-construction, Occupancy

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-11: The Project Applicant shall consult with the LAPD Crime Prevention Unit on any additional crime prevention features appropriate to the design of the Proposed Project, and shall incorporate such measures to the extent feasible and practical.

Monitoring Phase: Pre-construction

Monitoring Agency: LAPD Enforcement Agency: LAPD

MM PSP-12: Upon completion of the Proposed Project, the Fashion Square Mall Association shall provide the Van Nuys Division Commanding Officer with a diagram of each portion of the property, including access routes and additional information that might facilitate police response.

Monitoring Phase: Occupancy
Monitoring Agency: LAPD
Enforcement Agency: LAPD

PUBLIC UTILITIES: SOLID WASTE

MM PU-1: The Proposed Project shall comply with the Countywide Integrated Waste

Management Plan and meet targeted waste stream reduction requirements as

provided in the plan.

Monitoring Phase: Construction

Monitoring Agency: Department of Public Works, Integrated Solid Waste

Management Office

Enforcement Agency: Department of Public Works, Integrated Solid Waste

Management Office

MM PU-2:

The Proposed Project shall develop and implement a construction waste management plan (CWMP) that identifies the materials to be diverted from disposal and whether the materials will be sorted on-site or commingled. A minimum of 50% of the construction and demolition debris (exclusive of excavated soils and organic debris) shall be recycled and/or salvaged. Excavated/exported soil shall be transferred off-site as clean fill rather than landfilled. Organic landclearing debris (i.e., trees to be removed) shall be processed as greenwaste. The CWMP include measures for the recycling cardboard, metal, brick, acoustical tile, concrete, plastic, clean wood, glass, gypsum wallboard, carpet and insulation and other similar materials used during the construction phase. The CWMP shall designate a specific area(s) on the construction site for segregated or commingled collection of recyclable materials, The CWMP and track recycling efforts throughout the construction process. shall identify construction haulers and recyclers to handle the designated materials. Consistent with the intent to minimize waste, the CWMP shall also establish a minimum project goal of 10% (post-consumer and ½ pre-consumer) for recycled content construction materials and identify material suppliers that can achieve this goal. During construction, the developer shall ensure that the specified recycled content materials would be installed. The CWMP shall also establish a project goal (10% minimum) for locally sourced construction materials and would identify materials and material suppliers that can achieve this goal. During construction, the developer shall ensure that the specified local materials would be installed and quantify the total percentage of local materials installed.

Monitoring Phase: Pre-construction, Construction

Monitoring Agency: Department of Public Works, Integrated Solid Waste

Management Office

Enforcement Agency: Department of Public Works, Integrated Solid Waste

Management Office

MM PU-3:

The Proposed Project shall designate an area for recyclable collection and storage that is appropriately sized and located in a convenient area to serve mall tenants. As feasible, the Fashion Square Mall Association shall employ cardboard balers, aluminum can crushers, recycling chutes and other waste management technologies to further enhance and manage a recycling program at the shopping center

Monitoring Phase: Pre-construction

Monitoring Agency: Department of Public Works, Integrated Solid Waste

Management Office

Enforcement Agency: Department of Public Works, Integrated Solid Waste

Management Office

MM PU-4:

The Proposed Project shall be designed, built and operated in a manner consistent with the requirements to achieve LEED certification. The Proposed Project will implement a variety of design and operational features, including waste recycling and stream reduction programs, to achieve LEED certification.

Monitoring Phase: Pre-construction, Construction, Occupancy

Monitoring Agency: Department of Public Works, Integrated Solid Waste

Management Office

Enforcement Agency: Department of Public Works, Integrated Solid Waste

Management Office

TRAFFIC, CIRCULATION AND ACCESS

Construction

MM TRF-1:

In accordance with LAMC Section 91.70067, hauling of construction materials shall be restricted to a haul route approved by the City. The City of Los Angeles will approve specific haul routes for the transport of materials to and from the site during demolition and construction. This process includes a public hearing and opportunities for the public to comment on the proposed route.

Monitoring Phase: Pre-construction, Construction

Monitoring Agency: Department of Building and Safety, Department of

Public Works- Bureau of Street Services

Enforcement Agency: Department of Building and Safety, Department of

Public Works-Bureau of Street Services

MM TRF-2:

Prior to obtaining a demolition and/or grading permit, the Project Applicant shall prepare a Construction Traffic Control Plan (Construction TCP) for review and approval by the LADOT. The Construction TCP shall include the designated haul route and staging area, traffic control procedures, emergency access provisions, and construction crew parking to mitigate the traffic impact during construction. The Construction TCP will identify a designated off-site parking lot at which construction workers will be required to park.

Monitoring Phase: Pre-construction

Monitoring Agency: LADOT Enforcement Agency: LADOT

Long-Term Operational

- MM TRF-3: The Proposed Project shall comply with Section 12.26 J of the Los Angeles Municipal Code for purposes of implementing a Transportation Demand Management (TDM) plan. The following outlines the minimum measures that the project will undertake in compliance with the Code section.
 - Employee Transportation Center and Transportation Coordinator. The project shall designate an area within the building to be the Transportation Center. The Employee Transportation Center shall be maintained by the center's Transportation Coordinator, who will be employed by Westfield. The Transportation Coordinator will assist employees in seeking out and arranging for commute alternatives. This includes carpool and vanpool formation, assisting employees with planning trips to work via bus, and locating bike or walking routes to work. The Employee Transportation Center shall provide a bulletin board, display case, or kiosk displaying transportation information where the greatest number of employees are likely to see it. The transportation information displayed should include, but is not limited to, the following:
 - Current routes and schedules for public transit serving the site;
 - Telephone numbers for referrals on transportation information including numbers for the regional ridesharing agency and local transit operations;
 - Ridesharing promotion material supplied by commuter-oriented organizations;
 - Regional/local bicycle route and facility information; and
 - A listing of on-site services or facilities which are available for carpoolers, vanpoolers, bicyclists, and transit riders.
 - Preferential Parking Spaces. The project will provide designated parking areas for employee carpools and vanpools as close as practical to the main pedestrian entrance(s) of the building(s). The spaces shall be signed and striped sufficient to meet the employee demand for such spaces. The carpool/vanpool parking area shall be identified on the driveway and circulation plan upon application for a building permit.
 - <u>Bicycle Parking Spaces</u>. Bicycle parking shall be provided in conformance with Section 12.21 A 16 of the Los Angeles Municipal Code. The project will provide safe and convenient access from the external circulation system to bicycle parking facilities on-site.
 - <u>Carpool/Vanpool Loading Area</u>. The project shall provide a safe and convenient area in which carpool/vanpool vehicles may load and unload passengers other than in their assigned parking area.

- <u>Pedestrian Access</u>. The project shall provide sidewalks or other designated pathways following direct and safe routes from the external pedestrian circulation system to the center.
- <u>Transit Stop Enhancements</u>. In coordination with LADOT and the Department of City Planning, the project will consult with local bus service providers in determining appropriate improvements to transit stops, such as installation of benches, shelters, and schedule information.

Monitoring Phase: Pre-construction, Construction

Monitoring Agency: LADOT Enforcement Agency: LADOT

MM TRF-4: The Project Applicant shall seek LADOT approval to install two new traffic signals at the two new Riverside Drive driveways to facilitate vehicular movements to and from the project site.

Monitoring Phase: Pre-construction

Monitoring Agency: LADOT Enforcement Agency: LADOT

MM TRF-5: The Project Applicant shall install a pedestrian crossing at the Riverside Drive/Matilija Avenue intersection.

Monitoring Phase: Construction
Monitoring Agency: LADOT
Enforcement Agency: LADOT

- MM TRF-6: In addition to the TDM measures described above that satisfy the requirements of Section 12.26 J, the Proposed Project shall voluntarily implement the following demand management services to further reduce vehicle trips and parking demand at the site:
 - Orange Line Shuttle. The project shall provide a shuttle service connecting the site to a nearby Orange Line station (e.g., Van Nuys Boulevard). This service could be provided by either the provision of a private shuttle or the funding of extended hours for the existing LADOT DASH line. The Orange Line shuttle would complement existing transit services (i.e., the LADOT DASH service) such that the shuttle would operate during hours when other public transit services connecting the site to the Orange Line are not available (e.g., evenings during the work week and certain weekend hours). The shuttle would operate during regular shopping center hours corresponding with periods of peak parking demand at the site (i.e., everyday during the holiday shopping period between November 15 and January 1, and every Saturday/Sunday throughout the year).

Monitoring Phase: Pre-construction

Monitoring Agency: LADOT Enforcement Agency: LADOT

MM TRF-7: The Proposed Project applicant, in consultation with LADOT, shall fund the development and implementation of a Neighborhood Traffic Management Plan (NTMP) to address potential existing and future regional "cut-through" traffic on residential streets north of the project site, which may encompass the area generally bounded by Magnolia Boulevard to the north, Riverside Drive to the south, Hazeltine venue to the west and Woodman Avenue to the east. The following is a discussion of the sequential steps typically followed by LADOT in implementing the NTMP.

- Deposit Funds. Prior to issuance of a Building Permit for the Proposed Project, the project applicant will be required to deposit funds in a separate account maintained by LADOT designated for use in funding the NTMP. The exact amount will be determined by LADOT and will reasonably cover the likely costs of the measures.
- Stakeholders Meeting. Following establishment of the NTMP account, a group consisting of representatives from LADOT, the Council Office, and the residential community north of the project site will meet to discuss the goals, opportunities and constraints of the NTMP. As needed, follow-up meetings may be conducted with other City departments (Public Works, Fire Department, Police Department, etc.).
- Data Collection and Initial Plan Formulation. Based on the input received at the stakeholders meeting, LADOT will commence with conducting appropriate studies (traffic observations, traffic counts, vehicle speed surveys, accident research, commercial parking intrusion, etc.) to assess existing traffic conditions on the residential streets north of the project site. The studies will be based on studies conducted for the EIR as well as other studies deemed necessary by LADOT. Following collection of the data and based on their professional experience, LADOT will prepare for the stakeholders an initial NTMP for implementation prior to completion of the Proposed Project.
- Neighborhood Concurrence. As some of the measures that may be recommended within the initial NTMP (e.g., installation of speed humps, implementation of permit parking districts) may, by LADOT policy, require majority or super-majority consent of affected property owners (at least two-thirds), LADOT will work with the stakeholders to survey the appropriate residents to determine if there is support to implement the specific measures.
- Implementation and Follow-Up Studies. LADOT will implement the initial NTMP (including those measures authorized by the affected residents) prior to the completion of the Proposed Project. Following a reasonable period of time after opening of the Proposed Project, LADOT will meet with the stakeholders to review traffic experiences since the implementation of the NTMP and opening of the Proposed Project. As needed, additional review

and studies may be conducted by LADOT based on the effectiveness of the initial NTMP and/or traffic and parking issues related to the shopping center.

• Updated NTMP. Based on the follow-up studies, LADOT will present to the stakeholders their recommendations for an updated NTMP. Following review by the stakeholders, and with consent of the affected residents (if required), the updated NTMP will be implemented.

Monitoring Phase: Pre-construction, Occupancy

Monitoring Agency: LADOT Enforcement Agency: LADOT

MM TRF-8: To further alleviate potential inconvenience existing in the area which lead to non-project related cut-through traffic the Proposed Project shall install protected/permissive left-turn traffic signal phasing for Hazeltine Avenue at its intersection with Riverside Drive to improve current safety and traffic flow at this intersection.

Monitoring Phase: Pre-construction

Monitoring Agency: LADOT Enforcement Agency: LADOT

MM TRF-9: The Project Applicant will prepare and implement an Interim Traffic Control Plan (TCP) during construction. The Interim TCP shall address interim traffic staging and parking for shopping center patrons that would continue to shop at the shopping center during the construction phase. To maintain the required parking and adequate access during the construction stage, the Proposed Project will include a plan to implement a number of strategies to temporarily address parking on the site and ensure safe and functional access. These strategies are anticipated to include the use of valet parking, stacked parking, shuttles from the eastern most parking lot, and if necessary off-site parking for employees.

Monitoring Phase: Construction
Monitoring Agency: LADOT
Enforcement Agency: LADOT

MM TRF-10: Prior to issuance of building permit, the Project Applicant shall contribute prorated funding for the installation of LADOT's Victory ATSAC system at the following seven intersections: (1) Van Nuys Boulevard/Riverside Drive; (2) Tyrone Avenue/Moorpark Street; (3) Hazeltine Avenue/Riverside Drive; (4) Hazeltine Avenue/Fashion Square Lane: (5) Woodman Avenue/Riverside Drive; (6) Woodman Avenue/US 101 Westbound Ramps; and (7) Woodman Avenue/Moorpark Street.

Monitoring Phase: Pre-construction

Monitoring Agency: DOT Enforcement Agency: DOT

MM TRF-11: Prior to project occupancy, the LADOT shall redesignate the curb lane on the southbound approach on Woodman Avenue to an optional through/right-turn lane so that the resultant lane configurations at the southbound approach will be one left-turn lane, two through lanes and one optional through/right-turn lane. If required by LADOT, the existing four-foot wide median island on the south leg of the intersection would be replaced by striping and/or lane delineators (e.g., two feet wide or less) so that additional width could be provided to the existing three southbound Woodman Avenue through lanes on the departure side of the intersection. The Project Applicant shall pay all expenses for these improvements.

Monitoring Phase: Construction
Monitoring Agency: LADOT
Enforcement Agency: LADOT